



REPORT

Tyrone Stockpile Stability Analysis for 2013 Closure Close-Out Plan Update

Tyrone, New Mexico

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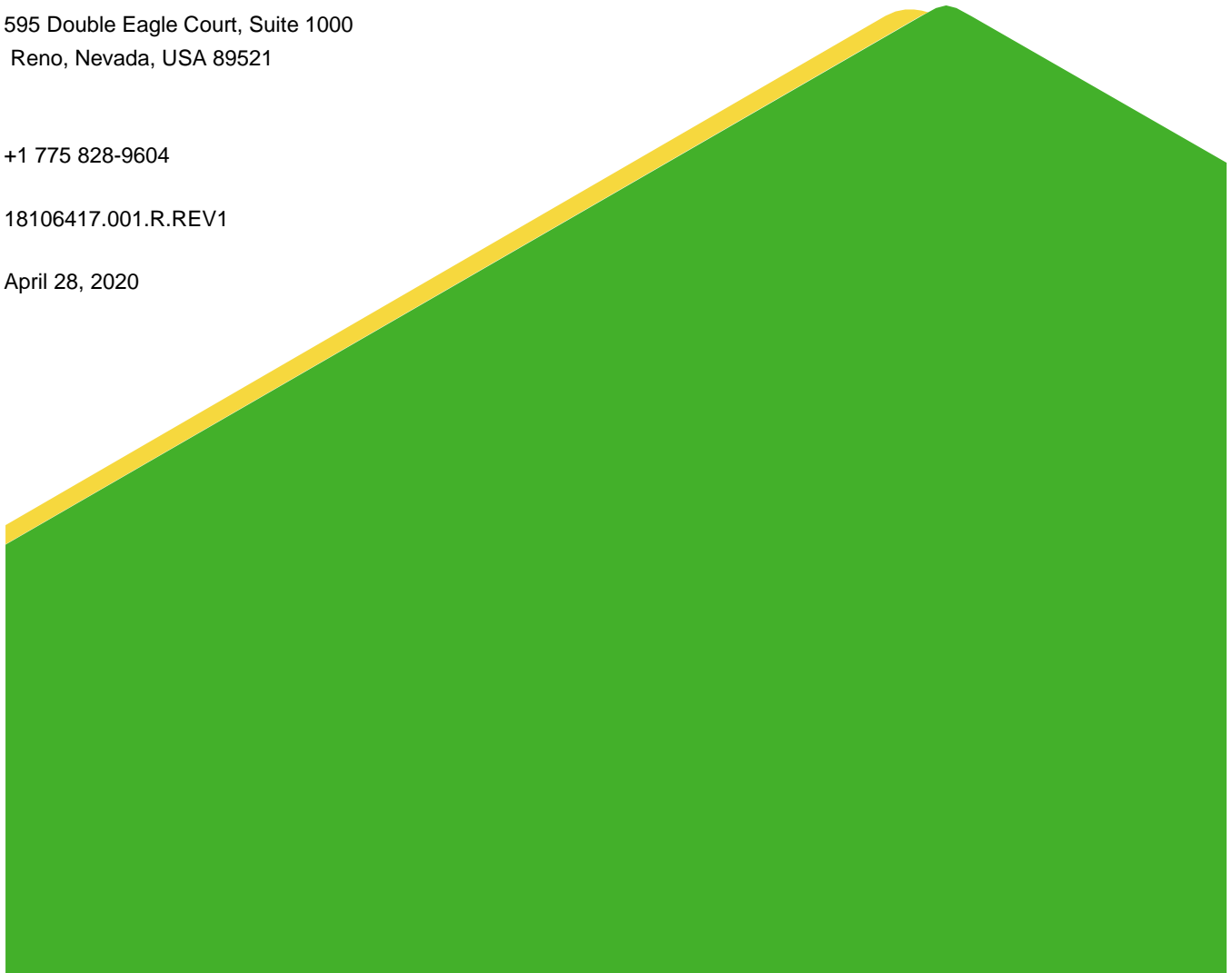


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1.0 INTRODUCTION

New Mexico Environment Department (NMED) issued a Supplemental Discharge Permit for closure (DP-1341) to Freeport-McMoRan Tyrone Inc. (Tyrone) on April 8, 2003. Per DP-1341 Condition 78, Tyrone submitted a Work Plan for Supplemental Slope Stability Analyses dated December 12, 2003. Tyrone submitted a series of stability assessment memorandums during 2006 and 2007 that addressed the stability of the Tyrone stockpiles.

The purpose of this report is to update the previous stockpile stability assessments to specifically address the Copper Rule criteria. The Copper Rule states that:

At closure, tailing impoundment(s) not regulated by the office of the state engineer, leach stockpile(s) or waste rock stockpile(s) shall be constructed to promote the long-term stability of the structure. Closure of all critical structures at a copper mine facility shall be designed for a long-term static factor of safety of 1.5 or greater and non-critical structures shall be designed for a long-term static factor of safety of 1.3 or greater. The units being closed shall also be designed for a factor of safety of 1.1 or greater under pseudo-static analysis. A stability analysis shall be conducted for the unit and shall include evaluation for static and seismic induced liquefaction.

“Critical Structure” means earthen or rock structures or embankments (such as the outslope of a rock stockpile), that are likely to cause an exceedance of applicable groundwater standards or undue risk to property in the event of a significant unexpected slope movement

This report provides a comprehensive report of the stability of the reclaimed configurations of the Tyrone leach and waste rock stockpiles, including stockpiles not addressed by the previous memorandums. The stability assessment applies consistent criteria for all stockpiles and updates data applied during the 2006-2007 stability assessments where new information is available.

2.0 SUMMARY OF PREVIOUS WORK COMPLETED

New Mexico Environmental Department (NMED) issued a Supplemental Discharge Permit for Closure (DP-1341) dated April 8, 2003 to Phelps Dodge Tyrone Inc. (now Freeport-McMoRan Tyrone Inc. [Tyrone]) for the Tyrone Mine. To comply with the New Mexico Water Quality Act (WCA) and the New Mexico Water Quality Control Commission (WQCC) regulations, Tyrone was required to conduct scientific studies that address a number of conditions specified under Section III of DP-1341. A slope stability study commenced, and the results were submitted in the memorandums issued during 2006-2007. These memorandums addressed Condition 78. Condition 78 states:

Tyrone shall perform a supplemental stability study on the Waste Rock Piles and Leach Ore Stockpiles at the Tyrone Mine Facility. In accordance with the schedule approved under Condition 74, Tyrone shall submit to NMED for approval a work plan including an implementation schedule for the supplemental stability study to evaluate the long-term physical stability of Waste Rock Piles and Leach Ore Stockpiles after closure. The study shall evaluate and quantify changes in the engineering parameters resulting from the natural weathering process of the Waste Rock Pile and Leach Ore Stockpile materials that may ultimately affect long-term stability. At a minimum, the work plan shall propose methods and analyses to account for changes in chemical and physical properties of the stockpile materials from the time of deposition to present day and to a specified time during post-closure. The study shall include an evaluation of the recently reported data for materials interior to the stockpiles and whether additional data collection is warranted to evaluate long-term stability.

A Work Plan for Supplemental Slope Stability Analyses dated December 12, 2003 was submitted which was intended to address the supplemental stability requirements of Condition 78. The Work Plan had four primary objectives.

- Further characterize the internal composition, structure, chemical and physical state and engineering parameters of the leach ore stockpiles and waste stockpiles.
- Further characterize the geological and engineering parameters of the foundation materials of the leach stockpiles and waste stockpiles.
- Quantify the effects of chemical weathering/cementation on the engineering parameters of the stockpile materials with respect to long-term slope stability.
- Re-evaluate the stability of the stockpiles based on the more detailed geologic/geotechnical model and engineering parameters.

The following sections summarize the various investigations and studies that have been completed to address Condition 78. These included a number of memorandums that addressed the stability of individual stockpile facilities that were included as appendices to the construction design quality assurance plans (CDQAP) for the reclamation designs of those facilities as discussed in Section 2.6.

Several stockpile characterization programs and slope stability assessments of the waste rock piles and leach stockpiles at the Tyrone mine have been completed per Condition 78. Much of the information collected from those studies was summarized in the Interim Report for DP-1341, Condition 78 (Golder, 2005) which is included in Appendix I of this report. The drill hole and test pit locations are provided on the Figures 2 through 5. The previous stockpile assessment activities are summarized in the following sections. In 2019, Tyrone performed an additional test pit investigation at the 9A Waste and 9AX Waste stockpiles. This investigation is summarized in Section 2.7.

2.1 Closure/Closeout Plan Addendum

Golder completed a slope stability analysis of stockpiles at the Tyrone Mine and reported the results of those analyses in a report titled; Closure/Closeout Plan Addendum, Slope Stability Analysis, Phelps Dodge Tyrone Inc. dated February 24, 2000. The Closure/Closeout Plan Addendum study provided a description of the existing site conditions, stockpile descriptions, characterization of stockpile materials, determination of mechanical properties of the stockpile materials, and assessment of the stability of the stockpile slopes in terms of a factor of safety.

The stockpile materials that were characterized during this study were obtained from 14 surface excavations that were typically 15 feet deep. The test pit locations are shown on Figures 2 through 5 (GTP series). The percentage of oversized (>3 inch) rock fragments in each test pit was estimated visually. The stockpile soils (minus 3-inch fraction) at each site were logged according to ASTM (D2488) standards. Point load tests were performed on oversize rock fragments to quantify the strength of the cobbles. Approximately 10 point-load tests were performed for each test pit. Nuclear gauge moisture and density measurements were obtained from each test pit to develop stockpile density values. Bulk samples were collected from seven of the test pits and were subjected to grain size analyses and Atterberg limits testing. Two samples were also selected for triaxial shear testing. Staged triaxial shear testing was performed in a four-inch diameter triaxial cell, scalped of fragments larger than ¾ inch, under consolidated, undrained conditions with pore pressure measurements to allow the determination of both the drained and undrained shear strength parameters.

2.2 Characterization of Re-Mined Stockpiles

During closure hearings with the State of New Mexico, Tyrone received comments concerning the lack of geotechnical characterization of the interior portions of the stockpiles. The comments addressed the impact that leaching by low pH process solutions and long-term weathering may have on the long-term shear strength and, consequently, the long-term stability of the stockpiles and that the interior portions of the stockpiles may be more impacted by leaching and weathering than the surface. To address these comments, Golder conducted a field investigation during October 2001 to augment the previous stockpile characterization study.

Tyrone identified several stockpiles that had been partially re-mined, exposing the interiors. This work was reported previously in an Interim Report (Golder, 2005) which is included as Appendix I of this current report. Golder collected samples and classified the stockpile soils at 12 sites. Sample locations are shown on the figures (TYTP01 series). The percentage of oversize material (plus 3-inch fraction) was estimated visually and the visual estimates were verified or adjusted using scaled photographic images. The stockpile soils (minus 3-inch fraction) at each site were logged according to ASTM (D2488) standards.

Soil samples collected from each site were subjected to sieve analysis (ASTM C117/C136), and Atterberg Limits determination (ASTM D-4318). Extended grain size curves were prepared by combining the laboratory grain size curves with the visual estimates of the gradation of the oversize fraction. Four staged, consolidated, undrained triaxial shear tests were performed using a four-inch diameter triaxial cell with the soil scalped of fragments larger than $\frac{3}{4}$ inch.

During 2004, Tyrone re-mined a portion of the 1C Waste stockpile exposing the interior portions of this stockpile. Golder performed a site investigation to characterize the interior portions of the 1C Waste stockpile in September 2004. The characterization was completed in the same manner as the October 2001 program. Soil samples were collected from eight locations in the interior of the 1C Waste stockpile. The locations are shown on Figure 2 (GA04-TY series). The samples were subjected to sieve analysis (ASTM C117/C136), hydrometer analysis (ASTM D422) and Atterberg Limits determination (ASTM D-4318). Large-scale (6-inch shear box) direct shear tests were performed under saturated conditions. The direct shear test samples were scalped of fragments larger than one inch.

Observations of the stockpile interiors showed that the internal layering on dump faces varied in orientation on each successive, approximately 50-foot lift so that there is not continuous slope-parallel layering over the entire slope height.

2.3 2004 Rotosonic Stockpile Drilling Program

A rotosonic drilling program was completed during the fall of 2004. The locations of the drill holes are provided on Figures 2 through 5. Boreholes TSGT-1 through TSGT-3 were terminated within the stockpile. Borehole TSGT-4 extended approximately 15 feet into the foundation soils.

The drill holes were logged for geologic and geotechnical information and samples were collected for laboratory testing. Geotechnical and geological logs and laboratory testing results were provided in the 2005 Interim Report (Appendix I). Estimates of the moisture content were made but do not reflect in situ conditions because the samples were stored for seven months prior to geotechnical logging. The core recovery and percentage of fragments greater than 3-inches were also recorded. The estimates of the oversize fraction from the rotosonic

core are considered less representative than from the test pits because of the relatively small diameter of the core.

After completion of each roto sonic core hole, a down-hole geophysical survey was performed. Geophysical logging included:

- Cased Density Log
- Natural Gamma Ray Spectrometry Tool
- Epithermal Neutron Log

The results of the geophysical logging were reported in Greystone, 2004.

2.4 2005 1A Leach Stockpile Rotosonic Drilling Program

During September 2005, Tyrone undertook a roto sonic drilling program of the 1A Leach stockpile to verify the condition prior to re-mining the slopes back to reclamation slope angles. Five roto sonic drill holes (S1A-1 through S1A-5 [referred to as T-1 through T-5 in the laboratory test results]) were completed to depths of 110 to 200 feet. Borehole locations are shown on the figures. A Golder engineer was present during drilling of roto sonic holes S1A-3 through S1A-5 to complete geotechnical logging and sampling of those holes. Hole S1A-3 penetrated through the base of the stockpile into the underlying Gila Conglomerate and encountered two feet of organic soil. The geotechnical logs are provided in Appendix B.3 of the Interim Report (Appendix I). Geologic logs prepared by Tyrone are provided in Appendix B.4. Nineteen samples were collected and subjected to sieve analysis (ASTM C117/C136), hydrometer analysis (ASTM D422) and Atterberg Limits determination (ASTM D-4318). The laboratory test results are provided in Appendix C.5 of the Interim Report (Appendix I) and are summarized in Table IV-1 of Appendix IV.

2.5 Materials Characterization Studies

EnviroGroup (2005) summarized materials characterization studies completed on the leached ore and waste rock stockpiles at the Tyrone mine to fulfil the requirements of the Condition 80 of DP-1341.

Golder reviewed stockpile cross sections prepared by EnviroGroup (formally Greystone) from historical mine maps to determine the year that various stockpiles were placed, and the mineral assemblages placed in the stockpiles. An abbreviated compilation of the compositional model cross sections is provided in Appendix II. Cross sections that have been included in Appendix II were selected to show the test pit and borehole locations which have been projected into the cross sections. The complete compositional model cross-sections were provided in EnviroGroup (2005). We have not use updated compositional models for this stability update as the drilling and sampling occurred on the older stockpile surfaces. The compositional models are based on a mineral assemblage (MA) classification system applied by Tyrone geologists that is summarized in Table 1.

Table 1: Tyrone Mineral Assemblage Classifications

Mineral Assemblage	Description
MA-0	Gila Conglomerate
MA-1	Leach Capping
MA-2	Oxide Copper
MA-3	Mixed Oxide and Chalcocite
MA-4	Chalcocite and Pyrite
MA-5	Mixed Chalcocite and Chalcopyrite
MA-6	Chalcopyrite and Pyrite
MA-7	Mixed Oxide and Chalcopyrite
MA-8	Native Copper and Cuprite

Table IV-1 in Appendix IV summarizes the sample locations, the age, and depth in the stockpiles. A characterization of mineral assemblage type in each stockpile was provided in Appendix E of the Preliminary Materials Characterization Report (DBS&A, 1997a) and the Supplemental Materials Characterization Report (DBS&A, 1997b). Those reports also include pit development maps, plan view stockpile distribution maps, and stockpile development cross sections. This information was used for determination of the placement history of each stockpile.

The materials near the base of the 1A Leach stockpile have been subjected to leaching for the longest duration of any of the stockpiles at Tyrone.

2.6 2006 Stability Memorandums

Golder prepared a number of stockpile stability reports in 2006 and 2007 for the stockpiles at Tyrone to provide an assessment of the stability of the stockpiles for the CDQAPs for stockpile reclamation and to address supplemental stability analyses requirements of Condition 78 of DP-1341. Stockpile names have been revised since the creation of these reports, and original names are used to reference these old reports for clarity.

Reports prepared included:

- Reclaimed 1 Stockpile (1 Leach)
- Reclaimed 1C Stockpile and 7A Waste Stability Analysis, dated May 4, 2006
- 1A Leach and 1B Leach Stockpiles Stability Analyses, dated July 14, 2006

- 2A Leach – 2B Waste Stockpile Stability Analysis, dated April 6, 2007
- 3A Leach Stockpile Stability Analysis, dated April 6, 2007
- 4C Leach Stockpile Stability Analysis, dated May 11, 2007
- 5A Waste Stockpile Stability Analyses, dated May 11, 2007
- Stability of Interior and In-Pit Stockpiles (1A Leach-1B Leach, 2B Waste, 2C Leach, 5A Waste, 3B Waste, 7B Leach, 8C Leach), dated May 11, 2007
- Addendum to the 1A and 1B Stockpile Stability Analysis, dated September 15, 2008

In addition to these reports, several State comment letters and response to comments letters were issued including:

- Response to NMED Comments Dated May 11, 2007, Conditional Approval, 3A Stability Analysis, dated June 18, 2007
- Response to NMED Comments Dated May 11, 2007 Conditional Approval No, 2A-2B Stockpile Stability Analysis, dated June 18, 2007

2.7 9A Waste and 9AX Waste Test Pit Investigation

Placement of waste rock at the 9A Waste and 9AX Waste stockpiles began after completion of the previous stockpile stability analyses. Two test pits were completed in December 2018 at the 9A Waste and 9AX Waste stockpiles to support this CCP update. Samples were analyzed for grain size, Atterberg limits, and a direct shear test was completed on a composite sample from the two test pits. The laboratory test results are provided in Appendix III. The sample locations are shown on Figure 5.

3.0 SITE CONDITIONS

3.1 Stockpile Descriptions

The current mode of open pit mining began in 1967 and waste rock stockpile placement began at that time. The Tyrone Stockpiles are generally situated near and within the Main, Gettysburg, Valencia, Little Rock, and other mine pits. The Reclaimed 1 Leach stockpile is located approximately one-mile east of the Main Pit. Reclamation of that stockpile is complete, and the stability is not addressed in this report. The stockpile crest elevations of the mine area stockpiles are generally between 6,200 and 6,700 feet and the stockpiles range in height between 350 to 600 feet from crest to toe. Stockpiles were placed at angle-of-repose with occasional setbacks resulting in overall slopes typically between 30° and 35°. The stockpiles were generally constructed by end dumping the materials in 30 to 50-foot lifts from the bottom up.

Limited leaching operations began in 1972, coincident with the opening of the precipitation plant. Mine-wide leaching operations began in 1984 after the commissioning of the solution extraction electrowinning (SX-EW) plant.

The following stockpiles which are designated on Figure 1, comprise the Tyrone Stockpile system addressed in this report. For naming consistency, only the current stockpile names are listed below.

- 1A Leach, 1B Leach, and Reclaimed 1C Waste

- 2A Leach and 2B Waste
- 3A Leach and 3B Waste
- 2 Leach (Areas 1 and 2), San Salvador Waste Backfill, and 7B Leach
- 7B Waste (South Rim Pit)
- 5A Waste
- 6B Leach and 6D Leach
- Reclaimed 7A Waste
- 9A Waste and 9AX Waste
- West In-Pit Waste Stockpile (at Little Rock)
- North In-Pit Waste Stockpile (at Little Rock)

Cross sections from the compositional model (Appendix II) located near drill hole and other sampling points are included in Appendix II.

Figures 1 through 6 illustrate the existing topography with the planned final reclamation grading of the stockpiles, overlain on the bedrock geology map. Cross sections on Figures 7 through 9 show the final reclaimed ground surfaces with the original ground surface and underlying geology.

3.2 Climate

The Tyrone Mine area is in a semi-arid region with elevations ranging from about 5,800 to 6,300 feet above mean sea level (amsl). The climate is warm and dry with mean annual precipitation of about 16 inches and a mean annual temperature near 50° F (Golder, 2007g). Precipitation falls mainly as rain, but snow may occur from November to March. Most precipitation falls during monsoon period from July through October in the form of short intense thunderstorms. About 60% of the precipitation falls during the monsoon. Annual evaporation greatly exceeds annual precipitation.

3.3 Geology

The geologic base map shown on Figures 1 through 6 was prepared by Golder (2007g) from a variety of sources that are listed on Figure 1. The mining district is in the northwest-striking Burro Mountain uplift. The mineralization is in and around the Quartz Monzonite of Tyrone (Tqm) stock, a 53 to 57 million-year-old Paleocene quartz monzonite porphyry (DuHamel et al., 1995) emplaced into the Precambrian Burro Mountain Granite (pCg). Paleozoic strata that are present north and east of Silver City and Cretaceous units present elsewhere in the Burro Mountains are not present in the Tyrone Mine area. Miocene-Pliocene-Pleistocene fan, sheet flood deposits, and older fan deposits (Qfo/Qtg), which includes the Gila Conglomerate (also referred to locally as the Mangas Conglomerate) are in direct contact with the crystalline basement rocks.

3.3.1 Lithology

The distribution of the lithologic units is shown on Figure 1. The bedrock units that are present below the Tyrone stockpiles include the Precambrian-age Burro Mountain Granite, containing dikes of quartz monzonite, rhyolite

and diabase. The Burro Mountain Granite is described by Paige (1922) as a light-gray, medium grained, granular granite containing 20-40% perthitic microcline, 30-50% sodic oligoclase, 30-38% quartz and 2% biotite.

The Burro Mountain Granite is intruded by the Paleocene Quartz Monzonite of Tyrone. The quartz monzonite is a very light gray to pinkish-gray, medium-grained, granular rock containing 15% orthoclase, 60% oligoclase 20% quartz 4% biotite. The quartz monzonite is locally porphyritic.

The Pliocene-Pleistocene Gila Conglomerate (Qfo/QTg) is present around the margins of the crystalline basement exposed in the Burro Mountain uplift. The Gila Conglomerate is a well-consolidated basin fill and fan deposit ranging from sand to conglomerate. It is often cemented by caliche. Where exposed in the east wall of the main pit it forms steep bench slopes and maintains stable 50° slope angles.

The Mangas Conglomerate and Gila Conglomerate have been used interchangeably by various workers. Over most of the mine area where these units occur, Tyrone identifies the Upper and Lower Mangas units and assigns them a late Tertiary to Quaternary age. Hedlund identified Gila Conglomerate only in localized exposures northeast of the mine area, while he mapped the majority of the cemented alluvium and conglomerate in and adjacent to the mine area as older conglomerate deposits (Qfo), Hedlund reports the Qfo as being derived from the underlying Gila Conglomerate. Where Hedlund mapped Qfo, others have mapped the Upper and Lower Mangas Conglomerate.

Griffin (2001) described the Lower Mangas as sediments eroded from the Big Burro Mountains and Silver City Range that were deposited in a graben system during the late Neogene. The Upper Mangas fan deposits were formed upon reactivation of basin and range faults which bisected the older graben forming the Mangas half-graben as described by Griffin.

Younger alluvium (Qa) is present along alluvial valleys in Brick Kiln Wash, Oak Grove Wash, Niagara Wash, and Mangas Wash and their tributaries. The alluvium is typically a relatively loose to compact sand to clayey sand. Local occurrences of Quaternary Fan (Qf) and Talus (Qt) are included in the Qa unit.

3.3.2 Structure

The main fault systems in the Tyrone Mine area trend northwest and northeast and are shown on Figure 1. The main northwest trending faults include the Mangas and Townsite Faults. Northeast trending faults include the West Main, Gettysburg Entry, Crusher, and San Salvador Faults. The northwest trending Mangas Fault is southwest dipping normal fault that has preserved a wedge of the Gila Conglomerate in the down-dropped block, being thickest at the fault and thinning to the southwest.

3.3.3 Alteration

Porphyry copper mineralization is related to the intrusion of the quartz monzonite with phyllic, propylitic, and argillic primary alteration zoned around the intrusion. The primary alteration is overprinted by supergene alteration and secondary copper enrichment.

Sericite is the most abundant hydrothermal alteration product, Intensive kaolinization appears to have obliterated hypogene clay minerals in the higher levels of the ore body. Propylitic alteration has been observed at the periphery of the deposit in the Gettysburg area. Argillization occurs in the southeast corner of the Racket area but appears localized. Silicification is prevalent in the granite -quartz monzonite contact zone. A zone of clay may be present at the base of the enrichment zone (Kolessar, 1982). The crystalline bedrock units are generally competent, brittle rock units

4.0 TEST DATA

4.1 Field Testing Data

Point load testing was performed on stockpile rock fragments during the test pit program carried out for the 2000 Closure/Closeout Addendum and the results were contained in Appendix B.2 of that report (Golder, 2000). Approximately 10 tests were performed at each of the 14 test pits. The point load index was multiplied by a typical conversion factor of 24 (ASTM, 2016) to estimate the uniaxial compressive strength (UCS). The averaged values from each test pit ranged from a UCS of 4,621 pounds per square inch (psi) corresponding to a medium strong rock according to the International Society for Rock Mechanics (ISRM, 1985) to 22,481 psi which corresponds to a very strong rock.

Field nuclear density testing was also performed during the 2000 program. Three tests were completed at each test pit and the density values for test pits completed on each stockpile were average. The average moist density ranged from 123.4 pounds per cubic foot (pcf) to 133.8 pcf and moisture contents ranged from 4.4% to 6.7%.

4.2 Laboratory Testing Data

The laboratory testing data collected from the previous stockpile stability assessments as well as the 2018 9AX Waste test pit program, are compiled on Table IV-1 in Appendix IV.

4.2.1 Grain Size

Grain size data is available from a total of 81 stockpile samples that have been collected during the various stockpile stability assessments described in Section 2.0. These include 18 waste rock samples and 63 leached ore samples. Hydrometer analyses were completed for 60 of the samples providing size gradation data for the fines fraction (<0.075 mm) to 1.4 microns.

Figures IV-1 and IV-2 in Appendix IV plot the grain size distribution for leach and waste stockpile material, respectively. The curves on the figures are for reference to compare the gradation of the leach and waste materials. The leach material has a larger percentage of test results above the reference line indicating the leach material is generally finer grained than the waste material. Although, the waste material generally falls within the size gradation range of the leach material.

Figures IV-3 and IV-4, in Appendix IV, plot the variations in the percent fines (<0.075 mm) and percent sand plus fines (<9.5 mm) respectively as a function of depth. Leach and waste materials are distinguished by different symbols. Linear trend lines are included. The results show no recognizable systematic trend in the grain size with the depth in the heap. Depth in the heap is also a relative measure of the time since placement.

4.2.1.1 Extended Grain Size Curves

The distribution of the various-sized particles plays a significant role in determining the physical properties of stockpiles. The strength of a soil with oversized particles may be characterized by the strength of the matrix material if the oversized particles are in a floating state. Conversely, the strength of the soil may be characterized by the properties of the oversized material if there is sufficient oversized particle to particle contact. The strength properties of a soil having less than 40% oversized material are controlled primarily by the soil matrix and that the strength properties of a soil with more than 65% oversized material is controlled primarily by the properties of the oversized material. The strength properties of soils having between 40% and 65% oversized material are influenced by both the soil matrix and the oversized material (Fragaszy, et al. 1992).

For the purposes of assessing the strength of a rockfill material and the influence of the oversize fraction, the oversize fraction is defined as the size larger than that included in laboratory testing apparatus. Triaxial testing was performed on materials finer than $\frac{3}{4}$ inch. The large-scale direct shear tests were performed on the minus 1-inch fraction. The percentages of the fragments larger than 3-inches (cobbles and boulders) were visually estimated from test pits completed in the re-mined leach stockpiles that were mapped in 2001 and the 1C Waste Stockpile in 2004. The logs from the mapping are included in Appendix A-2 and A-3 of the Interim Report included as Appendix I of the current report.

Extended grain size curves were prepared that shift the laboratory plots and include the greater than 3-inch fraction. The extended grain size curves for the leach ore and waste material are provided in Figures IV-5 and IV-6, respectively, in Appendix IV. Strength tests on leach ore samples were done entirely using the triaxial cell.

Four out of five of the strength tests performed on the waste material were done in the shear box. Therefore, oversize fraction for leach ore is $>\frac{3}{4}$ inch and for waste rock it is >1 inch. Estimates of the oversize fraction for the sonic boreholes are considered less reliable because of the relatively small diameter of the sonic core compared to the grain size of the stockpile and were not included in the extended grain size plots.

The oversize fraction was estimated to generally fall in the transitional range with 40% to 65% larger than the matrix size, i.e. $<\frac{3}{4}$ inch or <1 inch for the leach ore and waste rock material. Therefore, the laboratory shear strength tests results from of the stockpile matrix is considered to underestimate the actual stockpile shear strength.

4.2.2 Atterberg Limits

Atterberg limits testing was performed on all 81 of the stockpile samples. Atterberg limits testing is performed on the <0.425 mm size fraction and is used to assess the plasticity of the soil. More plastic soils are composed of higher percentage of clay minerals and/or more active clay minerals. Clay minerals adsorb water into their mineral structure and more active clays (e.g. montmorillonite) can adsorb more water than less active clays (e.g. kaolinite). Soils with higher percentages of clay or active clays tend to have lower shear strengths.

The average plasticity index (PI) of the leach material is 13 with a standard deviation of 4, indicating fairly consistent PI values. Waste materials have a slightly higher average plasticity index value of 16 with a standard deviation of 2. These results suggest that leaching stockpiles has not caused the formation of clay or more active clay minerals which might cause an associated reduction in the shear strength.

Figure IV-7 in Appendix IV shows the variability of the PI as a function of depth (age). No recognizable systematic trend in the PI with depth in the stockpiles is evident.

The activity of a clay mineral is taken as the PI divided by the percentage of the clay size fraction (i.e. <2 micron). Activity has been used for engineering property correlations especially for inactive and active clays. Higher activities are indicative of more active clay minerals.

Figure IV-8 plots the clay activity with respect to depth with different symbols used for leach and waste samples. There is no tendency for higher activities for leach materials or with increase in depth (age) of the stockpile material, also suggesting that leaching or weathering in acid conditions does not lead to formation of more active clay minerals that might lead to a reduction in the shear strength of the stockpile materials.

4.2.3 Shear Strength

A total of 11 laboratory shear strength tests have been completed during the course of the various stockpile assessments. As discussed previously, seven tests were completed in a four-inch diameter triaxial cell and four were done in a 6-inch shear box. Six strength tests were completed on leach ore material and five strength tests were done on waste material. Each test method has advantages and disadvantages. The direct shear tests can accommodate larger size fragments and can be carried out to larger displacements. The triaxial tests better model higher confinement stresses. All six tests on leach ore material were triaxial tests. Four of the five tests on waste rock were direct shear tests. The shear strength test results are provided in Table 2 below. The Mohr-Coulomb (M-C) parameters friction angle (ϕ) and, cohesion (c) for the triaxial tests are reported at 5% strain. The direct shear test results reported in Table 2 are residual, large displacement, results. The laboratory reports from SGI (Appendix D-3 of the Interim Report included as Appendix I of this report) do not specify the amount of displacement that occurred at the reported residual strength, although the Golder direct shear test laboratory reports in Appendix III were run to a 20% strain and the SGI tests were likely run to that amount of displacement as well. The tested samples do not exhibit brittle behavior and the large-strain residual strengths are close to the peak strength.

Triaxial tests were performed in a 4-inch diameter cell on remolded samples that were scalped of material larger than $\frac{3}{4}$ inch. Samples were remolded to a dry density of generally between 120 and 125 pcf. Triaxial tests were performed under consolidated, undrained conditions with pore pressure measurements at confining loads of 25 to 100 psi simulating failure surface depths of 27 to 110 feet.

The direct shear tests were performed on remolded samples that were scalped of material larger than 1-inch. The material was nominally compacted and tested under inundated conditions and were allowed to consolidate at each normal load. Tests were done at normal loads of 40, 100, and 300 psi simulating failure surface depths of approximately 45 to 325 feet.

The direct shear test results from primarily waste rock samples shows considerable curvilinear strength envelop shape and were performed to a higher confining load compared to the triaxial tests. Therefore, the resulting linear M-C fit to the curvilinear envelopes for waste rock samples has lower ϕ angles and higher c values than the triaxial test results performed on leach ore samples.

Table 2: Laboratory Shear Strength Test Results

Sample Number	Stockpile ¹	Year Placed	Depth (ft)	Soil Classification	Method	Effective Friction Angle (ϕ°)	Effective Cohesion (c [psi])
GTP-03/02	5A Waste	1998	10	SC	Triaxial	34.6	5.8
GTP-06/03	2A Leach	1998	10	GP-GC	Triaxial	32.8	8.3
TYTP1-2	Copper Mtn. Leach	1978	400	GW-GC	Triaxial	36.2	0.6
TYTP1-4	Copper Mtn. Leach	1986	100	GC	Triaxial	35.5	0.4
TYTP1-7	6A Leach	1989	300	SC	Triaxial	36.9	0.6
TYTP1-9	Gettysburg Waste	1967	40	GC	Triaxial	34.1	2.2

Sample Number	Stockpile ¹	Year Placed	Depth (ft)	Soil Classification	Method	Effective Friction Angle (ϕ°)	Effective Cohesion (c [psi])
TSGT-4	2A Leach	1973	265	SM	Triaxial	38.4	0
GA04-TY-1	Reclaimed 1C Waste	1998	50	GC	DS	31.0	10.4
GA04-TY-5	Reclaimed 1C Waste	1978	250	GC	DS	32.0	11.8
GA04-TY-8	Reclaimed 1C Waste	1978	150	GC	DS	29.0	8.8
TY18-01/02	9A Waste, 9AX Waste	2017	10	GW-GC	DS	28.1	20.5
Average Leach Material						35.65	2.02
Average Waste Material						30.9	11.5

Notes:

1 - Stockpile names presented in this table are those used in the previous sampling efforts and associated reports and may not reflect the current stockpile names presented in the CCP Update.

4.3 Summary of the Impacts of Weathering

Condition 78 required an evaluation of the changes in the stockpile strength parameters and long-term stability resulting from the natural weathering processes. Key attributes that can affect the strength of stockpile materials include the gradation, durability (i.e. resistance to degradation over time), strength of rock fragments, fines content, clay content, and density (Hawley and Cuning, 2017). A well-graded soil gradation improves stockpile shear strength. The strength, angularity and durability of the rock fragments also tends to increase the shear strength. High fines and clay content tend to reduce rockfill strength. Potential causes of reductions in the shear strength over time due to weathering include weakening of inter-mineral bonds resulting in the physical breakdown of rock fragments and reduction of the oversize fraction, increase in the clay content or formation of more active clay minerals, and increase in the fines fraction. Potential causes of increases in the shear strength of stockpiles over time include settlement and increase in the density and cementation.

The assessment of the long-term impacts of chemical weathering were addressed by the Supplemental Materials Characterization study prepared by EnviroGroup (2005). The effects of stockpile weathering and leaching were investigated by examination of the material characteristic trends as a function of age, roughly translated as depth in the stockpiles. Trends in pH, acid-base potential, pyritic fraction, sulphate fraction, and meteoritic water mobility procedure extract parameters; acidity, Al, Cu, Fe, and SO₄ were plotted as functions of stockpile age. The pyritic sulfur fraction diminished with age of the stockpile material indicating that pyrite is reacting in the stockpiles and is being depleted as they age. No other distinct trends in these parameters could be discerned as a function of age.

X-ray diffraction studies by Envirogroup showed there is a weak trend for muscovite and kaolinite to increase as feldspar decreases and the abundance of these minerals appears to be controlled by the degree of hydrothermal alteration that occurred during the time of mineralization rather than due to changes after placement in the stockpile. Muscovite (illite) is formed under conditions of hydrothermal alteration and is not a product of stockpile weathering and leaching. Kaolinite can form under weathering conditions, but the downhole trends seen in the boreholes did not appear to be related to progressive weathering of the stockpile materials.

The depth trends with respect to amorphous clay, goethite, jarosite and pyrite were also evaluated by EnviroGroup. The amorphous clay in the stockpile samples was compared to the age (depth) and there was no evidence for a correlation of amorphous clay abundance with pyrite or jarosite content. Furthermore, the amorphous clay content in the waste rock stockpile samples range as high as those found in the leach ore stockpiles. This suggests that acidic conditions and leaching reactions are not resulting in the breakdown of the rock to amorphous clay components.

Golder evaluated the impacts of weathering on the physical parameters of the stockpile materials by evaluating the trends in the grain size, Atterberg limits, and shear strength as functions of depth in the stockpiles. The results presented in this section and the figures in Appendix IV indicate that there is no clear relationship between the fines content, PI, clay activity, or other factors potentially attributable to weathering that might negatively influence the stockpile shear strength with the age or leach history of the stockpiles. The results also do not indicate significant formation of clay minerals, formation of more plastic fines, or more active clay minerals with time or leach history.

The gradation curves in Figures IV-1 and IV-2 in Appendix IV show the leach stockpiles to be more fine-grained which may be the result of chemical weathering resulting in a breakdown in the rock fragment size. However, EnviroGroup have also noted that the finer grained nature of the leach ore compared to the waste rock may result from different blasting parameters that increase fragmentation to enhance ore recovery. Similar studies completed at the Chino mine stockpiles (Golder, 2008b) noted some tendency for the stockpiles to become finer grained with depth and this was attributed to particle crushing, downward migration of fines, and/or chemical weathering in the older (deeper) parts of the stockpiles. However, this tendency is not apparent from the Tyrone data. The Stockpiles at Tyrone are relatively young compared to the Chino stockpiles and the trends may not be as apparent.

Golder has concluded that the soil matrix fraction of leached material or material weathered for long periods of time remains similar in character to the matrix fraction of the less weathered material but may become higher in proportion due to the physical breakdown of the rock fragments. The fact that the PI and clay activity values are indistinguishable for leached and unleached material and for material that has been placed in the stockpiles for longer and shorter periods of time suggest that the process of weathering does not lead to the formation of additional clay fraction or more active clay minerals.

Both the waste rock and leach ore stockpiles have historically been stable and no significant stockpile failures have occurred to our knowledge. We are also not aware of the occurrence of instability developing in stockpiles at any of the porphyry copper mines in the southwest USA after long periods due to weathering and reduction in the shear strength over time.

5.0 DEVELOPMENT OF MODEL PARAMETERS

5.1 Summary of Material Parameters Applied in the Stability Analyses

Table 3 summarizes the material parameters applied in the stability analyses. The basis for the development of these parameters is described in the following sections.

Table 3: Summary of Material Parameters

Material	Strength Model	Moist Unit Weight (pcf)	Saturated Unit Weight (pcf)	ϕ (°)	c (psi)
Leached Ore	M-C	125	138	35.5	2.0
Waste Rock	M-C	125	138	30.9	11.5
Alluvium	M-C	125	138	29.0	0
Liquified Alluvium	M-C	125	138	8.0	0
Gila Conglomerate	M-C	125	138	35	6.94
Quartz Monzonite	M-C	160	160	43	669
Precambrian Granite	M-C	160	160	35	340

5.2 Material Parameters for Leach Ore and Waste Rock Stockpiles

The results of the laboratory testing and geotechnical logging indicate that the leach ore and waste stockpile materials generally classify as clayey gravels (GC) or clayey sand (SC) with occasional poorly graded gravel with clay and sand. The soil has a low to moderate plasticity with PI values generally between 10 and 20. The oversize fraction varies considerably, comprising 10% to 50% of the material. There is no recognized differentiation in the soil classifications, PI, or oversize fraction between the leached materials and waste rock.

Leps (1970) published the results of large-diameter triaxial testing, capable of including particles up to 8-inch diameter, on 15 different rockfill materials that represent a range of grain size distributions, particle strength, and density.

Leps identified a range of rockfill material strength envelopes depending on the density, gradation, shape, and intact strength of the particles and provided curvilinear envelopes representing Strong Rockfill, Average Rockfill, and Weak Rockfill. These curves are often applied to assist in the selection of rockfill strengths for the design of rockfill structures and stockpiles. The laboratory strength test data from the Tyrone stockpiles are plotted on normal stress versus shear strength plots on Figures IV-10 and IV-11 in Appendix IV for leach ore and waste rock, respectively. These results are compared to curves for Weak, Average, and Strong rock fill provided by Leps (1970). The leach ore samples plot close to the average Lep's strength and the waste rock generally falls between weak and average Lep's strength.

The grain size distribution data and soil classifications for the stockpile samples show the material is generally well graded. The major components of the stockpiles are blasted, competent bedrock units that form strong angular fragments with limited fines content. The Gila Conglomerate, placed mainly on waste rock piles, has rounded fragments and higher fines content. The 5A Waste stockpile is primarily composed of Gila Conglomerate. Leaching and wetting of the leach ore stockpiles is expected to have induced settlement of the stockpiles and the leach ore stockpiles are likely in a more-dense state than the waste rock stockpiles. As discussed in Section 4.3, neither the waste rock or the leach ore stockpiles are predicted to develop higher clay contents or generate more active clay minerals that would cause a reduction in the shear strength over time due to long-term weathering. Therefore, it is reasonable that the waste rock stockpiles would have lower shear strengths compared to the leach ore stockpiles.

The laboratory-derived Mohr-Coulomb shear strengths of the stockpile samples range from $\phi = 29.0^\circ$ and $c = 8.8$ psi to $\phi = 38.4^\circ$ with $c = 0.0$ psi cohesion. The shear strength results were summarized in Table 2. The average Mohr-Coulomb strength parameters for leach ore are $\phi = 35.5^\circ$ and $c = 2.0$ psi. The average strength parameters for the waste rock are $\phi = 30.9^\circ$ and $c = 11.5$ psi.

Due to the limitation of particle size in the test apparatus (i.e., $\frac{3}{4}$ -inch for triaxial cell and 1-inch for direct shear box) the impact of the larger size fragments is not included in the laboratory-derived shear strength estimates.

As discussed in Section 4.2.1.1, the boundaries provided on Figures IV-5 and IV-6 in Appendix IV between oversized controlled shear strength, matrix-controlled strength, and transitional behavior indicate the stockpiles are generally in the transitional region and the laboratory-derived shear strengths do not account for the influence the oversize fragments have on the shear strength of the stockpile materials.

Soil strengths assessed from the laboratory testing of remolded samples reflect the matrix strength without influence from the oversize fraction or from the in-situ cohesion resulting from cementation (e.g., gypsum precipitates). The shear strength values derived from the soil matrix are considered to reflect the fully weathered strength of the stockpile materials.

The use of effective stress (as opposed to total stress) strength parameters for stockpile materials is appropriate based on the unsaturated condition of the stockpiles and the granular nature of the stockpile materials. The average apparent cohesion, resulting from the linear best fit to the curvilinear strength envelope in the stress range under consideration has been applied. Cohesion due to cementation by sulphate and other minerals formed in the stockpile has been disregarded. The presence of sulphate minerals in the stockpiles derived from weathering after placement is documented in EnviroGroup (2005). The presence of cohesion from cementation is also evidenced by excavations into the re-mined 1C Waste stockpile material (Golder, 2005) that stood at near vertical slope angles and remained stable for years. The application of a stockpile shear strength represented by the average of the laboratory strength tests completed on remolded samples i.e. without the cementation from sulphate minerals or the influence of oversize fraction provides a reasonable estimate of the long-term, weathered stockpile strength. These strengths fall within the general range of average to weak Leps rockfill strengths showing the laboratory test results are in the expected range.

A M-C shear strength represented by an effective stress $\phi = 35.5^\circ$ and $c = 2.0$ psi, corresponding to the average of all triaxial and direct shear test results completed on leach ore samples has been applied for the weathered strength of the leach ore stockpiles.

The waste rock stockpiles are assigned a strength represented by an effective stress $\phi = 30.9^\circ$ and $c = 11.5$ psi corresponding to the average values from laboratory strength testing of waste stockpile samples.

Unit weight information collected and reported in the CCP Addendum (Golder, 2000) from nuclear density testing from stockpile surfaces was used as a basis for the selection of unit weight. The measured densities from the stockpile surfaces were likely affected by compaction by haul traffic but do not reflect compaction by burial in the stockpile. The range in variability of the test results from a stockpile are as variable as the results from all stockpiles and there is little basis to apply different values for different stockpiles. The design basis moist unit weight selected for the stockpiles is 125 pcf and the saturated unit weight is 138 pcf.

5.3 Material Parameters for Stockpile Foundations

Bedrock units underlying the Tyrone stockpiles are Precambrian Burro Mountain Granite and Paleocene Quartz Monzonite of Tyrone. Gila Conglomerate overlies the crystalline basement rocks along the east and north sides of the mine complex. There is no evidence that a weak interface is present at the base of the stockpiles. Alluvium is present in the Oak Grove, Brick Kiln, Niagara, Mangas, and Deadman Washes and their tributaries.

Two of the sonic boreholes (TSGT4, S1A Leach-3) were interpreted to have penetrated the base of the 2A Leach and 1A Leach stockpiles respectively. There was no evidence of a zone of clay soil or highly weathered weak interface at the base of the 2A Leach stockpile, underlain by Precambrian Granite. A thin 2-foot zone of organic soil layer was encountered at the base of the 1A Leach stockpile above the Gila Conglomerate. A triaxial test was taken from the interface zone at the base of the 2A Leach stockpile that yielded a M-C strength of $\phi = 38.4^\circ$ and $c = 0$ psi, higher than the average strength of the stockpile materials. Therefore, a weak basal zone has not been included in the cross-sectional models of the stockpiles.

The following sections describe the data available and applied to the development of engineering parameters for the foundation units.

5.3.1 Precambrian Granite

Precambrian Granite underlies the foundation of stockpiles in the northern portion of the mine area (Figure 1). CNI (1982) used uniaxial compression and Brazilian disk tests to estimate the intact strength of mine area granitic rocks for pit slope stability studies. Minimum reported estimates for intact bedrock cohesion and internal friction angle are $c = 669$ psi and $\phi = 43.4^\circ$, respectively. Strength testing along fractures resulted in $\phi = 26^\circ$ to 28° and an apparent $c = 13$ psi to 16 psi. Applying the intact strength listed above, a fracture strength of $\phi = 26^\circ$ and 16 psi cohesion, and assuming that failure surfaces involve 50% intact material with the remainder following pre-existing fractures, CNI estimated a M-C strength of $\phi = 35.6^\circ$ and $c = 340$ psi for the Precambrian Granite. For the stockpile stability analyses, the strength of the Precambrian Granite bedrock has been assigned a M-C strength $\phi = 35.0^\circ$ and $c = 340$ psi.

Soils developed on the surface of the Precambrian Granite are shallow and gravelly.

5.3.2 Monzonite

CNI (1982) reported strengths for the Quartz Monzonite unit. The minimum reported M-C strength parameters are $\phi = 43^\circ$ and $c = 669$ psi and this value has been applied for the foundation strength.

Soils developed on the surface of the Quartz Monzonite are similarly shallow and gravelly.

5.3.3 Gila Conglomerate

CNI (1982) reported a peak shear strength of $\phi = 40.89^\circ$ from large-scale, direct shear testing of disturbed samples of Gila Conglomerate. MWH applied a strength of $\phi = 35$, $c = 6.94$ psi (1,000 pcf). We have applied the strength parameters applied by MWH and moist and saturated unit weights of 120 pcf and 133 pcf respectively to the QTg in the stability analyses.

5.3.4 Quaternary Alluvium

Golder tested two samples of alluvium recovered from the 3A Leach Stockpile seepage collection area from Boreholes 11-9 and 10-4 using staged consolidated, undrained (CU) triaxial tests (Golder, 2007b). Triaxial test specimens were remolded to field-measured in-situ density and moisture content. Effective $\phi = 38.8^\circ$ and 37.5° were measured in staged CU triaxial tests. These were performed at confining pressures of 40 to 125 psi.

Several geotechnical drilling programs have been completed in the toe area of the stockpiles that yielded information on Quaternary alluvium including standard penetration testing (SPT) blow count data. These programs include:

- 1B Leach Collection System Relocation
- 1C Waste Toe Investigation, January 2005
- 3A Leach Soil Boring Investigation for PLS Catchments in Canyons 7 through 11, July 2006

The soils encountered in areas designated as Alluvium were typically classified silty to gravelly sand or poorly graded sand and are generally less than 30 feet thick.

A standard correlation between corrected blow count values to the relative density and friction angle of sands is provided in Table 4 (Das, 1985).

Table 4: Correlation of SPT Blow Counts to Compactness and Friction Angle

Corrected Standard Penetration Number	Compactness	ϕ (°)
0 - 5	Very Loose	26 – 30
5 - 10	Loose	28 – 35
10 - 30	Compact	35 – 42
30 – 50	Dense	38 - 46
>50	Very Dense	38 – 46

The boring logs with measured SPT blow counts and tables with the corrected blow counts are included in Appendix V. Based on the corrected blow count results, the alluvium encountered generally classifies as compact to dense. Lower blow count values reported in the 1B Leach stockpile toe area (Appendix V-1) were related to soft fill (i.e. sediment pond accumulations). As-built documentation shows that those soft fill deposits were removed and replaced with compacted fill. The corresponding ϕ angle for compact to dense cohesionless soils can be

expected to range from 35° to 42° based on empirical values reported by Das (1985). However, these correlations are for clean sands and the presence of silts and fines will lower the shear strength.

For the analysis of the stability of stockpiles the alluvium has been assigned a Mohr-Coulomb strength of $\phi = 29^\circ$ and $c = 0$ which is considered conservative based on the results of the SPT testing and laboratory testing information.

Moist and saturated unit weights of 125 pcf and 138 pcf, respectively, have been assumed for the alluvium.

Where saturated alluvium is present below the toe of the stockpiles and available standard penetration test (SPT) blow count data yields factors of safety against liquefaction of less than approximately 1.3 (see Section 6.3), the stability of the stockpiles is evaluated applying liquified strength parameters to the zone of saturated alluvium. Vaid and Thomas (1994) found that the residual strength of loose clean sand samples subjected to extension tests ranged from 0.1 to 0.18 times the effective overburden stress (σ_{vo}). This is approximately equivalent to an internal friction angle of 5° to 11°. For the analysis of the stability with liquefied alluvium, the zones of alluvium below the modelled seasonal average groundwater table (discussed in Section 5.4.2) were assigned an internal friction angle of 8° representative of a residual, post-liquefaction shear strength. The liquefied stability analyses were performed applying pseudo-static loading conditions.

5.4 Hydrogeologic Conditions

5.4.1 Stockpile Moisture Conditions

Information regarding moisture conditions in the stockpiles at Tyrone is available from downhole geophysical logging in sonic drill holes completed in the 3A Leach, 6B Leach and the 5A Waste stockpiles, and gravimetric moisture content testing in the 1A Leach stockpile. Conditions within 3A Leach, 5A Waste, 6B Leach, and 1A Leach stockpiles are considered to be indicative of conditions in waste rock and leached ore stockpiles in general.

The 3A Leach stockpile was under active leaching at the time of geophysical logging. Logging results (EnviroGroup, 2005a) from sonic borehole TSGT-1 indicate a volumetric moisture content between 3 and 19 percent (ft^3/ft^3), averaging approximately 12%. Applying a dry unit weight of 114 pcf, this represents an average gravimetric moisture content of 1.6 to 10 percent (lbs/lb), averaging approximately 6.6 percent. Applying a specific gravity of soil solids of 2.765 (the average from available laboratory testing), saturated conditions would occur at a gravimetric moisture content of 19 percent. Geophysical logging in drill hole TBGC-6 in the 6B Leach stockpile indicated an average gravimetric moisture content of approximately 5 percent, applying the same criteria as described above, and ranged from 2 percent to 7.5 percent. The results from the 6B Leach stockpile also indicate drained conditions. Although the dry unit weight applied in these conversions are assumed values, consideration of a range reasonable dry unit weights indicates that the measured moisture contents from geophysical logs are generally below saturation levels and generally unsaturated conditions are indicated, even while under leach.

Geophysical logging conducted in drill hole TSGT-3 in the 5A Waste stockpile indicates a volumetric moisture content of 2 to 15 percent. These are approximately equivalent to gravimetric moisture contents of 1 to 7 percent and indicate that the waste stockpiles can be considered to be unsaturated.

Moisture content testing (ASTM D2216) of roto-sonic borehole samples collected in October 2005 and reported in Appendix C-5 of the Interim Report (Appendix I) from the 1A Leach stockpile indicated gravimetric moisture contents ranging from 4.3 to 22.5 percent (a high clay content sample), averaging 10.1 percent. Stockpile material properties are expected to vary; however, unsaturated conditions are indicated within the leached ore stockpiles.

The potential for saturation to occur will be lower under post-closure conditions when leaching is terminated and following placement of a soil cover and surface water controls.

Elevated groundwater levels and local groundwater mounds in the stockpiles that would impact stability are not expected because of the drainage capacity of the stockpiles. In particular, the ore stockpiles have previously been leached at rates that exceed 100-year storm rainfall amounts on a daily basis. Saturation and instability did not occur under these conditions. The potential for elevated groundwater levels will be further reduced upon cessation of leaching operations, cover placement, and implementation of surface water management. In the waste rock piles, surface water management measures instituted at reclamation will reduce long term infiltration rates and further reduce the potential for the development of saturated conditions that could impact stability.

These data and conclusions are consistent with EnviroGroup (2005) findings, which indicate that the stockpiles are drained, that moisture content correlates with the grain size of the materials, with sands and gravels having low moisture content and zones with higher clay content having higher retained moisture. Overall, the stockpiles are indicated to be unsaturated.

5.4.2 Perched Alluvial and Regional Bedrock Groundwater Conditions

Groundwater levels applied in the stability models are based on the quarterly sitewide groundwater models provided by Daniel B. Stevens and Associates (DBS&A, 2018). Models are developed for the perched water table present in the alluvium within the East Side Area (Oak Grove Wash) and Deadman Canyon and included in bi-annual reports. Contoured perched water level data is not available for the alluvium in the Mangas and Niagara Wash areas and the perched groundwater levels were determined from individual well data. Site-wide groundwater models are also provided by DBS&A for the deeper regional water table in the mine and stockpile area.

Monitoring well data are available in the east side area, the 3A Leach area, and the Deadman Canyon Wash area from quarterly reporting for the period 2011 to 2018. These were used to define an “Average” (average over the period of record) and a “High” (the average positive deviation from the average water level for the wells in a given area) perched water table condition. The “High” water table condition applied in the liquefaction susceptibility analyses to identify zones where liquified strengths may be applied. The high perched water table levels are between 5.3 to 10.9 m above the average perched water table elevation.

Where water levels were measured in geotechnical boreholes and recorded on the drill logs (e.g. 1B Leach stockpile toe area), these levels were applied for the assessment of the liquefaction potential (Section 6.3). These measured water levels are significantly higher than the current water levels indicated in the quarterly monitoring.

There are a number of interceptor/barrier trenches in the alluvium that affect the perched groundwater levels. Perched models are not available in the 3A Leach stockpile area from the DBS&A quarterly monitoring reports.

The regional water table is in the basement below the stockpiles generally 200 to 500 feet below the stockpiles and is intercepted by the open pits.

5.5 Seismic Coefficient

Accepted geotechnical engineering practice indicates that certain types of embankments such as water retention dams and tailings dams must consider the potential impact of earthquakes on their stability. While there is less agreement among practitioners as to the impact of earthquakes on waste rock dumps and stockpiles, regulatory agencies in some jurisdictions require that earthquake ground motions be explicitly considered in the design of

these facilities (Hawley and Cuning, 2017). For example, New Mexico Copper Rule specifies “a stability analysis be conducted for closure and the mine units being closed shall be designed for a factor of safety of 1.1 or greater under pseudo-static analysis and shall include evaluation for static and seismic induced liquefaction”.

A pseudo-static analysis requires selection of the seismic coefficient, which is estimated as a fraction of the horizontal peak ground acceleration (PGA) that the structure is expected to experience for a specified annual exceedance probability (AEP) or its inverse, return period. The PGA AEP value is developed through probabilistic seismic hazard analysis (PSHA). Based on the previous criteria applied for the stockpile stability assessments at Tyrone, an earthquake ground motion with a 2% probability of exceedance in 50 years (i.e., a 2,475-year return period) is applied for the stockpile stability assessment for closure conditions. The PGA at a site may be influenced by the type of soils overlying the bedrock. The stockpiles at Tyrone are predominantly underlain by competent crystalline bedrock (Burro Mountain Granite and Quartz Monzonite of Tyrone). However, the stockpiles located on the east and north sides of the mining complex (i.e., 1A Leach, 1B Leach, Reclaimed 1C Waste, 3A Leach, 3B Waste, 5A Waste, 9A Waste, and 9AX Waste) are in places underlain by Gila Conglomerate.

Previous stability assessments for Tyrone completed in 2006 and 2007 used the PGA with a 2,500- year return period based on a site-specific PSHA completed by URS Corporation (2005). Earthquake ground motion models and seismic hazard estimation methods, however, have been updated several times since the 2005 site-specific seismic hazard study was completed for Tyrone.

Golder has used the United States Geological Survey (USGS) Unified Hazard Tool to identify the site PGA for use in the development of the pseudo-static seismic coefficient (<https://earthquake.usgs.gov/hazards/interactive/>) and any liquefaction analysis. The USGS Unified Hazard Tool provides PGA and select spectral accelerations for sites from the 2014 US National Seismic Hazard Model (v4.0). For the Tyrone site, Golder used the USGS Unified Hazard Tool to obtain a PGA with a 2% probability of exceedance in 50 years (2,475-year return period) based on the Tyrone site location at 32.657591°North and 108.391275°West.

PGA was calculated, for Burro Mountain Granite and Quart Monzonite, assuming a ground condition equivalent to the ASCE 7-16 soil Site Class B/C boundary or time-averaged shear wave velocity ($V_{S,100}$) of 2,500 feet/second (760 m/s) for the upper 100 ft of the column under the stockpile. The 2,475-year return period PGA obtained from the USGS Unified Hazard Tool is 0.1088 g.

The other areas that are underlain by the Gila Conglomerate, Golder considers the ground conditions beneath these stockpiles are likely equivalent to a soil Site Class C, with $1,200 \text{ ft/s} \geq V_{S,100} \leq 2,500 \text{ ft/s}$. based on the physical descriptions of the Gila Conglomerate. The 0.1088 g PGA calculated for a soil site Class B/C site was converted to a soil Site Class C (i.e., $V_{S,100}$ of 1,760 ft/s) by applying a 1.3 amplification factor as indicated in ASCE-7 16 Table 11.4.1 for a short period site coefficient with S_s of < 0.5 . The resulting site PGA is 0.141 g.

The deaggregation data from the 2014 USGS model is not yet available, so it is not possible at this stage to select directly the dominate earthquake magnitude and distance that contributes the greatest to the 2,475-year return period PGA hazard. For this study, Golder used the deaggregation of the 2008 USGS National model which indicates that a moment magnitude (**M**)6 earthquake at 14 miles (23 km) from the site is the mean magnitude distance earthquake for the 2,475-year return period PGA hazard. There is little difference between the 2008 (0.1079 g) and 2014 (0.1088 g) 2,475-year return period PGA values for the site, so an **M**6 at 23 km can be used to estimate any liquefaction hazard at the site.

The stability analyses of the stockpiles simulate the seismic loading using a pseudo-static coefficient that is applied as a constant horizontal force. Therefore, the pseudo-static coefficient applied in limit equilibrium stability analyses are taken as a fraction of the PGA. Hynes and Franklin (1984) discuss the selection of pseudo-static coefficients for use in dam design and recommend the use of one-half of the peak acceleration with a 20-percent reduction of the shear strength and a target factor of safety of 1.0. Bray et al. (1993) provides recommendations for seismic design of landfills and notes that “the normalized fundamental periods of many solid waste landfills are greater than two-seconds, and that for these cases, the maximum horizontal equivalent acceleration value used to represent the seismic loading will be less than one-half of the bedrock maximum horizontal acceleration.” Jansen (1985) states an acceleration of 0.4 to 0.7 times peak ground acceleration is typically suitable for computing the sustained effect of an earthquake on embankment stability.

For stockpiles underlain by Gila Conglomerate, a seismic acceleration equal to two-thirds of the amplified peak ground acceleration (i.e., 0.094) for an event with a 2,500-year return period is appropriate for the pseudo-static analyses of these facilities. We have conservatively retained the same pseudo-static coefficient for stockpiles underlain by Burro Mountain Granite and Quart Monzonite. Golder believes this approach to be conservative and consistent with standard industry practice.

6.0 STABILITY ANALYSIS METHOD

Golder used the two-dimensional, limit-equilibrium, method of slices analysis program, Slide version 2018 (RocScience, 2018) for the stability analyses. This program provides for various failure surface types, including circular and non-circular (block), and various failure surface search methods. Slide2018 allows the use of multiple methods of analyses. Golder applied Morgenstern-Price’s Method of Slices which satisfies conditions of static horizontal and vertical equilibrium, as well as moment equilibrium.

Analyses considered both circular and block type failure surfaces. Circular failure surface searches were generally used to identify the most critical failure surface (i.e. lowest factor of safety) for failures through the stockpile materials. The circular surfaces also evaluate failures through the stockpile foundation. Block type failures are typically used to identify critical failure surfaces that develop along preferential zones of weakness, such as thin layers of weak alluvium or through liner systems. Where more complex failure modes may occur such as a combination of failure along a weak layer and circular failure through the upper slope, optimized path search methods (Cuckoo method) available in SLIDE2018 are used to identify the most critical failure surface.

The stability analyses cross section output for each analyzed failure mode is included in Appendix VI. The geotechnical units are indicated by colors with the color legend at the front of Appendix VI. The circular failure searches show the limits of the failure searches and the 10 lowest failure surfaces with the factor of safety for the lowest surface reported. Block failure surface search windows shown as red polygons. The perched and regional water table is shown on the stability output as blue lines.

Two-dimensional cross-sectional models were prepared based on pre-mining topography (digitized from early topographic maps), recent aerial surveys, and stockpile reclamation designs prepared by MWH Global (MWH). Stability analyses were performed for the reclaimed stockpile configuration. The geologic units present below the stockpiles is taken from the geological site map.

Stockpiles along the east and south sides of the mine area stockpile complex locally overlie alluvium present in the tributary channels to the Mangas, Brick Kiln, Niagara, and Oak Grove washes. Depths of the alluvium in the foundations shown in the model were based on the conditions encountered by previous geotechnical investigations where available and applying conservative estimates where site specific data is absent. Failure

surface searches evaluated overall slope heights and included focused failure surface searches in the toe area where weak foundation materials are present.

The stockpiles were assumed to be fully drained for the closure conditions analyzed. In alluvium deposits, monitoring wells indicate perched water in the alluvium unit. The quarterly perched alluvial groundwater data from monitoring wells described in Section 5.4.2 were the basis for the perched water table levels used in the stability analyses.

In the sections that have alluvium below the perched water level, the potential for liquefaction was evaluated based on methods described in Section 6.3.

6.1 Selection of Critical Cross Sections

One to two sections were selected for the evaluation of the stability of each stockpile. Selection of the most-critical sections was based on the planned closure facility design slope gradient, slope height, subsurface geology, and hydrogeologic conditions. The critical cross section of each stockpile is described in more detail in Section 7.0. The closure stockpile surficial geology and planned closure geometry is shown in Figures 2 through 5 and sections are presented in Figures 6 through 8.

6.2 Loading Conditions

The stability of the reclaimed stockpile configurations was evaluated considering static conditions and pseudo-static loading cases. Based on the level of detail of the investigation and amount of available laboratory testing, use of average long-term shear strengths, targeting factors of safety as defined by Copper rule and DP-1341 have been applied. For the seismic case, Golder evaluated pseudo-static earthquake loading applying a pseudo-static coefficient of 0.094 as discussed in Section 5.5. A factor-of-safety of 1.5 for critical structures and 1.3 for non-critical structures is considered suitable under the Copper rule for static loading and minimum target factors of safety for pseudo-static loading are 1.1 or greater.

6.3 Evaluation of Liquefaction Potential

The potential for liquefaction of zones of saturated alluvium that locally underlie the toe of the stockpile was assessed using the Seed-Idriss Simplified Liquefaction Procedure (Seed and Idriss, 1971), and the various updates to the method (Boulanger and Idriss, 2014), a stress-based approach that compares the earthquake-induced cyclic stress ratios (CSR) with the cyclic resistance ratios (CRR) of the soil to determine a factor of safety against liquefaction. The Gila Conglomerate is considered to be non-liquefiable. The corrected SPT blow count results for all intervals of alluvium below the “High” perched groundwater level (discussed in Section 5.4.2) were used to develop the CRR. Where water levels were measured in geotechnical boreholes and recorded on the drill logs (e.g. 1B Leach stockpile toe area), these levels were applied for the assessment of the liquefaction potential. Otherwise, the water levels from the quarterly monitoring data was applied (Section 5.4.2).

A design earthquake of magnitude 6.7, imparting a peak amplified acceleration of 0.14 g was applied to determine CSR. Samples of alluvium subjected to grain size testing indicate they are granular soils with fines (<#200 sieve) fraction of 16% to 27%. Therefore, calculation of the $CRR_{7.5}$ applied the 10% fines content curve (Boulanger and Idriss, 2014).

Where liquefaction potential is indicated (factors of safety below approximately 1.3), additional stability analyses were performed to evaluate the potential impacts of seismic loading and liquified conditions during and after shaking. Liquefied material strengths as discussed in Section 5.2.4 were applied for the alluvium below the

“average” perched water table level as described in Section 5.4.2 using static loading. Use of a “high” water table condition and pseudo-static loading is considered to be overly conservative.

The tabulated SPT data and the liquefaction potential calculations are contained in Appendix V.1 through V.3 with the associated boring logs. Where the SPT tests were above the modeled “high” water table condition the alluvium is considered non-liquefiable. The water levels that are currently being measured in wells near the leach stockpile toes are mainly leach solutions. Water table levels are expected to be further reduced at closure when leaching ceases and the stockpiles are capped and surface water controls are in place. Therefore, the perched water table conditions applied in the assessment of liquefaction are considered conservative.

7.0 STABILITY ANALYSIS RESULTS

The results of the stability analyses provided in this section were based on the parameters and methods described in the preceding sections. All the calculated factors of safety were found to be above the minimum required factor of safety criterion, and the stockpiles are predicted to maintain long-term stability for the planned closure geometries. Table 5 summarizes the minimum factors of safety obtained for each stockpile for static and pseudo-static loading conditions. Where alluvium is present underlying the toe area of the reclaimed stockpiles and SPT blow count data is lacking or indicates some potential for liquefaction exists, the factor of safety assuming liquefied strengths for alluvium below the water table is reported.

The following sections describe the individual stability models prepared for each stockpile, the analyses completed and resulting factors of safety. The factors of safety are provided for each of the failure surface search methods analyzed (e.g. block, circular, path). Output from all stability analysis models are provided in Appendix VI.

Unless otherwise noted the reclaimed slope geometry consists of overall 3.5H to 1V overall slopes consisting of 3H to 1V interbench slopes and benches spaced every 200 feet of slope length.

Table 5: Stability Analysis Results Summary

Stockpile	Minimum Static FOS	Minimum Pseudo-static FOS	Liquefied FOS
1A Leach	2.65	2.01	2.01
1B Leach	1.95	1.32	No liquefiable soils present
Reclaimed 1C Waste	3.52	2.40	1.56
2A Leach	2.78	2.02	No liquefiable soils present
2B Waste	3.45	2.54	No liquefiable soils present
3A Leach	1.85	1.63	1.51
3B Waste	5.80	2.15	No liquefiable soils present
2 Leach (Area 1)	2.56	1.92	No liquefiable soils present
2 Leach (Area 2)	2.28	1.70	No liquefiable soils present

Stockpile	Minimum Static FOS	Minimum Pseudo-static FOS	Liquefied FOS
(San Salvador Waste Backfill, 7B Leach)			
7B Waste	2.45	1.86	No liquefiable soils present
7B Waste and 7B Leach (South Rim Pit)	2.47	1.79	NA
5A Waste	2.33	1.71	1.71
6B Leach	2.98	2.20	No liquefiable soils present
6D Waste	3.00	2.24	No liquefiable soils present
Reclaimed 7A Waste (eastern)	3.16	2.33	No liquefiable soils present
Reclaimed 7A Waste (central)	3.13	2.44	No liquefiable soils present
Reclaimed 7A Waste (western)	3.22	2.37	No liquefiable soils present
9A Waste	3.31	2.43	No liquefiable soils present
9AX Waste	2.83	2.24	1.42
West In-Pit Waste (at Little Rock)	2.55	1.88	No liquefiable soils present
North In-Pit Waste (at Little Rock)	2.56	1.95	No liquefiable soils present

7.1 1A Leach, 1B Leach, and Reclaimed 1C Waste Stockpiles

Stockpiles 1A Leach, 1B Leach, and Reclaimed 1C Waste are located in the southeast area of the Tyrone mine. Highway 90 runs north-south along the east of the 1A Leach Stockpile, a minimum of 300 feet from the toe of the 1A Leach stockpile (Figure 2). The critical sections and descriptions of the analyses are described in the following sections. The minimum factors of safety results from the SLIDE2 models are summarized in Table 6.

7.1.1 1A Leach Stockpile

The 1A Leach stockpile is bounded by stockpiles 1B Leach and Reclaimed 1C Waste to the north and south, respectively and the Gettysburg Pit to the west (Figure 1). The 1A Leach stockpile has an overall slope height of approximately 340 feet. The reclamation slopes are being developed by holding the toe and mining the stockpile back to the reclamation configuration.

The critical stability section was selected to run through a finger of alluvium that extends up a tributary of Brick Kiln Wash under the stockpile. The location of the critical cross-section is shown on Figure 2 and the stability

cross section model is shown on Figure 7. The alluvium is estimated to be approximately 10 to 40 feet deep under the stockpile and saturated below depths of approximately 25 feet based on the quarterly groundwater monitoring data. Stability analyses included overall circular failure, block failure along the alluvium, and local stability in the toe area overlying the alluvium.

The potential for development of instability related to liquefaction was assessed. Site specific geotechnical borehole data and SPT blow count data are lacking in this zone of alluvium. A stability analysis was completed assuming loose sands (i.e. low blow count) are present in the alluvium below the perched (average) groundwater level. A liquefied strength was applied to these soils and the stability was computed applying pseudo-static loading conditions. The underlying native material below the alluvium is Gila conglomerate. The occurrence of liquefaction does not lower the factor of safety because of the distance from the stockpile toe to the zone of saturated alluvium.

Results of the stability analyses are summarized in Table 5. The most critical failure surface would be a global failure from crest to toe of the slope and has a minimum static factor of safety is 2.65 and the minimum pseudo-static factor of safety is 2.01.

7.1.2 1B Leach Stockpile

The 1B Leach stockpile is bounded by the 1A Leach stockpile on the south, 5A Waste stockpile to the north, and the Gettysburg Pit to the southwest (Figure 2). The 1B Leach stockpile has a maximum stockpile height of approximately 475 feet. The reclamation slope flattening is being accomplished by mining the upper slopes and placing material at the toe in compacted lifts.

The critical stability cross-section was selected to run through the relocated 1B Leach main seepage collection system. Previous stability analyses of the 1B Leach Stockpile (Golder, 2006b, Golder, 2008b) modeled the stockpile placed above low strength clay accumulations in a storm water sedimentation pond. As-built documentation Golder 2007h indicates the pond sediments were removed and replaced with common fill. A geomembrane-lined collection area is located below the final reclaimed toe of the 1B Leach stockpile. The collection system is underlain by native alluvium. The alluvium along this section is estimated to be approximately 25 feet, thickening to 50 feet thick toward the northeast and saturated below depths of approximately 25 feet.

Geotechnical drill data collected to support the relocation of the seepage collection pond was available to assess subsurface conditions. The location of the boreholes is shown on Figure 2. Table V.1-1 in Appendix V.1 provides the factors of safety against liquefaction for all SPT tests from these drill holes. Drill logs for these boreholes are included in Appendix V. A factor of safety of 1.5 against liquefaction was obtained from borehole GA-05-01 at a depth of 15 feet. The material was logged as fill (pond sediments) which as-built documentation shows was removed and replaced with compacted fill. Borehole GA-05-02 has a factor of safety of 1.0 at a depth of 10 feet. Borehole logs note the test encountered a void (PLS line). Borehole GA-05-05 has a factor of safety of 1.5 at a depth of 17.5 feet in material logged as PLS pond sediment which was also removed during relocation of the PLS Collection System. All other SPT intervals yielded factors of safety greater than 2.0 and there is a very low potential for liquefaction to occur. Therefore, no stability analyses that apply liquefied strength for alluvium below the perched water table are reported for this section.

The critical stability section evaluates potential sliding along the geomembrane interface applying a conservative interface friction angle $\phi = 10^\circ$. This section also represents the greatest overall slope height.

The most critical failure modes are global failure from crest to toe of the slope and a small localized failure through the toe along the geomembrane. The minimum static factor of safety is 1.95 and the minimum pseudo-static factor of safety is 1.32.

7.1.3 Reclaimed 1C Waste Stockpile

The Reclaimed 1C waste stockpile is bounded by stockpiles 1A Leach and 7A East (Reclaimed 7A Waste) to the north and southwest, respectively, and the Gettysburg Pit to the northwest (Figure 1 and 2). The southeast slope has a crest elevation of 6260 feet and slopes at 3.9H to 3.5H:1V with a maximum stockpile height of 250 feet. The eastern and northern slopes have crest elevations of 6260 feet and overall slopes of 3.5H:1V and was reclaimed using “ridge-valley” reclamation grading. The Reclaimed 1C Stockpile was originally placed over Oak Grove Wash and the toe was mined-back to the northwest up to 1,200 feet prior to 2004 and prior to the reclamation “ridge-valley configuration” that was completed in approximately 2012.

The critical section was selected to run through a deposit of alluvium that extends up a drainage under the stockpile. The location of the critical section is shown on Figure 2 and the cross-section model is shown on Figure 7. The alluvium is estimated to be approximately 15 to 20 feet deep. Stability analyses included overall circular failure, block failure along the alluvium, and local stability in the toe area overlying the alluvium. The underlying native material is Quartz monzonite under the main heap with a local exposure of Precambrian granite.

The most critical failure surface is a small localized failure through the toe. The minimum static factor of safety is 3.52 and the minimum pseudo-static factor of safety is 2.40.

Due to the fact that the Reclaimed 1C waste stockpile was mined back to the current toe, the alluvium below the current toe was previously under much higher stockpile loads and it could be reasoned that the alluvium is non-liquefiable due to the high loads the alluvial soils have been subject to. However, the potential for development of instability related to liquefaction was assessed. Site specific geotechnical borehole data and SPT blow count data are lacking in this zone of alluvium. A stability analysis was completed assuming loose sands (i.e. low blow count) are present in the alluvium below the perched (average) groundwater level. A liquefied strength was applied to these soils and the stability was computed using pseudo-static loading conditions. The underlying native material is Gila conglomerate. In the scenario in which the alluvium liquefies below the water table, the minimum pseudo-static factor of safety is 1.56.

Table 6: 1A Leach, 1B Leach and Reclaimed 1C Waste Stability Analysis Summary

Stockpile	Failure Type	Static Condition	Pseudo-static Condition (k = 0.094g)	
			No liquefaction	Liquefaction below average perched water table
1A Leach	Block	2.70	2.04	2.04
	Circular	2.65	2.01	2.01
1B Leach	Block	2.82	2.08	No liquefiable soils present
	Circular	2.01	2.06	

Stockpile	Failure Type	Static Condition	Pseudo-static Condition ($k = 0.094g$)	
			No liquefaction	Liquefaction below average perched water table
	Block Liner	1.95	1.32	
Reclaimed 1C Waste	Block	3.52	2.40	1.56
	Circular	3.73	2.59	2.01

7.2 2A Leach and 2B Waste Stockpiles

Stockpiles 2A Leach and 2B Waste are located in the northwest portion of the Tyrone mine area (Figure 5). Deadman Canyon runs north-south along the west of the stockpiles. The critical sections and descriptions of the analyses are described in the following sections. The factor of safety results from the SLIDE models are summarized in Table 7.

7.2.1 2A Leach Stockpile

The 2A Leach stockpile is bounded by stockpiles 9A Waste to the north, 2B Waste to the south, and the Valencia Pit to the east (Figures 1 and 5). The eastern slope has a crest elevation of 6300 feet. Two critical stability sections were selected to run through the west (2A-1) and east (2A-2) slopes of the stockpile perpendicular to the topography. The location of the critical cross-section is shown on Figure 5 and the stability cross section model is shown on Figure 7. The 2A Leach stockpile has an overall slope height of approximately 395 feet. The reclamation slopes are being developed primarily by holding the toe and mining the stockpile back to the reclamation configuration.

Stability analyses included overall circular failure and block type failure. The most critical failure surface is a global failure from crest to toe of the slope and has a minimum static factor of safety is 2.78 and the minimum pseudo-static factor of safety is 2.02.

7.2.2 2B Waste Stockpile

The 2B Waste stockpile is bounded by the 2A Leach stockpile on the north and the Copper Mountain Pit and 2B Leach to the south (Figure 1). The eastern and western slopes have a crest elevation of 6450 feet and a maximum stockpile height of 375 feet with a 200-foot wide bench at elevation 6350 feet breaking the slope into a 150-foot high upper slope and a 200-foot high lower slope.

The critical stability section was selected to run perpendicular to the topography at its greatest slope height. The location of the critical cross-section is shown on Figure 5 and the stability cross section model is shown on Figure 7. Stability analyses included overall circular failure and block type failure. The most critical failure surface is a global failure of the lower bench from bench crest to toe of the slope. The minimum static factor of safety is 3.45 and the minimum pseudo-static factor of safety is 2.54.

Table 7: 2A Leach and 2B Waste Stability Analysis Summary

Stockpile	Failure Type	Static Condition	Pseudo-static Condition ($k = 0.094g$)
2A-1 Leach	Block	2.89	2.10
	Circular	2.87	2.09
2A-2 Leach	Block	2.80	2.05
	Circular	2.78	2.02
2B Waste	Block	3.63	2.64
	Circular	3.45	2.54

7.3 3A Leach and 3B Waste Stockpiles

Stockpiles 3A Leach and 3B Waste stockpiles are located in the northeast Tyrone mine Area (Figure 4). Brick Kiln Gulch runs north-south along the northeast side of the stockpiles. The critical sections and descriptions of the analyses are described in the following sections. The engineering FOS results from the SLIDE models are summarized in Table 8.

7.3.1 3A Leach Stockpile

The 3A Leach stockpile is bounded by the 3B Waste stockpile on the south and the Brick Kiln Gulch to the northwest (Figure 1). The northern slope has a crest elevation of 6300 feet with a maximum stockpile height of approximately 640 feet. The toe of the slope in the alluvium is locally steepened to accommodate the pond systems. The reclamation slope flattening will be accomplished by a combination of by holding the toe fixed and mining the crest back and placement of material at the toe in compacted lifts.

The critical stability section was selected to run through a finger of alluvium that extends up a tributary of Mangas Wash under the stockpile and through the existing process ponds at the toe. The location of the critical cross-section is shown on Figure 4 and the stability cross section model is shown on Figure 7. The alluvium is estimated to be approximately 20 to 70 feet deep under the stockpile and saturated below depths of 20 feet based on water levels in nearby wells. Stability analyses included overall circular failure, block failure along the alluvium, and local stability in the toe area overlying the alluvium. The most critical failure surface is a localized failure through the toe of the slope in the localized steepened section and has a minimum static factor of safety is 1.85 and the minimum pseudo-static factor of safety is 1.63.

The potential for development of instability related to liquefaction was assessed. Site specific geotechnical borehole data and SPT blow count data are lacking in this zone of alluvium. However, geotechnical borehole data is available in the alluvium in several nearby tributary channels as shown on Figure 4. The results of liquefaction potential analyses are provided in Appendix V.3 and show the factors of safety against liquefaction are greater than 1.6 and the potential for liquefaction is low. Due to the lack of site-specific geotechnical borehole data, a stability analysis was completed assuming loose sands (i.e. low blow count) are present in the alluvium below the estimated perched groundwater level. A liquefied strength was applied to these soils and the stability was

computed using pseudo-static loading conditions. The underlying native material below the alluvium is Gila Conglomerate. In the scenario in which the alluvium liquefies below the water table, the minimum pseudo-static factor of safety is 1.51.

7.3.2 3B Waste Stockpile

The 3B Waste stockpile is bounded by the 3A Leach stockpile on the north and the Main Pit to the southwest (Figure 1). The eastern slope has a crest elevation of 6320 feet and a maximum stockpile height of 485 feet with a 100-foot bench at elevation 6000 feet breaking the slope into a 350-foot high upper slope and a 100-foot high lower slope. Stockpile 5A Waste lies to the east of the critical section. The reclamation slope flattening will be accomplished by constructing at the final reclaimed slope angles.

The critical stability section was selected to run through a finger of alluvium that extends up a tributary of Mangas Wash under the stockpile. The location of the critical cross-section is shown on Figure 2 and the stability cross section model is shown on Figure 7. The alluvium is estimated to be approximately 10 to 40 feet deep under the stockpile and saturated below depths of 30 feet based on the quarterly monitoring data. As the alluvium is not under the toe of the slope and is under stockpile height of approximately 100 feet at confining loads that would preclude liquefaction. Therefore, the stability applying liquefied strengths for the saturated alluvium was not assessed. Stability analyses included overall circular and block type failure. The most critical failure surface is a shallow local failure through the lower bench and has a minimum static factor of safety is 5.80 and the minimum pseudo-static factor of safety is for a block failure in the upper slope with a factor of safety of 2.24.

Table 8: 3A Leach and 3B Waste Stability Analysis Results

Stockpile	Failure Type	Static Condition	Pseudo-static Condition (k = 0.094g)	
			No liquefaction	Liquefaction below average perched water table
3A Leach	Block	2.26	1.96	1.97
	Circular	1.85	1.63	1.51
3B Waste	Block	7.65	2.15	No liquefiable soils present
	Circular	5.80	4.08	

7.4 2 Leach, Waste Backfill, 7B Leach, and 7B Waste Stockpiles

Stockpiles 2 Leach (Area 1 and 2), San Salvador Waste Backfill, and South Rim Pit (7B Waste and 7B Leach) are located in the south Tyrone mine area (Figure 3). The critical sections and descriptions of the analyses are described in the following sections. The engineering FOS results from the SLIDE models are summarized in Table 9.

7.4.1 2 Leach (Area 1) Stockpiles

The 2 Leach (Area 1) stockpile is bounded by the Copper Mountain Pit to the north and stockpile the San Salvador Waste Backfill to the east. (Figure 1). The western and southern slopes have a crest elevation of 6725

feet and overall slope height of approximately 200 to 250 feet. The reclamation slope flattening is being accomplished by mining the upper slopes and placing material at the toe in compacted lifts.

Two critical stability sections were selected to run through the east and west slopes of the stockpile perpendicular to the topography. Section 2-1 evaluates the stability of the western slope, and Section SS evaluates the eastern slope through the backfilled San Salvador Pit. The location of the critical cross-section is shown on Figure 3 and the stability cross section model is shown on Figure 8. Stability analyses included overall circular failure and block type failure. The most critical failure surface in 2-1 is a global failure of the lower bench from bench crest to toe of the slope and in SS is a failure of the upper bench from stockpile crest to the bench. The minimum static factor of safety is 2.56 and the minimum pseudo-static factor of safety is 1.92.

7.4.2 2 Leach (Area 2) Stockpile

The 2 Leach (Area 2) ore stockpile was placed over the backfilled San Salvador Pit and bounded by the 7B Leach stockpile on the east and the Valencia Pit to the north and the Copper Mountain Pit to the west (Figure 1). The southern slope has a crest elevation of 6700 feet and overall slope height of 325 feet. The reclamation slope flattening is being accomplished by mining the upper slopes and placing material at the toe in compacted lifts.

The critical stability section was selected to run perpendicular to the topography at its greatest height and cuts through the backfilled San Salvador Pit and 7B Leach approximately 150 ft from the toe of the slope. The location of the critical cross-section 7B-2 is shown on Figure 3 and the stability cross section model is shown on Figure 8. Stability analyses included overall circular failure and block type failure. The most critical failure surface is a global failure from crest to toe of the slope and has a minimum static factor of safety is 2.78 and the minimum pseudo-static factor of safety is 2.03.

7.4.3 7B Leach Stockpile

The 7B Leach stockpile was originally placed over the backfilled South Rim Pit and is bounded by the 2C Leach and Reclaimed 7A Waste stockpiles on the north and south and 6B Leach 6C Leach, and 6D Leach stockpiles to the east (Figure 1). The southern slope has a crest elevation of 6700 feet and has a maximum stockpile height of 325 feet. The reclamation slope flattening is being accomplished by mining the upper slopes and placing material at the toe in compacted lifts.

The critical stability section 7B-1 was selected to run through the east and west slopes of the stockpile perpendicular to the topography and through the backfilled South Rim Pit. The location of the critical cross-section is shown on Figure 3 and the stability cross section model is shown on Figure 8. Stability analyses included overall circular failure and block type failure. The most critical failure surface would be a global failure of the leached ore from crest to toe of the slope. The minimum static factor of safety is 2.45 and the minimum pseudo-static factor of safety is 1.86.

7.4.4 San Salvador Waste Backfill

One stockpile in the San Salvador Pit is located in the south Tyrone mine area. The engineering factor of safety results from the SLIDE models are summarized in Table 9.

The San Salvador Waste Backfill is bounded by the 2 Leach (Area 2), Reclaimed 7A Waste and 2 Leach (Area 1) stockpiles on the north, east, and west and the backfilled South Rim Pit to the northeast (Figure 3). The backfilled slopes have a crest elevation of 6700 in the upper leached ore slope and 6490 feet in the lower waste slopes. The waste stockpile has a maximum stockpile slope height of approximately 240 feet. The reclamation slope flattening is being accomplished by mining the upper slopes and placing material at the toe in compacted lifts.

The critical stability section SS and 2-2 was selected to run perpendicular to the topography at its greatest height. The location of the critical cross-sections are shown on Figure 3 and the stability cross section model is shown on Figure 9. Stability analyses included overall circular failure and block type failure. The most critical failure surface would be a global failure of the upper leached ore bench from crest to toe or of the lower waste rock stockpile. The minimum static factor of safety is 2.28 and the minimum pseudo-static factor of safety is 1.70.

7.4.5 7B Waste Stockpile

One stockpile in the South Rim Pit is located in the south Tyrone mine area (Figure 3). The critical sections and descriptions of the analyses are described in the following sections. The engineering factor of safety results from the SLIDE models are summarized in Table 9.

The 7B Waste stockpile backfilled in the South Rim Pit and is bounded by the 2 Leach (Area 2) and Reclaimed 7A Waste stockpile on the north and east and the San Salvador Pit to the southwest (Figure 3). The backfilled slopes have a crest elevation of 6700 in the upper leached ore slope and 6550 feet in the lower waste slopes. Overall slopes are a maximum height of 230 feet and there is a 200-foot bench at elevation 6400, breaking the slope into an approximately 150-foot-high upper slope and a 100-foot lower slope. The reclamation slopes will be accomplished by constructing to the reclamation slope angles.

The critical stability section SR was selected to run perpendicular to the topography at its greatest height. The location of the critical cross-section is shown on Figure 3 and the stability cross section model is shown on Figure 9. Stability analyses included overall circular failure and block type failure. The most critical failure surface would be a global failure of the upper waste rock bench from crest to bench. The minimum static factor of safety is 2.47 and the minimum pseudo-static factor of safety is 1.79

Table 9: 2 Leach, 7B Leach, and 7B Waste Stability Analysis Summary

Stockpile	Failure Type	Static Condition	Pseudo-static Condition (k = 0.094g)
7B-1 Waste	Block	2.54	1.93
	Circular	2.45	1.86
7B-2 Leach	Block	2.82	2.06
	Circular	2.78	2.03
2 Leach, Area 1 (2-1)	Block	2.63	1.96
	Circular	2.56	1.92
2 Leach, Area 2 (2-2)	Block	2.55	1.87
	Circular	2.28	1.70
San Salvador Waste Backfill	Block	2.76	2.08
	Circular	2.71	2.04

Stockpile	Failure Type	Static Condition	Pseudo-static Condition (k = 0.094g)
South Rim Pit (7B Waste, 7B Leach)	Block	2.47	1.79
	Circular	2.48	1.79

7.5 5A Waste Stockpile

Stockpile 5A Waste is located in the northeast Tyrone mine area (Figure 4). Mangas Wash runs northeast along the east side of the stockpiles. The south portion of the 5A Waste stockpile is designated here as the 5A-2 Waste. The critical sections and descriptions of the analyses are described in the following sections. The engineering FOS results from the SLIDE models are summarized in Table 10.

7.5.1 5A Waste Stockpile North

The 5A Waste stockpile is bounded by the 3B Waste stockpile on the north and the Main Pit to the southwest (Figure 4). Mangas Wash lies to the northeast. The northern portion of the 5A Waste stockpile has been designated with a representative cross section 5A-1 Waste in this report. The eastern and northern slope has a crest elevation of 6240 feet with a maximum stockpile height of 375 feet. The reclamation slope flattening is being accomplished by mining the upper slopes and placing material at the toe in compacted lifts.

The critical stability section was selected to run through a finger of alluvium that extends up a tributary of Brick Kiln Gulch under the stockpile and several mine facilities lie at the toe. The location of the critical cross-section is shown on Figure 4 and the stability cross section model is shown on Figure 8. The alluvium is estimated to be approximately 40 to 70 feet deep under the stockpile and saturated below depths of 30 feet. Stability analyses included overall circular failure, block failure and a path search to evaluate the local stability in the toe area overlying the alluvium. The critical failure surface is a global failure from the crest to the toe of the slope through the underlying alluvium and has a minimum static factor of safety is 2.33 and the minimum pseudo-static factor of safety is 1.71.

The potential for development of instability related to liquefaction was assessed. Site specific geotechnical borehole data and SPT blow count data are lacking in this zone of alluvium. A stability analysis was completed assuming loose sands (i.e. low blow count) are present in the alluvium below the perched (average) groundwater level. A liquefied strength was applied to these soils and the stability was computed using pseudo-static loading conditions. The underlying native material is Gila conglomerate. In the scenario in which the alluvium liquefies below the water table, the critical failure surface is a localized toe failure through the liquefied alluvium and has a minimum pseudo-static factor of safety is 1.71. Liquefaction does not result in a reduction in the pseudo-static factor of safety because of the distance of the liquefiable alluvium from the toe of the slope.

7.5.2 5A Waste Stockpile South

The southern portion of the 5A Waste stockpile has been designated with a representative cross section 5A-2 Waste in this report. It is bounded by the 5A Waste stockpile on the north and the Main Pit to the southwest (Figure 4). Mangas Wash lies to the northeast. The eastern and northern slope has a crest elevation of 6240 feet and a maximum stockpile height of 360 feet. The reclamation slope flattening is being accomplished by mining the upper slopes and placing material at the toe in compacted lifts.

The critical stability section was selected to run through a finger of alluvium that extends up a tributary of Mangas Wash under the stockpile. The location of the critical cross-section is shown on Figure 4 and the stability cross section model is shown on Figure 8. The alluvium is estimated to be approximately 20 to 60 feet deep under the stockpile and saturated below depths of 35 feet. Stability analyses included overall circular failure, block failure along the alluvium, and local stability in the toe area overlying the alluvium. The most critical failure surface would be a global failure from crest to toe of the slope or a block type, shallow, localized failure through the underlying alluvium at the toe of the stockpile. The minimum static factor of safety is 2.65 and the minimum pseudo-static factor of safety is 1.94.

The potential for development of instability related to liquefaction was assessed. Site specific geotechnical borehole data and SPT blow count data are lacking in this zone of alluvium. A stability analysis was completed assuming loose sands (i.e. low blow count) are present in the alluvium below the perched (high) groundwater level. A liquefied strength was applied to these soils and the stability was computed using static and pseudo-static loading conditions. The underlying native material is Gila conglomerate. In the scenario in which the alluvium liquefies below the water table, the minimum static factor of safety is 2.65 and the minimum pseudo static factor of safety is 1.94.

Table 10: 5A Waste Stability Analysis Summary

Stockpile	Failure Type	Static Condition	Pseudo-static Condition ($k = 0.094g$)	
			No liquefaction	Liquefaction below average perched water table
5A-1 Waste	Cuckoo	2.33	1.71	1.71
	Circular	2.42	1.75	1.75
5A-2 Waste	Block	2.80	2.06	2.06
	Circular	2.65	1.94	1.94

7.6 6B Leach and 6D Leach Stockpiles

Stockpiles 6B Leach and 6D Leach stockpiles are located in the south Tyrone mine area (Figure 2). The critical sections and descriptions of the analyses are described in the following sections. The factor of safety results from the SLIDE models are summarized in Table 11.

7.6.1 6B Leach Stockpile

The 6B Leach stockpile is bounded by the Valencia Pit and Gettysburg Pit to the north and east and stockpiles 7B Leach and 6D Leach to the west and south. (Figure 1). The southern slope has a crest elevation of 6450 feet and overall maximum stockpile slope height of 100 feet. The reclamation slope flattening is being accomplished by mining the upper slopes and placing material at the toe in compacted lifts.

The critical stability section was selected to run perpendicular to the topography at its greatest height. The location of the critical cross-section is shown on Figure 2 and the stability cross section model is shown on Figure 8. Stability analyses included overall circular failure and block type failure. The most critical failure surface is a

global failure of the upper slope from crest to toe. The minimum static factor of safety is 2.98 and the minimum pseudo-static factor of safety is 2.20.

7.6.2 6D Leach Stockpile

The 6D Leach stockpile is bounded by the stockpile 6B Leach and Gettysburg Pit to the north and east and stockpiles 7B Leach and 7B Waste to the west and south. (Figure 2). The southern slope has a crest elevation of 6720 feet and with a maximum stockpile slope height of approximately 115 feet. The reclamation slope flattening will be accomplished by mining the upper slopes and placing material at the toe in compacted lifts.

The critical stability section was selected to run perpendicular to the topography at its greatest height. The location of the critical cross-section is shown on Figure 2 and the stability cross section model is shown on Figure 8. Stability analyses included overall circular failure and block type failure. The most critical failure surface is a global failure of the upper slope from crest to toe. The minimum static factor of safety is 3.00 and the minimum pseudo-static factor of safety is 2.24.

Table 11: 6B Leach and 6D Leach Stability Analysis Summary

Stockpile	Failure Type	Static Condition	Pseudo-static Condition (k = 0.094g)
6B Leach	Block	3.08	2.28
	Circular	2.98	2.20
6D Leach	Block	3.09	2.28
	Circular	3.00	2.24

7.7 Reclaimed 7A Waste Stockpiles

Three stockpiles consisting of 7A Waste are located in the south Tyrone mine area (Figures 2 and 3). The critical sections and descriptions of the analyses are described in the following sections. The engineering factor of safety results from the SLIDE models are summarized in Table 12.

7.7.1 Eastern Reclaimed 7A Waste Stockpile

The eastern Reclaimed 7A Waste closure stockpile is bounded by stockpiles Reclaimed 1C Waste and 7B Waste to the northeast and north, respectively (Figure 2). The south-eastern slope has a crest elevation of 6260 feet and with overall slopes of 3.9H to 3.5H:1V with a maximum stockpile height of 230 feet. The Oak Grove Wash has extending tributaries below the footprint of the stockpile. However, the alluvium is under stockpile thickness of 100 feet or more and is not considered to be susceptible to liquefaction under those confining loads. The reclamation slope flattening is being accomplished by mining the upper slopes and placing material at the toe in compacted lifts.

The critical stability section 7A-E was selected to run perpendicular to the topography at its greatest height. The location of the critical cross-section is shown on Figure 2 and the stability cross section model is shown on Figure 8. Stability analyses included overall circular failure and block type failure. The most critical failure surface would

be a global failure from crest to toe of the slope. The minimum static factor of safety is 3.16 and the minimum pseudo-static factor of safety is 2.33.

7.7.2 Central Reclaimed 7A Waste Stockpile

The central Reclaimed 7A Waste stockpile is bounded by the Reclaimed 7A Waste and 7B Waste stockpiles on the east and north, and the South Rim pit to the west (Figures 2 and 3). The southern slope has a crest elevation of 6410 feet and overall stockpile slope height of approximately 190 feet. The reclamation slope flattening is being accomplished by mining the upper slopes and placing material at the toe in compacted lifts.

The critical stability section 7A-W was selected to run perpendicular to the topography at its greatest height. The location of the critical cross-section is shown on Figure 3 and the stability cross section model is shown on Figure 8. Stability analyses included overall circular failure and block type failure. The most critical failure surface would be a global failure from crest to toe of the slope. The minimum static factor of safety is 3.13 and the minimum pseudo-static factor of safety is 2.44.

7.7.3 Western Reclaimed 7A Waste Stockpile

The western Reclaimed 7A Waste western stockpile is bounded by the South Rim Pit to the north (Figure 3). The eastern and northern closure slopes have a crest elevation of 6450 feet and a maximum stockpile height of 210 feet. The eastern, southern, and northern slopes have crest elevations of 6440 feet and overall slopes of 3.5H:1V and were developed using “ridge-valley” reclamation grading. The stockpile was placed just north of the Oak Grove Wash.

The critical stability section 7A-FW was selected to run perpendicular to the topography at its greatest height. The location of the critical cross-section is shown on Figure 3 and the stability cross section model is shown on Figure 8. Stability analyses included overall circular failure and block type failure. The most critical failure surface would be a global failure from crest to toe of the slope. The minimum static factor of safety is 3.22 and the minimum pseudo-static factor of safety is 2.37.

Table 12: Reclaimed 7A Waste Stability Analysis Summary

Stockpile Section	Failure Type	Static Condition	Pseudo-static Condition (k = 0.094g)
Reclaimed 7A Waste (7A-E)	Block	3.30	2.43
	Circular	3.16	2.33
Reclaimed 7A Waste (7A-W)	Block	3.37	2.49
	Circular	3.13	2.44
Reclaimed 7A Waste (7A-FW)	Block	3.27	2.42
	Circular	3.22	2.37

7.8 9A Waste and 9AX Waste Stockpiles

Stockpiles 9A Waste and 9AX Waste stockpiles are located in the northwest of the Tyrone mine area (Figure 5). The critical sections and descriptions of the analyses are described in the following sections. The engineering factor of safety results from the SLIDE models are summarized in Table 13.

7.8.1 9A Waste Stockpile

The 9A waste stockpile is bounded by the 2A Leach stockpile on the south (Figure 5). The northern slope has a crest elevation of 6360 feet and a maximum stockpile height of 500 feet. The reclamation grading is being accomplished by constructing the stockpile at the reclamation slope angle.

The critical stability section was selected to run perpendicular to the topography at its greatest height. The location of the critical cross-section is shown on Figure 5 and the stability cross section model is shown on Figure 8. Stability analyses included overall circular failure and block type failure. The most critical failure surface would be a local, shallow failure from the middle of the slope to toe roughly parallel the contours of the native ground. The minimum static factor of safety is 3.31 and the minimum pseudo-static factor of safety is 2.40.

7.8.2 9AX Waste Stockpile

The 9AX waste stockpile is bounded by the 9A Waste stockpile on the south and the 3A Leach stockpile lies to the east (Figure 5). Tributaries of the Mangas Wash extend from the north under the toe of the stockpile. The western, eastern and northern slopes have a crest elevation of 6080 feet and a maximum stockpile height of 265 feet. The reclamation grading is being accomplished by constructing the stockpile at the reclamation slope angle.

Two critical stability sections were selected to run through the north and east slopes of the stockpile perpendicular to the topography. Section 9AX-1 was selected as the critical section for the northern slope and 9AX-2 was selected to run through a small section of potentially liquefiable alluvium in the eastern slope. The location of the critical cross-sections is shown on Figure 5 and the stability cross section models are shown on Figure 9. The alluvium is estimated to be approximately 10 to 25 feet deep in the tributaries and saturated below depths of 10 feet. Stability analyses included overall circular failure, block failure along the alluvium, and local stability in the toe area overlying the alluvium. The most critical failure surface for Section 9AX-1 is a global failure from crest to toe of the slope and for Section 9AX-2 is a global failure through native Gila conglomerate outcrop and terminating in the alluvium at the toe of the slope. The minimum static factor of safety is 2.83 and the minimum pseudo-static factor of safety is 2.24.

The potential for development of instability related to liquefaction was assessed. Site specific geotechnical borehole data and SPT blow count data are lacking in this zone of alluvium. A stability analysis was completed assuming loose sands (i.e. low blow count) are present in the alluvium below the perched groundwater level. A liquefied strength was applied to these soils and the stability was computed using static and pseudo-static loading conditions. The underlying native material is Gila conglomerate. In the scenario in which the alluvium liquefies below the water table, the most critical failure surface is a shallow, localized toe failure through the liquefied alluvium and has a minimum static factor of safety is 2.83 and the minimum pseudo-static factor of safety is 1.42.

Table 13: 9A Waste and 9AX Waste Stability Analysis Summary

Stockpile	Failure Type	Static Condition	Pseudo-static Condition (k = 0.094g)	
			No liquefaction	Liquefaction below average perched water table
9A Waste	Block	3.42	2.46	No liquefiable soils present
	Circular	3.31	2.40	
9AX Waste (1 of 2)	Block	3.10	2.28	No liquefiable soils present
	Circular	3.04	2.24	
9AX Waste (2 of 2)	Block	3.12	2.32	2.30
	Circular	2.83	2.26	1.42

7.9 Little Rock In-Pit Stockpiles

Two stockpiles in the Little Rock pit are located to the west of the Tyrone mine (Figure 6). Deadman Canyon runs north-south along the east of the pit. The critical sections and descriptions of the analyses are described in the following sections. The engineering factor of safety results from the SLIDE models are summarized in Table 14.

7.9.1 West In-pit Waste Stockpile

The West In-Pit Waste stockpile is contained to the north, south, and west by the pit walls (Figure 6). The eastern and northern slope has a crest elevation of 6200 feet and a maximum stockpile height of 560 feet. At closure, a pit lake may be allowed to form up to an estimated elevation of 5700 ft.

The critical stability section was selected to run perpendicular to the topography at its greatest height. The location of the critical cross-section is shown on Figure 6 and the stability cross section model is shown on Figure 9. The presence of a pit lake has the potential to affect the stability of the waste stockpile, so analyses included two conditions: one with a pit lake at 5700 ft and one with dry pit lake conditions. Stability analyses included overall circular failure and block type failure. The most critical failure surface would be a global failure from crest to toe of the slope. The minimum static factor of safety is 2.55 and the minimum pseudo-static factor of safety is 1.88.

7.9.2 North In-pit Waste Stockpile

The North In-Pit Waste stockpile is contained to the north and east by the pit walls (Figure 1). The eastern and northern slope has a crest elevation of 6140 feet and overall slopes height of 310 feet. At closure, a pit lake may be allowed to form up to an estimated elevation of 5700 ft.

The critical stability section was selected to run perpendicular to the topography at its greatest height. The location of the critical cross-section is shown on Figure 6 and the stability cross section model is shown on Figure 9. The presence of a pit lake has the potential to affect the stability of the waste stockpile, so analyses included

two conditions: one with a pit lake at 5700 ft and one with dry pit lake conditions. Stability analyses included overall circular failure and block type failure. The most critical failure surface would be a global failure from crest to toe of the slope above the southern containment outcrop of Burro Mountain granite. The minimum static factor of safety is 2.56 and the minimum pseudo-static factor of safety is 1.95.

Table 14: Little Rock In-Pit Stability Analysis Summary

Stockpile	Failure Type	Static Condition		Pseudo-static Condition (k = 0.094g)	
		No Pit Lake Condition	Pit Lake at elevation 5700 feet	No Pit Lake Condition	Pit Lake at elevation 5700 feet
West In-Pit Waste	Block	2.68	2.68	1.98	1.98
	Circular	2.55	2.55	1.88	1.88
North In-Pit Waste	Block	2.63	2.75	2.00	2.00
	Circular	2.57	2.56	1.95	1.95

8.0 CONCLUSIONS

Stability evaluations incorporating the design parameters outlined in this report indicate long-term factors of safety for the reclaimed stockpile configurations of at least 1.85 under static conditions and 1.48 under seismic loading. The stability analyses included an evaluation of the effects of liquefaction on the stockpile stability. These safety factors meet the minimum factor of safety criteria provided in the Copper Rule for all facilities (regardless of whether they would be categorized as non-critical or critical).

The stockpiles have been stable during operations. The reduction in the slope angles as part of reclamation will increase the degree of stability. The long-term effects of weathering on the shear strength of the stockpiles was assessed from the geochemical studies by EnviroGroup and evaluation of the engineering properties summarized in this report. These studies conclude the development of additional clays or more active clay minerals that might lead to a reduction in the stockpile shear strength is not predicted.

Reductions in the percentages of cobbles and boulders in the stockpile may occur as a result of long-term weathering, although we do not see evidence of it in the Tyrone stockpiles. It is recognized that the Tyrone stockpiles are relatively young compared to the Chino stockpiles. A correlation of reduced grain size of the stockpile material with the age of the material was recognized in a similar stockpile stability study completed at the Chino Mine (Golder, 2008) which are as much as 100 years old. The percentage of cobbles and boulders present in the Tyrone stockpiles is considered to contribute to the strength of the stockpile and potential long-term reductions in the percentage of these oversize particles can be expected to reduce the stockpile shear strength. The shear strengths assigned to the Tyrone stockpiles are based on laboratory testing of samples without oversize material that exists in the actual stockpile and reflect the stockpile matrix (or finer fraction) strength. Therefore, the laboratory derived shear strengths are lower than actual stockpile material strengths as they exist

today but are considered to reasonably reflect the long-term strengths of the stockpile materials. Long term increases in the strength of the stockpiles can also be anticipated due to long term settlement and densification of the stockpiles and cementation, mainly by sulphate minerals.

The potential for earthquake induced instability was evaluated using pseudo-static analyses. The pseudo-static coefficient applied considered the peak ground acceleration associated with a design basis earthquake with a 2500- year return period and applied an amplification factor appropriate for the sites underlain by the Gila Formation. The factors of safety applying the pseudo-static loads met the minimum factor of safety. All were above 1.4.

The stability analyses also considered the potential for liquefaction. The potential for liquefaction of Quaternary alluvium deposits below the water table in the toe areas of stockpiles was assessed using available subsurface geotechnical information where available. Where site specific geotechnical information is not available, the alluvium below the perched water table was assumed to be susceptible to liquefaction. If liquefaction potential was indicated, an additional stability analysis was performed applying a liquefied strength to the saturated alluvium. The resulting factors of safety indicate that liquefaction is not predicted to lead to the instability of the reclaimed stockpiles.

The stockpiles are currently indicated to be generally unsaturated. We expect moisture contents in the stockpile and in the alluvium in the toe areas of the stockpiles will decrease further after closure as a result of the cessation of leaching, addition of cover placement, and implementation of surface water management measures. The development of elevated groundwater levels in the stockpiles that could impact the stockpile's long-term stability is not expected. The potential for liquefaction of the alluvium will be further reduced as a result of cover placement and surface water management.

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project management/500_reporting/510_reports/512_finals/18106417.001.r.rev0/01 updated stockpile names

APPENDIX I

2005 Interim Report

FOR CLIENT REVIEW

**SUPPLEMENTAL STABILITY STUDY OF
WASTE ROCK PILES
AND LEACH ORE STOCKPILES
INTERIM REPORT FOR DP-1341, CONDITION 78
TYRONE MINE**

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2 Copies - Phelps Dodge Tyrone, Inc.
3 Copies - New Mexico Environment Department
2 Copies - Mining and Minerals Division
1 Copy - EnviroGroup Limited
2 Copies - Golder Associates Inc.

December 19, 2005

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1.0 INTRODUCTION

The New Mexico Environment Department (NMED) issued a Supplemental Discharge Permit for Closure (DP-1341) dated April 8, 2003 to Phelps Dodge Tyrone, Inc. (PDTI) for the Tyrone Mine. To comply with the New Mexico Water Quality Act and the New Mexico Water Quality Control Commission regulations, PDTI is required to conduct scientific studies that address a number of conditions specified under Section III of DP-1341. This Interim Report addresses Condition 78, which states:

Tyrone shall perform a supplemental stability study on the Waste Rock Piles and Leach Ore Stockpiles at the Tyrone Mine Facility. In accordance with the schedule approved under Condition 74, Tyrone shall submit to NMED for approval a work plan including an implementation schedule for the supplemental stability study to evaluate the long-term physical stability of Waste Rock Piles and Leach Ore Stockpiles after closure. The study shall evaluate and quantify changes in the engineering parameters resulting from the natural weathering process of the Waste Rock Pile and Leach Ore Stockpile materials that may ultimately affect long-term stability. At a minimum, the work plan shall propose methods and analyses to account for changes in chemical and physical properties of the stockpile materials from the time of deposition to present day and to a specified time during post-closure. The study shall include an evaluation of the recently reported data for materials interior to the stockpiles and whether additional data collection is warranted to evaluate long-term stability. (Golder Associates Inc. [Golder], 2003)

A Work Plan for Supplemental Slope Stability Analyses dated December 12, 2003 was submitted that is intended to address the supplemental stability requirements of Condition 78. The work plan has four primary objectives. They are:

- further characterize the internal composition, structure, chemical and physical state, and engineering parameters of the leach ore stockpiles and waste stockpiles;
- further characterize the geological and engineering parameters of the foundation materials of the leach stockpiles and waste stockpiles;
- quantify the effects of chemical weathering/cementation on the engineering parameters of the stockpile materials with respect to long-term slope stability; and

- re-evaluate the stability of the stockpiles based on the more detailed geologic/geotechnical model and engineering parameters.

A similar work plan was issued for Condition 80 of DP-1340 for the Chino Mine. NMED issued a “Request for Additional Information” as a condition of acceptance of the Chino Mine Condition 78 work plan. A request for additional information was not issued in response to the Tyrone Mine Condition 78 work plan. However, we have assumed that the additional requirements for Chino will apply to the Tyrone Mine as well.

This Interim Report has been prepared to provide a summary of the stockpile characterization data that have been collected to date for use as reference for other ongoing studies related to other conditions.

The following stockpiles comprise the Tyrone Mine Stockpile system

- No. 1 Waste
- No. 1A Leach
- No. 1B Leach
- No. 1C Waste
- No. 2A Leach
- No. 2B Waste
- No. 3A Leach
- No. 3C Waste
- No. 4A Leach
- No. 5A Waste
- No. 6B Leach
- No. 7A Waste

The stockpiles at the Tyrone Mine were placed by end dumping in lifts generally 50 to 200 feet high. There is considerable primary slope-parallel layering and segregation in the stockpiles.

2.0 SUMMARY OF WORK COMPLETED

A number of stockpile characterization programs and slope stability assessments of the waste rock piles and leach stockpiles at the Tyrone Mine have been completed. Characterization activities have consisted of test pit excavations, rotosonic borehole drilling, and laboratory testing of samples collected from the test pits and boreholes. Test pit logs are provided in Appendix A. Borehole logs are provided in Appendix B. Golder logged the rotosonic borehole cores for geotechnical information and the geotechnical logs are provided in Appendix B-1. PDTI prepared geologic logs of the rotosonic boreholes and these are provided in Appendix B-2. Laboratory testing results are provided in Appendices C and D. This Interim Report has been prepared to summarize the information compiled to date.

2.1 Closure/Closeout Plan Addendum

Golder completed a slope stability analysis of stockpiles at the Tyrone Mine and reported the results of those analyses in Golder (2000). The Closure/Closeout Plan Addendum provided a description of the existing site conditions, stockpile descriptions, characterization of stockpile materials, determination of mechanical properties of the stockpile materials, and computation of the stability of the stockpile embankment slopes.

The stockpile materials characterized during this study were collected from 14 surface excavations that were typically 15-feet deep. Golder visually estimated the percentage of oversized (>3-inch) material in each test pit. Golder logged the stockpile soils (minus 3-inch fraction) at each site according to American Society for Testing and Materials (ASTM) (D2488) standards. The test pit locations are provided in Table 1. Golder performed point load tests on oversize rock fragments to quantify the strength of the cobbles. Approximately 10 point load tests were performed for each test pit and the results were averaged to provide the results listed in Table 2. Additionally, Golder obtained three nuclear gauge moisture and density measurements from each test pit, and the values were averaged to provide the dry densities listed in Table 2. The test pit logs are provided in Appendix A-1. Bulk samples were collected from seven of the test pits and were subjected to grain-size analyses and Atterberg limits testing. Grain-size test results are provided in Appendix C-1. Two samples were also selected for triaxial shear testing. Staged triaxial shear testing was performed under consolidated, undrained conditions with pore pressure measurements. The results are provided in Appendix D-1. A 4-inch diameter triaxial cell was used, and the soil was scalped of fragments larger than $\frac{3}{4}$ inch. The laboratory test results are summarized in Table 3.

2.2 Stockpile Characterization Activities at the Tyrone Mine

During closure hearings with the state of New Mexico, PDTI received comments concerning the lack of geotechnical characterization of the interior portions of the stockpiles. The comments addressed the impact that leaching by low pH process solutions and long-term weathering may have on the long-term shear strength and, consequently, the long-term stability of the stockpiles and that the interior portions of the stockpiles may be more impacted by leaching and weathering than the surface. To address these comments, Golder conducted a field investigation during October 11 and 12, 2001 to augment the previous stockpile characterization study. PDTI identified several stockpiles that had been partially re-mined, exposing the interiors. This work was reported previously in Golder (2002). We collected samples and classified the stockpile soils at 12 sites. The sampled sites are identified on Figure 1 and the locations are described in Table 1. One of the difficulties in characterizing coarse stockpile materials is collecting a sufficiently large sample to provide a valid representation of large (up to approximately 4-foot diameter) fragments. At each sample site Golder defined a cell approximately the width of the bench height (typically 50-feet high) and characterized the stockpile material within each cell. Golder visually estimated the percentage of oversize material (plus 3-inch fraction), and the visual estimates were verified using scaled photographic images. Golder took three photographs at each sample location: one across the entire cell width, one approximately 10-feet wide, and one approximately 18 inches across to allow resolution of particles from several feet to 3 inches in diameter. The stockpile soils (minus 3-inch fraction) at each site were logged according to ASTM (D2488) standards. Geotechnical logs with a photograph for each cell are provided in Appendix A-2. Golder reviewed stockpile cross-sections prepared by Greystone from historical mine maps to determine the year that various stockpiles were placed. Table 1 summarizes the sample locations and age and leaching history of the stockpiles

Approximately 60-pound soil samples were collected from each cell. The samples were sent to Golder's laboratory in Denver, Colorado and subjected to sieve analysis (ASTM C117/C136) and Atterberg limits determination (ASTM D4318). The grain-size and Atterberg limits test results are provided in Appendix C-2. Golder prepared extended grain-size curves by combining the laboratory grain-size curves with the visual estimates of the gradation of the oversize fraction. Figure 2 provides a plot of all of the extended grain-size curves that have been prepared from samples collected from the Tyrone Mine Stockpiles. Four staged, consolidated, undrained triaxial shear tests were performed, and the results are provided in Appendix D-2. A 4-inch diameter triaxial cell was used, and the soil was scalped of fragments larger than $\frac{3}{4}$ inch. Laboratory test results are summarized in Table 3.

2.3 2004 1C Stockpile Sampling Program

During 2004, PDTI re-mined a portion of the 1C Stockpile exposing the interior portions of the stockpile.

Golder performed a site investigation to characterize the interior portions of the 1C Stockpile on September 8 and 9, 2004. Golder's Senior Project Engineer, Thomas Wythes, performed the site investigation. Golder collected samples and classified the stockpile soils at eight sites. The sampled sites are identified on Figure 1, and approximate coordinates are provided in Table 1. The topography shown on Figure 1 does not reflect the topography at the time of sampling because the 1C Stockpile was being actively mined at that time and has since been reclaimed. At each sample site we defined a cell approximately the width of the bench height (typically 50-feet high) and characterized the stockpile material within each cell. The stockpile soils (i.e., the minus 3 inch fraction) at each site were logged according to ASTM (D2488) standards. Golder visually estimated the percentage of oversize material (plus 3-inch fraction), and the visual estimates were verified using scaled photographic images. Golder took three photographic images at each sample location: one across the entire the cell width, one approximately 10- to 20-feet wide, and one approximately 2- to 5-feet across to allow resolution of particles from several feet to 3 inches in diameter. Extended grain-size curves were prepared and are illustrated on Figure 2. Geotechnical logs for each cell are provided in Appendix A-3.

Approximately 60-pound soil samples were collected from each cell. The samples were sent to Golder's laboratory in Denver, Colorado and subjected to sieve analysis (ASTM C117/C136), hydrometer analysis (ASTM D422), and Atterberg limits determination (ASTM D4318). The grain-size and Atterberg limit test results are provided in Appendix C-3. Large scale (6-inch) direct shear tests were performed under saturated conditions. The direct shear test samples were scalped of fragments larger than 1 inch. The laboratory testing results are provided in Appendix D-3 and summarized in Table 3. An additional 30-pound sample was also collected from each cell for geochemical testing in support of studies related to Condition 80. Geochemical characterization is being performed by EnviroGroup Limited (formerly Greystone) under the direction of Dr. Drummond Early.

Golder reviewed stockpile cross-sections prepared by Greystone from historical mine maps to determine the year that various stockpiles were placed. Table 1 summarizes the sample locations, and the age and depth of the stockpiles. A characterization of mineral assemblage type in each stockpile

was provided in Appendix E of the Preliminary Materials Characterization Report (Daniel B. Stephens & Associates, Inc. [DBS&A], 1997a) and the Supplemental Materials Characterization Report (DBS&A, 1997b). These reports also include pit development maps, plan view stockpile distribution maps, and stockpile development cross-sections. This information was useful in determining the history of each stockpile.

2.4 2004 Rotosonic Stockpile Drilling Program

PDTI completed a rotosonic drilling program between September 28 and October 13 2005. The locations of the drillholes are provided in Table 4 and illustrated on Figure 1. Boreholes TSGT-1 through TSGT-3 were terminated within the stockpile. Borehole TSGT-4 extended approximately 15 feet into the foundation soils.

The drill holes were logged for geologic and geotechnical information. Golder completed geotechnical logging after the drilling was completed and the rotosonic drill core was laid out in the core shed. Geotechnical logs are provided in Appendix B-1. PDTI performed geologic logging, and the geologic logs are provided in Appendix B-2. Information collected during geotechnical logging included a soil description and classification according to ASTM D2488. A color designation was also applied based on the Munsel color code. Estimates of the moisture content were made but are of little value because samples were stored for 7 months prior to geotechnical logging. The core recovery and percentage of fragments greater than 3 inches were also recorded. The estimates of the oversize fraction from the rotosonic core are considered less representative than those from the test pits because of the relatively small diameter of the core. The strength of the clasts was estimated based on the International Society of Rock Mechanics manual index test procedures as summarized on Table 5. Photographs of the rotosonic core are provided on the CD included in Appendix C.

Samples were collected from the rotosonic core during geotechnical logging at typical intervals of 25 to 50 feet. Samples were typically approximately 25 pounds, and fragments larger than 3 inches were generally excluded. The samples were sent to Golder's laboratory in Denver, Colorado and subjected to sieve analysis (ASTM C117/C136), hydrometer analysis (ASTM D422), and Atterberg limits determination (ASTM D4318). The grain-size and Atterberg limit test results are provided in Appendix C-4 and are summarized in Table 6.

After completion of each rotosonic corehole, a downhole geophysical survey was performed. Geophysical logging included:

- Cased Density Log,
- Natural Gamma Ray Spectrometry Tool, and
- Epithermal Neutron Log.

The results of the geophysical survey are reported in Greystone (2004).

2.5 2005 1A Stockpile Rotosonic Drilling Program

PDTI performed a rotosonic drilling program of the 1A Stockpile to verify their condition prior to re-mining from September 13 through 17, 2005. Five rotosonic drillholes (S1A-1 through S1A-5) were completed to depths of 110 to 200 feet. The locations of the 2005 rotosonic boreholes are shown on Figure 1 and the coordinates are provided on Table 4. Golder Engineer, Gene Muller, was present during drilling of rotosonic holes S1A-3 through S1A-5 and completed geotechnical logging and sampling of those holes. Hole S1A-3 penetrated through the base of the stockpile into the underlying Gila Conglomerate. The geotechnical logs are provided in Appendix B-3. Geologic logs prepared by PDTI are provided in Appendix B-4. Nineteen samples were collected and were sent to Golder's laboratory in Denver, Colorado and subjected to sieve analysis (ASTM C117/C136), hydrometer analysis (ASTM D422), and Atterberg Limits determination (ASTM D4318). The laboratory test results are provided in Appendix C-5 and are summarized in Table 6. The materials near the base of the 1A Stockpile have been subjected to leaching for the longest period of time of any stockpiles at the Tyrone Mine.

3.0 PRELIMINARY RESULTS

The results of the laboratory testing and geotechnical logging indicate that the stockpile materials typically classify as clayey gravels or clayey sand with occasional poorly graded gravel with clay and sand. The soil has a low to moderate plasticity generally between 10 and 20 percent. The oversize fraction varies considerably, comprising 10 to 50 percent of the material. The extended grain size curves provided on Figure 2 illustrate the range of grain sizes from the stockpile test pit samples. Localized clay zones 1- to 6-feet thick have been recognized in the rotosonic cores that are 1- to 4-feet thick.

The laboratory-derived Mohr-Coulomb shear strengths of the stockpile samples range from 29 degree friction with 8.8 pounds per square inch (psi) cohesion to 36.2 degrees friction with 0.6 psi cohesion. The shear strength results are summarized in Table 2. Due to the limitation of particle size in the test apparatus (i.e., $\frac{3}{4}$ inch for triaxial cell and 1 inch for direct shear box) the impact of the larger size fragments is not included in the laboratory-derived shear strength estimates. The distribution of the various sized particles plays a significant role in determining the physical properties of the stockpile materials. Research conducted by Frigaszy, et al. (1992) suggests that the strength of a soil with oversize particles (larger than the limitations of the testing apparatus) may be characterized by the strength of the matrix material if the oversized particles are in a floating state. Conversely, the strength of the soil may be characterized by the properties of the oversized material if there is sufficient oversized particle to particle contact. The research suggests that the strength properties of a soil having less than 40 percent oversized material are controlled primarily by the soil matrix and that the strength properties of a soil with more than 65 percent oversized material is controlled primarily by the properties of the oversized material. The strength properties of soils having between 40 and 65 percent oversized material are influenced by both the soil matrix and the oversized material. Boundaries have been provided on Figure 2 between oversized controlled shear strength, matrix controlled strength, and transitional behavior. The stockpiles are generally transitional. Laboratory-derived shear strengths can be considered to underestimate the shear strength of the stockpile materials.

The stockpiles are unsaturated, and moisture content variability is related to grain size of the soils. Coarse-grained materials are dry, and fine-grained zones have higher moisture retention.

4.0 REMAINING WORK

Additional work that will be completed to assess the stockpile stability is the characterization of the foundation conditions underlying the stockpiles. This work will be completed during early 2006 and will include identification of areas with potentially weak foundation materials based on available data and field reconnaissance. Where weak foundation materials are identified or suspected, the field characteristics will be recorded and samples will be collected and subjected to laboratory testing as determined to be appropriate.

The long-term impacts of weathering on the stability of the stockpiles will also be considered through weathering studies being performed by EnviroGroup Limited.

Two-dimensional, cross-sectional, slope stability models will be developed for critical cross-sections. Critical cross-sections will be selected based on consideration of slope height, slope angle, natural ground surface topography, groundwater conditions, the compositional model and assigned material parameters. Slope regrading is ongoing and the slope stability analyses will be performed for slope configurations that reflect the final conditions. Limit equilibrium slope stability analyses will be performed for the critical cross-sections, and the results will be reported in terms of a factor of safety.

The methodology and results will be described in a final report that will be prepared after the completion of the slope stability analyses and is scheduled for completion prior to September 2006.

5.0 USE OF THIS REPORT

This report has been prepared exclusively for the use of Phelps Dodge Tyrone, Inc. for specific application to the Tyrone Project. No third party engineer or consultant shall be entitled to rely on any of the information, conclusions, or opinions contained in this report without the prior written approval from PDTI and Golder.

The conclusions in this report have been prepared in a manner consistent with that level of care and skill ordinarily exercised by professionals currently practicing in this field. In preparing our conclusions and recommendations, Golder has relied upon information provided by the client and other parties involved in the study and Golder is not responsible for errors or omissions in the information provided by the client or the other parties.

Respectfully submitted,

GOLDER ASSOCIATES INC.

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TABLES

TABLE 1
SUMMARY OF STOCKPILE TEST PIT LOCATIONS

Sample #	Approximate Mine Coordinates			Stockpile Name	Year Stockpile Placed	Depth Below Stockpile Surface (feet)	% >3-inch
	Easting	Northing	Elev.				
2000 Closure/Closeout Plan Addendum ^a							
GTP-01	21349	11668	6123	No. 1		10	15
GTP-02	22620	11316	6150	No. 1		10	15
GTP-03	15937	14890	6250	No. 5A		10	20
GTP-04	15252	15617	6130	No. 5A		10	10
GTP-05	13426	16559	6180	No. 5A		10	20
GTP-06	4038	11230	6310	No. 2A		10	20
GTP-07	4694	13436	6380	No. 2A		10	20
GTP-08	4408	9437	6270	No. 2A		10	15
GTP-09	6075	6910	6400	No. 2A		10	45
GTP-10	11416	3546	6450	No. 7A		10	25
GTP-11	13141	4636	6400	No. 7A		10	20
GTP-12	16737	5541	6250	No. 1C		10	25
GTP-13	18450	6622	6250	No. 1C		10	20
GTP-14	10230	19063	6150	No. 3A		10	35
2001 Test Pit Program							
TYTP01-1	4757	10882	6270	No. 2B	1975	100	40
TYTP01-2	6335	8367	6100	No. 2C	1978	400	30
TYTP01-3	6643	8275	6200	No. 2C	1982	250	50
TYTP01-4	5945	9155	6350	No. 2C	1986	100	15
TYTP01-5	5766	6482	6400	No. 4C	1975	100	10
TYTP01-6	13643	9578	5925	No. 6B	1989	300	50
TYTP01-7	13282	9913	5975	No. 6B	1989	300	40
TYTP01-8	13523	9429	6025	No. 6B	1990	250	30
TYTP01-9	17378	8051	6150	No. 1A	1967	40	30
TYTP01-10	14429	12245	6200	No. 5A	1967	50	50
TYTP01-11	4473	10612	5850	No. 2B	1973	100	10
TYTP01-12	10770	15320	6275	No. 3B	1975	100	40
2004 1C Stockpile Sampling							
GA04-TY-1	17364	6537	6200	1C	1998	50.0	10
GA04-TY-2	18049	7149	6200	1C	1998	50.0	10
GA04-TY-3	17366	6367	6150	1C	1982	100.0	50
GA04-TY-4	18112	6939	6150	1C	1982	100.0	40
GA04-TY-5	18417	6253	6000	1C	1982	250.0	15
GA04-TY-6	18220	7116	6150	1C	1982	100.0	20
GA04-TY-7	18158	6690	6100	1C	1982	150.0	20
GA04-TY-8	18404	7044	6100	1C	1982	150.0	30

Note:

^a Golder, 2000

TABLE 2
SUMMARY OF STOCKPILE SAMPLE FIELD TEST RESULTS

Sample #	Approximate Mine Coordinates			Stockpile Name	Average Clast UCS (psi)	Average Field Dry Density	Average Field Moisture
	Easting	Northing	Elev.				
GTP-01	21349	11668	6123	No. 1	9,436	121.3	5.0
GTP-02	22620	11316	6150	No. 1	14,187	118.7	6.8
GTP-03	15937	14890	6250	No. 5A	13,155	125.1	5.3
GTP-04	15252	15617	6130	No. 5A	8,006	121.1	3.3
GTP-05	13426	16559	6180	No. 5A	14,109	108.4	4.8
GTP-06	4038	11230	6310	No. 2A	9,268	127.5	6.8
GTP-07	4694	13436	6380	No. 2A	8,052	117.9	5.8
GTP-08	4408	9437	6270	No. 2A	5,119	121.7	9.8
GTP-09	6075	6910	6400	No. 2A	15,856	116.3	6.7
GTP-10	11416	3546	6450	No. 7A	4,621	127.1	6.0
GTP-11	13141	4636	6400	No. 7A	10,593	117.0	3.2
GTP-12	16737	5541	6250	No. 1C	7,906	128.3	5.0
GTP-13	18450	6622	6250	No. 1C	6,507	127.3	3.8
GTP-14	10230	19063	6150	No. 3A	22,481	119.6	5.3

Notes:

UCS = uniaxial compressive strength

psi = pounds per square inch

TABLE 3
SUMMARY OF STOCKPILE TEST PIT LABORATORY TEST RESULTS

Sample #	USCS Soil Classification	Atterberg Limits			Grain-size Distribution			Effective Stress Parameters	
					% Finer	% Finer	% Finer	φ	C
		LL	PL	PI	3/4"	#4	#200	(degrees)	(psi)
GTP-01	GC	47	22	25	81	49	17		
GTP-03	SC	33	20	13	84	60	18	34.6 ^a	5.8 ^a
GTP-06	GP-GC	42	20	22	69	38	10	32.8 ^a	8.3 ^a
GTP-09	GP-GC	39	20	19	59	37	8		
GTP-10	GC	47	21	26	70	38	12		
GTP-13	GP-GC	38	19	19	74	49	11		
GTP-14	GP-GC	28	15	13	66	31	8		
TYTP01-1	GC	38	17	21	77	48	13		
TYTP01-2	GW-GC	30	14	16	78	48	10	36.2 ^a	0.6 ^a
TYTP01-3	GP-GC	36	18	18	75	38	9		
TYTP01-4	GC	40	17	23	76	53	15	35.5 ^a	0.4 ^a
TYTP01-5	GC	39	16	23	79	49	15		
TYTP01-6	GW-GC	29	16	13	70	42	8		
TYTP01-7	SC	28	15	13	81	63	15	36.9 ^a	0.6 ^a
TYTP01-8	SC	24	16	8	85	60	15		
TYTP01-9	GC	30	18	12	80	58	24	34.1 ^a	2.2 ^a
TYTP01-10	SC	30	18	12	96	80	17		
TYTP01-11	GC	37	17	20	85	50	13		
TYTP01-12	GC	30	16	14	77	46	14		
GA-04-TY-1	GC	34	20	14	70	41	18	31.0 ^b	10.4 ^b
GA-04-TY-2	GC	35	21	14	80	52	22		
GA-04-TY-3	GC	40	21	19	69	40	17		
GA-04-TY-4	GP-GC	50	17	33	46	15	6		
GA-04-TY-5	GC	33	19	14	59	33	14	32.0 ^b	11.8 ^b
GA-04-TY-6	GC	35	19	16	66	42	20		
GA-04-TY-7	GC	36	19	17	64	36	14		
GA-04-TY-8	GC	30	19	11	68	42	17	29.0 ^b	8.8 ^b
<i>Average</i>		<i>35.5</i>	<i>18.1</i>	<i>17.3</i>	<i>73.5</i>	<i>45.9</i>	<i>14.1</i>	<i>33.6</i>	<i>5.4</i>
<i>Standard Deviation</i>		<i>6.4</i>	<i>2.1</i>	<i>5.6</i>	<i>10.1</i>	<i>12.3</i>	<i>4.5</i>	<i>2.6</i>	<i>4.6</i>

Notes:

^a consolidated-undrained triaxial shear with pore pressure measurements^b large-scale (6 in x 6 in) direct shear

USCS = Unified Soil Classification System

LL = liquid limit

PL = plastic limit

PI = plasticity index

psi = pounds per square inch

TABLE 4
ROTOSONIC BOREHOLE LOCATIONS

Drillhole	Easting	Northing	Elevation	Depth
TSGT-01	9537	19405	6208	410
TSGT-02	17009	11550	6378	281
TSGT-03	15999	14300	6298	250
TSGT-04	4156	10839	6311	2,730
S1A-1	18931	8274	6164	150
S1A-2	18744	9432	6159	110
S1A-3	18991	10092	6170	110
S1A-4	18606	8849	6304	115
S1A-5	18441	9895	6374	200

TABLE 5
INTERNATIONAL SOCIETY OF ROCK MECHANICS MANUAL INDEX TEST

Grade	Description	Field Identification	Approximate Range of Uniaxial Compressive Strength	
			(MPa)	(psi)
S1	Very Soft Clay	Easily penetrated several inches by fist	<0.025	<4
S2	Soft Clay	Easily penetrated several inches by thumb	0.025 – 0.05	4 – 7
S3	Firm Clay	Can be penetrated several inches by thumb with moderate effort	0.05 – 0.1	7 – 15
S4	Stiff Clay	Readily indented by thumb but penetrated only with great effort	0.1 – 0.25	15 – 35
S5	Very Stiff Clay	Readily indented by thumbnail	0.25 – 0.50	35 – 70
S6	Hard Clay	Indented with difficulty by thumbnail	>0.50	>70
R0	Extremely Weak Rock	Indented by thumbnail	0.25 – 1.0	35 – 150
R1	Very Weak Rock	Crumbles under firm blows with point of geological hammer, can be pealed by a pocket knife	1.0 – 5.0	150 – 725
R2	Weak Rock	Can be pealed by a pocket knife with difficulty, shallow indentation made by firm blow with point of geological hammer	5.0 – 25	725 – 3,500
R3	Medium Strong Rock	Cannot be scraped or pealed with a pocket knife, specimen can be fractured with a single firm blow of geological hammer	25 – 50	3,500 – 7,500
R4	Strong Rock	Specimen requires more than one blow of geological hammer to fracture it	50 – 100	7,500 – 15,000
R5	Very Strong Rock	Specimen requires many blows of geological hammer to fracture it	100 – 250	15,000 – 35,000
R6	Extremely Strong Rock	Specimen can only be chipped with geological hammer	>250	>35,000

Notes:

MPa = megaPascals

psi = pounds per square inch

TABLE 6
SUMMARY OF LABORATORY TEST RESULTS FROM
ROTOSONIC BOREHOLE SAMPLES

Drillhole	Depth (feet)	USCS Soil Classi- fication	Atterberg Limits			Grain-size Distribution		
			LL	PL	PI	% Finer 3/4"	% Finer #4	% Finer #200
TSGT-1	19-21	SC	25	14	11	86	65	23
TSGT-1	47-48.5	GC	29	16	13	61	41	15
TSGT-1	75-77	SC-SM	26	19	7	81	60	17
TSGT-1	88-90	GC	28	18	10	84	58	23
TSGT-1	102-104	GC	33	20	13	80	55	21
TSGT-1	140-142	GC	29	18	11	66	46	21
TSGT-1	158.5-160	GC	34	19	15	64	41	18
TSGT-1	184-186	GC	30	20	10	72	50	17
TSGT-1	211-214	SC	27	16	11	93	76	32
TSGT-1	228-230	GC	23	15	8	72	52	21
TSGT-1	250.5-253	SC	27	16	11	89	69	30
TSGT-1	298-300	SC	28	14	14	95	82	38
TSGT-1	310-312	GC	29	16	13	78	55	21
TSGT-1	327-329	SC-SM	21	15	6	83	67	24
TSGT-1	356-358	SC	32	16	16	80	65	26
TSGT-1	388-390	SC	26	15	11	93	82	35
TSGT-2	18-20	GC	27	16	11	72	47	16
TSGT-2	42-44	GP-GC	22	14	8	64	37	12
TSGT-2	140-142	SC	34	20	14	86	63	18
TSGT-2	267-268.5	GC	29	19	10	66	46	12
TSGT-3	18-20	SC	28	17	11	94	66	16
TSGT-3	58-60	GC	29	17	12	85	56	15
TSGT-3	156-158	SC	29	15	14	94	75	28
TSGT-3	248-250	GC	28	17	11	74	56	22
TSGT-4	12-14	GC	34	18	16	78	55	21
TSGT-4	69-71	GC	36	20	16	64	34	12
TSGT-4	117-119	GC	35	20	15	68	38	14
TSGT-4	152-154	GC	31	18	13	88	55	18
TSGT-4	183-185	SC	31	19	12	94	78	24
TSGT-4	221-223	GC	29	17	12	78	47	16
TSGT-4	265-269	SM	NP	NP	NP	97	90	22
T3	11.0-12.5	GC	32	17	15	71	41	16
T3	31.0-32.5	GC	34	18	16	69	56	28
T3	51.0-52.5	GC	31	19	12	71	55	21
T3	71.0-72.5	SC	33	20	13	91	76	35
T3	76.5-77.5	SC-SM	23	16	7	88	67	28
T3	91.0-92.5	GC	31	18	13	79	62	26
T4	49.0-50.0	GC	31	17	14	77	55	22
T4	69.0-70.0	SC	31	16	15	86	63	25
T4	89.0-90.0	SC	36	20	16	86	66	30
T4	109.0-110.0	GC	35	20	15	74	55	24

Drillhole	Depth (feet)	USCS Soil Classi- fication	Atterberg Limits			Grain-size Distribution		
			LL	PL	PI	% Finer 3/4"	% Finer #4	% Finer #200
T4	113.0-114.0	SC	36	19	17	97	76	33
T4	129.0-130.0	GC	34	23	11	72	49	24
T5	16.5-17.5	GM	NP	NP	NP	69	49	15
T5	36.5-37.5	GC	30	16	14	56	37	13
T5	56.5-57.5	GC	34	18	16	58	38	13
T5	96.5-97.5	GC	37	21	16	71	55	20
T5	116.5-117.5	GC	33	19	14	84	62	27
T5	136.5-137.5	GC	33	18	15	67	47	19
T5	164.0-165.0	GC	35	17	18	75	53	20
<i>Average</i>			29.7	17.2	12.5	78.4	57.4	21.7
<i>Standard Deviation</i>			5.8	3.2	3.3	11.0	13.2	6.5

Notes:

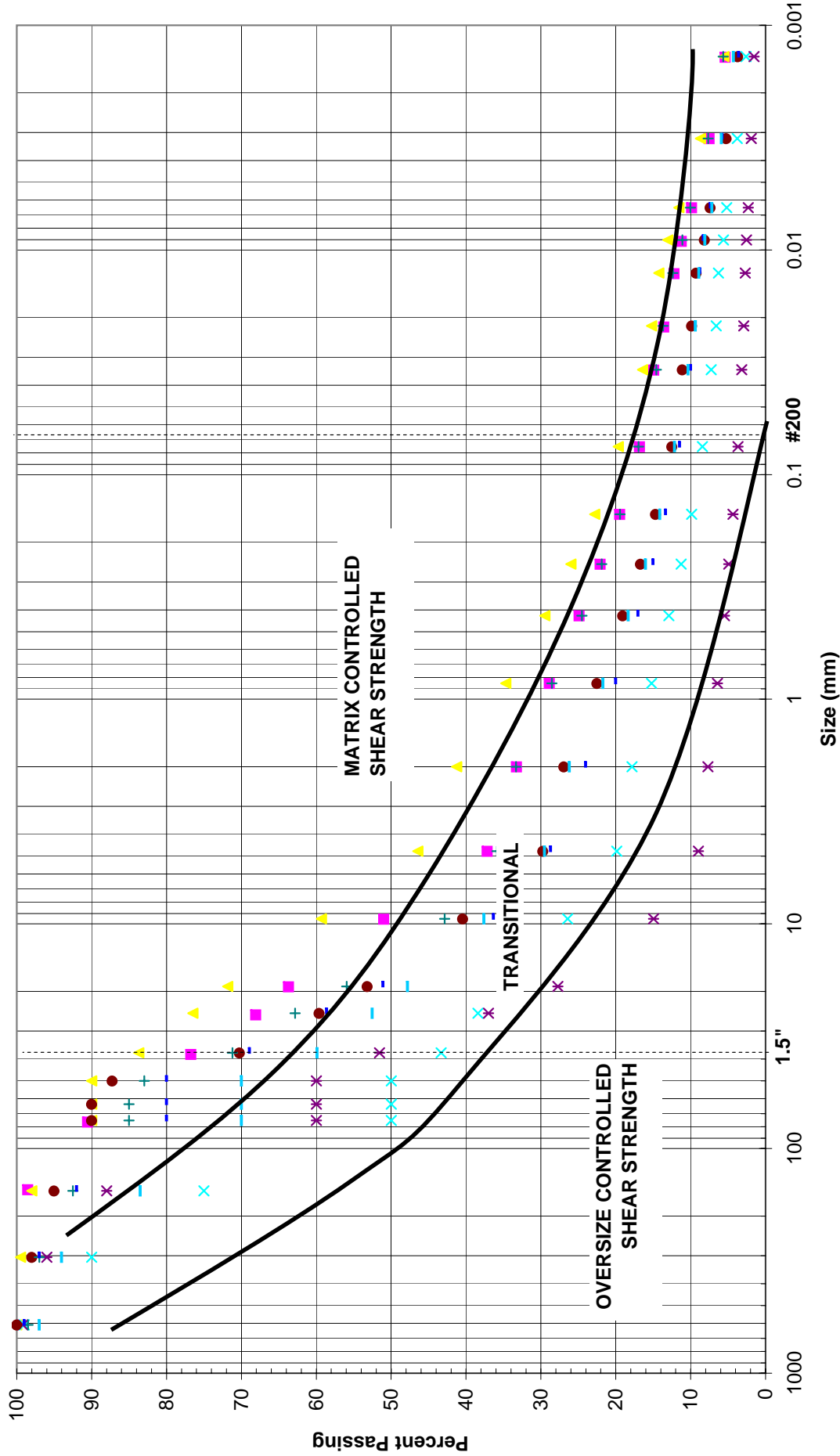
USCS = Unified Soil Classification System

LL = liquid limit

PL = plastic limit

PI = plasticity index

FIGURES



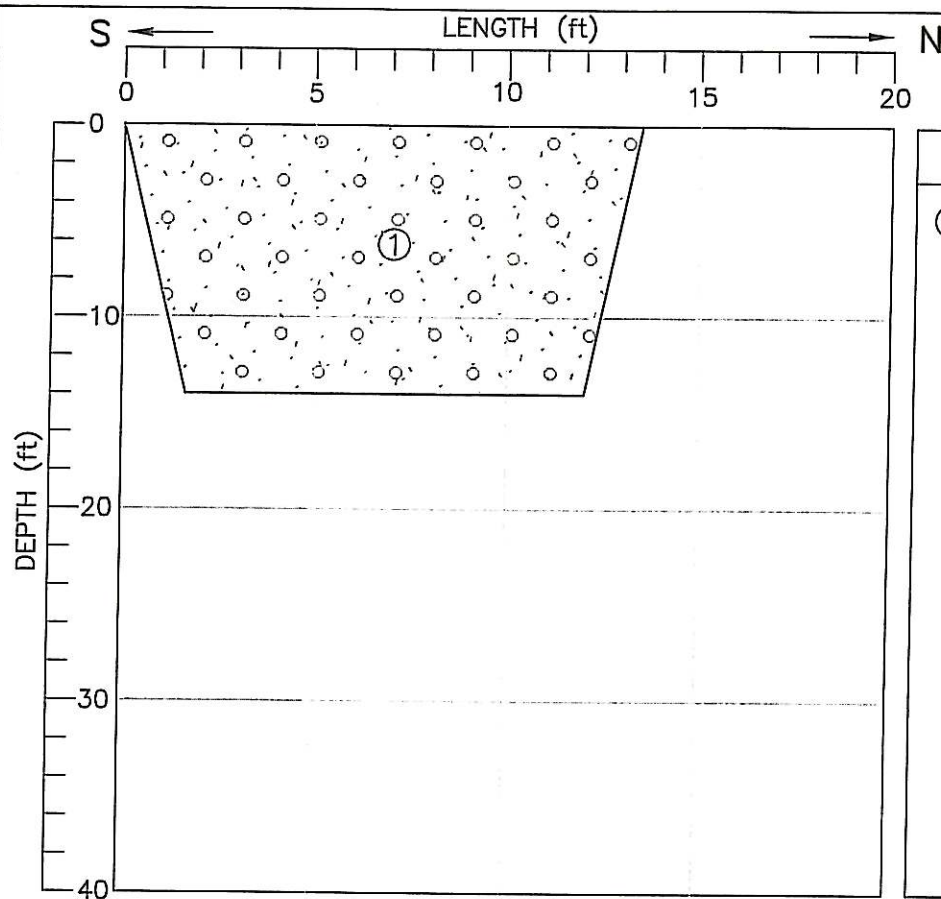
CLIENT/PROJECT				TITLE			
Phelps Dodge Tyrone Inc. DP-1341, Condition 78				Interim Report 1C Stockpile Extended Grain Size Curves			
DRAWN				Tucson, Arizona			
CHECKED				FILE NO. gures.xls			
REVIEWED				N.T.S.			
DATE				SCALE			
11-27-05				JOB NO. 053-2550			
dak				REV. NO. A			
tjw				FIGURE 2			

APPENDIX A
TEST PIT LOGS

APPENDIX A-1
2000 TEST PIT LOGS

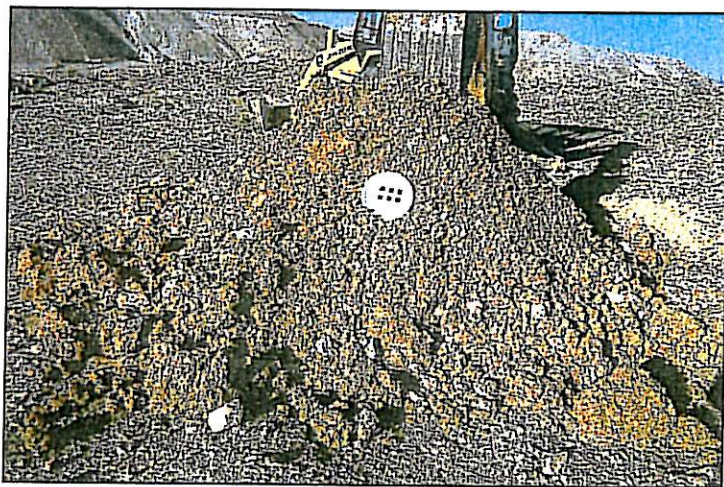
FIELD TEST PIT LOG

TEMP 30 °F WEATHER CLEAR TEST PIT GTP-01
 EQUIPMENT DEERE 510C BACKHOE ENGINEER G. TORTELLI OPERATOR SAL
 ELEVATION 6123.5 FT. CONTRACTOR HAMILTON DATE JANUARY 7, 2000
 LOCATION 11668.64 N, 21349.19 E DATUM MSL JOB 993-2546
 NO. 1 STOCKPILE



LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Well Graded Gravel with Sand and Clay. Few boulders to 12 inches. Fine to coarse, angular gravel. Some fine to coarse sand. Little medium plasticity fines. Slightly moist-moist. Medium dense.



SAMPLES

NO.	DESCRIPTION
S-01	Bulk
-	Point Load Clasts

NOTES:

85% Soil Matrix
15% Oversize Clast

Excavation terminated at 14 foot depth.

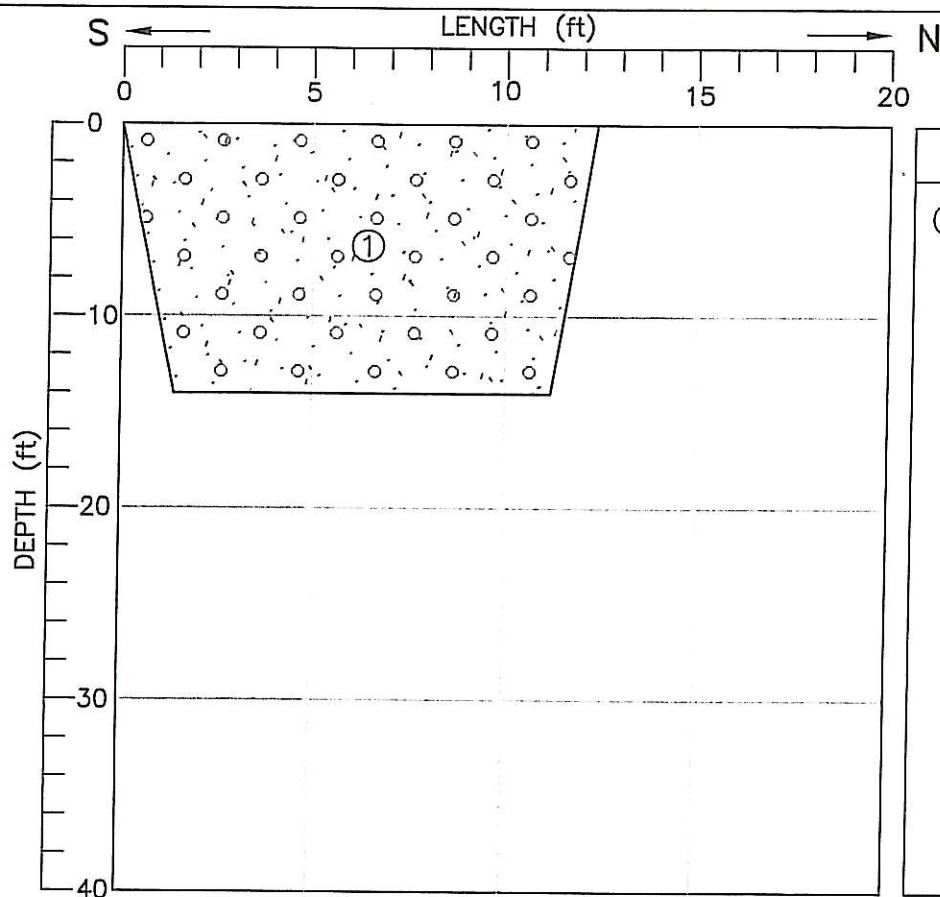
No groundwater encountered during excavation.

Test pit backfilled with excavated material.

Field density tests performed in vicinity of test pit.

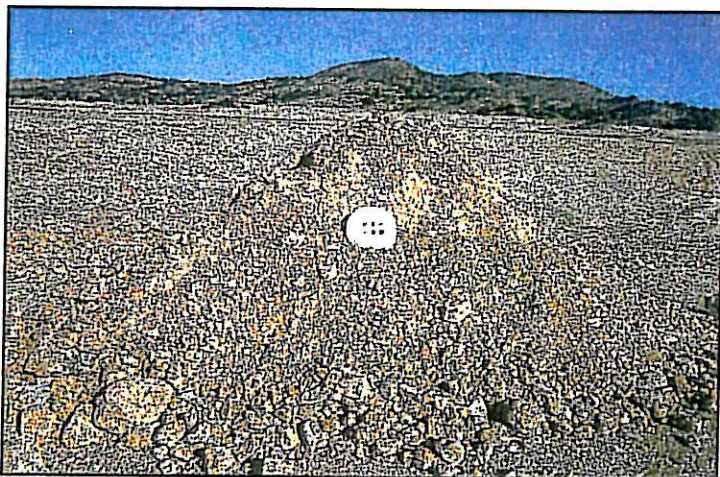
FIELD TEST PIT LOG

TEMP 30 °F WEATHER CLEAR TEST PIT GTP-02
 EQUIPMENT DEERE 510C BACKHOE ENGINEER G. TORTELLI OPERATOR SAL
 ELEVATION 6150 FT. CONTRACTOR HAMILTON DATE JANUARY 7, 2000
 LOCATION 11315.70 N, 22620.44 E DATUM MSL JOB 993-2546
 NO. 1 STOCKPILE



LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Well Graded Gravel with Sand and Clay. Few boulders to 36 inches. Fine to coarse angular gravel. Some fine to coarse sand. Little medium plasticity fines. Moist. Medium dense.



SAMPLES

NO.	DESCRIPTION
—	Point Load Clasts

NOTES:

85% Soil Matrix
15% Oversized Clasts

Excavation terminated at 13.5 foot depth.

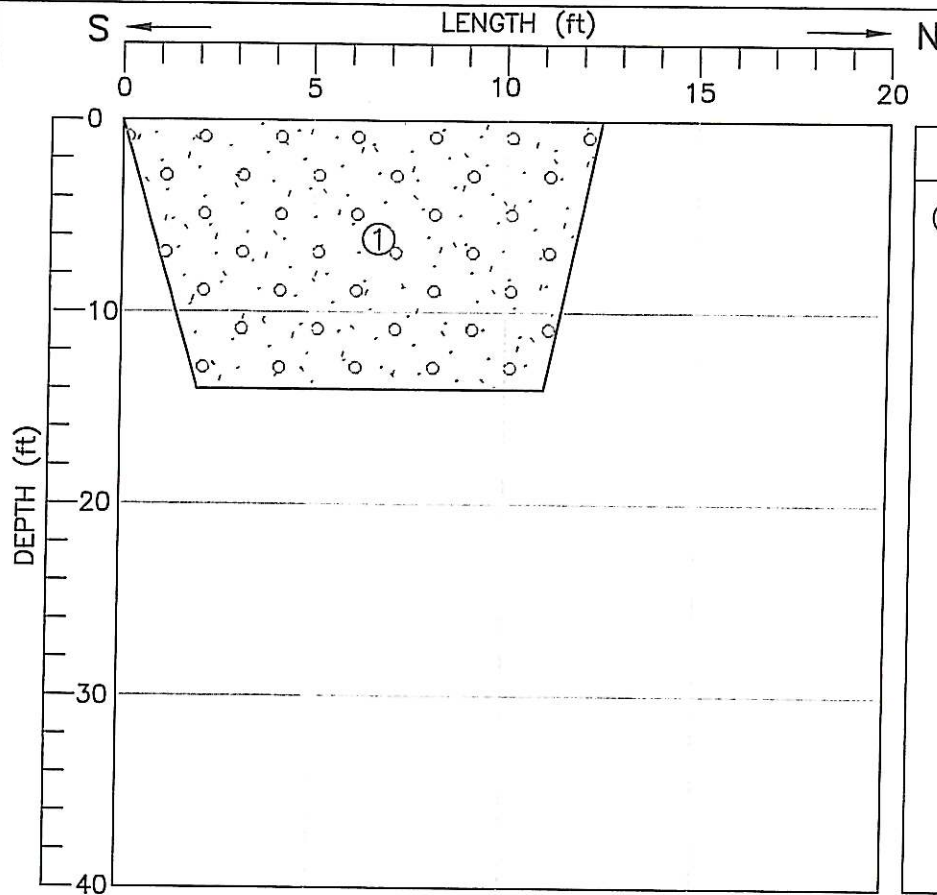
No groundwater encountered during excavations.

Test pit backfilled with excavated material.

Field density tests performed in vicinity of test pit.

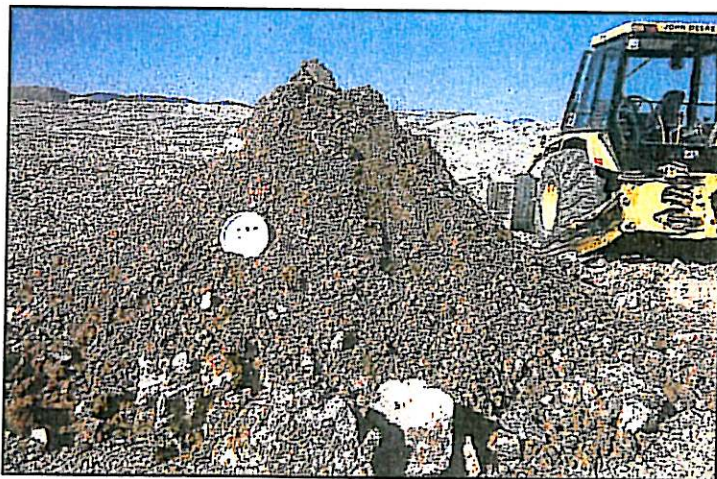
FIELD TEST PIT LOG

TEMP <u>32</u> °F	WEATHER <u>CLEAR</u>	TEST PIT <u>GTP-03</u>	
EQUIPMENT <u>DEERE 510C BACKHOE</u>	ENGINEER <u>G. TORTELLI</u>	OPERATOR <u>SAL</u>	
ELEVATION <u>6250 FT.</u>	CONTRACTOR <u>HAMILTON</u>	DATE <u>JANUARY 7, 2000</u>	
LOCATION <u>14889.58 N, 15936.84 E</u>	DATUM <u>MSL</u>	JOB <u>993-2546</u>	
	<u>1D Stockpile</u>		



LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Well Graded Gravel with Sand and Clay. Little cobbles and boulders to 3/6 inches. Fine to coarse angular gravel. Some fine to coarse sand. Little medium plasticity fines. Moist. Medium Dense.



SAMPLES

NO.	DESCRIPTION
S-02	Bulk
-	Point Load Clasts

NOTES:

80% Soil Matrix
20% Oversized Clasts

Excavation terminated at 15 foot depth.

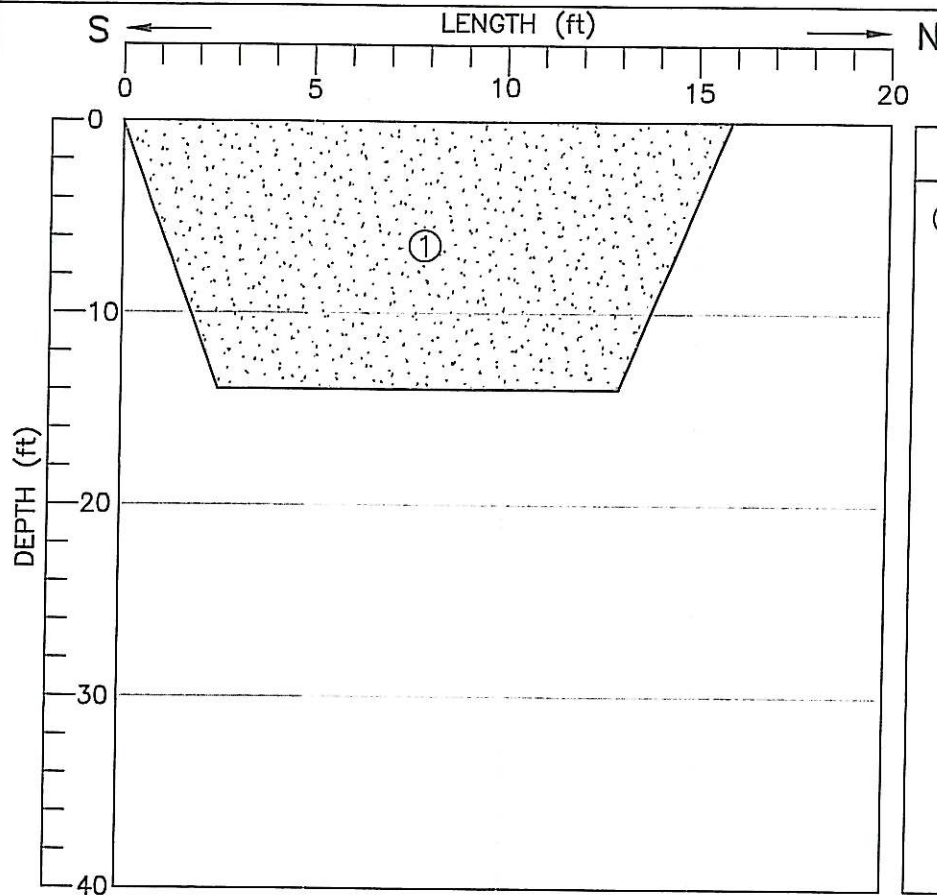
No groundwater encountered during excavation.

Test pit backfilled with excavated material.

Field density tests performed in vicinity of test pit.

FIELD TEST PIT LOG

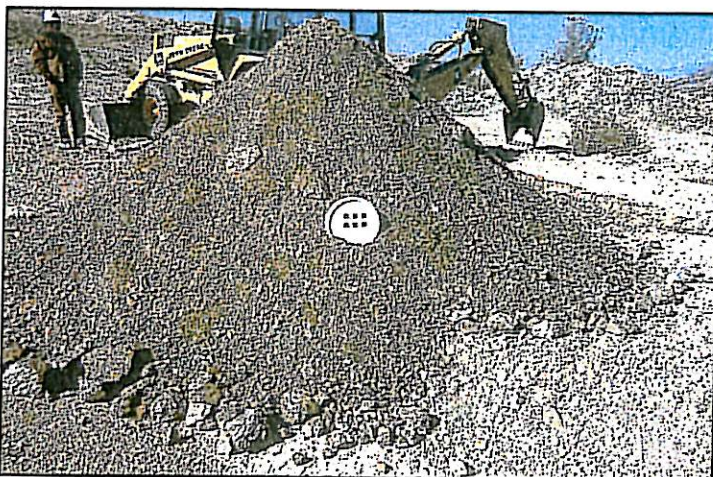
TEMP 35 °F WEATHER CLEAR TEST PIT GTP-04
 EQUIPMENT DEERE 510 C BACKHOE ENGINEER G. TORTELLI OPERATOR SAL
 ELEVATION 6130 FT. CONTRACTOR HAMILTON DATE JANUARY 7, 2000
 LOCATION 15616.96 N, 15252.46 E DATUM MSL JOB 993-2546
 1D STOCKPILE (MID BENCH)



LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Well Graded Sand with Gravel. Fine to coarse sand. Some Fine to coarse angular gravel. Few cobbles and boulders to 24 inches. Few low plasticity fines. Moist. Medium dense.

Mostly moderately to severely weathered granitic rock material.



SAMPLES

NO.	DESCRIPTION
—	Point Load Clasts

NOTES:

90% Soil Matrix
10% Oversized Clasts

Excavation terminated at 15 foot depth.

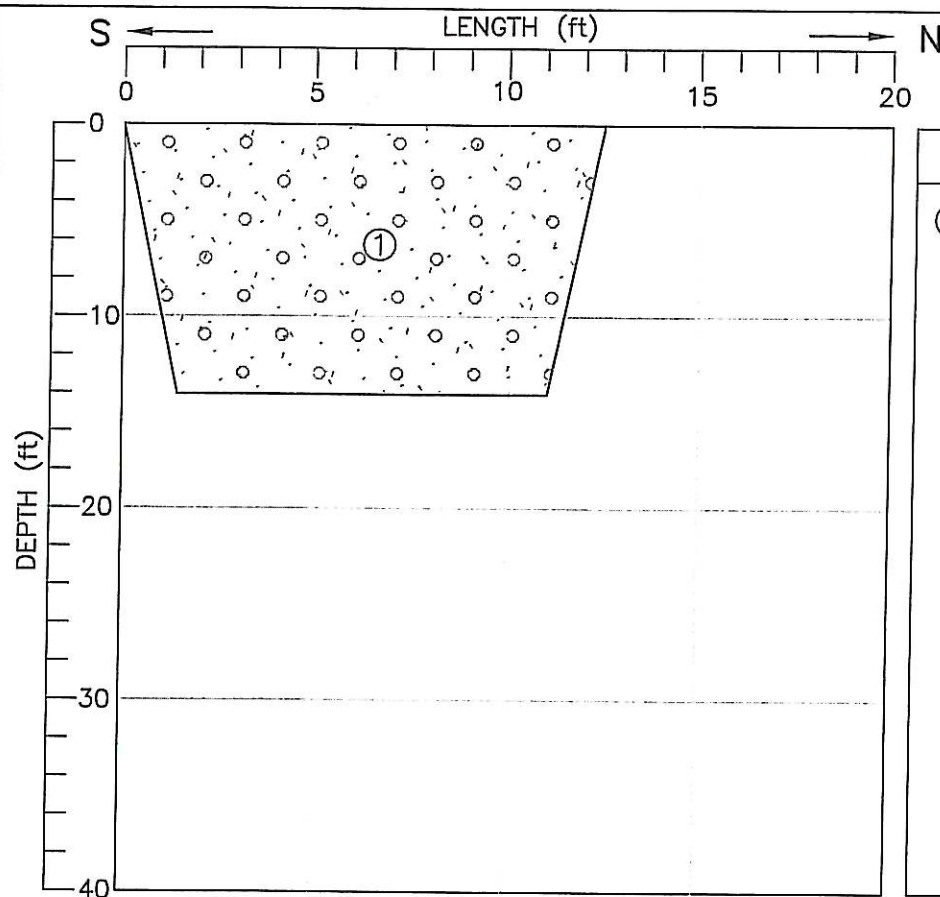
No groundwater encountered during excavation.

Test pit backfilled with excavated material.

Field density tests performed in vicinity of test pit.

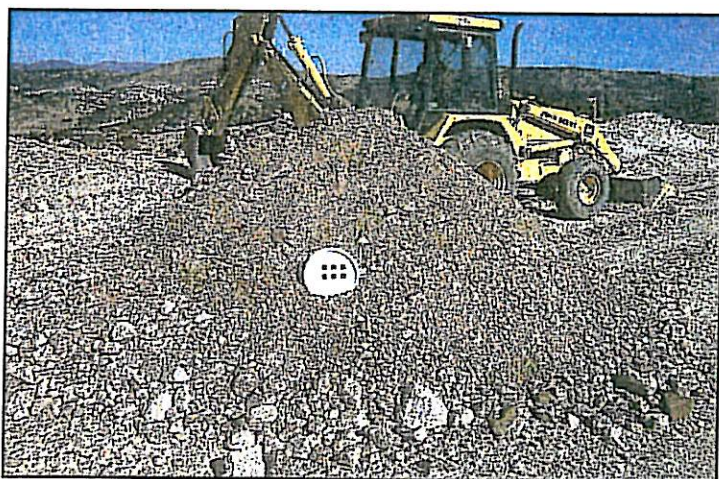
FIELD TEST PIT LOG

TEMP 40 °F WEATHER CLEAR TEST PIT GTP-05
 EQUIPMENT DEERE 510C BACKHOE ENGINEER G. TORTELLI OPERATOR SAL
 ELEVATION 6180 FT. CONTRACTOR HAMILTON DATE JANUARY 7, 2000
 LOCATION 16559.15 N, 13426.10 E DATUM MSL JOB 993-2546
1D STOCKPILE (NORTH SIDE)



LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Well Graded Gravel with Sand. Little cobbles and boulders to 24 inches. Fine to coarse angular gravel. Some fine to coarse sand. Few low plasticity fines. Slightly moist-moist. Medium dense.



SAMPLES

NO.	DESCRIPTION
—	Point Load Clasts

NOTES:

80% Soil Matrix
20% Oversized Clasts

Excavation terminated at 14 foot depth.

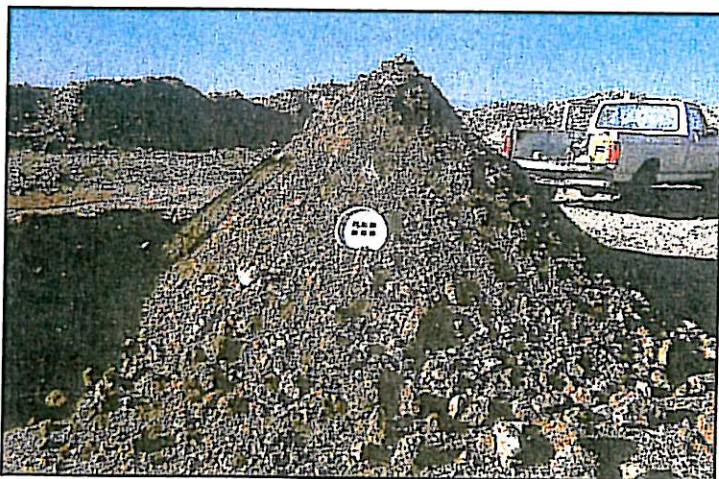
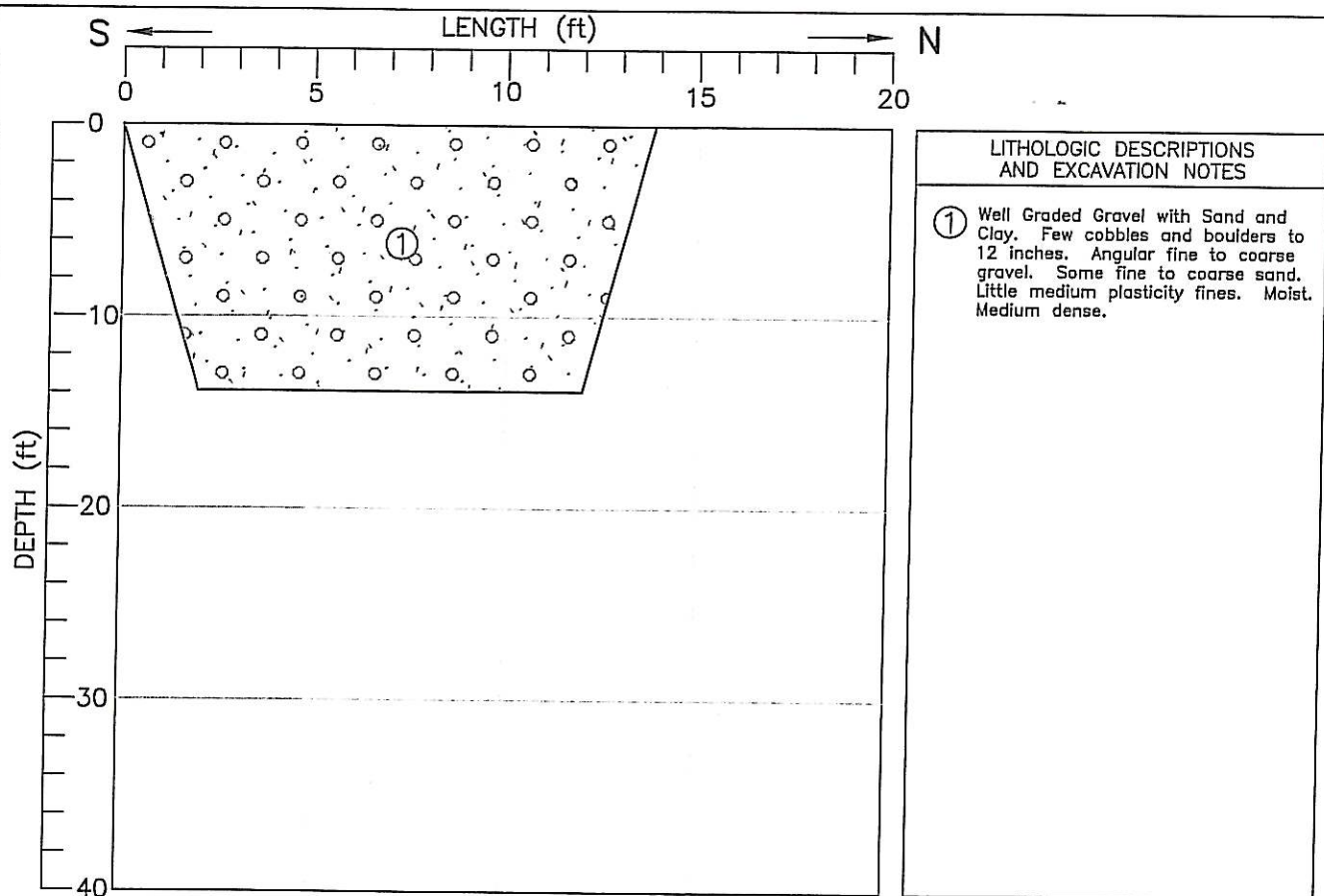
No groundwater encountered during excavation.

Test pit backfilled with excavated material.

Field density tests performed in vicinity of test pit.

FIELD TEST PIT LOG

TEMP 45 °F WEATHER CLEAR TEST PIT GTP-06
 EQUIPMENT DEERE 510C BACKHOE ENGINEER G. TORTELLI OPERATOR SAL
 ELEVATION 6310 FT. CONTRACTOR HAMILTON DATE JANUARY 7, 2000
 LOCATION 11230.62 N, 4038.42 E DATUM MSL JOB 993-2546
2, 2A STOCKPILE GROUP (NORTH)



SAMPLES	
NO.	DESCRIPTION
S-03	Bulk
-	Point Load Clasts

NOTES:

80% Soil Matrix
 20% Oversized Clasts

Excavation terminated at 14.5 foot depth.

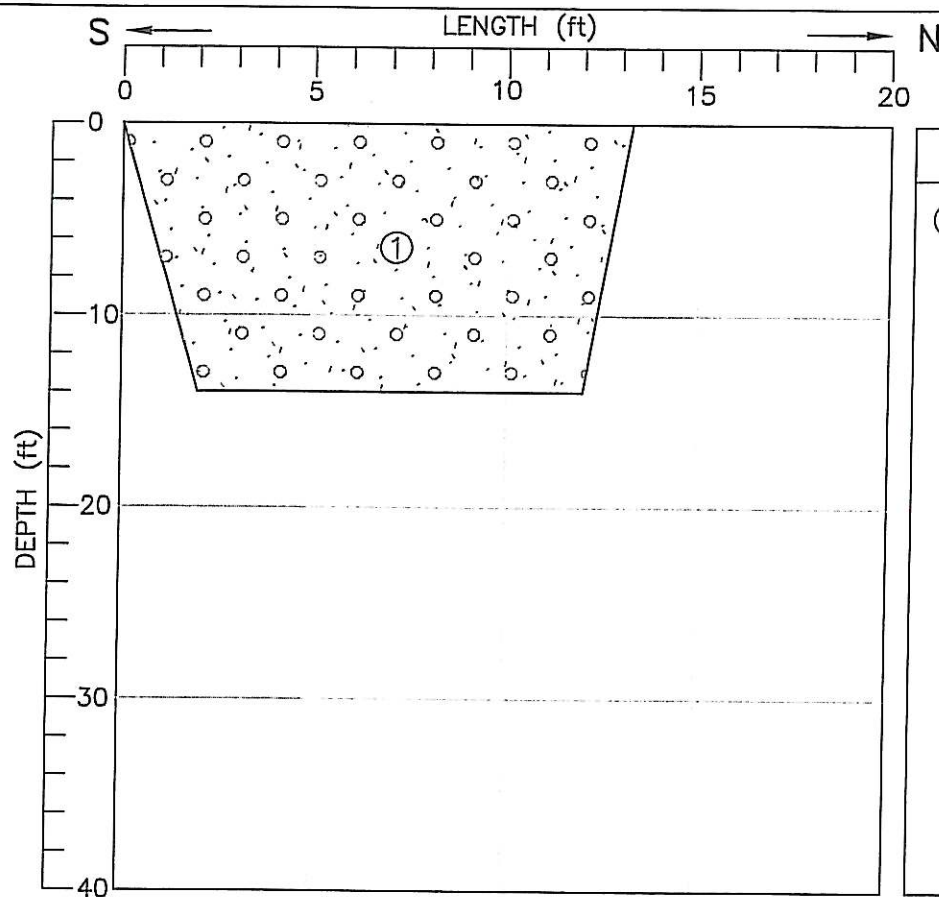
No groundwater encountered during excavation.

Test pit backfilled with excavated material.

Field density tests performed in vicinity of test pit.

FIELD TEST PIT LOG

TEMP 30 °F WEATHER CLEAR TEST PIT GTP-07
 EQUIPMENT DEERE 510C BACKHOE ENGINEER G. TORTELLI OPERATOR SAL
 ELEVATION 6380 FT. CONTRACTOR HAMILTON DATE JANUARY 8, 2000
 LOCATION 13435.50 N, 4693.92 E DATUM MSL JOB 993-2546
2, 2A STOCKPILE GROUP (NORTH)



LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Well Graded Gravel with Sand and Clay. Occasional cobbles and boulders to 24 inches. Fine to coarse angular gravel. Some fine to coarse sand. Little medium plasticity fines. Moist. Medium dense.



SAMPLES

NO.	DESCRIPTION
-	Point Load Clasts

NOTES:

80% Soil Matrix
20% Oversized Clasts

Excavation terminated at 14.5 foot depth.

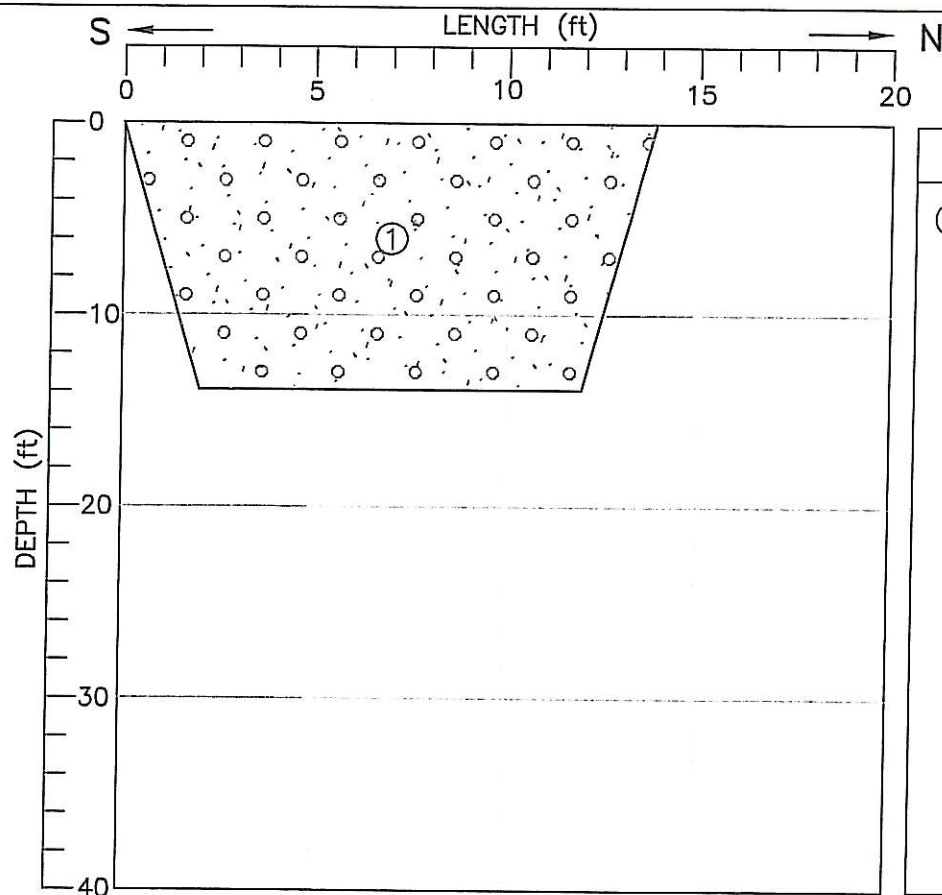
No groundwater encountered during excavation.

Test pit backfilled with excavated material.

Field density tests performed in vicinity of test pit.

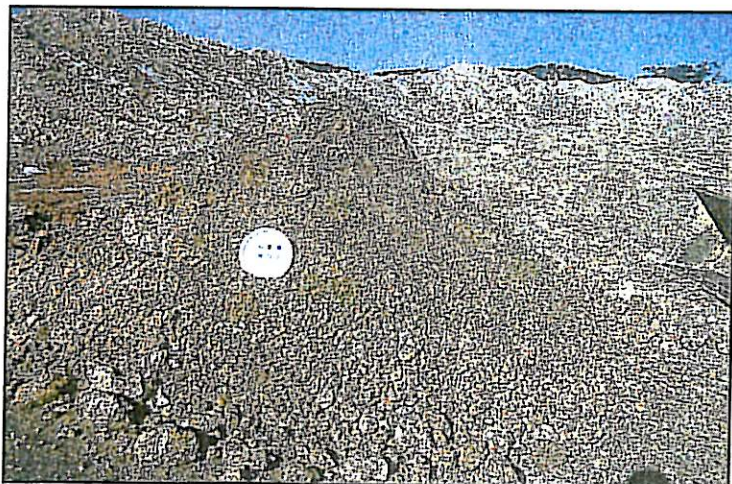
FIELD TEST PIT LOG

TEMP 30 °F WEATHER CLEAR TEST PIT GTP-08
 EQUIPMENT DEERE 510C BACKHOE ENGINEER G. TORTELLI OPERATOR SAL
 ELEVATION 6270 FT. CONTRACTOR HAMILTON DATE JANUARY 8, 2000
 LOCATION 9437.31 N, 4407.96 E DATUM MSL JOB 993-2546
2, 2A STOCKPILE GROUP (NORTH)



LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Well Graded Gravel with Sand and Clay. Occasional Cobbles and boulders to 24 inches. Fine to coarse angular gravel. Some fine to coarse sand. Little medium plasticity fines. Moist. Medium dense.



SAMPLES

NO.	DESCRIPTION
—	Point Load Clasts

NOTES:

85% Soil matrix.
15% Oversized Clasts.

Excavation terminated at 14 foot depth.

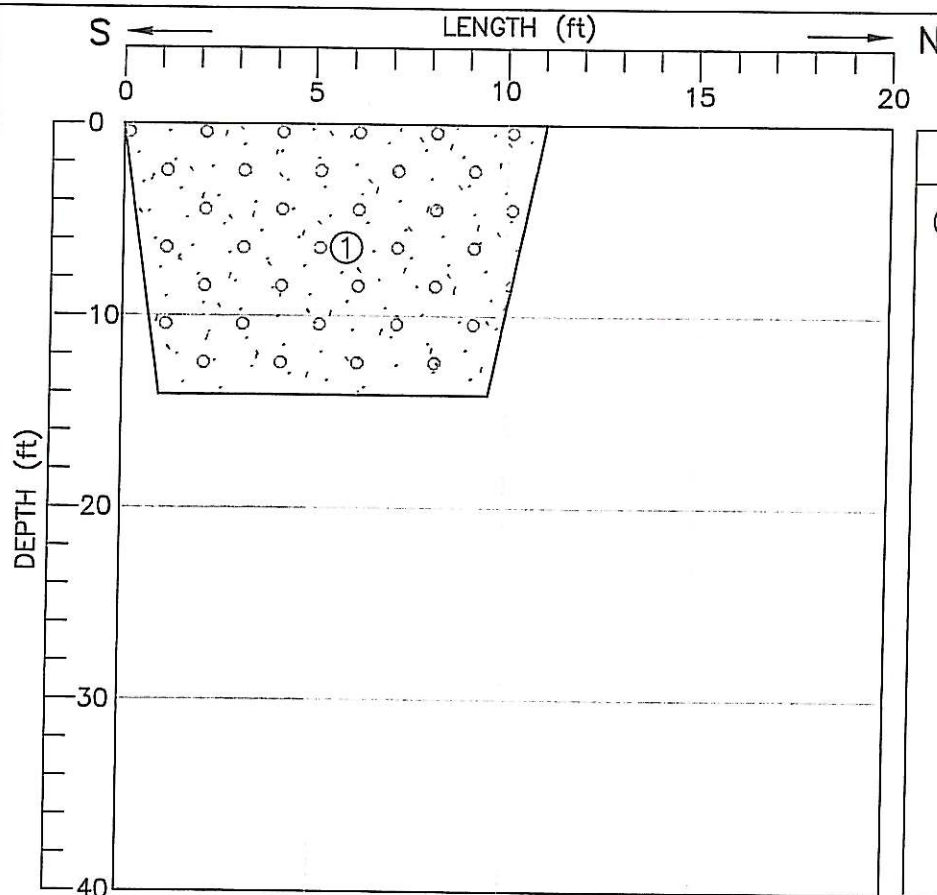
No groundwater encountered during excavation.

Test pit backfilled with excavated material.

Field density tests performed in vicinity of test pit.

FIELD TEST PIT LOG

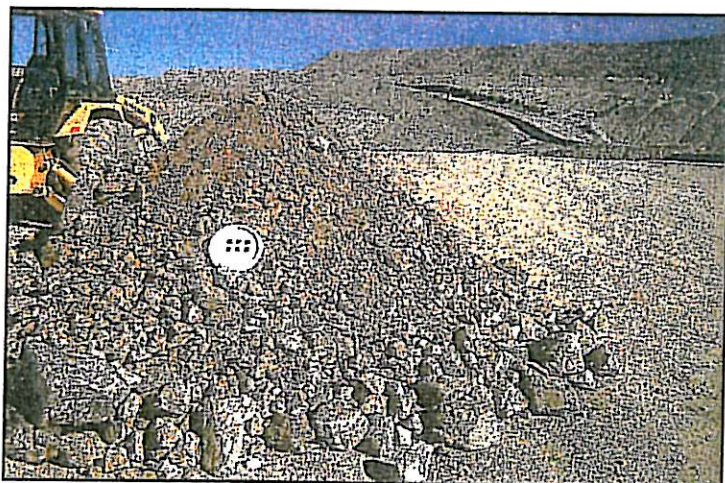
TEMP 40 °F WEATHER CLEAR TEST PIT GTP-09
 EQUIPMENT DEERE 510C BACKHOE ENGINEER G. TORTELLI OPERATOR SAL
 ELEVATION 6400 FT. CONTRACTOR HAMILTON DATE JANUARY 8, 2000
 LOCATION 6909.68 N, 6075.43 E DATUM MSL JOB 993-2546
2, 2A STOCKPILE GROUP (SOUTH)



LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Well Graded Gravel with Sand. Frequent cobbles and boulders to 24". Fine to coarse angular gravel. Some fine to coarse sand. Little medium plasticity fines. Moist. Medium Dense.

Cracking and vertical displacement of 12 to 18 inches observed near the crest of slope. Displacement is located at contact between stockpile and pit wall. Displacement most likely due to settlement.



SAMPLES

NO.	DESCRIPTION
—	Point Load Clasts
S-05	Bulk

NOTES:

55% Soil Matrix
45% Oversized Clasts

Excavation terminated at 14 foot depth.

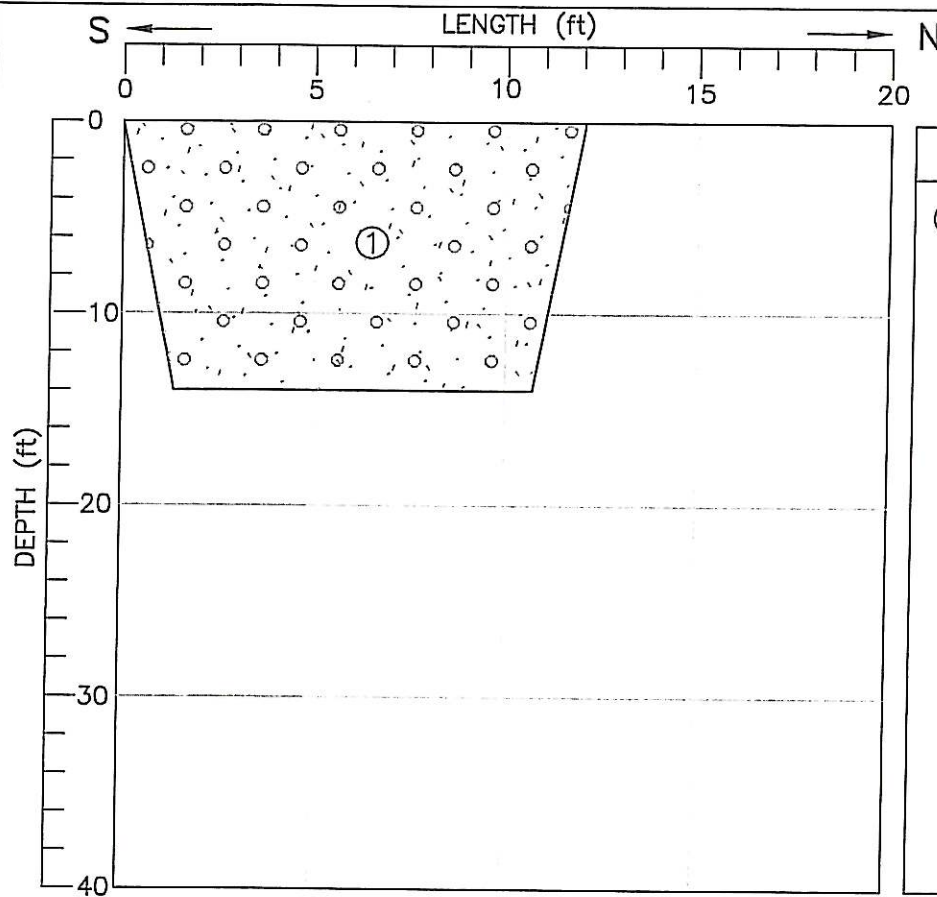
No groundwater encountered during excavation.

Test pit backfilled with excavated material.

Field density tests performed in vicinity of test pit.

FIELD TEST PIT LOG

TEMP 40 °F WEATHER CLEAR TEST PIT GTP-10
 EQUIPMENT DEERE ENGINEER G. TORTELLI OPERATOR SAL
 ELEVATION 6450 FT. CONTRACTOR HAMILTON DATE JANUARY 8, 2000
 LOCATION 3545.95 N, 11415.52 E DATUM MSL JOB 993-2546
 SOUTH RIM GROUP



LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Well Graded Gravel with Sand and Clay. Occasional cobbles and boulders to 24 inches. Fine to coarse angular gravel. Some fine to coarse sand. Some medium plasticity fines.



SAMPLES

NO.	DESCRIPTION
S-04	Bulk
-	Point Load Clasts

NOTES:

75% Soil Matrix
25% Oversized Clasts

Excavation terminated at 14.5 foot depth.

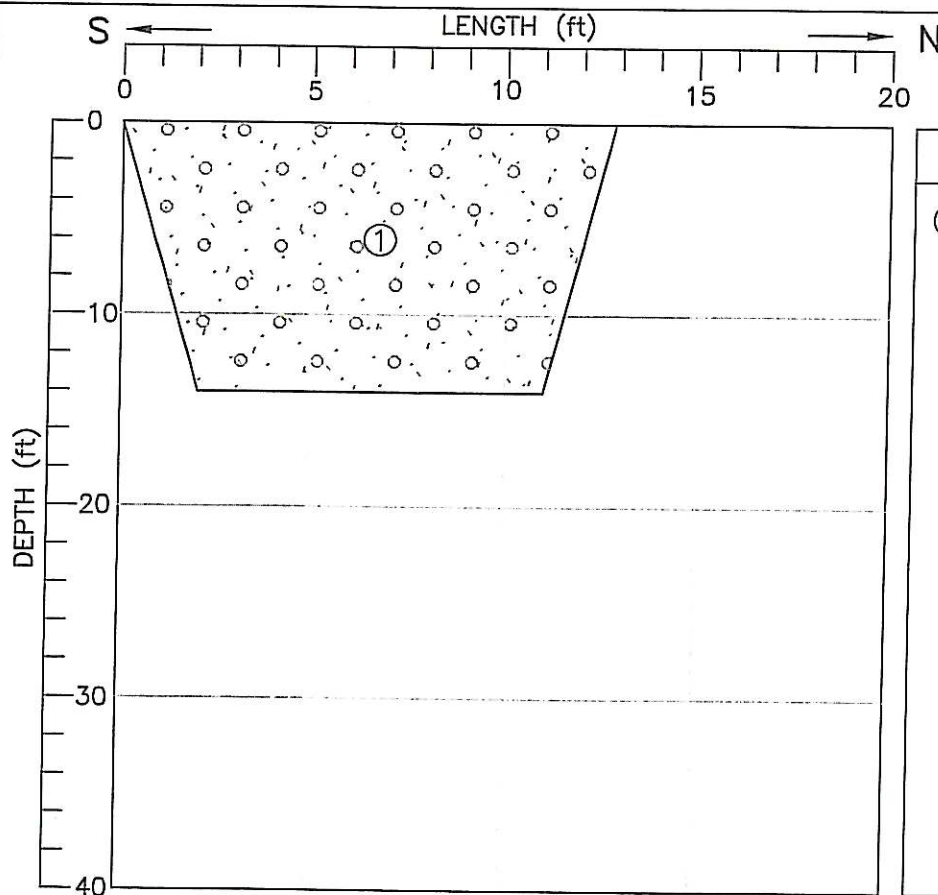
No groundwater encountered during excavation.

Test pit backfilled with excavated material.

Field density tests performed in vicinity of test pit.

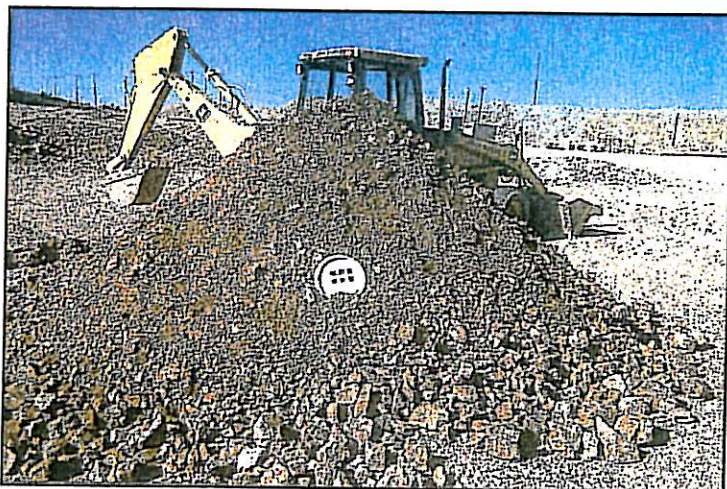
FIELD TEST PIT LOG

TEMP 40 °F WEATHER CLEAR TEST PIT GTP-11
 EQUIPMENT DEERE 510C BACKHOE ENGINEER G. TORTELLI OPERATOR SAL
 ELEVATION 6400 FT. CONTRACTOR HAMILTON DATE JANUARY 8, 2000
 LOCATION 4636.20 N, 13141.36 E DATUM MSL JOB 993-2546
 SOUTH RIM GROUP



LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Well Graded Gravel with Sand. Frequent cobbles and boulders to 24 inches. Fine to coarse angular gravel. Some fine to coarse sand. Little medium plasticity fines. Slightly moist. Dense.
- Occasional boulder/cobble clast supported pockets. Slight sloughing due to disturbance of clast pockets.



SAMPLES

NO.	DESCRIPTION
—	Point Load Clasts

NOTES:

80% Soil Matrix
20% Oversized Clasts

Excavation terminated at 14.5 foot depth.

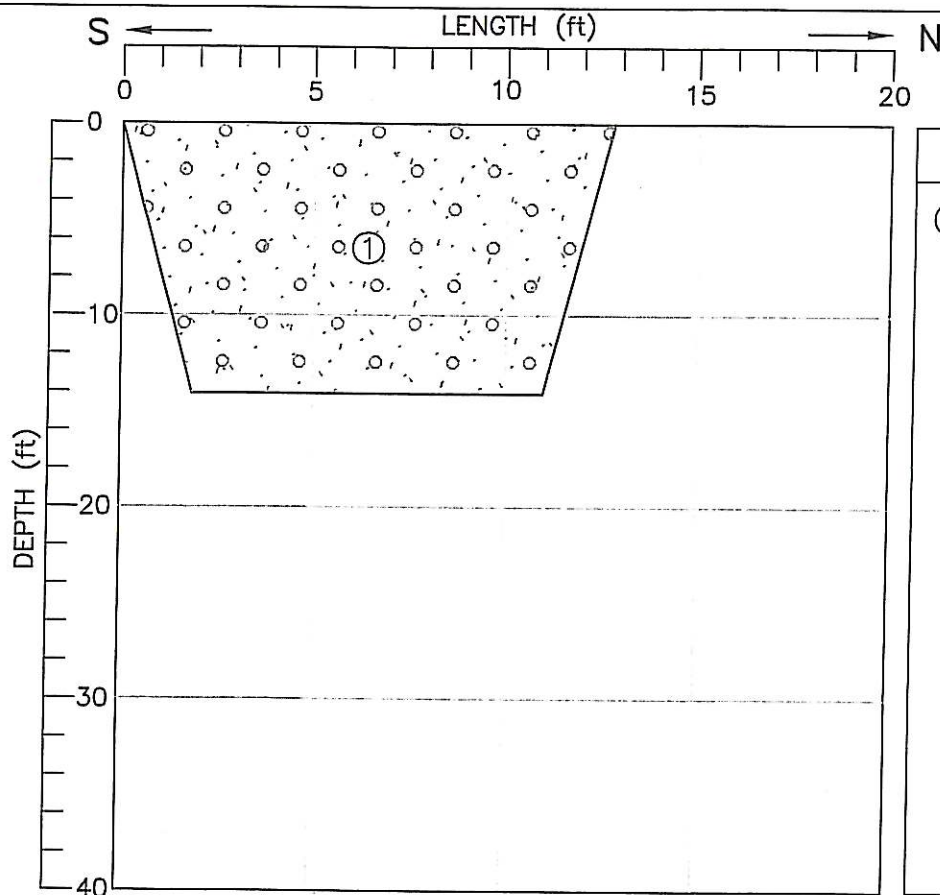
No groundwater encountered during excavation.

Test pit backfilled with excavated material.

Field density tests performed in vicinity of test pit.

FIELD TEST PIT LOG

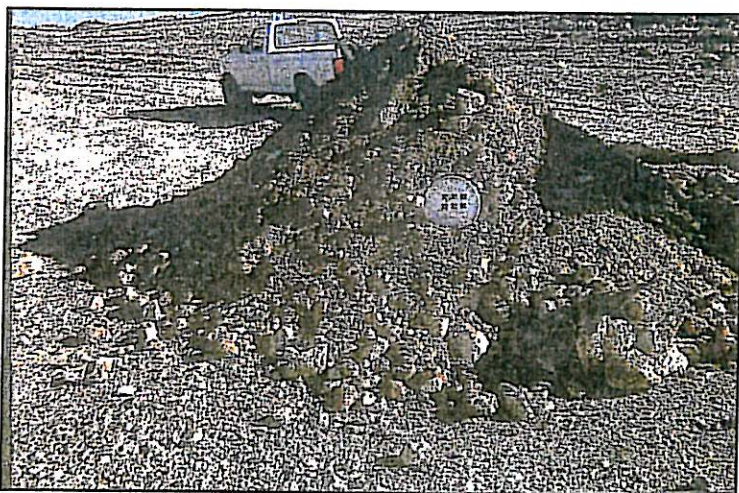
TEMP 45 °F WEATHER CLEAR TEST PIT GTP-12
 EQUIPMENT DEERE 510C BACKHOE ENGINEER G. TORTELLI OPERATOR SAL
 ELEVATION 6250 FT. CONTRACTOR HAMILTON DATE JANUARY 8, 2000
 LOCATION 5541.28 N, 16737.02 E DATUM MSL JOB 993-2546
 1 A,B,C STOCKPILE GROUP



LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Well Graded Gravel with Sand. Occasional cobbles and boulders to 36 inches. Fine to coarse angular gravel. Some fine to coarse sand. Little medium plasticity fines. Moist. Dense.

Hard digging due to several large boulders.



SAMPLES

NO.	DESCRIPTION
—	Point Load Clasts

NOTES:

75% Soil Matrix
25% Oversized Clasts

Excavation terminated at 14.5 foot depth.

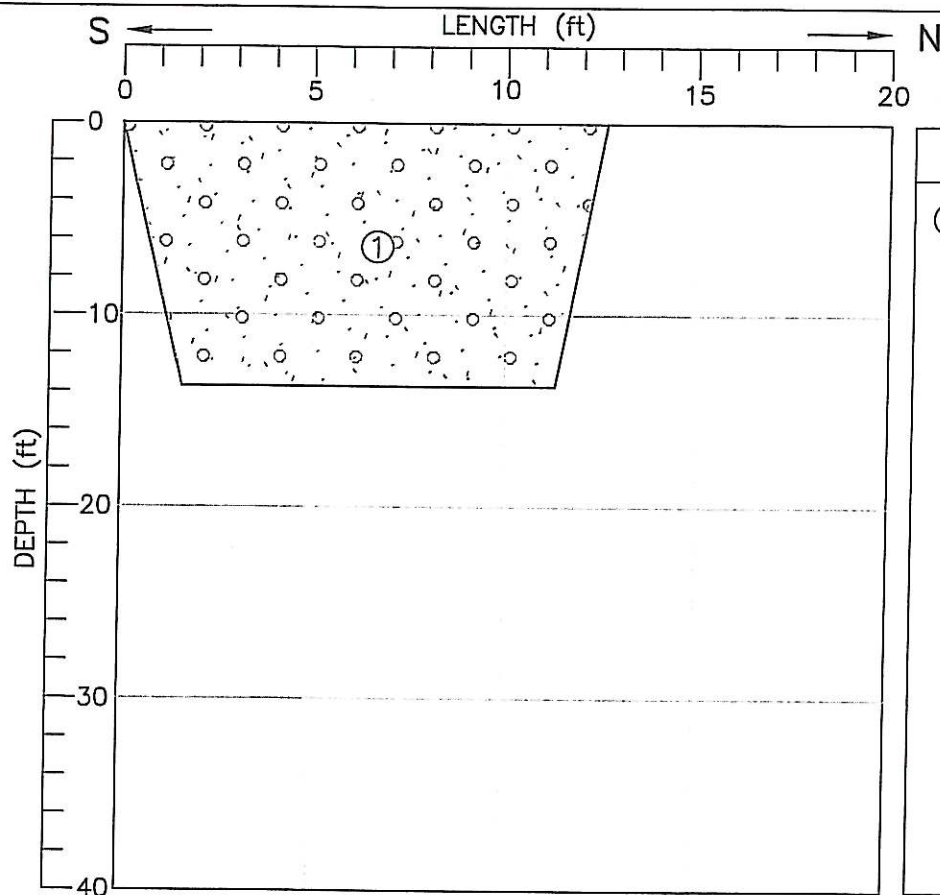
No groundwater encountered during excavation.

Test pit backfilled with excavated material.

Field density tests performed in vicinity of test pit.

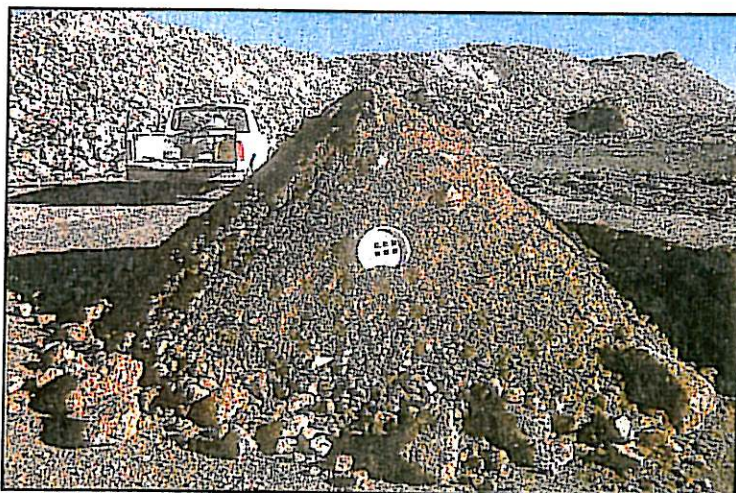
FIELD TEST PIT LOG

TEMP 45 °F WEATHER CLEAR TEST PIT GTP-13
 EQUIPMENT DEERE 510C BACKHOE ENGINEER G. TORTELLI OPERATOR SAL
 ELEVATION 6250 FT. CONTRACTOR HAMILTON DATE JANUARY 8, 2000
 LOCATION 6622.14 N, 18449.57 E DATUM MSL JOB 993-2546
 1 A,B,C STOCKPILE GROUP



LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Well Graded Gravel with Sand. Occasional cobbles and boulders to 24 inches. Fine to coarse angular gravel. Some fine to coarse sand. Little medium plasticity fines.



SAMPLES

NO.	DESCRIPTION
S-06	Bulk
-	Point Load Clasts

NOTES:

80% Soil Matrix
20% Oversized Clasts

Excavation terminated at 14.5 foot depth.

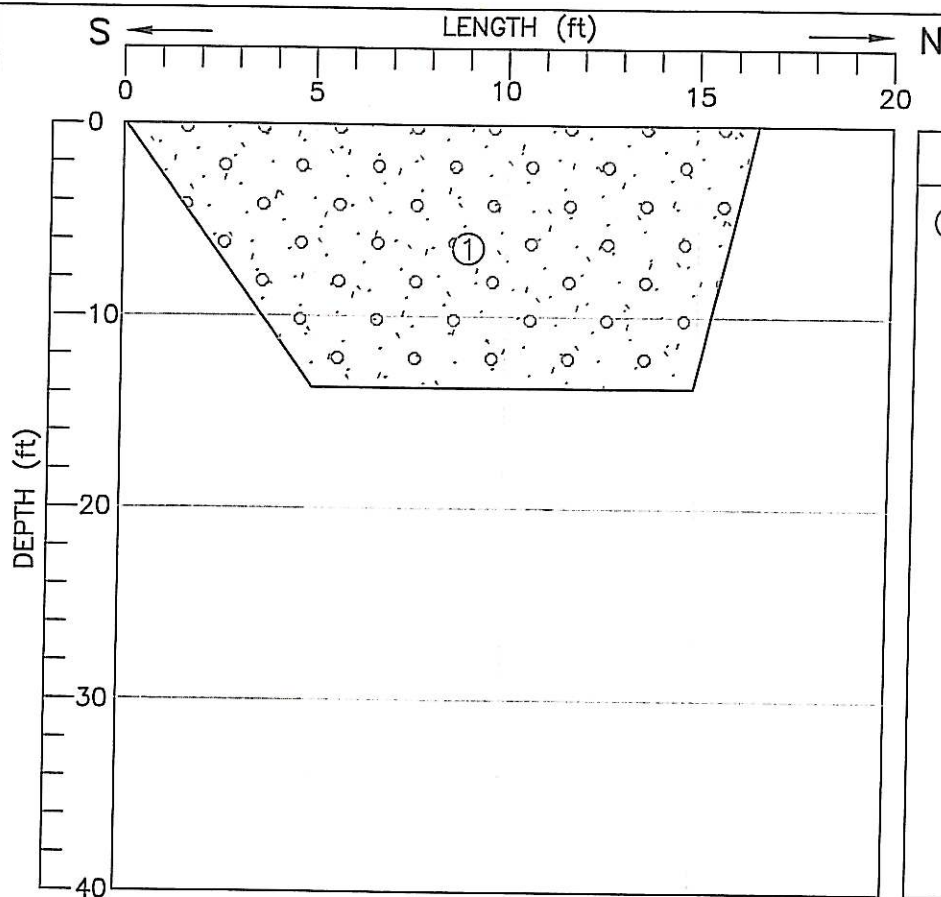
No groundwater encountered during excavation.

Test pit backfilled with excavated material.

Field density tests performed in vicinity of test pit.

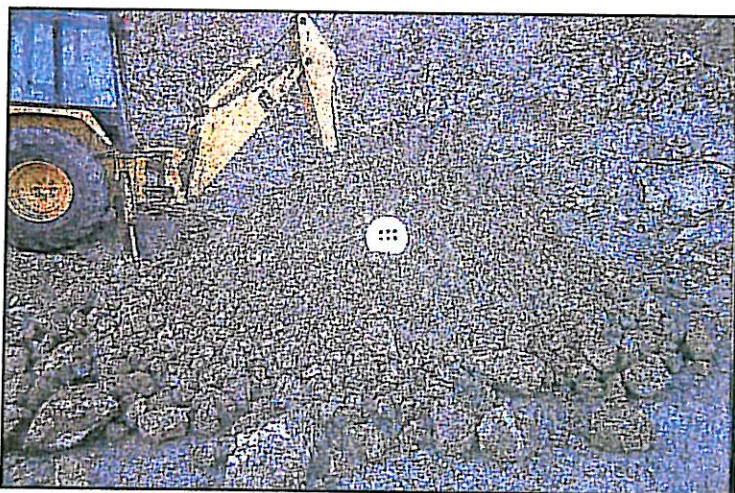
FIELD TEST PIT LOG

TEMP 40 °F WEATHER CLEAR TEST PIT GTP-14
 EQUIPMENT DEERE 510C BACKHOE ENGINEER G. TORTELLI OPERATOR SAL
 ELEVATION 6150 FT. CONTRACTOR HAMILTON DATE JANUARY 8, 2000
 LOCATION 19063.47 N, 10230.42 E 3A STOCKPILE DATUM MSL JOB 993-2546



LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Well Graded Gravel with Sand. Frequent cobbles and boulders at 36 inches. Fine to coarse angular gravel. Some fine to coarse sand. Little medium plasticity fines. Moist to very moist. Dense.
- Currently under leach.



SAMPLES

NO.	DESCRIPTION
S-07	Bulk
-	Point Load Clasts

NOTES:

65% Soil Matrix
 35% Oversized Clasts
 Excavation terminated at 14.5 foot depth.
 No groundwater encountered during excavation.
 Test pit backfilled with excavated material.
 Field density tests performed in vicinity of test pit.

APPENDIX A-2
2001 TEST PIT LOGS

FIELD TEST PIT LOG

TEMP 75 °F WEATHER SUNNY TEST PIT TYTP01-1
 EQUIPMENT NONE ENGINEER M. GRASS OPERATOR NONE
 ELEVATION ± 6270 CONTRACTOR NONE DATE 10/11/01
 LOCATION SOUTH END OF 2, 2A STOCKPILE GROUP DATUM MSL JOB 013-1595

PHOTOGRAPH



SAMPLES	
NO.	DEPTH
TYTP01-1	BULK

LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Clayey GRAVEL with sand (GC), and little fines, angular to subangular gravel, c-f sand, and medium plasticity fines, light brown, moist, no HCl reaction, strong sulfate(?) cementation. 40 percent cobbles and boulders up to 30", porphyry, medium strong rock (R3), jarositic oxidation, brown, yellow to buff in color, minor copper oxides.

TIME	DEPTH OF HOLE	DEPTH TO W/L
13:10	---	---

SPECIAL NOTES:

GPS COORDINATES (NAD83, UTM):
 12 S 0744617
 3614571
 MINE COORDINATE CONVERSION
 10882N, 4748E

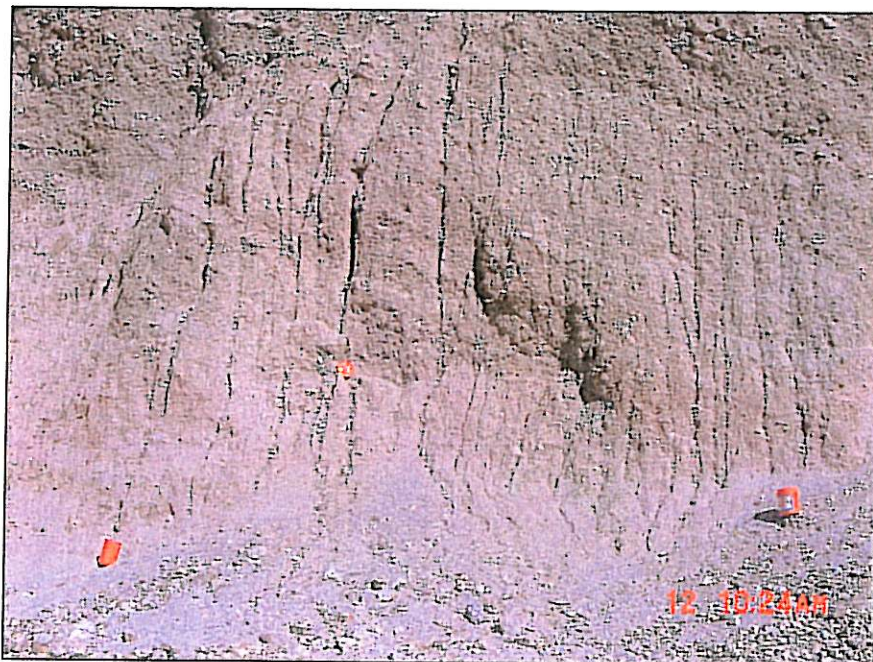
LOG OF EXISTING BENCH CUT

SAMPLES:
 2 5-GALLON BUCKETS TAKEN FROM
 SLOUGH AT TOE OF SLOPE OF -3"
 MATERIAL.

FIELD TEST PIT LOG

TEST PIT TYTP01-10
 TEMP 72 °F WEATHER SUNNY, WINDY ENGINEER M. GRASS OPERATOR NONE
 EQUIPMENT NONE CONTRACTOR NONE DATE 10/12/01
 ELEVATION ± 6200 DATUM MSL JOB 013-1595
 LOCATION TOP OF NORTH EAST WALL OF SAVANNA PIT

PHOTOGRAPH



SAMPLES	
NO.	DEPTH
TYTP01-10	BULK

LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Clayey SAND with gravel (SW), angular to subrounded gravel, c-f sand, and little low plasticity fines, light brown, moist, no HCl reaction, not cemented. 50 percent cobbles and boulders up to 24", granite and porphyry, weak to medium strong rock (R2-R3), leach cap.

TIME	DEPTH OF HOLE	DEPTH TO W/L
11:05	---	---

SPECIAL NOTES:

GPS COORDINATES (NAD83, UTM):
 12 S 0747561
 3615014
 MINE COORDINATE CONVERSION
 12245N, 14429E

LOG OF EXISTING BENCH CUT

SAMPLES:
 2 5-GALLON BUCKETS TAKEN FROM
 SLOUGH AT TOE OF SLOPE OF -3"
 MATERIAL.

FIELD TEST PIT LOG

TEMP 75 °F WEATHER SUNNY, WINDY TEST PIT TYTP01-11
 EQUIPMENT NONE ENGINEER M. GRASS OPERATOR NONE
 ELEVATION ± 5850 CONTRACTOR NONE DATE 10/12/01
 LOCATION SOUTH END OF 2, 2A STOCKPILE GROUP DATUM MSL JOB 013-1595

PHOTOGRAPH



SAMPLES	
NO.	DEPTH
TYTP01-11	BULK

LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Clayey GRAVEL with sand (GC), angular to subrounded gravel, c-f sand, and little medium plasticity fines, light brown, moist, no HCl reaction, not cemented. 10 percent cobbles and boulders up to 24", porphyry, medium strong rock (R3), leach cap.

TIME	DEPTH OF HOLE	DEPTH TO W/L
11:50	---	---

SPECIAL NOTES:

GPS COORDINATES (NAD83, UTM):
 12 S 0744531
 3614488
 MINE COORDINATE CONVERSION
 10612N, 4473E

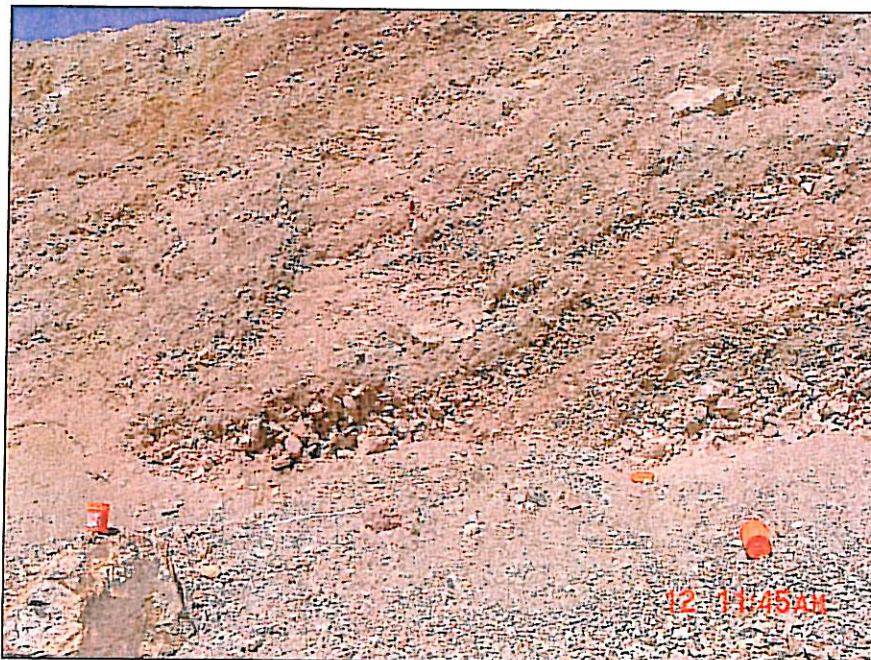
LOG OF EXISTING BENCH CUT

SAMPLES:
 2 5-GALLON BUCKETS TAKEN FROM
 SLOUGH AT TOE OF SLOPE OF -3"
 MATERIAL.

FIELD TEST PIT LOG

TEST PIT TYTP01-12
 TEMP 75 °F WEATHER SUNNY, WINDY ENGINEER M. GRASS OPERATOR NONE
 EQUIPMENT NONE CONTRACTOR NONE DATE 10/12/01
 ELEVATION ± 6275 DATUM MSL JOB 013-1595
 LOCATION NORTH WEST WALL OF MAIN PIT, NO. 3 LEACH STOCKPILE

PHOTOGRAPH



SAMPLES	
NO.	DEPTH
TYTP01-12	BULK

LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Clayey GRAVEL with sand (GC), angular to subangular gravel, c-f sand, little low plasticity fines, light brown, moist, no HCl reaction, strong sulfate(?) cementation. 40 percent cobbles and boulders up to 30", porphyry and granite, medium strong rock (R3), leach cap, copper percipitate on rock surfaces.

TIME	DEPTH OF HOLE	DEPTH TO W/L
12:30	---	---

SPECIAL NOTES:

GPS COORDINATES (NAD83, UTM):
 12 S 0746437
 3615941
 MINE COORDINATE CONVERSION
 15321N, 10770E

LOG OF EXISTING BENCH CUT

SAMPLES:
 2 5-GALLON BUCKETS TAKEN FROM
 SLOUGH AT TOE OF SLOPE OF -3"
 MATERIAL.

FIELD TEST PIT LOG

TEMP 75 °F WEATHER SUNNY TEST PIT TYTP01-2
 EQUIPMENT NONE ENGINEER M. GRASS OPERATOR NONE
 ELEVATION 6100 CONTRACTOR NONE DATE 10/11/01
 LOCATION EAST END, BOTTOM OF COPPER MOUNTAIN PIT DATUM MSL JOB 013-1595

PHOTOGRAPH



SAMPLES	
NO.	DEPTH
TYTP01-2	BULK

LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Well graded GRAVEL with clay and sand (GW-GC), angular to subrounded gravel, c-f sand, and low to medium plasticity fines, light yellowish brown, moist, no HCl reaction, minor sulfate(?) cementation. 30 percent cobbles and boulders up to 24", porphyry, medium strong to strong rock (R3-R4), leached.

TIME	DEPTH OF HOLE	DEPTH TO W/L
14:15	---	---

SPECIAL NOTES:

GPS COORDINATES (NAD83, UTM):
 12 S 0745105
 3613809
 MINE COORDINATE CONVERSION
 8367N, 6335E

LOG OF EXISTING BENCH CUT

SAMPLES:
 2 5-GALLON BUCKETS TAKEN FROM
 SLOUGH AT TOE OF SLOPE OF -3"
 MATERIAL.

FIELD TEST PIT LOG

TEMP 75 °F WEATHER SUNNY TEST PIT TYTP01-3
 EQUIPMENT NONE ENGINEER M. GRASS OPERATOR NONE
 ELEVATION ± 6200 CONTRACTOR NONE DATE 10/11/01
 LOCATION EAST END, MID-HEIGHT OF COPPER MOUNTAIN PIT DATUM MSL JOB 013-1595

PHOTOGRAPH



SAMPLES	
NO.	DEPTH
TYTP01-3	BULK

LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Poorly Graded GRAVEL with clay and sand (GP-GC), angular to subangular gravel, c-f sand, and low to medium plasticity fines, light brown, moist, no HCl reaction, minor sulfate(?) cementation. 50 percent cobbles and boulders up to 36", porphyry and granite, medium strong to strong rock (R3-R4), leach cap, waste rock.

TIME	DEPTH OF HOLE	DEPTH TO W/L
15:00	---	---

SPECIAL NOTES:

GPS COORDINATES (NAD83, UTM):
 12 S 0745199
 3613782
 MINE COORDINATE CONVERSION
 8276N, 6643E

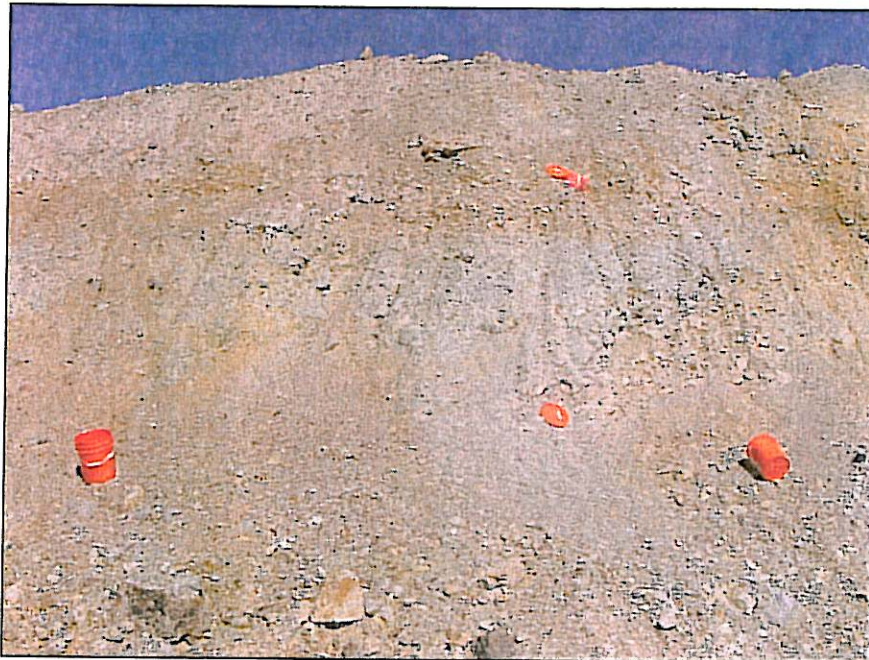
LOG OF EXISTING BENCH CUT

SAMPLES:
 2 5-GALLON BUCKETS TAKEN FROM
 SLOUGH AT TOE OF SLOPE OF -3"
 MATERIAL.

FIELD TEST PIT LOG

TEST PIT TYTP01-4
 TEMP 75 °F WEATHER SUNNY, WINDY ENGINEER M. GRASS OPERATOR NONE
 EQUIPMENT NONE CONTRACTOR NONE DATE 10/11/01
 ELEVATION ± 6350 DATUM MSL JOB 013-1595
 LOCATION EAST END, TOP OF COPPER MOUNTAIN PIT

PHOTOGRAPH



SAMPLES	
NO.	DEPTH
TYTP01-4	BULK

LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Clayey GRAVEL with sand (GC), angular to subangular gravel, c-f sand, and medium plasticity fines, light yellowish brown, moist, no HCl reaction, strong sulfate(?) cementation. 15 percent cobbles and boulders up to 30", porphyry, medium strong to strong rock (R3-R4), leached.

TIME	DEPTH OF HOLE	DEPTH TO W/L
15:35	---	---

SPECIAL NOTES:

GPS COORDINATES (NAD83, UTM):
 12 S 0744984
 3614048
 MINE COORDINATE CONVERSION
 9155N, 5946E

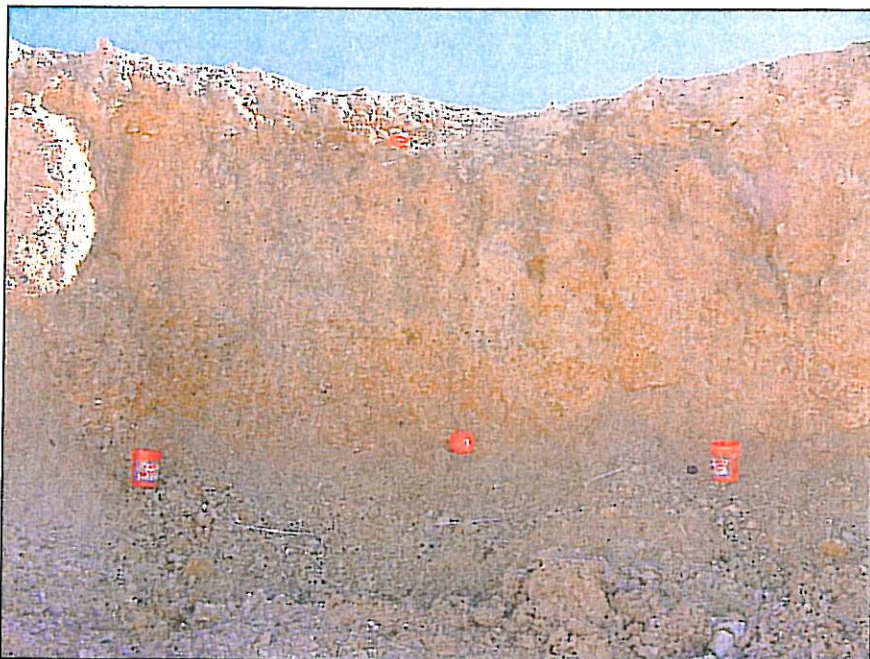
LOG OF EXISTING BENCH CUT

SAMPLES:
 2 5-GALLON BUCKETS TAKEN FROM
 SLOUGH AT TOE OF SLOPE OF -3"
 MATERIAL.

FIELD TEST PIT LOG

TEMP 75 °F WEATHER SUNNY, WINDY TEST PIT TYTP01-5
 EQUIPMENT NONE ENGINEER M. GRASS OPERATOR NONE
 ELEVATION 6400 CONTRACTOR NONE DATE 10/11/01
 LOCATION NORTH END OF 4C STOCKPILE DATUM MSL JOB 013-1595

PHOTOGRAPH



SAMPLES	
NO.	DEPTH
TYTP01-5	BULK

LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Clayey GRAVEL with sand (GC), angular to subangular well graded gravel, c-f sand, and low to medium plasticity fines, light yellowish brown, moist, no HCl reaction, strong sulfate(?) cementation. 10 percent cobbles and boulders up to 24", porphyry, weak rock (R2), leached.

TIME	DEPTH OF HOLE	DEPTH TO W/L
16:15	---	---

SPECIAL NOTES:

GPS COORDINATES (NAD83, UTM):
 12 S 0744937
 3613233
 MINE COORDINATE CONVERSION
 6483N, 5767E

LOG OF EXISTING BENCH CUT

SAMPLES:
 2 5-GALLON BUCKETS TAKEN FROM
 SLOUGH AT TOE OF SLOPE OF -3"
 MATERIAL.

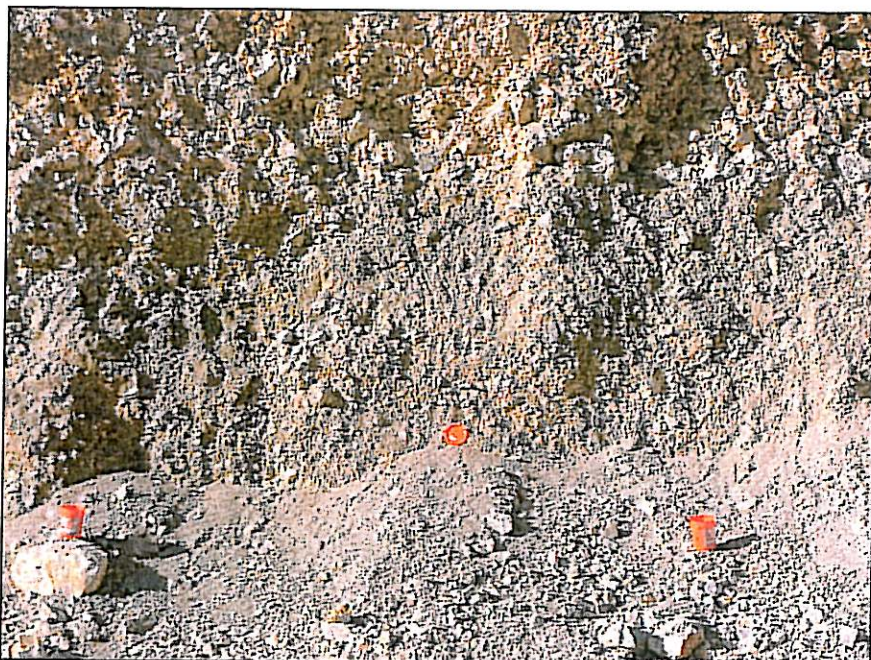
APPENDIX A-3

2004 1C STOCKPILE TEST PIT LOGS

FIELD TEST PIT LOG

TEMP 55 °F WEATHER SUNNY, WINDY TEST PIT TYTP01-6
 EQUIPMENT NONE ENGINEER M. GRASS OPERATOR NONE
 ELEVATION ± 5925 CONTRACTOR NONE DATE 10/12/01
 LOCATION WEST END, BOTTOM OF SAVANNA PIT DATUM MSL JOB 013-1595

PHOTOGRAPH



SAMPLES	
NO.	DEPTH
TYTP01-6	BULK

LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Well graded GRAVEL with clay and sand (GW-GC), angular to subrounded gravel, c-f sand, and low plasticity fines, yellowish brown, moist, no HCl reaction, strong sulfate(?) cementation. 50 percent cobbles and boulders up to 30", granite, medium strong to strong rock (R3-R4), leached.

TIME	DEPTH OF HOLE	DEPTH TO W/L
8:50	---	---

SPECIAL NOTES:

GPS COORDINATES (NAD83, UTM):
 12 S 0747329
 3614199
 MINE COORDINATE CONVERSION
 9579N, 13643E

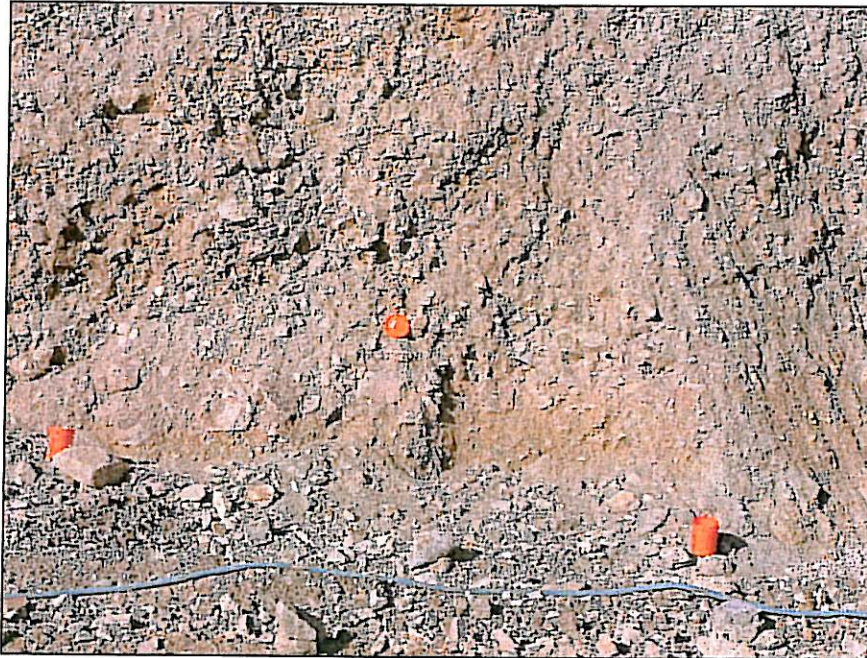
LOG OF EXISTING BENCH CUT

SAMPLES:
 2 5-GALLON BUCKETS TAKEN FROM
 SLOUGH AT TOE OF SLOPE OF -3"
 MATERIAL.

FIELD TEST PIT LOG

TEST PIT TYTP01-7
 TEMP 65 °F WEATHER SUNNY, WINDY ENGINEER M. GRASS OPERATOR NONE
 EQUIPMENT NONE CONTRACTOR NONE DATE 10/12/01
 ELEVATION ± 5975 DATUM MSL JOB 013-1595
 LOCATION WEST END, 2 BENCHES FROM BOTTOM OF SAVANNA PIT

PHOTOGRAPH



SAMPLES	
NO.	DEPTH
TYTP01-7	BULK

LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Clayey SAND with gravel (SC), subangular to subrounded well graded gravel, c-f sand, and low to medium plasticity fines, light brown, moist, no HCl reaction, strong sulfate(?) cementation. 40 percent cobbles and boulders up to 30", porphyry and granite, medium strong to strong rock (R3-R4) with minor weak rock (R2), leached.

TIME	DEPTH OF HOLE	DEPTH TO W/L
9:25	---	---

SPECIAL NOTES:

GPS COORDINATES (NAD83, UTM):
 12 S 0747218
 3614300
 MINE COORDINATE CONVERSION
 9914N, 13282E

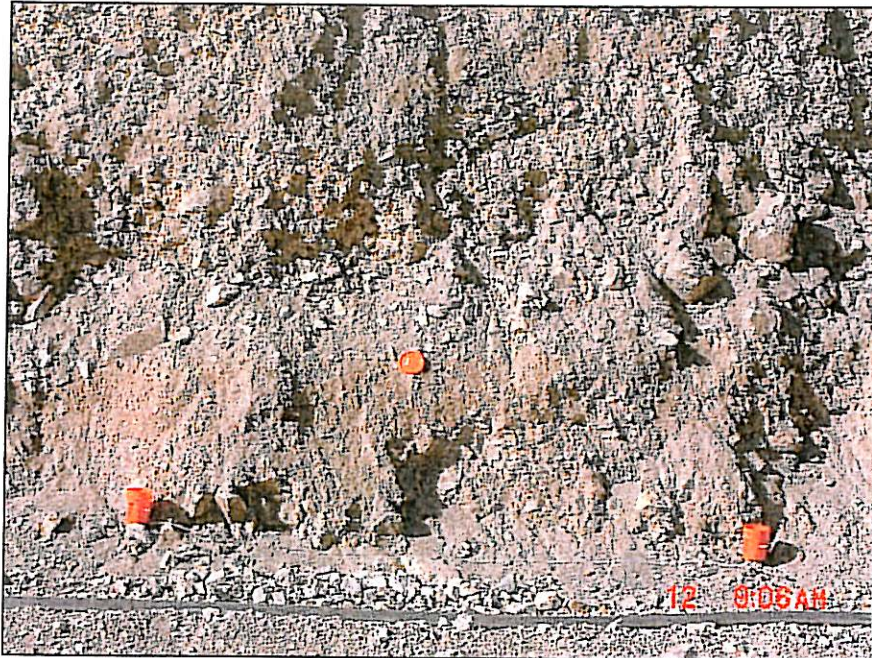
LOG OF EXISTING BENCH CUT

SAMPLES:
 2 5-GALLON BUCKETS TAKEN FROM
 SLOUGH AT TOE OF SLOPE OF -3"
 MATERIAL.

FIELD TEST PIT LOG

TEMP 70 °F WEATHER SUNNY, WINDY TEST PIT TYTP01-8
 EQUIPMENT NONE ENGINEER M. GRASS OPERATOR NONE
 ELEVATION ± 6025 CONTRACTOR NONE DATE 10/12/01
 LOCATION WEST END, 4 BENCHES FROM BOTTOM OF SAVANNA PIT DATUM MSL JOB 013-1595

PHOTOGRAPH



SAMPLES	
NO.	DEPTH
TYTP01-8	BULK

LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Clayey SAND with gravel (SC), subangular to subrounded gravel, c-f well graded sand, and low to medium plasticity fines, light brown, moist, no HCl reaction, strong sulfate(?) cementation. 30 percent cobbles and boulders up to 30", porphyry and granite, medium strong to strong rock (R3-R4), leached.

TIME	DEPTH OF HOLE	DEPTH TO W/L
9:50	---	---

SPECIAL NOTES:

GPS COORDINATES (NAD83, UTM):
 12 S 0747295
 3614153
 MINE COORDINATE CONVERSION
 9429N, 13524E

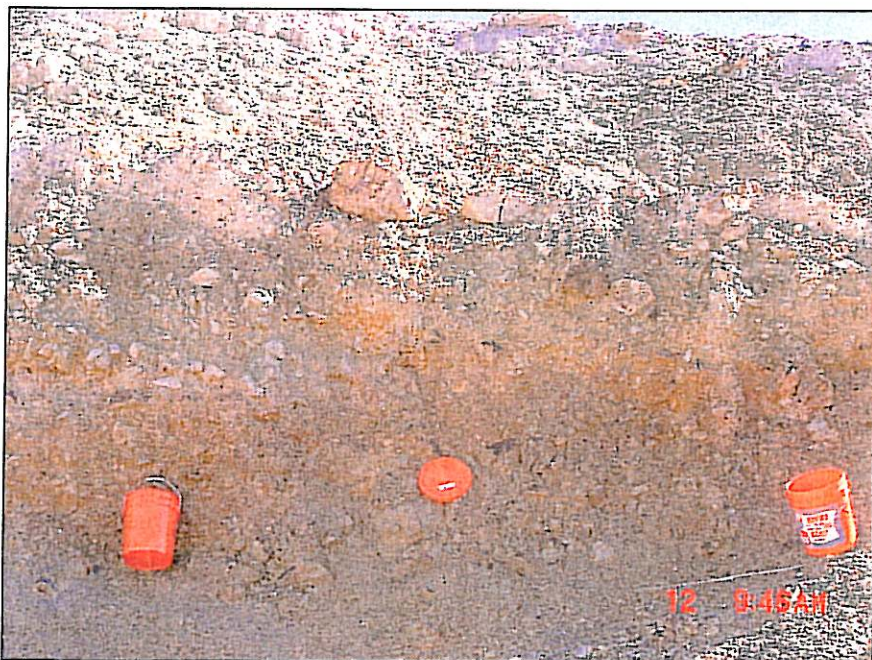
LOG OF EXISTING BENCH CUT

SAMPLES:
 2 5-GALLON BUCKETS TAKEN FROM
 SLOUGH AT TOE OF SLOPE OF -3"
 MATERIAL.

FIELD TEST PIT LOG

TEST PIT TYTP01-9
 TEMP 70 °F WEATHER SUNNY, WINDY ENGINEER M. GRASS OPERATOR NONE
 EQUIPMENT NONE CONTRACTOR NONE DATE 10/12/01
 ELEVATION ± 6150 DATUM MSL JOB 013-1595
 LOCATION TOP OF NORTH EAST WALL OF GETTYSBURG PIT

PHOTOGRAPH



SAMPLES	
NO.	DEPTH
TYTP01-9	BULK

LITHOLOGIC DESCRIPTIONS AND EXCAVATION NOTES

- ① Clayey GRAVEL with sand (GC), angular to subrounded gravel, c-f sand, and low to medium plasticity fines, light yellowish brown, moist, no HCl reaction, strong sulfate(?) cementation. 30 percent cobbles and boulders up to 30", porphyry and granite, medium strong rock (R3), leach cap.

TIME	DEPTH OF HOLE	DEPTH TO W/L
10:30	----	----

SPECIAL NOTES:

GPS COORDINATES (NAD83, UTM):
 12 S 0748472
 3613744
 MINE COORDINATE CONVERSION
 8051N, 17379E

LOG OF EXISTING BENCH CUT

SAMPLES:
 2 5-GALLON BUCKETS TAKEN FROM
 SLOUGH AT TOE OF SLOPE OF -3"
 MATERIAL.

STOCKPILE BENCH FACE LOG

053-2550

Project: Tyrone Stockpile Characterization
Project No.:
Location: Tyrone Mine 1 C Stockpile
Cell No. GA04-TY-1

Date
Coordinates: 3613259 N 748460 E
Elevation:



Soil Fraction

60% fine to coarse gravel

25% fine to coarse sand

15% low to medium plastic fines

Clayey Gravel with Sand (GC). Reddish brown (10R4/6), dry becoming moist 6" below surface. Slough at toe of the cut has cemented surface crust (sulfate?). Noncalcareous.

Oversize

10% >3"

80% 3" to 6"

15% 6" to 12"

5% >12"

Leach Cap. Bleached porphyry. 80%. R3, 2-10 mm feldspar in fine grained tan to greenish groundmass w/ Tan to black fracture coating. 10% is R4 w/ qtz selvages, py casts and partial silicification. 10% R2, pervasive argillic

Maximum vertical slopes in cut approx. 18 feet.



STOCKPILE BENCH FACE LOG

053-2550

Project: Tyrone Stockpile Characterization
Project No.:
Location: Tyrone Mine 1 C Stockpile
Cell No. GA04-TY-2

Date
Coordinates: 3613446 N 748669 E
Elevation:



Soil Fraction

50% fine to coarse gravel

30% fine to coarse sand

20% low to medium plastic fines

Clayey Gravel with Sand (GC). Reddish brown (10R4/6), dry becoming moist 6" below surface. Noncalcareous.

Oversize

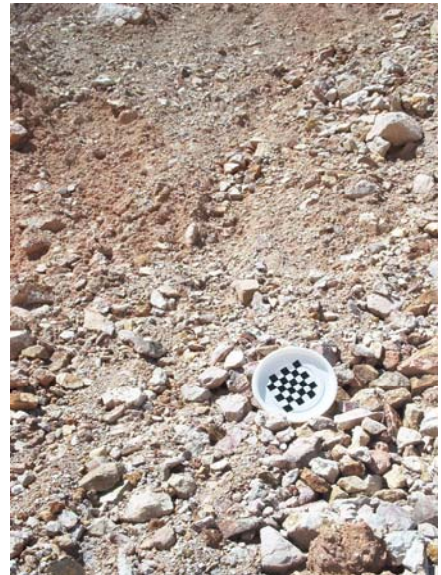
10% >3"

3" to 6" 80%

6" to 12" 15%

>12" 5%

Leach Cap. Bleached porphyry. Typ 1-5 mm feldspar phenos in lt. gray matrix w/ tan to black fracture oxides, variably bleached and silicified, R2 to R4. Trace gossan (R0). Trace holocrystalline granite w. K-spar, qtz, hb, and fg plag. (R5). Typically R3.



STOCKPILE BENCH FACE LOG

053-2550

Project: Tyrone Stockpile Characterization
Project No. 043-2572
Location: Tyrone Mine 1 C Stockpile
Cell No. GA04-TY-3

Date Sept. 8, 2004
Coordinates: 3613207 N 748461 E
Elevation:



Soil Fraction

60% fine to coarse gravel

25% fine to coarse sand

15% low to medium plastic fines

Clayey Gravel with Sand (GC). Yellowish orange (10YR 8/6). dry becoming moist 6" below surface. Noncalcareous, nonmagnetic.

Oversize

50% >3"

3" to 6" 50%

6" to 12" 30%

>12" 20%

Max 72"

Pervasively silicified groundmass, clay altered feldspar phenos, tan to black fract oxide, black to brassy cubic py < 1 mm, other finer grained black specks disseminated near py. Typ. R3.



Samples GA04-TY-3-GC 1, 5 gal bucket
GA04-TY-3-GT 2, 5 gal buckets + 1, 1 gal ziplock

Notes:

Max. vertical cut slope 32'

STOCKPILE BENCH FACE LOG

053-2550

Project: Tyrone Stockpile Characterization
Project No. 043-2572
Location: Tyrone Mine 1 C Stockpile
Cell No. GA04-TY-4

Date Sept. 8, 2004
Coordinates: 3613382 N 748688 E
Elevation:



Soil Fraction

85% fine to coarse gravel

10% fine to coarse sand

5% low to medium plastic fines

Poorly graded Gravel with Clay and Sand. Reddish (10YR 6/6). dry becoming moist 6" below surface. Noncalcareous, nonmagnetic.

Oversize

40% >3"

3" to 6" 70%

6" to 12" 20%

12" to 24" 8%

>24 2%

max 36"

Porphyry w/ lt. gray siliceous matrix, dism. oxidized sulfides, milky euhedral feldspar, some black tabular sulfides(?). Fracture surfaces have lt. reddish brown to tan and dk. brown. Consistent R4.



Samples GA04-TY-4-GC 1, 5 gal bucket
GA04-TY-4-GT 2, 5 gal buckets

Notes:

Max. vertical cut slope 25'

STOCKPILE BENCH FACE LOG

053-2550

Project: Tyrone Stockpile Characterization
Project No. 043-2572
Location: Tyrone Mine 1 C Stockpile
Cell No. GA05-TY-5

Date Sept. 8, 2004
Coordinates:
Elevation:

3613172 N

748781 E



Soil Fraction

55% fine to coarse gravel

25% fine to coarse sand

20% low to medium plastic fines

Clayey Gravel with Sand. Pale reddish brown (10YR 6/6). Dry. Mod. calcareous, nonmagnetic.

Oversize

15% >3"

3" to 6" 50%

6" to 12" 30%

12" to 24" 10%

>24" 8%

max 72"

Typically bleached granite w/ 50% pink feldspar 40% greenish gray feldspar intergrown, anhedral to subhedral, 10% euhedral to subhedral biotite, R3). Unbleached is R4.



Samples GA04-TY-5-GC 1, 5 gal bucket
GA04-TY-5-GT 2, 5 gal buckets + 1, 1 gal ziplock

Notes:

STOCKPILE BENCH FACE LOG

053-2550

Project: Tyrone Stockpile Characterization
Project No. 043-2572
Location: Tyrone Mine 1 C Stockpile
Cell No. GA04-TY-6

Date: Sept. 8, 2004
Coordinates: 3613436 N 748721 E
Elevation:



Soil Fraction

50% fine to coarse gravel

30% fine to coarse sand

20% low to medium plastic fines

Clayey Gravel with Sand. Reddish orange (10YR 6/6). Dry to moist. Noncalcareous, nonmagnetic.

Oversize

20% >3"

3" to 6" 85%

6" to 12" 10%

12" to 24" 3%

>24" 2%

max. 36"

Porphyry. White feldspathic matrix. Locally 50% euhedral qtz eyes. 3% to 5% fine grained dism. black spherical oxidized sulfides, anhedral biotite and biotite books up to 2 mm. Tan to black fracture oxides, R3.



Samples GA04-TY-6-GC 1, 5 gal bucket
GA04-TY-6-GT 2, 5 gal buckets + 1, 1 gal ziplock

Notes:

STOCKPILE BENCH FACE LOG

053-2550

Project: Tyrone Stockpile Characterization
Project No. 043-2572
Location: Tyrone Mine 1 C Stockpile
Cell No. GA04-TY-7

Date Sept. 8, 2004
Coordinates: 3613306 N 748702 E
Elevation:



Soil Fraction

50% fine to coarse gravel
30% fine to coarse sand
20% low to medium plastic fines
Clayey Gravel with Sand. Reddish orange (10YR 6/6). Dry to moist. Noncalcareous, nonmagnetic.

Oversize

20% >3"
3" to 6" 60%
6" to 12" 35%
12" to 24" 10%
>24" 5
max. 36"

Granite. 2 feldspar w/ qtz eyes and dism. py and biotite approx. 1 mm. Also propyry with a silicic greenish matrix w/ remnant feldspar phenos and abun. sulfides disseminated and along fract. Tan and black fract. oxides.



Samples GA04-TY-7-GC 1, 5 gal bucket
GA04-TY-7-GT 2, 5 gal buckets + 1, 1 gal ziplock

Notes:

STOCKPILE BENCH FACE LOG

053-2550

Project: Tyrone Stockpile Characterization
Project No. 043-2572
Location: Tyrone Mine 1 C Stockpile
Cell No. GA04-TY-8

Date Sept. 8, 2004
Coordinates: 3613414 N 748777 E
Elevation:



Soil Fraction

55% fine to coarse gravel

25% fine to coarse sand

20% low to medium plastic fines

Clayey Gravel with Sand. Reddish brown (10YR 6/6). Dry to moist. Noncalcareous, nonmagnetic.

Oversize

30% >3"

3" to 6" 45%

6" to 12" 35%

12" to 24" 10%

>24" 10%

max. 60"

Predominantly greenish silicic matrix w/ remnant cloudy feldspar phenos. 3-5% dism. and fract. coating sulfides (py, cpy).



Samples GA04-TY-8-GC 1, 5 gal bucket
GA04-TY-8-GT 2, 5 gal buckets + 1, 1 gal ziplock

Notes:

APPENDIX B
BOREHOLE LOGS

APPENDIX B-1

2004 ROTOSONIC DRILLING LOGS

ROTOSONIC COREHOLE LOG - BOREHOLE TSGT- 01

Project: G&K/Stockpile Characterization

Datum:

Collar Elev: 6207.6

Project No.: 043-2572

Drill date: 10/13/2004

Coordinates N: 19405.4 E: 9536.6

Sheet 1 of 11

Location: Tyrone Mine

Drill rig:

Azimuth: N/A

Inclination: 90°

Depth (m)	Soil / Rock Type		Group Symbol	Color code (Munsell)	D-dry SM-slightly moist M-moist VM-very moist S-saturated	Core recovery (%)					% 3-inch plus (%)					Maximum particle size in.					ISRM Strength Index for CLASTS						D-Point Load-diametral A-Point Load-axial S-Seive PI-Plasticity Index	Notes Test Results																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																				
	Description (i.e. Group Name, % fines/sand/gravel, plasticity, color, HCL reaction, cementation, angularity, odor, structure)	Moisture																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																														
						0-20	20-40	40-60	60-80	80+	0-3	3-8	8-20	20-50	50+	1	2	3	4	5+	R0	R1	R2	R3	R4	R5				R6																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																		
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0	0'-8': Clayey GRAVEL with sand, GC, 40% gravel, 30% sand, 30% clay, yellowish gray (5Y 7/2), slightly moist and loose. Coarse: subangular. Fines: medium plasticity.	GC	5Y 7/2	SM																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																												</

Scale:

Drilling Contractor:

Driller:

USC (MPa)

R0	0.25-1.0
R1	1.0-5.0
R2	5.0-25
R3	25-50
R4	50-100
R5	100-250
R6	>250



ROTOSONIC COREHOLE LOG - BOREHOLE TSGT- 01

Project: G&K/Stockpile Characterization

Datum:

Collar Elev: 6207.6

Project No.: 043-2572

Drill date: 10/13/2004

Coordinates N: 19405.4 E: 9536.6

Sheet 2 of 11

Location: Tyrone Mine

Drill rig:

Azimuth: N/A

Inclination: 90°

Depth (m)	Soil / Rock Type		Group Symbol	Color code (Munsell)	D-dry SM-slightly moist M-moist VM-very moist S-saturated	Core recovery (%)				% 3-inch plus (%)				Maximum particle size in.				ISRM Strength Index for CLASTS						Physical Testing	Notes Test Results																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																															
	Description (i.e. Group Name, % fines/sand/gravel, plasticity, color, HCL reaction, cementation, angularity, odor, structure)	Moisture				0-20	20-40	40-60	60-80	80+	0-3	3-8	8-20	20-50	50+	1	2	3	4	5+	R0	R1	R2				R3	R4	R5	R6																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																										
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ROTOSONIC COREHOLE LOG - BOREHOLE TSGT- 01

Project: G&K/Stockpile Characterization

Datum:

Collar Elev: 6207.6

Project No.: 043-2572

Drill date: 10/13/2004

Coordinates N: 19405.4 E: 9536.6

Sheet 3 of 11

Location: Tyrone Mine

Drill rig:

Azimuth: N/A

Inclination: 90°

Depth (m)	Soil / Rock Type		Group Symbol	Color code (Munsell)	D-dry SM-slightly moist M-moist VM-very moist S-saturated	Core recovery (%)					% 3-inch plus (%)					Maximum particle size in.					ISRM Strength Index for CLASTS						D-Point Load-diametral	Notes Test Results																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																	
	Description (i.e. Group Name, % fines/sand/gravel, plasticity, color, HCL reaction, cementation, angularity, odor, structure)	Moisture																									A-Point Load-axial																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																		
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Scale:

Drilling Contractor:


Driller:

USC (MPa)

R0	0.25-1.0
R1	1.0-5.0
R2	5.0-25
R3	25-50
R4	50-100
R5	100-250
R6	>250



Project:	G&K/Stockpile Characterization			Datum:				Collar Elev:	6207.6	
Project No.:	043-2572	Drill date:	10/13/2004	Coordinates	N:	19405.4	E:	9536.6	Sheet	4 of 11
Location:	Tyrone Mine	Drill rig:		Azimuth:	N/A				Inclination:	90°

		USC (MPa)		
Scale:		R0	0.25-1.0	
		R1	1.0-5.0	
Drilling Contractor:		R2	5.0-25	
		R3	25-50	
Driller:		R4	50-100	
		R5	100-250	
		R6	>250	

ROTOSONIC COREHOLE LOG - BOREHOLE TSGT- 01

Project: G&K/Stockpile Characterization

Datum:

Collar Elev: 6207.6

Project No.: 043-2572

Drill date: 10/13/2004

Coordinates N: 19405.4 E: 9536.6

Sheet 5 of 11

Location: Tyrone Mine

Drill rig:

Azimuth: N/A

Inclination: 90°

Depth (m)	Soil / Rock Type		Group Symbol	Color code (Munsel)	D-dry SM-slightly moist M-moist VM-very moist S-saturated	Core recovery (%)					% 3-inch plus (%)					Maximum particle size in.					ISRM Strength Index for CLASTS						D-Point Load-diametral				Notes Test Results				
	Description (i.e. Group Name, % fines/sand/gravel, plasticity, color, HCL reaction, cementation, angularity, odor, structure)																										Physical Testing								
						Moisture	0-20	20-40	40-60	60-80	80+	0-3	3-8	8-20	20-50	50+	1	2	3	4	5+	R0	R1	R2	R3	R4						R5	R6		

160	Continued	CH	5Y 7/2	SM																												160		
-																																		-
-																																		-
-																																		-
165	164'-178': Clayey GRAVEL, GC, 60% gravel, 10% sand, 30% clay, yellowish gray to light brown (5Y 7/2 to 5YR 5/6), slightly moist and medium dense. Coarse: angular. Fines: high plasticity.	GC	5Y 7/2 - 5YR 5/6	SM																												165		
-																																		-
-																																		-
-																																		-
170	164'-178': Clayey GRAVEL, GC, 60% gravel, 10% sand, 30% clay, yellowish gray to light brown (5Y 7/2 to 5YR 5/6), slightly moist and medium dense. Coarse: angular. Fines: high plasticity.	GC	5Y 7/2 - 5YR 5/6	SM																												170		
-																																		-
-																																		-
-																																		-
175	164'-178': Clayey GRAVEL, GC, 60% gravel, 10% sand, 30% clay, yellowish gray to light brown (5Y 7/2 to 5YR 5/6), slightly moist and medium dense. Coarse: angular. Fines: high plasticity.	GC	5Y 7/2 - 5YR 5/6	SM																												175		
-																																		-
-																																		-
-																																		-
180	178'-190': Gravelly CLAY, CH, 40% gravel, 10% sand, 50% clay, yellowish gray to light brown (5Y 7/2 to 5YR 5/6), slightly moist and loose to medium dense. Coarse: subangular. Fines: high plasticity.	CH	5Y 7/2 - 5YR 5/6	SM																												180		
-																																		-
-																																		-
-																																		-
185	178'-190': Gravelly CLAY, CH, 40% gravel, 10% sand, 50% clay, yellowish gray to light brown (5Y 7/2 to 5YR 5/6), slightly moist and loose to medium dense. Coarse: subangular. Fines: high plasticity.	CH	5Y 7/2 - 5YR 5/6	SM																												185		
-																																		-
-																																		-
-																																		-
190	178'-190': Gravelly CLAY, CH, 40% gravel, 10% sand, 50% clay, yellowish gray to light brown (5Y 7/2 to 5YR 5/6), slightly moist and loose to medium dense. Coarse: subangular. Fines: high plasticity.	CH	5Y 7/2 - 5YR 5/6	SM																												190		
-																																		-
-																																		-
-																																		-
195	190'-194': Clayey GRAVEL with sand, GC, 70% gravel, 15% sand, 15% clay, light brown (5YR 5/6), dry and loose. Coarse: subangular. Fines: high plasticity.	GC	5YR 5/6	D																												195		
-																																		-
-																																		

Scale:

Drilling Contractor:

Driller:

USC (MPa)

R0	0.25-1.0
R1	1.0-5.0
R2	5.0-25
R3	25-50
R4	50-100
R5	100-250
R6	>250



ROTOSONIC COREHOLE LOG - BOREHOLE TSGT- 01

Project: G&K/Stockpile Characterization

Datum:

Collar Elev: 6207.6

Project No.: 043-2572

Drill date: 10/13/2004

Coordinates N: 19405.4 E: 9536.6

Sheet 6 of 11

Location: Tyrone Mine

Drill rig:

Azimuth: N/A

Inclination: 90°

Depth (m)	Soil / Rock Type		Group Symbol	Color code (Munsell)	D-dry SM-slightly moist M-moist VM-very moist S-saturated	Core recovery (%)						% 3-inch plus (%)					Maximum particle size in.					ISRM Strength Index for CLASTS						D-Point Load-diametral A-Point Load-axial S-Seive PI-Plasticity Index	Physical Testing	Notes Test Results	
	Description (i.e. Group Name, % fines/sand/gravel, plasticity, color, HCL reaction, cementation, angularity, odor, structure)	Moisture				0-20	20-40	40-60	60-80	80+	0-3	3-8	8-20	20-50	50+	1	2	3	4	5+	R0	R1	R2	R3	R4	R5	R6				
200	Continued	CH	5Y 7/2	SM																										200	
-																															-
-																															-
-	201'-205': Gravelly CLAY with sand, CH, 25% gravel, 15% sand, 60% clay, moderate yellowish brown (10YR 5/4), dry and loose. Coarse: subangular. Fines: low to medium plasticity.	CH	10YR 5/4	D																											-
205																															205
-																															-
-																															-
-	205'-211': Clayey GRAVEL with sand, GC, 60% gravel, 15% sand, 25% clay, yellowish gray (5Y 7/2), dry and medium dense. Coarse: subangular. Fines: high plasticity.	GC	5Y 7/2	D																											-
210																															210
-																															-
-	211'-214': Clayey GRAVEL with sand, GC, 40% gravel, 20% sand, 40% clay, light brown (5YR 5/6), slightly moist, dense and low HCL reaction. Coarse: angular. Fines: Medium to high plasticity.	GC	5YR 5/6	SM																							Sample Bucket: 211'-214'				-
215																															215
-	214'-217': Gravelly CLAY with sand, CH, 25% gravel, 25% sand, 50% clay, light brown (5YR 5/6), slightly moist and medium dense. Coarse: subangular. Fines: high plasticity.	CH	5YR 5/6	SM																											-
-																															-
-	217'-222': Clayey GRAVEL with sand, GC, 40% gravel, 20% sand, 40% clay, light brown (5YR 5/6), slightly moist and medium dense. Coarse: subangular. Fines: high plasticity.	GC	5YR 5/6	SM																											-
220																															220
-																															-
-																															-
-																															-
225																															225
-																															-
-	222'-232': Clayey GRAVEL with sand, GC, 40% gravel, 15% sand, 45% clay, light brown to pale yellowish brown (5YR 5/6 to 10YR 6/2), slightly moist and dense. Coarse: subangular. Fines: high plasticity.	GC	5YR 5/6 - 10YR 6/2	SM																											-
230																											Sample Bucket: 228'-230'				230
-																															-
-																															-
235																															235
-	232'-240': Clayey GRAVEL, GC, 50% gravel, 10% sand, 40% clay, yellowish gray (5Y 7/2), slightly moist and loose to medium dense. Coarse: subangular. Fines: medium plasticity.	GC	5Y 7/2	SM																											-
-																															-
-																															-
240																															240

Scale:

Drilling Contractor:

Driller:

USC (MPa)

R0	0.25-1.0
R1	1.0-5.0
R2	5.0-25
R3	25-50
R4	50-100
R5	100-250
R6	>250



ROTOSONIC COREHOLE LOG - BOREHOLE TSGT- 01

Project: G&K/Stockpile Characterization

Datum:

Collar Elev: 6207.6

Project No.: 043-2572

Drill date: 10/13/2004

Coordinates N: 19405.4 E: 9536.6

Sheet 7 of 11

Location: Tyrone Mine

Drill rig:

Azimuth: N/A

Inclination: 90°

Depth (m)	Soil / Rock Type		Group Symbol	Color code (Munsel)	D-dry SM-slightly moist M-moist VM-very moist S-saturated	Core recovery (%)				% 3-inch plus (%)				Maximum particle size in.				ISRM Strength Index for CLASTS						D-Point Load-diametral A-Point Load-axial S-Seive PI-Plasticity Index	Notes Test Results			
	Description (i.e. Group Name, % fines/sand/gravel, plasticity, color, HCL reaction, cementation, angularity, odor, structure)																											
																											Moisture	
					0-20	20-40	40-60	60-80	80+	0-3	3-8	8-20	20-50	50+	1	2	3	4	5+	R0	R1	R2	R3	R4	R5	R6	Physical Testing	
240	- - 240'-245': Clayey GRAVEL with sand, - GC, 40% gravel, 20% sand, 40% clay, light brown (5YR 5/6), slightly moist, dense and low HCL reaction. Coarse: angular. Fines: Medium to high plasticity. - 																											

Scale:

Drilling Contractor:

Driller:

USC (MPa)

R0	0.25-1.0
R1	1.0-5.0
R2	5.0-25
R3	25-50
R4	50-100
R5	100-250
R6	>250



ROTOSONIC COREHOLE LOG - BOREHOLE TSGT- 01

Project: G&K/Stockpile Characterization

Datum:

Collar Elev: 6207.6

Project No.: 043-2572

Drill date: 10/13/2004

Coordinates N: 19405.4 E: 9536.6

Sheet 8 of 11

Location: Tyrone Mine

Drill rig:

Azimuth: N/A

Inclination: 90°

Depth (m)	Soil / Rock Type		Group Symbol	Color code (Munsel)	D-dry	Core recovery (%)					% 3-inch plus (%)					Maximum particle size in.					ISRM Strength Index for CLASTS						D-Point Load-diametral		Notes Test Results
	SM-slightly moist	A-Point Load-axial																											
	M-moist	S-Seive																											
	VM-very moist	PI-Plasticity Index																											
Description (i.e. Group Name, % fines/sand/gravel, plasticity, color, HCL reaction, cementation, angularity, odor, structure)		Moisture	0-20	20-40	40-60	60-80	80+	0-3	3-8	8-20	20-50	50+	1	2	3	4	5+	R0	R1	R2	R3	R4	R5	R6	Physical Testing				
280	- 																												

Scale:

Drilling Contractor:

Driller:

USC (MPa)

R0	0.25-1.0
R1	1.0-5.0
R2	5.0-25
R3	25-50
R4	50-100
R5	100-250
R6	>250



ROTOSONIC COREHOLE LOG - BOREHOLE TSGT- 01

Project: G&K/Stockpile Characterization

Datum:

Collar Elev: 6207.6

Project No.: 043-2572

Drill date: 10/13/2004

Coordinates N: 19405.4 E: 9536.6

Sheet 9 of 11

Location: Tyrone Mine

Drill rig:

Azimuth: N/A

Inclination: 90°

Depth (m)	Soil / Rock Type		Group Symbol	Color code (Munsel)	D-dry	Core recovery (%)					% 3-inch plus (%)					Maximum particle size in.					ISRM Strength Index for CLASTS						D-Point Load-diametral A-Point Load-axial S-Seive PI-Plasticity Index				Notes Test Results																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																										
	Description (i.e. Group Name, % fines/sand/gravel, plasticity, color, HCL reaction, cementation, angularity, odor, structure)	Moisture			SM-slightly moist M-moist VM-very moist S-saturated	0-20	20-40	40-60	60-80	80+	0-3	3-8	8-20	20-50	50+	1	2	3	4	5+	R0	R1	R2	R3	R4	R5	R6	Physical Testing																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																													
320	Continued																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																								</

Scale:

Drilling Contractor:

Driller:

USC (MPa)

R0	0.25-1.0
R1	1.0-5.0
R2	5.0-25
R3	25-50
R4	50-100
R5	100-250
R6	>250



ROTOSONIC COREHOLE LOG - BOREHOLE TSGT- 01

Project: G&K/Stockpile Characterization

Datum:

Collar Elev: 6207.6

Project No.: 043-2572

Drill date: 10/13/2004

Coordinates N: 19405.4 E: 9536.6

Sheet 10 of 11

Location: Tyrone Mine

Drill rig:

Azimuth: N/A

Inclination: 90°

Depth (m)	Soil / Rock Type		Group Symbol	Color code (Munsell)	D-dry	Core recovery (%)					% 3-inch plus (%)					Maximum particle size in.					ISRM Strength Index for CLASTS						Physical Testing	Notes Test Results																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																								
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Description (i.e. Group Name, % fines/sand/gravel, plasticity, color, HCL reaction, cementation, angularity, odor, structure)		Moisture	0-20	20-40	40-60	60-80	80+	0-3	3-8	8-20	20-50	50+	1	2	3	4	5+	R0	R1	R2	R3	R4	R5	R6																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																												
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Scale:

Drilling Contractor:


Driller:

USC (MPa)

R0	0.25-1.0
R1	1.0-5.0
R2	5.0-25
R3	25-50
R4	50-100
R5	100-250
R6	>250



Project:	G&K/Stockpile Characterization			Datum:				Collar Elev:	6207.6	
Project No.:	043-2572	Drill date:	10/13/2004	Coordinates	N:	19405.4	E:	9536.6	Sheet	11 of 11
Location:	Tyrone Mine	Drill rig:		Azimuth:	N/A				Inclination:	90°

Scale:	USC (MPa)	
	R0 0.25-1.0	
Drilling Contractor:	R1 1.0-5.0	
	R2 5.0-25	
Driller:	R3 25-50	
	R4 50-100	
	R5 100-250	
	R6 >250	

ROTOSONIC COREHOLE LOG - BOREHOLE TSGT-2																																					
Project: G&K/Stockpile Characterization										Datum:										Collar Elev:																	
Project No.: 043-2572										Drill date:										Coordinates N: E:																	
Location: Tyrone Mine										Drill rig:										Azimuth									Inclination: vertical								
Depth (m)	Soil / Rock Type		USCS	Run No.	Moisture	D-dry SM-slightly moist M-moist VM-very moist S-saturated				Core recovery (%)				% 3-inch plus (%)				Maximum particle size in.				ISRM Strength Index				D-Point Load-diametral A-Point Load-axial S-Sieve PI-Plasticity Index		Physical Testing	Notes Test Results								
	Description	Run No.				Moisture	0-20 20-40 40-60 60-80 80+				0-3 3-8 8-20 20-50 50+				1 2 3 4 5+ R6 R5 R4 R3 R2 R1 R0				D-Point Load-diametral A-Point Load-axial S-Sieve PI-Plasticity Index																		
0	SANDY GRAVEL, small amount to some silt and clay, well graded angular gravel, predominantly fine- to medium-grained sand, non-plastic to low plasticity, brown (7.5YR 7/2)		GW		SM																						pyrite throughout (<1mm)	0									
5	GRAVEL, some sand and cobbles, small amount of silt and clay, predominantly coarse, angular, low plasticity, brown (10 Y 8/)		GP		SM																								5								
10					M																									10							
15					D																									15							
20	SILTY GRAVEL, considerable sand, some clay, well graded, sub-angular to angular, low plasticity, light brown to yellow-brown (10 Y 8/)		GP		D																									20							
25					SM																									25							
30	gravel from 26 to 28'				SM																										30						
35	gravelly silt from 31 to 32'		ML		D																									35							
40	SANDY GRAVEL, considerable clay and cobble, well graded angular sand and gravel, low to medium plasticity, brown (2.5Y 7/3)		GW		D																									40							
																		USC (MPa)				Golder Associates															
																		R0-0.25-1.0 R1-1.0-5.0 R2-5.0-25 R3-25-50				R4-50-100 R5-100-250 R6->250															
Scale																																					
Drilling Contractor																																					
Driller																																					

ROTOSONIC COREHOLE LOG - BOREHOLE TSGT-2																										
Project: G&K/Stockpile Characterization			Datum:			Collar Elev:																				
Project No.: 043-2572			Drill date:			Coordinates N:			E:			Sheet 2 of 7														
Location: Tyrone Mine			Drill rig:			Azimuth			Inclination:			vertical														
Depth (m)	Soil / Rock Type	USCS	Run No.	Moisture	Core recovery (%)				% 3-inch plus (%)				Maximum particle size in.	ISRM Strength Index						D-Point Load-diametral A-Point Load-axial S-Sieve PI-Plasticity Index	Notes Test Results					
					0-20	20-40	40-60	60-80	80+	0-3	3-8	8-20		20-50	50+	1	2	3	4			5+	R6	R5	R4	R3
40	SILTY GRAVEL , some sand and clay, predominantly coarse, angular, non-plastic, light brown	GM		D																						
	GRAVEL , considerable sand, some cobbles, silt and clay, low plasticity, gray (2.5Y 7/2)	GW		SM																						
45	sandy gravel from 44' to 49'			D																						
				D																						
	silty gravel from 49' to 50'	GM		SM																						
				D																						greenish-gray to yellow-brown (2.5Y 8/4) from 50 to 56'
				D																						smectite coatings at 52'
55	silty gravel from 55' to 58'	ML		D																						
				D																						
	CLAYEY SAND , considerable gravel, small amount of silt, angular, medium plasticity, reddish-brown (10R 6/4)	SC		M																						
				M																						
				M																						
65	sandy gravel from 64' to 66'	GP		SM-M																						
				D																						
																										sample missing
70				M																						
				M																						
	SANDY GRAVEL , small amount of silt, clay and cobbles, well graded angular gravel, well graded sand, low plasticity, brown to light brown	GW		D																						
				D																						
				SM																						
80	CLAYEY to GRAVELLY SAND , fine gravel, fine- to medium-grained sand, low to medium plasticity, red- to yellow-brown	SP		M																						

Scale

Drilling Contractor

Driller

USC (MPa)

R0-0.25-1.0
R1-1.0-5.0
R2-5.0-25
R3-25-50

R4-50-100
R5-100-250
R6->250

ROTOSONIC CORELOG LOG - BOREHOLE TSGT-2																							
Project: G&K/Stockpile Characterization						Datum:						Collar Elev:											
Project No.: 043-2572						Drill date:						Coordinates N: E:											
Location: Tyrone Mine						Drill rig:						Azimuth											
												Inclination: vertical											
Depth (m)	Soil / Rock Type	USCS	Run No.	Moisture	D-dry SM-slightly moist M-moist VM-very moist S-saturated	Core recovery (%)				% 3-inch plus (%)				Maximum particle size in.				ISRM Strength Index				Physical Testing	Notes Test Results
						0-20	20-40	40-60	60-80	80+	0-3	3-8	8-20	20-50	50+	1	2	3	4	5+	R6		
80	CLAYEY to GRAVELLY SAND (cont.)	SP		M																			80
-	clayey sand	SC		M																			-
-	gravel from 83 to 84'			M																			-
-	GRAVELLY SAND, considerable clay, some silt, predominantly fine- to medium-grained sand, non-plastic to low plasticity, bluish-gray (10B 6/)	SP		M																			-
85				SM																			85
-				M																			-
-				SM																			-
90				D																			90
-	sandy gravel from 92 to 94'	GP		D																			-
-				D																			-
95	CLAYEY SAND, some gravel, well graded angular sand, medium plasticity, reddish-brown (7.5YR 6/4) to yellow-brown (2.5Y 8/4)	SC		M																			95
-				M																			-
-				M																			-
100				M																			100
-	clayey gravel 100 to 101'	GC		SM																			-
-				M																			-
105	CLAYEY SAND, some gravel, predominantly fine- to medium-grained angular sand, medium plasticity, bluish-gray (10B 7/)	SC		M																			105
-				M																			-
-				M																			-
110				M																			110
-				M																			-
-	GRAVELLY SAND, considerable clay and silt, well graded sand, low plasticity, yellow-brown (2.5Y 7/6)	SW																					-
115																							115
-																							-
-																							-
120																							120

Scale

Drilling Contractor

Driller

USC (MPa)

R0-0.25-1.0
R1-1.0-5.0
R2-5.0-25
R3-25-50

R4-50-100
R5-100-250
R6->250

[illegible]

ROTOSONIC COREHOLE LOG - BOREHOLE TSGT-2

Project: G&K/Stockpile Characterization

Datum:

Collar Elev:

Project No.: 043-2572

Drill date:

Coordinates N:

E:

Sheet

5 of 7

Location: Tyrone Mine

Drill rig:

Azimuth

Inclination:

vertical

Depth (m)	Soil / Rock Type		USCS	Run No.	Moisture	Core recovery (%)			% 3-inch plus (%)			Maximum particle size in.			ISRM Strength Index			Physical Testing	Notes Test Results										
	Description	D-dry SM-slightly moist M-moist VM-very moist S-saturated				0-20	20-40	40-60	60-80	80-100	0-3	3-8	8-20	20-50	50+	1	2			3	4	5+	R6	R5	R4	R3	R2	R1	R0
D-Point Load-diametral	A-Point Load-axial	S-Sieve	PI-Plasticity Index																										

160	GRAVELLY SAND (cont.)	SW		M																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																									
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brown (10R 5/3) from 166 to 167'

sandy gravel from 193 to 196'

GRAVEL, considerable sand, some silt and clay, predominantly coarse-grained, angular, non-plastic, yellow-brown (2.5Y 7/3)

Scale
Drilling Contractor
Driller

USC (MPa)
R0-0.25-1.0
R1-1.0-5.0
R2-5.0-25
R3-25-50
R4-50-100
R5-100-250
R6->250



ROTONSONIC COREHOLE LOG - BOREHOLE TSGT-2																		
Project: G&K/Stockpile Characterization Project No.: 043-2572 Location: Tyrone Mine						Datum: Drill date: Coordinates N: E: Azimuth:						Collar Elev: Sheet 6 of 7 Inclination: vertical						
Depth (m)	Soil / Rock Type	USCS	Run No.	Moisture	Core recovery (%)										Maximum particle size in.	ISRM Strength Index	D-Point Load-diametral A-Point Load-axial S-Sieve PI-Plasticity Index	Notes Test Results
200	GRAVEL (cont.), considerable sand, some silt and clay, predominantly coarse-grained, angular, non-plastic, yellow-brown (2.5Y 7/3)	GP		M														
				SM														
205	gray gravelly silt from 206 to 207' (N 6/)	GP		D														
	GRAVEL , small amount to considerable gray silt, some cobbles, predominantly coarse-grained, angular, non-plastic, brown (10YR 7/3) to reddish-brown (10R 7/4)			D														
210	GRAVELLY SAND , considerable clay, some silt, predominantly fine- to medium-grained angular sand, low plasticity, yellow-brown (10YR 6/6)	SP		D														
				SM														
				M														
215			M															
		M																
		M																
220			M															
		M																
225			SM															
	S																	
	S																	
230		M																
	M																	
235	CLAYEY SAND , considerable fine angular gravel and silt, predominantly fine- to medium-grained angular sand, medium plasticity, yellow-brown (10YR 6/8)	SC		M														
	GRAVELLY SAND , considerable clay, some silt, predominantly fine- to medium-grained angular sand, low plasticity, yellow-brown (10YR 7/4)	SP		M														
240																		

Scale

Drilling Contractor

Driller

USC (MPa)

R0-0.25-1.0
R1-1.0-5.0
R2-5.0-25
R3-25-50

R4-50-100
R5-100-250
R6->250

ROTOSONIC COREHOLE LOG - BOREHOLE TSGT-2

Project: G&K/Stockpile Characterization

Datum:

Collar Elev:

Project No.: 043-2572

Drill date:

Coordinates N:

E:

Sheet

7 of 7

Location: Tyrone Mine

Drill rig:

Azimuth

Inclination:

vertical

Depth (m)	Soil / Rock Type		USCS	Run No.	Moisture	Core recovery (%)					% 3-inch plus (%)					Maximum particle size in.					ISRM Strength Index					Physical Testing	Notes Test Results																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																															
	Description	D-dry SM-slightly moist M-moist VM-very moist S-saturated				0-20	20-40	40-60	60-80	80+	0-3	3-8	8-20	20-50	50+	1	2	3	4	5+	R6	R5	R4	R3	R2			R1	R0																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																													
240	GRAVELLY SAND (cont.), considerable clay, some silt, predominantly fine- to medium-grained angular sand, low plasticity, yellow-brown (10YR 7/4)	SP	M																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							</

END OF RECORD (281')

Scale
Drilling Contractor
Driller

USC (MPa)
R0-0.25-1.0
R1-1.0-5.0
R2-5.0-25
R3-25-50
R4-50-100
R5-100-250
R6->250



[illegible]

[illegible]

ROTOSONIC COREHOLE LOG - BOREHOLE TSGT-3

Project: G&K/Stockpile Characterization

Datum:

Collar Elev:

Project No.: 043-2572

Drill date:

Coordinates N:

E:

Sheet

3 of 7

Location: Tyrone Mine

Drill rig:

Azimuth

Inclination:

vertical

Soil / Rock Type		USCS	Run No.	Moisture	D-dry SM-slightly moist M-moist VM-very moist S-saturated			Core recovery (%)	% 3-inch plus (%)	Maximum particle size in.	ISRM Strength Index	D-Point Load-diametral A-Point Load-axial S-Sieve PI-Plasticity Index		Notes Test Results
Depth (m)	Description				Physical Testing	Notes								
80	GRAVELLY SAND (cont.)	SP		M	0-20 20-40 40-60 60-80 80-100 100+	0-3 3-8 8-20 20-50 50+	0-3 3-8 8-20 20-50 50+	0-3 3-8 8-20 20-50 50+	0-3 3-8 8-20 20-50 50+	0-3 3-8 8-20 20-50 50+	0-3 3-8 8-20 20-50 50+	0-3 3-8 8-20 20-50 50+		80
-	SANDY GRAVEL, some silt and clay, small amount cobbles, well graded angular gravel, non-plastic to low plasticity, reddish-brown (10 R 5/4) to gray (10 BG 5/)			M										-
-				M										-
-	(10 R 5/4) to gray (10 BG 5/)			M										thin (2") layers of open gravel at 83, 90'
85				M										85
-	clayey sand from 86-87'	SC		M										-
-				M										-
-		SP		M										-
90				M										solid core from 87 to 92'
-				M										90
-				M										-
-				M										-
95				M										95
-				M										-
-				M										-
100	GRAVEL, some silt and fine sand, angular, predominantly coarse-grained, non-plastic, gray (5PB 7/)	GP		M										solid core from 98 to 99'
-				D										100
-				M										-
-	SANDY GRAVEL, considerable silt and clay, some cobbles, predominantly coarse angular gravel, well graded sand, low plasticity, reddish- to purplish-brown (10 R 7/3)	GP		M										-
105				M										105
-				M										-
-				M										color change to orange-brown (2.5 YR 6/6) from 107 to 108'
110				M										110
-				M										-
-				M										orange-brown (2.5 YR 6/6) from 110 to 111', 114 to 115'
115				M										115
-				M										-
-	GRAVEL to SANDY GRAVEL, small amount of silt and clay, predominantly coarse angular to sub-angular gravel, non-plastic, orange-brown (2.5 YR 5/6)	GP		M										solid core from 112 to 115'
120				SM										120
-				SM										-
-				SM										-

Scale


Drilling Contractor

Driller

USC (MPa)

R0-0.25-1.0
R1-1.0-5.0
R2-5.0-25
R3-25-50

R4-50-100
R5-100-250
R6->250



Scale
Drilling Contractor
Driller

USC (MPa)
R0-0.25-1.0
R1-1.0-5.0
R2-5.0-25
R3-25-50
R4-50-100
R5-100-250
R6->250



ROTOSONIC CORELOG - BOREHOLE TSGT-3																												
Project: G&K/Stockpile Characterization						Datum:						Collar Elev:																
Project No.: 043-2572						Drill date:				Coordinates N:				E:				Sheet 4 of 7										
Location: Tyrone Mine						Drill rig:				Azimuth				Inclination: vertical														
Depth (m)	Soil / Rock Type		USCS	Run No.	Moisture	Core recovery (%)				% 3-inch plus (%)				Maximum particle size in.				ISRM Strength Index				D-Point Load-diametral A-Point Load-axial S-Sieve PI-Plasticity Index		Notes Test Results				
	Description					0-20	20-40	40-60	60-80	80+	0-3	3-8	8-20	20-50	50+	1	2	3	4	5+	R6	R5	R4		R3	R2	R1	R0
120	GRAVEL to SANDY GRAVEL (cont.)		GP		M																							
-					M																							
-	GRAVELLY SAND , some silt and clay, predominantly fine- to medium-grained sand, low plasticity, dark reddish-brown (5YR 6/6)		SW		M																							
125					M																							
-					M																							
-					M																							
130	SANDY GRAVEL , some to considerable clay, some silt and cobbles, well graded angular gravel, low plasticity, reddish-brown to orange-brown		GW		M																							
-					M																							
135					M																				dark reddish-brown (2.5 YR 4/6) at 134' - small piece of solid core			
-					M																							
-					M																							
140					M																							
-					SM																							
145					M																							
-					M																							
-					M																							
150	SAND and GRAVEL , small amount of silt and clay, well graded sand, predominantly fine gravel, low plasticity, brown (7.5 TR 7/3)		SW-GP		M																							
-					M																							
-					SM																							
-					SM																							
155					M																				sand, reddish-brown (10 R 5/6) from 156 to 157'			
-					M																							
-																									some solid core from 158 to 163'			
160																												

Scale

Drilling Contractor

Driller

USC (MPa)
R0-0.25-1.0
R1-1.0-5.0
R2-5.0-25
R3-25-50
R4-50-100
R5-100-250
R6->250

ROTOSONIC COREHOLE LOG - BOREHOLE TSGT-3

Project: G&K/Stockpile Characterization

Datum:

Collar Elev:

Project No.: 043-2572

Drill date:

Coordinates N:

E:

Sheet

5 of 7

Location: Tyrone Mine

Drill rig:

Azimuth

Inclination:

vertical

Depth (m)	Soil / Rock Type		USCS	Run No.	Moisture	Core recovery (%)				% 3-inch plus (%)				Maximum particle size in.				ISRM Strength Index				Physical Testing	Notes Test Results						
	Description	D-dry SM-slightly moist M-moist VM-very moist S-saturated				0-20	20-40	40-60	60-80	80+	0-3	3-8	8-20	20-50	50+	1	2	3	4	5+	R6			R5	R4	R3	R2	R1	R0
D-Point Load-diametral	A-Point Load-axial	S-Sieve	PI-Plasticity Index																										

160	SAND and GRAVEL (cont.)	SW-GP		M																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																						</
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very few fines from 186 to 187.5'

GRAVEL, considerable sand, considerable silt (195'), small amount of clay, predominantly coarse-grained, angular, non-plastic, gray (5 PB 7/)

Scale

Drilling Contractor

Driller

USC (MPa)

R0-0.25-1.0
R1-1.0-5.0
R2-5.0-25
R3-25-50
R4-50-100
R5-100-250
R6->250



ROTOSONIC CORELOG LOG - BOREHOLE TSGT-3																															
Project: G&K/Stockpile Characterization Project No.: 043-2572 Location: Tyrone Mine						Datum: Drill date: Coordinates N: Azimuth						Collar Elev: E: Sheet 6 of 7 Inclination: vertical																			
Depth (m)	Soil / Rock Type	USCS	Run No.	Moisture	Core recovery (%)									% 3-inch plus (%)			Maximum particle size in.	ISRM Strength Index							D-Point Load-diametral	A-Point Load-axial	S-Sieve	Pt-Plasticity Index	Notes Test Results		
																							Physical Testing								
					0-20	20-40	40-60	60-80	80+	0-3	3-8	8-20	20-50	50+	1	2		3	4	5+	R6	R5					R4	R3		R2	R1
200	SANDY GRAVEL , considerable silt and clay, some cobbles, predominantly coarse angular gravel, non-plastic, gray (2.5 Y 6/1)	GP		M																						Cu sulfates at 200.5' much solid core below 200'	200				
-				M																											
-				M																											
205				M																											
-				M																											
-				M																											
210				M																											
-				M																											
-				M																											
215				SM																											
-		SM																													
-		SM																													
220	predominantly fine gravel below 221'			M																											
-				M																											
-				M																											
-				M																											
225				M																											
-				SM																											
-		SM																													
230	GRAVELLY SAND , considerable clay, some silt, predominantly fine gravel, predominantly fine- to medium-grained sand, low plasticity, reddish-brown (2.5 YR 6/3)	SP		M																											
-				M																											
-				M																											
-				M																											
235				M																											
-		M																													
-		M																													
240																															
												Scale				Drilling Contractor				Driller				USC (MPa) R0-0.25-1.0 R4-50-100 R1-1.0-5.0 R5-100-250 R2-5.0-25 R6->250 R3-25-50							

ROTOSONIC COREHOLE LOG - BOREHOLE TSGT-3

Project: G&K/Stockpile Characterization

Datum:

Collar Elev:

Project No.: 043-2572

Drill date:

Coordinates N:

E:

Sheet

7 of 7


Location: Tyrone Mine

Drill rig:

Azimuth

Inclination:

vertical

Depth (m)	Soil / Rock Type		USCS	Run No.	Moisture	D-dry SM-slightly moist M-moist VM-very moist S-saturated				Core recovery (%)	% 3-inch plus (%)	Maximum particle size in.	ISRM Strength Index	D-Point Load-diametral A-Point Load-axial S-Sieve PI-Plasticity Index		Notes Test Results		
	Description					0-20 20-40 40-60 60-80 80+	0-3 3-8 8-20 20-50 50+	1 2 3 4 5+ R6 R5 R4 R3 R2 R1 R0	Physical Testing									
240	GRAVELLY SAND (cont.)		SP		M												240	
-	GRAVEL, small amount of sand, trace silt and clay, predominantly coarse-grained, angular, non-plastic, brown (5YR 7/3)		GP		M												-	
SM																-		
M																-		
M																245		
245	GRAVELLY SAND, considerable silt and clay, predominantly fine- to medium-grained angular sand, predominantly fine angular gravel, low plasticity, dark reddish-brown (10R 6/6)		SP		M												-	
250	END OF RECORD																250	
-																	-	
-																	-	
-																	-	
255																	255	
-																	-	
-																	-	
-																	-	
260																	260	
-																	-	
-																	-	
265																	265	
-																	-	
-																	-	
270																	270	
-																	-	
-																	-	
275																	275	
-																	-	
-																	-	
280																	280	
													USC (MPa)					
Scale													R0-0.25-1.0				R4-50-100	
Drilling Contractor													R1-1.0-5.0				R5-100-250	
Driller													R2-5.0-25				R6->250	

ROTOSONIC COREHOLE LOG - BOREHOLE TSGT- 04

Project: G&K/Stockpile Characterization

Datum:

Collar Elev: 6310.7

Project No.: 043-2572

Drill date: 9/28/2004

Coordinates N: 10838.6 E: 4155.5

Sheet 1 of 7

Location: Tyrone Mine

Drill rig:

Azimuth

Inclination: 90°

Depth (m)	Soil / Rock Type		Group Symbol	Color code (Munsell)	D-dry SM-slightly moist M-moist VM-very moist S-saturated	Core recovery (%)				% 3-inch plus (%)				Maxiumum particle size in.				ISRM Strength Index for CLASTS						D-Point Load-diametral A-Point Load-axial S-Sieve PI-Plasticity Index	Notes Test Results							
	Description (i.e. Group Name, % fines/sand/gravel, plasticity, color, HCL reaction, cementation, angularity, odor, structure)	Moisture																														
						0-20	20-40	40-60	60-80	80+	0-3	3-8	8-20	20-50	50+	1	2	3	4	5+	R0	R1	R2				R3	R4	R5	R6	Physical Testing	
0	0'-2': Gravelly to sandy CLAY, CH, 20% gravel, 20% sand, 60% clay, light brown (5YR 5/6), slightly moist and dense. Coarse: subangular. Fines: high plasticity.	CH	5YR 5/6	SM																											0	
-																																-
-																																-
-																																-
5																																5
-																																-
-																																-
-																																-
10	2'-18': Clayey GRAVEL with sand, GC, 40% gravel, 15% sand, 45% clay, dusky yellow (5Y 6/4), slightly moist and medium dense. Coarse: subangular. Fines: high plasticity.	GC	5Y 6/4	SM																												10
-																																-
-																																-
-																																-
15																																15
-																																-
-																																-
20	18'-24': Gravelly CLAY, CH, 40% gravel, 10% sand, 50% clay, light brown (5YR 5/6), slightly moist and dense. Coarse: subangular. Fines: high plasticity.	CH	5YR 5/6	SM																												20
-																																-
-																																-
25																																25
-																																-
-																																-
30	24'-32': Clayey GRAVEL with sand, GC, 40% gravel, 15% sand, 45% clay, dark yellowish orange (10YR 5/6), slightly moist and dense. Coarse: subangular. Fines: high plasticity.	GC	10YR 5/6	SM																												30
-																																-
-																																-
35	32'-36': Gravelly CLAY, CH, 40% gravel, 10% sand, 50% clay, light brown (5YR 5/6), slightly moist and dense. Coarse: subangular. Fines: high plasticity.	CH	5YR 5/6	SM																												35
-																																-
40	36'-42': Clayey GRAVEL with sand, GC, 35% gravel, 25% sand, 40% clay, light brown (5YR 6/4), slightly moist and medium dense. Coarse: subangular. Fines: high plasticity.	GC	5YR 6/4	SM																												-
-																																-
-																																-
40																																40



Scale:

Drilling Contractor:

Driller:

USC (MPa)

R0	0.25-1.0
R1	1.0-5.0
R2	5.0-25
R3	25-50
R4	50-100
R5	100-250
R6	>250

ROTOSONIC COREHOLE LOG - BOREHOLE TSGT- 04

Project: G&K/Stockpile Characterization

Datum:

Collar Elev: 6310.7

Project No.: 043-2572

Drill date: 9/28/2004

Coordinates N: 10838.6 E: 4155.5

Sheet 2 of 7

Location: Tyrone Mine

Drill rig:

Azimuth

Inclination: 90°

Depth (m)	Soil / Rock Type		Group Symbol	Color code (Munsell)	D-dry SM-slightly moist M-moist VM-very moist S-saturated	Core recovery (%)				% 3-inch plus (%)				Maximum particle size in.				ISRM Strength Index for CLASTS						D-Point Load-diametral A-Point Load-axial S-Sieve PI-Plasticity Index	Notes Test Results					
	Description (i.e. Group Name, % fines/sand/gravel, plasticity, color, HCL reaction, cementation, angularity, odor, structure)																													
		Moisture																												
						0-20 20-40 40-60 60-80 80+	0-3 3-8 8-20 20-50 50+	1 2 3 4 5+	R0 R1 R2 R3 R4 R5 R6	Physical Testing																				
40	-	Continued	GC	5YR 6/4	SM																								40	
-																														-
-																														-
45				5YR 6/4																										45
-																														-
-																														-
-																														-
50																														50
-																														-
-																														-
55				5YR 5/6																										55
-																														-
-																														-
60		42'-80': Clayey GRAVEL with sand, GC, 45% gravel, 25% sand, 30% clay, light brown (5YR 6/4 to 5YR 5/6), slightly moist and medium dense. Coarse: subangular. Fines: high plasticity.	GC		SM																									60
-																														-
-																														-
65																														65
-																														-
-																														-
70																											Sample Bucket: 69' 71'			70
-				5YR 6/4																										-
-																														-
-																														-
75																														75
-																														-
-																														-
-																														-
80																														80

Scale:

Drilling Contractor:


Driller:

USC (MPa)

R0	0.25-1.0
R1	1.0-5.0
R2	5.0-25
R3	25-50
R4	50-100
R5	100-250
R6	>250



Project:	G&K/Stockpile Characterization			Datum:				Collar Elev:	6310.7	
Project No.:	043-2572	Drill date:	9/28/2004	Coordinates	N:	10838.6	E:	4155.5	Sheet	3 of 7
Location:	Tyrone Mine	Drill rig:		Azimuth				Inclination:	90°	

		USC (MPa)		
Scale:		R0	0-25-1.0	
Drilling Contractor:		R1	1.0-5.0	
		R2	5.0-25	
		R3	25-50	
		R4	50-100	
		R5	100-250	
Driller:		R6	>250	

ROTOSONIC COREHOLE LOG - BOREHOLE TSGT- 04

Project: G&K/Stockpile Characterization

Datum:

Collar Elev: 6310.7

Project No.: 043-2572

Drill date: 9/28/2004

Coordinates N: 10838.6 E: 4155.5

Sheet 4 of 7

Location: Tyrone Mine

Drill rig:

Azimuth

Inclination: 90°

Depth (m)	Soil / Rock Type		Group Symbol	Color code (Munsell)	D-dry SM-slightly moist M-moist VM-very moist S-saturated	Core recovery (%)					% 3-inch plus (%)					Maximum particle size in.					ISRM Strength Index for CLASTS						D-Point Load-diametral A-Point Load-axial S-Sieve PI-Plasticity Index	Notes Test Results							
	Description (i.e. Group Name, % fines/sand/gravel, plasticity, color, HCL reaction, cementation, angularity, odor, structure)	Moisture																																	
						Physical Testing																													
120	-	Continued	GC	10 YR 6/6-5YR 5/6	SM																										120				
125	-	122'-129': Gravelly CLAY, CH, 30% gravel, 20% sand, 50% clay, light brown (5YR 5/6), slightly moist, well graded and medium dense. Coarse: subangular. Fines: high plasticity.	CH	5YR 5/6	SM																											125			
130	-	129'-136': Clayey GRAVEL with sand, GC, 40% gravel, 20% sand, 40% clay, light brown (5YR 5/6), slightly moist, well graded and medium dense. Coarse: subangular. Fines: high plasticity.	GC	5YR 5/6	SM																												130		
135	-																																	135	
140	-	136'-146': Clayey SAND with gravel, SC, 25% gravel, 35% sand, 40% clay, light brown (5YR 5/6), slightly moist, well graded and medium dense. Coarse: subangular. Fines: high plasticity.	SC	5YR 5/6	SM																													140	
145	-																																		145
150	-	146'-156': Clayey SAND with gravel, SC, 30% gravel, 35% sand, 35% clay, pale reddish brown (10R 5/4), dry, well graded and loose to medium dense. Coarse: subangular. Fines: medium plasticity.	SC	10R 5/4	D																													150	
155	-																																		155
160	-	156'-162': Gravelly to Sandy CLAY, CH, 25% gravel, 25% sand, 50% clay, light brown (5YR 5/6), dry and loose. Coarse: subangular. Fines: high plasticity.	CH	5YR 5/6	D																														160

Scale:

Drilling Contractor:

Driller:

USC (MPa)

R0	0.25-1.0
R1	1.0-5.0
R2	5.0-25
R3	25-50
R4	50-100
R5	100-250
R6	>250



ROTOSONIC COREHOLE LOG - BOREHOLE TSGT- 04

Project: G&K/Stockpile Characterization

Datum:

Collar Elev: 6310.7

Project No.: 043-2572

Drill date: 9/28/2004

Coordinates N: 10838.6 E: 4155.5

Sheet 5 of 7

Location: Tyrone Mine

Drill rig:

Azimuth

Inclination: 90°

Depth (m)	Soil / Rock Type		Group Symbol	Color code (Munsell)	D-dry SM-slightly moist M-moist VM-very moist S-saturated	Core recovery (%)					% 3-inch plus (%)					Maximum particle size in.					ISRM Strength Index for CLASTS						D-Point Load-diametral				Notes Test Results																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																												
	Description (i.e. Group Name, % fines/sand/gravel, plasticity, color, HCL reaction, cementation, angularity, odor, structure)	Moisture																									A-Point Load-axial																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																
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Scale:

Drilling Contractor:

Driller:

USC (MPa)

R0	0.25-1.0
R1	1.0-5.0
R2	5.0-25
R3	25-50
R4	50-100
R5	100-250
R6	>250



ROTOSONIC COREHOLE LOG - BOREHOLE TSGT- 04

Project: G&K/Stockpile Characterization

Datum:

Collar Elev: 6310.7

Project No.: 043-2572

Drill date: 9/28/2004

Coordinates N: 10838.6 E: 4155.5

Sheet 6 of 7

Location: Tyrone Mine

Drill rig:

Azimuth

Inclination: 90°

Depth (m)	Soil / Rock Type		Group Symbol	Color code (Munsell)	D-dry SM-slightly moist M-moist VM-very moist S-saturated	Core recovery (%)				% 3-inch plus (%)				Maximum particle size in.				ISRM Strength Index for CLASTS						D-Point Load-diametral A-Point Load-axial S-Sieve PI-Plasticity Index	Notes Test Results								
	Description (i.e. Group Name, % fines/sand/gravel, plasticity, color, HCL reaction, cementation, angularity, odor, structure)																																
		Moisture																															
						0-20 20-40 40-60 60-80 80+	0-3 3-8 8-20 20-50 50+	1 2 3 4 5+	R0 R1 R2 R3 R4 R5 R6																								
Physical Testing																																	
200	-	-	Continued	SC	5YR 5/6	D																									200		
205	-	-	204'-208': Clayey GRAVEL with sand, GC, 50% gravel, 20% sand, 30% clay, light brown (5YR 5/6), dry and loose. Coarse: subangular to angular. Fines: medium plasticity.	GC	5YR 5/6	D																									205		
210	-	-	208'-213': Clayey GRAVEL with sand, GC, 40% gravel, 20% sand, 40% clay, light brown (5YR 6/4), dry and loose. Coarse: subangular. Fines: high plasticity.	GC	5YR 6/4	D																									210		
215	-	-	213'-217': Gravelly to Sandy CLAY, CH, 25% gravel, 25% sand, 50% clay, light brown (5YR 5/6), dry and loose. Coarse: subangular. Fines: high plasticity.	CH	5YR 5/6	D																									215		
	-	-	From 215' to 216', Clayey Gravel with sand.	CH																													
220	-	-	217'-220': Clayey GRAVEL with sand, GC, 40% gravel, 20% sand, 40% clay, light brown (5YR 6/4), dry and loose. Coarse: subangular. Fines: high plasticity.	GC	5YR 6/4	D																									220		
	-	-	220'-236': Clayey GRAVEL with sand, GC, 40% gravel, 25% sand, 35% clay, light brown (5YR 5/6), dry and loose. Coarse: subangular. Fines: high plasticity.	GC	5YR 5/6	D																											
	-	-																															
	-	-																															
225	-	-																															
230	-	-																													230		
235	-	-																													235		
240	-	-	236'-240': Clayey GRAVEL with sand, GC, 45% gravel, 15% sand, 40% clay, light brown to grayish orange (5YR 5/6 to 10YR 7/4), slightly moist and medium dense. Coarse: subangular to angular. Fines: high plasticity.	GC	5YR 5/6-10YR 7/4	SM																									240		

Scale:

Drilling Contractor:

Driller:

USC (MPa)

R0	0.25-1.0
R1	1.0-5.0
R2	5.0-25
R3	25-50
R4	50-100
R5	100-250
R6	>250



ROTOSONIC COREHOLE LOG - BOREHOLE TSGT- 04

Project: G&K/Stockpile Characterization

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Project No.: 043-2572

Drill date: 9/28/2004

Coordinates N: 10838.6 E: 4155.5

Sheet 7 of 7

Location: Tyrone Mine

Drill rig:

Azimuth

Inclination: 90°

Depth (m)	Soil / Rock Type		Group Symbol	Color code (Munsell)	D-dry SM-slightly moist M-moist VM-very moist S-saturated	Core recovery (%)					% 3-inch plus (%)					Maximum particle size in.					ISRM Strength Index for CLASTS						D-Point Load-diametral				Notes Test Results	
	Description (i.e. Group Name, % fines/sand/gravel, plasticity, color, HCL reaction, cementation, angularity, odor, structure)	Physical Testing																									A-Point Load-axial					S-Sieve
					Moisture	0-20	20-40	40-60	60-80	80+	0-3	3-8	8-20	20-50	50+	1	2	3	4	5+	R0	R1	R2	R3	R4	R5	R6					
240	240'-251': Clayey SAND with gravel, SC, 30% gravel, 35% sand, 45% clay, dark yellowish orange to light brown (10YR 6/6 to 5YR 5/6), dry, well graded and loose. Coarse: subangular. Fines: high plasticity	SC	10YR 6/6-5YR 5/6	D																									240			
-																																-
-																																-
-																																-
-																																-
245	251'-255': Clayey GRAVEL with sand, GC, 40% gravel, 30% sand, 30% clay, pale reddish brown (10R 5/4), slightly moist, well graded and medium dense. Coarse: subrounded to subangular. Fines: high plasticity	GC	10R 5/4	SM																									245			
-																															-	
-																															-	
-																															-	
-																															-	
250	255'-259': Clayey GRAVEL, GC, 60% gravel, 10% sand, 30% clay, pale reddish brown to dark yellowish orange (10R 5/4 to 10YR 5/6), dry and loose. Coarse: subrounded to subangular. Fines: medium plasticity	GC	10R 5/4-10YR 5/6	D																									250			
-																														-		
-																														-		
-																														-		
-																														-		
255	251'-255': Clayey SAND with gravel, SC, 20% gravel, 50% sand, 30% clay, moderate brown (5YR 4/4 to 5YR 3/4), slightly moist and loose. Coarse: subrounded to subangular. Fines: low plasticity	SC	5YR 4/4-5YR 3/4	SM																									255			
-																														-		
-																														-		
-																														-		
-																														-		
260	251'-255': Clayey SAND with gravel, SC, 20% gravel, 50% sand, 30% clay, moderate brown (5YR 4/4 to 5YR 3/4), slightly moist and loose. Coarse: subrounded to subangular. Fines: low plasticity	SC	5YR 4/4-5YR 3/4	SM																									260			
-																														-		
-																														-		
-																														-		
-																														-		
265	251'-255': Clayey SAND with gravel, SC, 20% gravel, 50% sand, 30% clay, moderate brown (5YR 4/4 to 5YR 3/4), slightly moist and loose. Coarse: subrounded to subangular. Fines: low plasticity	SC	5YR 4/4-5YR 3/4	SM																									265			
-																														-		
-																														-		
-																														-		
-																														-		
270	251'-255': Clayey SAND with gravel, SC, 20% gravel, 50% sand, 30% clay, moderate brown (5YR 4/4 to 5YR 3/4), slightly moist and loose. Coarse: subrounded to subangular. Fines: low plasticity	SC	5YR 4/4-5YR 3/4	SM																									270			
-																														-		
-																														-		
-																														-		
-																														-		
275	251'-255': Clayey SAND with gravel, SC, 20% gravel, 50% sand, 30% clay, moderate brown (5YR 4/4 to 5YR 3/4), slightly moist and loose. Coarse: subrounded to subangular. Fines: low plasticity	SC	5YR 4/4-5YR 3/4	SM																									275			
-																														-		
-																														-		
-																														-		
-																														-		
280	251'-255': Clayey SAND with gravel, SC, 20% gravel, 50% sand, 30% clay, moderate brown (5YR 4/4 to 5YR 3/4), slightly moist and loose. Coarse: subrounded to subangular. Fines: low plasticity	SC	5YR 4/4-5YR 3/4	SM																									280			
-																														-		
-																														-		
-																														-		
-																														-		

Scale:

Drilling Contractor:

Driller:

USC (MPa)

R0	0.25-1.0
R1	1.0-5.0
R2	5.0-25
R3	25-50
R4	50-100
R5	100-250
R6	>250



APPENDIX B-2

**2004 PHELPS DODGE TYRONE, INC.
GEOLOGIC LOGS**

Orientation: -90

Easting: 9536.6

North: 19405.4

Graphic Log

Graphic Log

Graphic Log Notes

Alteration

Code

Alteration

WCI

XC

OS

Qtz

Ksp

Chl

FeOx

FeSt

CuOx

CuSt

Py

PySt

Cc

CcSt

PySt

Cc

CcSt

PySt

Cc

CcSt

PySt

Cc

CcSt

PySt

Cc

CcSt

PySt

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PySt

Cc

CcSt

PySt

Cc

CcSt

PySt

Cc

CcSt

PySt

Cc

Orientation: -90

Easting: 9536.6

North: 19405.4

Graphic Log

Graphic Log

Graphic Log Notes

Alteration

Code

Alteration

WCI

XC

OS

Qtz

Ksp

Chl

FeOx

FeSt

CuOx

CuSt

Py

PySt

Cc

CcSt

PySt

Cc

CcSt

PySt

Cc

CcSt

PySt

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Cc

Orientation: -90

Easting: 9536.6

North: 19405.4

Graphic Log

Graphic Log

Graphic Log Notes

Alteration

Code

Alteration

WCI

XC

OS

Qtz

Ksp

Chl

FeOx

FeSt

CuOx

CuSt

Py

PySt

Cc

CcSt

PySt

Cc

CcSt

PySt

Cc

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PySt

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Orientation: -90

Easting: 9536.6

North: 19405.4

Graphic Log

Graphic Log

Graphic Log Notes

Alteration

Code

Alteration

WCI

XC

OS

Qtz

Ksp

Chl

FeOx

FeSt

CuOx

CuSt

Py

PySt

Cc

CcSt

PySt

Cc

CcSt

PySt

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Orientation: -90

Easting: 9536.6

North: 19405.4

Graphic Log

Graphic Log

Graphic Log Notes

Alteration

Code

Alteration

WCI

XC

OS

Qtz

Ksp

Chl

FeOx

FeSt

CuOx

CuSt

Py

Project: TSGT-3

Hole Number: 14300.3

Date Drilled: 10-05-04

Type: SQUC

East: 6297.6

Logged by: R3W

15998.5

Orientation: R3W

14300.3

Hole Depth: 250

04-22-05

Notes

Analysis		Interval		Drill Log		Graphic Log		Graphic Log Notes		Alteration		Mineralization (vol%)										Enrich		Notes						
Tcu	Ox Cu	Q/LT	Elev	Fl	H ₂ O	Rock				WC	YC	QS	Qtz	Ksp	Chl	FeOx	FeSt	CuOx	CuSt	Py	PySt	Cc	CcSt		Cpy	CpySt	Omni	Omni2	Other	
			4716																											POORLY SORTED, ROCKS TO CORRECTION. SANDY, SANDY CLAY, 2-16. LATE KONGSLOAN CORREL. OF 110 W/SS ALTERATION U.S. STAY & 60 CHANGES FROM GILA TO KAMP. POORLY UP ALTERATION FRAGMENTS. WHITE SANDS STAYS ON V.M. CORE FROM 71 TO 75. ALL ROCKS BURNING & FINGERED NO VISIBLE TY OR CU MINERALS.
			116																											110 FRAGMENTS IN MINERALS & LIESSEM. MAJOR TR OF FINGER CU MINERALS OF 30V. NO VISIBLE CU OX MINERALS. 15% K-SPAC.
			4926																											ANGULAR FRAGMENTS OF 110 W/SS K-SPAC. VENTILATES DIS OF TY W/ VIGN. WEAK ENRICH OF CL. LIGHT BROWN IN COLOR.
			4816																											FRAGMENTS OF 110 W/SS K-SPAC. DIS OF TY W/ VIGN. WEAK ENRICH OF CL. LIGHT BROWN IN COLOR.
			4716																											ANGULAR FRAGMENTS OF 110 W/SS K-SPAC. DIS OF TY W/ VIGN. WEAK ENRICH OF CL. LIGHT BROWN IN COLOR.
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			4716										</																	

Project: TSGT-3
Hole Number: 14300.3
Date Drilled: 10-05-04
Type: S011C

Eastings: 1598.5
Orientation: -30°

C.E.: 62976

04-25-05

Analysis	Interval	Drill Log	Hole Depth	Graphic Log	Graphic Log Notes	Alteration										Mineralization (wt%)										Enrich		Notes
						Alteration	WC	YC	QS	QZ	Ksp	Chl	FeOx	FeSt	CuOx	CuSt	Py	PxSt	Cc	CaSt	Cpy	CpySt	Omn1	Omn2	Other			
Tcu	Ox/Cu	Q/LT Elev.	FL	H ₂ O	Rock	PALT: 5 SALT: 3 CLAY: 3 LCP:							28 HA	1			28	1	TR	1	TR	1						1-5% CARBONATE FRAGMENTS OF 110 - FRESH ALYDIT ALL DISPERSED. NO OXIDATION VIGIL LITTLE ENRICHED. NO CU OR MINERALS LITTLE CLAY.
						PALT: 3 SALT: 3 CLAY: 6 LCP:							28 HA	1														FROM 102.6 TO 103.5 SAME AS ABOVE. FROM 103.5 TO 107.6 - HIGHLY ALTERED SOOY 110 FRAGMENTS - PY COMPLETELY OXIDIZED TO H ₂ O. HIGH CLAY. NO VISIBLE CU OR OX.
						PALT: 3 SALT: 6 CLAY: 6 LCP:							28 HA	1														HIGHLY ALTERED MAP HIGH CLAY CONTAINING PY COMPLETELY OX TO H ₂ O. NO ENRICH OR CU OR MINERALS.
						PALT: 3 SALT: 6 CLAY: 6 LCP:							28 HA	1														HIGHLY ALTERED MAP HIGH CLAY ALL SOLIDS COMPLETELY OXIDIZED TO H ₂ O NO ENRICH NO VISIBLE CU OR MINERALS
						PALT: 3 SALT: 6 CLAY: 6 LCP:							28 HA	1														HIGHLY ALTERED 110 HIGH CLAY NO REMAINING SOLIDS - COMPLETELY OX TO AGGREGATE H ₂ O. NO VISIBLE CU OX.
						PALT: 3 SALT: 6 CLAY: 6 LCP:							28 HA	1														HIGHLY ALTERED 110 HIGH CLAY CONTAINING NO REMAINING SOLIDS - PY COMPLETELY OXIDIZED TO H ₂ O. NO VISIBLE CU OX
						PALT: 3 SALT: 6 CLAY: 6 LCP:							28 HA	1														HIGHLY ALTERED 110 HIGH CLAY CONTAINING NO REMAINING SOLIDS - PY COMPLETELY OX TO H ₂ O. SOME FRAGMENTS OF PY NO VISIBLE CU OX.
						PALT: 3 SALT: 6 CLAY: 6 LCP:							28 HA	1														HIGHLY ALTERED MAP HIGH CLAY CONTAINING MAJOR TR OF REMAINING SOOY - STEADY AM IN CLAY. NO VISIBLE CU OX
						PALT: 3 SALT: 6 CLAY: 6 LCP:							28 HA	1														HIGHLY ALTERED MAP W/ FRAG OF FRESH CARBONATE. NO REMAINING SOLIDS NO VISIBLE CU OX MINERALS
						PALT: 3 SALT: 6 CLAY: 6 LCP:							28 HA	1														HIGHLY ALTERED 110 W/ GT > H ₂ O. NO REMAINING SOLIDS. NO VISIBLE CU OX.
Composite:																												

75

50

Project: TSGT-3
Hole Number: 14500.3
Date Drilled: 0-05-04 Type: SONIC
Nothing: 250'
Hole Depth: 250'

Easting: 1598.5
C.E.: 6297.6
Logged by: RTU

04-25-05

Data Used		Interval	Drill Log	Graphic Log	Graphic Log Notes	Alteration		Mineralization (wt%)										Enrich		Notes							
Ten	OxCo	QLT Elev	Fl	H ₂ O	Rock	WC	YC	QS	Qtz	Ksp	Chl	FeOx	Fest	CuOx	QuSt	Py	PySt	Cc	CcSt		Cpy	CpySt	Omin1	Omin2	Other	Oxide	
			191.6									None				2%	1	TR	1	1	1	1					LARGE HARD FRAGMENTS OF MP CONTAINING SMALL DISS GRAINS OF PY NO VISIBLE ENRICH NO VISIBLE CU OX TR CC ENRICH IN CLAY.
			192.6									None				2%	1	TR	1	1	1	1					FRAGMENTS OF HARD MP w/ SMALL DISS GRAINS OF PY. MINOR CC ENRICH IN CLAY AREAS.
			201.6									None				1.5%	9	1	1	1	1	1					LARGE HARD FRAGMENTS OF MP CONTAINING SMALL DISS GRAINS OF PY NO ENRICH NO CU OX
			210.6									TR	1			1.5%	3	TR	1	1	1	1					ANGULAR FRAGMENTS OF ALTERED MP CONTAINING SMALL DISS GRAINS OF PY. VERY MINOR TR OF CC ENRICH NO CU OX
			211.6									TR	1			1.5%	1	1	1	1	1	1					HARD FRAGMENTS OF MP CONTAINING SMALL DISS GRAINS OF PY NO VISIBLE ENRICH NO CU OX
			222.6									1.5%	1			5%	1	TR	1	1	1	1					ANGULAR FRAGMENTS OF MP CONTAINING SMALL DISS GRAINS OF PY. VERY MINOR TR OF CC ENRICH OF PY NO VISIBLE CU OX
			227.6									5%	1			1%	1	1.5%	1	1	1	1					FRAGMENTS OF ALTERED MP CONTAINING SMALL DISS GRAINS OF PY NO VISIBLE CU OX
			232.6									2%	3			-	-	-	-	-	-	-					HIGHLY ALTERED MP. MINOR DISS. NO VISIBLE DISS. MINOR DISS. ON FINEST. NO VISIBLE CU OX
			237.6									1%	3			1%	1	-	-	-	-	-					ALTERED MP. MINOR PY AS DISS. MINOR DISS. NO VISIBLE ENRICH OR CU OX MINERALS.
			242.6									2.5%	3			1%	1	TR	1	1	1	1					ALTERED MP. MINOR DISS. PY w/ MINOR TR OF CC. MINOR DISS. MINOR DISS. VERY RESIDUAL CORE FROM 247.6 TO 250. NO VISIBLE CU OX MINERALS
			250																								

Composite

Phelps Dodge Tyrone Mine - Geological Services

Drill Hole Logging Form

Page: 2 of 6

Project: TSGT-4

Hole Number: 10038.6

Date Drilled: 07-30-04

Type: C-10

Orientation: 4155.5

East: 6310.7

Logged by:

North: 10038.6

Hole Depth: 215

Analysis		Interval		Drill Log		Graphic Log		Graphic Log Notes		Alteration										Mineralization (vol%)										Enrich		Notes
Tot	Oxide	Q	LT	Elev	Fl	H ₂ O	Rock			WC	VC	GS	Ox	Ksp	CH	FeOx	Fest	CuOx	CuSi	Py	PySi	Cc	CcSi	Cpy	CpySi	Omn1	Omn2	Other	Oxide			
				410													7%	3	0.4%	3											HIGHLY ALTERED MP. NO VISIBLE CLAYS. NO VISIBLE SULFIDES. TRACE SULFIDES REMAINING FROM 40' TO 41' SOLUBLE CU IN CLAY. LIGHT BROWNISH-BLUE	
				440													20%	3	0.4%	3											HIGHLY ALTERED MP. MOSTLY CLAYS. NO VISIBLE SULFIDES. TRACE SULFIDES. CU IN CLAYS. TRACE GYPSUM	
				510													15%	3		TR	1										HIGHLY ALTERED MP. HIGH CLAY. TR. DYES PY. NO VISIBLE ENRICH. NO VISIBLE CU OX	
				560													2%	3													HIGHLY ALTERED MP. HIGH CLAY. STRONG HMA FROM 51' TO 55'. NO VISIBLE PY OR OTHER SULFIDES. NO VISIBLE CU OX	
				610													20%	3													HIGHLY ALTERED MP. HIGH CLAY. STRONG HMA FROM 51' TO 61'. NO VISIBLE NO VISIBLE PY OR ENRICHMENT	
				650													2%	3		125	3	10%	3								HIGHLY ALTERED MP. HIGH CLAY. FROM 63' TO 64' DYES PY W/ WEAK ENRICHMENT. FROM 64' TO 66' GT > HMA. NO VISIBLE CU OX	
				710													2%	3													HIGHLY ALTERED MP. PY > GT. LIGHT YELLOW-BROWNISH GR. ON PERIPHERIES. NO VISIBLE PY OR ENRICH. NO VISIBLE CU OX.	
				750													15%	3													HIGHLY ALTERED MP. HIGH CLAY. NO VISIBLE PY OR ENRICH. NO VISIBLE CU OX. LIGHT BROWN-SLIGHTLY YELLOWISH	
				810													20%	3		16	1										HIGHLY ALTERED MP. HIGH CLAY. NO VISIBLE CU OX. SOFT GRABBY	
				850													15%	3													HIGHLY ALTERED MP. HIGH CLAY. NO VISIBLE SULFIDES. NO VISIBLE CU OX. LIGHT BROWN w/ SOME YELLOWISH	
Composite																																

Project: TSGT-4

Hole Number: 10338.6

Eastings: 4155.5

C.E.: 6810.7

Date Drilled: 07-23-01

Hole Depth: 273

Altitude: 70°

Logged by: RTN

05-02-05

Date Drilled: 07-25-00 Type: 201C										Hole Depth: 273		Graphic Log		Graphic Log Notes		Alteration										Mineralization (wt%)										Enrich										Notes																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																	
Analysis		Interval		Drill Log		Rock		H ₂ O		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL		FL	

[illegible]

APPENDIX B-3

2005 ROTOSONIC BOREHOLE LOGS

ROTOSONIC COREHOLE LOG - Tyrone Borehole S-3

Project: Tyrone 1A Stockpile

Datum:

Collar Elev:

Project No.: 013-1595-002

Drill date: 9/15/2005

Coordinates N:

E:

Sheet

1 of 3

Location: Tyrone Mine

Drill rig: Sonic

Azimuth: N/A

Inclination:

Depth (ft)	Soil / Rock Type		Group Symbol	Color code (Munsell)	D-dry SM-slightly moist M-moist VM-very moist S-saturated	Moisture	Core recovery (%)				% 3-inch plus (%)				Maximum particle size in.				ISRM Strength Index for CLASTS										Physical Testing	Notes Test Results
	Description (i.e. Group Name, % fines/sand/gravel, plasticity, color, HCL reaction, cementation, angularity, odor, structure)	D-Point Load-diametral A-Point Load-axial S-Sieve PI-Plasticity Index					0-20	20-40	40-60	60-80	80+	0-3	3-8	8-20	20-50	50+	1	2	3	4	5+	R6	R5	R4	R3	R2	R1	R0		

0	Red brn Clayey gravel (GC), cohesive, fine low to med plasticity	GC	10YR 5/4	M																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																										
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Scale:

Drilling Contractor:

Driller:

USC (MPa)

R0	0.25-1.0
R1	1.0-5.0
R2	5.0-25
R3	25-50
R4	50-100
R5	100-250
R6	>250



tyrone S3 ver 2.xls

ROTOSONIC COREHOLE LOG - Tyrone Borehole S-3

Project: Tyrone 1A Stockpile

Datum:

Collar Elev:

Project No.: 013-1595-002

Drill date:

Coordinates N: E:

Sheet 3 of 3

Location: Tyrone Mine

Drill rig:

Azimuth: N/A

Inclination:

Depth (ft)	Soil / Rock Type		Color code (Munsel)	D-dry SM-slightly moist M-moist VM-very moist S-saturated	Core recovery (%)				% 3-inch plus (%)				Maximum particle size in.				ISRM Strength Index for CLASTS				D-Point Load-diametral A-Point Load-axial S-Sieve PI-Plasticity Index				Notes Test Results				
	Description (i.e. Group Name, % fines/sand/gravel, plasticity, color, HCL reaction, cementation, angularity, odor, structure)	Group Symbol			Moisture	0-20	20-40	40-60	60-80	80+	0-3	3-8	8-20	20-50	50+	1	2	3	4	5+	R6	R5	R4	R3			R2	R1	R0
Physical Testing																													

80	Weathered zone, clayey sand, cohesive, variable plasticity, stiff to very stiff (S4 to S5), weathered rocky zone at 88'	SC	10R 4/6	M to SM																										80	
85																															85
90																															90
95	gravelly zone, clayey gravel (GC), variable plasticity, stiff to very stiff groundmass	GC	10R 4/6	M to SM																											95

Scale:

Drilling Contractor:

Driller:

USC (MPa)

R0	0.25-1.0
R1	1.0-5.0
R2	5.0-25
R3	25-50
R4	50-100
R5	100-250
R6	>250



ROTOSONIC COREHOLE LOG - Tyrone Borehole S-4

Project: Tyrone 1A Stockpile

Datum:

Collar Elev:

Project No.: 013-1595-002

Drill date: 9/16/2005

Coordinates N:

E:

Sheet

2 of 4

Location: Tyrone Mine

Drill rig:

Azimuth: N/A

Inclination:

Depth (ft)	Soil / Rock Type		Color code (Munsell)	D-dry SM-slightly moist M-moist VM-very moist S-saturated	Core recovery (%)					% 3-inch plus (%)					Maxiumum particle size in.					ISRM Strength Index for CLASTS										D-Point Load-diametral				Notes Test Results			
	Description (i.e. Group Name, % fines/sand/gravel, plasticity, color, HCL reaction, cementation, angularity, odor, structure)	Group Symbol																												A-Point Load-axial							
																														S-Seive	PI-Plasticity Index						
			Moisture	0-20	20-40	40-60	60-80	80+	0-3	3-8	8-20	20-50	50+	1	2	3	4	5+	R6	R5	R4	R3	R2	R1	R0	Physical Testing											
40	Clayey zone, clayey gravel to clayey sand (GC to SC), yellowish gray, matrix stiff to very stiff (S4 to S5), rocky at 45-47	GC to SC	5GY 6/1																																40		
-																																			-		
-																																				-	
-																																				-	
45																																				45	
-	Silty to clayey gravel (GC to GM), non plastic fines, stiff to very stiff,	GC to GM	5Y 6/1 to 5Y 5/2																																-		
-																																				-	
-																																					-
-																																					-
50																																					
-	rock/boulder		N7																																-		
-																																				-	
-																																					-
-																																					-
55																																					
-	Clayey gravel red brn, matrix stiff to very stiff, mod plasticity	GC	5YR 5/6																																-		
-																																				-	
-																																					-
-																																					-
60																																					
-	clayey gravel to sand (GC to SC), stiff to very stiff matrix, low plasticity	GC to SC																																	-		
-																																				-	
-																																				-	
-																																				-	
65																																					
-	Clayey gravel, lt gray to mottled red, low to mod plasticity fines, cohesive	GC	N7 locally 5YR 4/4																																-		
-																																				-	
-																																				-	
-																																				-	
70																																					
-	Clayey Gravel (GC), gray, mottled, locally plastic, groundmass S4 to S5	GC																																	-		
-																																				-	
-																																				-	
-																																				-	
75																																					
-	rock/boulder																																		-		
-																																				-	
-																																				-	
-																																				-	
80																																					
-	clayey gravel, brown	GC	5YR 4/4																																-		
-																																				-	
-																																				-	
-																																				-	
-																																					-

Scale:

Drilling Contractor:

Driller:

USC (MPa)

R0	0.25-1.0
R1	1.0-5.0
R2	5.0-25
R3	25-50
R4	50-100
R5	100-250
R6	>250



ROTOSONIC COREHOLE LOG - Tyrone Borehole S-4

Project: Tyrone 1A Stockpile

Datum:

Collar Elev:

Project No.: 013-1595-002

Drill date: 9/16/2005

Coordinates N:

E:

Sheet

3 of 4

Location: Tyrone Mine

Drill rig:

Azimuth: N/A

Inclination:

Depth (ft)	Soil / Rock Type		Group Symbol	Color code (Munsell)	D-dry SM-slightly moist M-moist VM-very moist S-saturated	Core recovery (%)				% 3-inch plus (%)				Maximum particle size in.				ISRM Strength Index for CLASTS				D-Point Load-diametral A-Point Load-axial S-Seive PI-Plasticity Index				Notes Test Results																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																			
	Description (i.e. Group Name, % fines/sand/gravel, plasticity, color, HCL reaction, cementation, angularity, odor, structure)	Moisture				0-20	20-40	40-60	60-80	80+	0-3	3-8	8-20	20-50	50+	1	2	3	4	5+	R6	R5	R4	R3	R2		R1	R0	Physical Testing																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																
80	Clayey gravel, brn, mottled, low to mod plasticity fines	GC	5YR 4/4	M																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																									

Scale:

Drilling Contractor:

Driller:

USC (MPa)

R0	0.25-1.0
R1	1.0-5.0
R2	5.0-25
R3	25-50
R4	50-100
R5	100-250
R6	>250



ROTOSONIC COREHOLE LOG - Tyrone Borehole S5

Project: Tyrone 1A Stockpile

Datum:

Collar Elev:

Project No.: 013-1595-002

Drill date: 9/17/2005

Coordinates N: E:

Sheet 2 of 5

Location: Tyrone

Drill rig:

Azimuth: N/A

Inclination:

Depth (ft)	Soil / Rock Type		Group Symbol	Color code (Munsell)	D-dry SM-slightly moist M-moist VM-very moist S-saturated	Core recovery (%)				% 3-inch plus (%)				Maximum particle size in.				ISRM Strength Index for CLASTS				D-Point Load-diametral A-Point Load-axial S-Seive PI-Plasticity Index				Notes Test Results										
	Description (i.e. Group Name, % fines/sand/gravel, plasticity, color, HCL reaction, cementation, angularity, odor, structure)	Moisture																				Physical Testing														
						0-20	20-40	40-60	60-80	80+	0-3	3-8	8-20	20-50	50+	1	2	3	4	5+	R6		R5	R4	R3		R2	R1	R0							
40	Clayey gravel, GC, lt brn, locally cohesive, 10% clay, NP to moderate plasticity		GC	5YR 5/6	M																											40	Bucket sample 40-42.5			
-																																			-	
-																																				-
-																																				-
45	Increasing clay, +/- 20%		GC	5YR 4/4	M																													45		
-																																		-		
-																																		-		
-																																		-		
50	Lt brn		GC	5YR 6/4	M																													50		
-																																		-		
-																																		-		
-																																		-		
55			GC	5YR 6/4	M																													55		
-																																		-		
-																																		-		
-																																		-		
60	Cohesive zone, S5, 25% clay, plastic fines		GC	5YR 4/4	M																													60		
-																																		-		
-																																		-		
-																																		-		
65			GC	5YR 5/4	M																													65		
-																																		-		
-																																		-		
-																																		-		
70			GC	5YR 5/4	M																													70		
-																																		-		
-																																		-		
-																																		-		
75			GP	N7/N6	SM																													75		
-																																		-		
-																																		-		
-																																		-		
80	Rock zone/boulder, mostly rock frags in sandy groundmass		GP	N7/N6	SM																													80		
-																																		-		
-																																		-		
-																																		-		

Scale:

Drilling Contractor:

Driller:

USC (MPa)

R0	0.25-1.0
R1	1.0-5.0
R2	5.0-25
R3	25-50
R4	50-100
R5	100-250
R6	>250



ROTOSONIC COREHOLE LOG - Tyrone Borehole S5

Project: Tyrone 1A Stockpile

Datum:

Collar Elev:

Project No.: 013-1595-002

Drill date: 9/17/2005

Coordinates N:

E:

Sheet

3 of 5

Location: Tyrone


Drill rig:

Azimuth: N/A

Inclination:

Depth (ft)	Soil / Rock Type		Group Symbol	Color code (Munsell)	D-dry SM-slightly moist M-moist VM-very moist S-saturated	Core recovery (%)		% 3-inch plus (%)		Maximum particle size in.					ISRM Strength Index for CLASTS						D-Point Load-diametral A-Point Load-axial S-Seive PI-Plasticity Index				Notes Test Results													
	Description (i.e. Group Name, % fines/sand/gravel, plasticity, color, HCL reaction, cementation, angularity, odor, structure)	Moisture				Core recovery (%)				% 3-inch plus (%)		Maximum particle size in.					ISRM Strength Index for CLASTS						Physical Testing															
						0-20	20-40	40-60	60-80	80+	0-3	3-8	8-20	20-50	50+	1	2	3	4	5+	R6	R5		R4		R3	R2	R1	R0									
80	Clayey gravel, pale lt. brn, cohesive zone, few large cobbles. Mostly -3" gravel, 20=30% clay, very stiff (S5), sandy fines	GC	10R 5/4	M																													80					
-																																					-	
-																																						-
-																																						-
-																																						
85	Lt Brn Decrepit rock zone, 87.5-111.5, rock structure altered	GC	5yr 6/1	M																														85				
-																																						-
-																																						-
-																																						-
-																																						
90	Cohesive zone, clayey gravel, very stiff (S5), lt brn, 30% clay, mod to high plasticity, 50 percent gravel	GC	10YR 4/6	M																														90				
-																																						-
-																																						-
-																																						-
-																																						
95	Sandy zone, GW, grayish red, fines non plastic, +/- 50% gravel, clasts deeply weathered, fines plastic at 102'	GW	5R 4/2	M																														95				
-																																						-
-																																						-
-																																						-
-																																						
100	Clayey gravel to clayey sand, red, with weathered decrepit clasts, cohesive zone	GC to SC	5R 4/6	M																														100				
-																																						-
-																																						-
-																																						-
-																																						
105	Clayey gravel to clayey sand, +/- 10% - 2" cobbles in sandy, clayey groundmass,	GC to SC	5Y 8/1	M																														105				
-																																						-
-																																						-
-																																						-
-																																						
110	Clayey gravel to clayey sand, yellow gray, mottled, +/-50% oversize	GC to SC	N6	M																														110				
-																																						-
-																																						-
-																																						-
-																																						
115	Clayey gravel, brn, 30% gravel, 15% clay, low plasticity	GC	5yr 4/4	M																														115				
-																																						-
-																																						-
-																																						-
-																																						
120																																		120				
-																																						-
-																																						-
-																																						-
-																																						

Scale:	USC (MPa)
Drilling Contractor:	R0 0.25-1.0
Driller:	R1 1.0-5.0
	R2 5.0-25
	R3 25-50
	R4 50-100
	R5 100-250
	R6 >250



Scale:

Drilling Contractor:

Driller:

USC (MPa)

R0	0.25-1.0
R1	1.0-5.0
R2	5.0-25
R3	25-50
R4	50-100
R5	100-250
R6	>250



ROTOSONIC COREHOLE LOG - Tyrone Borehole S5

Project: Tyrone 1A Stockpile

Datum:

Collar Elev:

Project No.: 013-1595-002

Drill date: 9/17/2005

Coordinates N:

E:

Sheet

4 of 5

Location: Tyrone

Drill rig:

Azimuth: N/A

Inclination:

Depth (ft)	Soil / Rock Type		Group Symbol	Color code (Munsell)	D-dry SM-slightly moist M-moist VM-very moist S-saturated	Core recovery (%)				% 3-inch plus (%)				Maximum particle size in.					ISRM Strength Index for CLASTS										D-Point Load-diametral				Notes Test Results																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																									
	Description (i.e. Group Name, % fines/sand/gravel, plasticity, color, HCL reaction, cementation, angularity, odor, structure)	Moisture																											A-Point Load-axial					S-Sieve				PI-Plasticity Index																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																				
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120	Yellowish gray sandy clay (SC) groundmass 30% residual (decrepit) cobbles, 20% clay	SC	5YR 7/2	moist																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																						

Scale:

Drilling Contractor:

Driller:

USC (MPa)

R0	0.25-1.0
R1	1.0-5.0
R2	5.0-25
R3	25-50
R4	50-100
R5	100-250
R6	>250



[illegible]

APPENDIX B-4

**2005 PHELPS DODGE TYRONE, INC.
GEOLOGIC LOGS**

Dr.: Ho's Logging Form
Project: LAS JB DUMP DISSEMINATION
Role Number: SIA-1
Date Ordered: 07-11-05
Nothing:
Held Cash:

Hole Number: SIA-1
Date Drilled: 09-11-01
Hole Depth: 10.00
North: 0.00

Case: 02-10073 Date Filed: 07/11/02 Page: 1 of 1

**Eastleigh
Creations**

C.E.:

Logged by: R.J. WARDEN

[illegible]

Project: 1A & 1B Bump Characteristics

Site Number: SIA-1

Date Drilled: 9-14-05

Type: Semi C

Drill Hole Logging Form

Geological Services

Page: 6 of 6

C.E.: A. Londe

Drilled by: A. Londe

Analysis		Interval		Elev. Ft.		Core Log		Grain Log		Grain Log Notes		Orientation		Alteration		Ageation		Notes	
Ten	ORC/OUT	Interval	Core Log	Grain Log	Grain Log Notes	WC	YC	OS	CS	KG	Ch	FC	FS	CL	CS	FC	FS	CL	CS
		1275																	
		130																	
		130																	
		132.5																	
		132.5																	
		135																	
		135																	
		137.5																	
		137.5																	
		140																	
		140																	
		142.5																	
		142.5																	
		145																	
		145																	
		147.5																	
		147.5																	
		150																	
		150																	

Core Log Notes:

Orange tan to grey boulders gravel (up to 20) clay, damp to clay minor clay, damp to dry will support shovel

Tan grey gravel with minor cobbles (up to 20) clay, damp to dry will support shovel

White red clay mixed sand & clay - 26% clay damp sand & local clay ball, consistency is consistency at consistency, shovel support & minor cobbles, boulders (up to 20) clay, damp to dry will support shovel, trace fines

Reddish grey gravel with minor cobbles (up to 20) clay, damp to dry will support shovel

Green gravel and cobbles (up to 20) clay, damp to dry will support shovel

Orange tan to grey gravel with some cobbles (up to 20) clay, damp to dry will support shovel

Alteration:

WC: 100%
YC: 100%
OS: 100%
CS: 100%
KG: 100%
Ch: 100%
FC: 100%
FS: 100%
CL: 100%
CS: 100%

Prepa Dodge Tyone Mine - Geological Services

Page 1 of 5

D-3 Hole Logging Form
Project: IAS 16 DUMP CHARACTERISTICS
Hole Depth: 9431.726
Scale Number: SIA-2
Date Drilled: 04-14-05

Easting: 18943.572
Northing: 9431.726
Core: 6158-877
Logged by: R.J. WARDLER

A-53's			Interval			Core Log			Gratic Log			Alteration										Mineralization Notes										Emch		Notes	
To	From	Core	Interval	Elev	Rt	H ₂ O	Reck	Gratic Log	Gratic Log	Gratic Log	Gratic Log	Code	Fe	Co	Si	Al	Ca	Mg	Na	K	P	S	Cl	Br	I	As	Sb	B	Cu	Zn	Pb	Ag	Other	Desc	
0			0									SALT																							MARON RED CLAY MATRIX ~15% CLAY W/ COARSE SAND GRAVEL & ROCK UP TO 5" ROCKS DAMP CLAY STICKY BUT DOESN'T QUITE FEEL BALL
2.5			2.5									CLAY																							MARON RED CLAY MATRIX ~15% CLAY W/ COARSE SAND GRAVEL & ROCK UP TO 5" ROCKS DAMP CLAY STICKY BUT DOESN'T QUITE FEEL BALL
5.0			5.0									CLAY																							MARON RED CLAY MATRIX ~15% CLAY W/ COARSE SAND GRAVEL & ROCK UP TO 5" ROCKS DAMP CLAY STICKY BUT DOESN'T QUITE FEEL BALL
5.0			5.0									CLAY																							MARON RED CLAY MATRIX ~15% CLAY W/ COARSE SAND GRAVEL & ROCK UP TO 5" ROCKS DAMP CLAY STICKY BUT DOESN'T QUITE FEEL BALL
7.5			7.5									CLAY																							MARON RED CLAY MATRIX ~15% CLAY W/ COARSE SAND GRAVEL & ROCK UP TO 5" ROCKS DAMP CLAY STICKY BUT DOESN'T QUITE FEEL BALL
7.5			7.5									CLAY																							MARON RED CLAY MATRIX ~15% CLAY W/ COARSE SAND GRAVEL & ROCK UP TO 5" ROCKS DAMP CLAY STICKY BUT DOESN'T QUITE FEEL BALL
10.0			10.0									CLAY																							MARON RED CLAY MATRIX ~15% CLAY W/ COARSE SAND GRAVEL & ROCK UP TO 5" ROCKS DAMP CLAY STICKY BUT DOESN'T QUITE FEEL BALL
10.0			10.0									CLAY																							MARON RED CLAY MATRIX ~15% CLAY W/ COARSE SAND GRAVEL & ROCK UP TO 5" ROCKS DAMP CLAY STICKY BUT DOESN'T QUITE FEEL BALL
12.5			12.5									CLAY																							MARON RED CLAY MATRIX ~15% CLAY W/ COARSE SAND GRAVEL & ROCK UP TO 5" ROCKS DAMP CLAY STICKY BUT DOESN'T QUITE FEEL BALL
15.0			15.0									CLAY																							MARON RED CLAY MATRIX ~15% CLAY W/ COARSE SAND GRAVEL & ROCK UP TO 5" ROCKS DAMP CLAY STICKY BUT DOESN'T QUITE FEEL BALL
15.0			15.0									CLAY																							MARON RED CLAY MATRIX ~15% CLAY W/ COARSE SAND GRAVEL & ROCK UP TO 5" ROCKS DAMP CLAY STICKY BUT DOESN'T QUITE FEEL BALL
17.5			17.5									CLAY																							MARON RED CLAY MATRIX ~15% CLAY W/ COARSE SAND GRAVEL & ROCK UP TO 5" ROCKS DAMP CLAY STICKY BUT DOESN'T QUITE FEEL BALL
17.5			17.5									CLAY																							MARON RED CLAY MATRIX ~15% CLAY W/ COARSE SAND GRAVEL & ROCK UP TO 5" ROCKS DAMP CLAY STICKY BUT DOESN'T QUITE FEEL BALL
20.0			20.0									CLAY																							MARON RED CLAY MATRIX ~15% CLAY W/ COARSE SAND GRAVEL & ROCK UP TO 5" ROCKS DAMP CLAY STICKY BUT DOESN'T QUITE FEEL BALL
20.0			20.0									CLAY																							MARON RED CLAY MATRIX ~15% CLAY W/ COARSE SAND GRAVEL & ROCK UP TO 5" ROCKS DAMP CLAY STICKY BUT DOESN'T QUITE FEEL BALL
22.5			22.5									CLAY																							MARON RED CLAY MATRIX ~15% CLAY W/ COARSE SAND GRAVEL & ROCK UP TO 5" ROCKS DAMP CLAY STICKY BUT DOESN'T QUITE FEEL BALL
22.5			22.5									CLAY																							MARON RED CLAY MATRIX ~15% CLAY W/ COARSE SAND GRAVEL & ROCK UP TO 5" ROCKS DAMP CLAY STICKY BUT DOESN'T QUITE FEEL BALL
25.0			25.0									CLAY																							MARON RED CLAY MATRIX ~15% CLAY W/ COARSE SAND GRAVEL & ROCK UP TO 5" ROCKS DAMP CLAY STICKY BUT DOESN'T QUITE FEEL BALL

GS-25819

Field Notes
Project: IASIS DUMP CHARACTERISTICS
Date: 07-19-05
Hole Depth: 159
Interval: 159
Core: 159
Type: 159
Notes: (Rock Description, Alteration, Mineralization, Structure)
LIGHT BROWN TO RED CLAY MATRIX
15-20% CLAY W/ ANGULAR COARSE
SAND, GRAVEL & ROCK FRAGMENTS
UP TO 3" ACROSS. CLAY DOES NOT
BALL-TOO MUCH SANDY
FROM 52.5 TO 54" CAMP GRANULAR
CRUMBLY SAND/ANGULAR CLAY
CLAYISH RED FROM 54" TO 55" HEAVY
STICKY CLAY W/ ANGULAR COARSE
SAND & GRAVEL - MOIST
MEDIUM RED HEAVY STICKY CLAY
MATRIX W/ ANGULAR COARSE SAND
& MINOR GRAVEL. MOIST TO WET
CLAY FORMS BALLS.
MEDIUM RED CLAY MATRIX - 10% CLAY
W/ ANGULAR COARSE SAND, GRAVEL
& ROCK FRAGMENTS - 50% CLAY
OF ALL TYPES UP TO 16" TO 18" BALLS
CLAY FORMS BALLS
REDDISH TO LIGHT BROWN SANDY CLAY
W/ ANGULAR LARGE ANGULAR ROCK
FRAGMENTS. DRY TO ALMOST DRY.
DOES NOT BALL
LIGHT BROWNISH RED CRUMBLY
SANDY TO GRANULAR CLAY DRY TO
ALMOST DRY W/ ANGULAR COARSE
SAND, GRAVEL & ROCK FRAGMENTS
UP TO 5" ACROSS
GRAY SANDY CLAY POWDERY &
MOSTLY DRY W/ ANGULAR COARSE
SAND, GRAVEL & ROCK FRAGMENTS
UP TO 3" ACROSS
MEDIUM RED SANDY CLAY MATRIX
W/ 15% CLAY W/ ANGULAR
COARSE SAND, GRAVEL & ROCK
FRAGMENTS UP TO 3" ACROSS.
DAMP CLAY DOES NOT BALL
REDDISH TO LIGHT BROWN SANDY
CLAY - GRANULAR BLOCK DOES NOT BALL
15% CLAY W/ ANGULAR COARSE
SAND, GRAVEL & ROCK FRAGMENTS
UP TO 4" ACROSS DRY TO ALMOST DRY
MEDIUM TO DARK RED BLOCKY
ANGULAR CLAY W/ COARSE SAND
GRAVEL & ROCK FRAGMENTS
CLAY BALLS DAMP

Phelps Dodge Tyro = Mine - Geological Services
Dull Hole Logging Form
Project: LA 228 Stable Characterization
Hole Number: SM-3
Depth: 249-15.05 Ties. Sonic Hole Depth: 110'

[illegible]

C.E. _____
 Witnessed by A. Landa

Easting: -900

Hole Number: SLA-4
 Date C-Ted: 9/15/05
 Type: Somic
 Northng: Hole Depth: 150'

[illegible]

Goldier removed material listed below page 304

Franks Dodge Type Wire - Geosocial Services
 Drill Hole 100010 5000
 Project: The 28 Stockpile Characterization
 Hole Number: 51A-4
 Date Drilled: 9-16-05
 Depth: 150'

Hole Number: 51A-4										Eastings: -90°										C.E.									
Date Drilled: 9-16-05										Type: Sonar										Location: A. Landa									
Hole Depth: 150'										Alteration										Mineralization (2.3.3)									
Alteration										Geochemical										Grain Size									
Interval										Depth										Notes									
Interval										Depth										Notes									
Interval										Depth										Notes									
Interval										Depth										Notes									
Interval										Depth										Notes									
Interval										Depth										Notes									
Interval										Depth										Notes									
Interval										Depth										Notes									
Interval										Depth										Notes									
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Page 5 of 5
Feb 11 10:00 AM

[illegible]

order took 43-44.5 for analysis, page 07
and removed sample from interval listed below.

C.E.:
Log by: A. Londe

Eastings:
Ore: 4900

Project: SIA-5 Stockpile Characterization
Date: 9-15-05
Time: 20:21.5

Notes

Interval	Depth	Core Log	Graphic Log	Graphic Log Notes	Alteration	Mineralization (wt%)										Other	Notes
						Fe	Co	Cr	Py	Py	Py	Py	Py	Py	Py		
25					PALE												tan light brown cobble (up to 2") gravel and minor clay damp up to mildly wet, sand with clay balls (20%) will compact under shovel.
27.5					PALE												crumbly grey cobbles (up to 2") gravel and clay (10%) mildly wet for shovel, mildly compact under shovel.
30					PALE												red grey cobbles (up to 2") gravel and clay (10%) for shovel, mildly compact under shovel.
32.5					PALE												red, orange grey cobbles (up to 4") gravel and clay (10%) mildly wet for shovel, mildly compact under shovel.
35					PALE												orange grey boulders (up to 5") cobbles of gravel and clay (20%) damp to mildly wet, water gritty, clay balls, will support shovel.
37.5					PALE												gray light brown boulders cobbles of gravel and minor clay (20%) damp to mildly wet, water gritty, clay balls, will support shovel.
40					PALE												brown, black, tan boulders (up to 5") cobbles gravel and clay (20%) for shovel, will support shovel.
42.5					PALE												light brown, over 10% cobbles of gravel and clay (20%) for shovel, will support shovel.
45					PALE												red brown, gravel and clay (20%) for shovel, will support shovel.
47.5					PALE												light brown, red black boulders (up to 5") cobbles of gravel and clay (20%) for shovel, will support shovel.
50					PALE												light brown, red black boulders (up to 5") cobbles of gravel and clay (20%) for shovel, will support shovel.

P.O. Box 1000, 21500
 P.O. Box 1000, 21500
 P.O. Box 1000, 21500

[illegible]

Finess Deep Tyons Mine - Geological Services														
Dredge Logs of Form														
Project: 1A & 1B Stakpile Characterization														
Hole Number: 9-17-55 Type: Sonic Northings: 7005'														
Core Depth: 9-17-55 Type: Sonic Northings: 7005'														
CE: A. L. Lank														
Orientation: -900														
Elev: 150														
Notes														
Rock Description, Alteration, Mineralization, Structure														
Grey, p. and Cobble (up to 2.5')														
Gravel & minor clay (2.5')														
No clay balls, dump, fill														
Support a shovel														
Orange yellow gravel with														
small clay (2.5')														
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Support a shovel														
Orange yellow gravel with														
small clay (2.5')														
clay balls, dump, fill														

* Silver removed sample from interval listed below.

CE: Logged by: A. Lande

Eastings: Orientation: -90°

Project: 8-116 Stockpile Characterization
 Hole Number: S1A-5
 Type: Sonar
 Date: 11-17-05
 Notes: Nothing
 Core Depth: 200.5'

Page 7 of 9

Interval	Core ID	Core Depth	Core Type	Core Material	Core Color	Core Texture	Core Hardness	Core Strength	Core Density	Core Porosity	Core Permeability	Core Conductivity	Core Resistivity	Core Magnetic Susceptibility	Core Radioactivity	Core Other	Notes
160																	Red brown cobbles (up to 3.5') gravel, some clay, will support a shovel dump
152.5																	Red brown cobbles (up to 3.5') gravel, some clay, will support a shovel dump
152.5																	Red brown cobbles (up to 3.5') gravel, some clay, will support a shovel dump
155																	Red brown cobbles (up to 3.5') gravel, some clay, will support a shovel dump
155																	Red brown cobbles (up to 3.5') gravel, some clay, will support a shovel dump
157.5																	Red brown cobbles (up to 3.5') gravel, some clay, will support a shovel dump
157.5																	Red brown cobbles (up to 3.5') gravel, some clay, will support a shovel dump
160																	Red brown cobbles (up to 3.5') gravel, some clay, will support a shovel dump
160																	Red brown cobbles (up to 3.5') gravel, some clay, will support a shovel dump
162.5																	Red brown cobbles (up to 3.5') gravel, some clay, will support a shovel dump
162.5																	Red brown cobbles (up to 3.5') gravel, some clay, will support a shovel dump
165																	Red brown cobbles (up to 3.5') gravel, some clay, will support a shovel dump
165																	Red brown cobbles (up to 3.5') gravel, some clay, will support a shovel dump
167.5																	Red brown cobbles (up to 3.5') gravel, some clay, will support a shovel dump
167.5																	Red brown cobbles (up to 3.5') gravel, some clay, will support a shovel dump
170																	Red brown cobbles (up to 3.5') gravel, some clay, will support a shovel dump
170																	Red brown cobbles (up to 3.5') gravel, some clay, will support a shovel dump
172.5																	Red brown cobbles (up to 3.5') gravel, some clay, will support a shovel dump
172.5																	Red brown cobbles (up to 3.5') gravel, some clay, will support a shovel dump
174																	Red brown cobbles (up to 3.5') gravel, some clay, will support a shovel dump

Site: 22 Decade Tyone M-r - Geology and Services

Project: 2R8 1B Stockpile Characterization
Hole Number: S1A-5
Date: 11/19/05
Hole Depth: 200.5

East: 200
C-Station: -000
Logged by: A. Lande

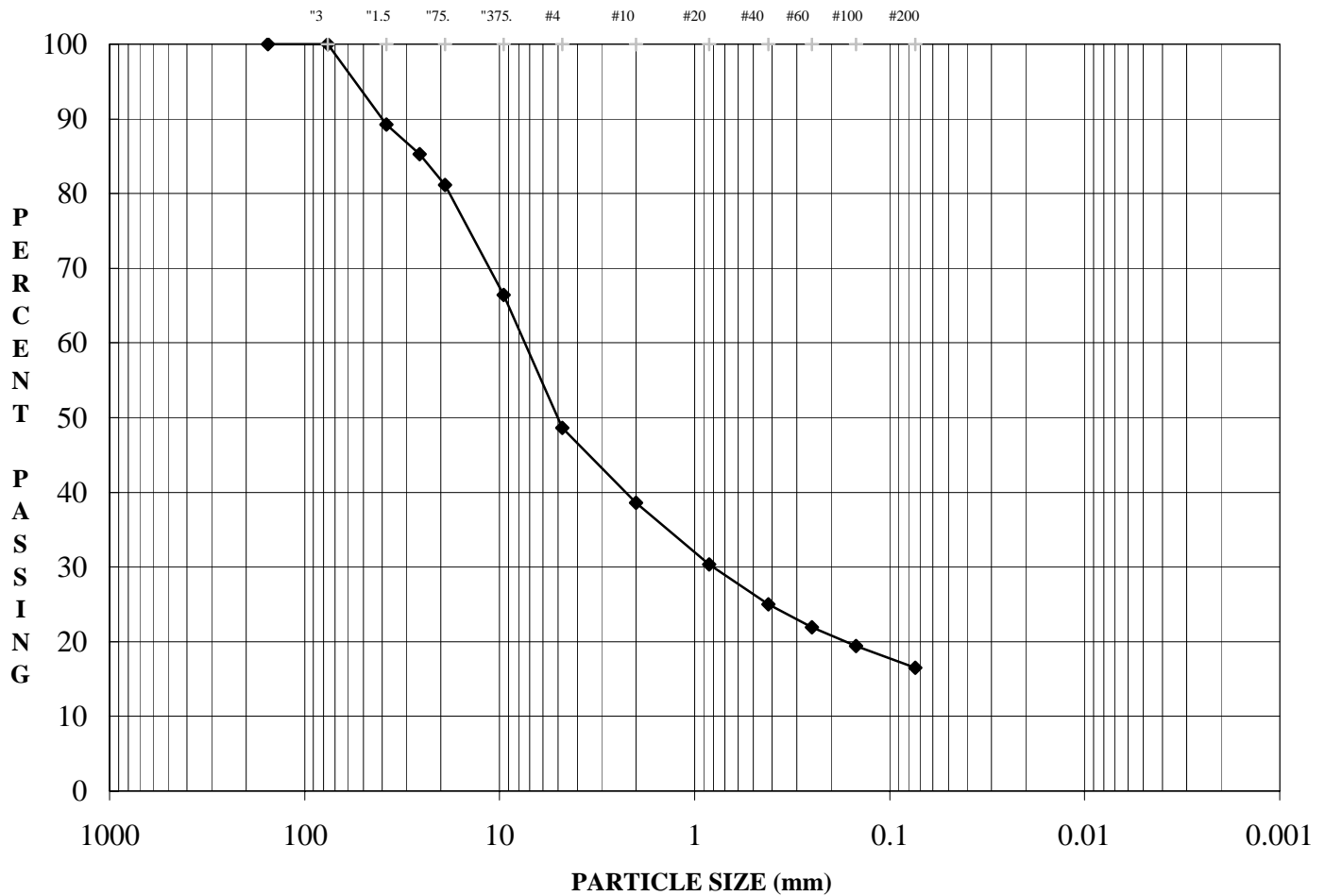
Core	Core ID	Core Elev.	Core Depth	Core Type	Core Notes	Magnetic Susceptibility										Unit	Notes
						WC	YC	OS	CS	MS	PS	GS	CS	GS	CS		
174																	Gravel and clay (10 ft) will form a fairly good wall. Compaction will be good.
175																	Gravel and clay (10 ft) will form a fairly good wall. Compaction will be good.
176																	Gravel and clay (10 ft) will form a fairly good wall. Compaction will be good.
177																	Gravel and clay (10 ft) will form a fairly good wall. Compaction will be good.
178																	Gravel and clay (10 ft) will form a fairly good wall. Compaction will be good.
179																	Gravel and clay (10 ft) will form a fairly good wall. Compaction will be good.
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198																	Gravel and clay (10 ft) will form a fairly good wall. Compaction will be good.
199																	Gravel and clay (10 ft) will form a fairly good wall. Compaction will be good.
200																	Gravel and clay (10 ft) will form a fairly good wall. Compaction will be good.

APPENDIX C
LABORATORY INDEX TESTING

APPENDIX C-1

2000 GRAIN-SIZE ANALYSIS

**PARTICLE-SIZE DISTRIBUTION ASTM D 421 AND D 422
US STANDARD SIEVE OPENING SIZES**



COBBLES	Coarse	Fine	Cor	Med	Fine	Silt or Clay Size
	GRAVEL		SAND			FINES

SAMPLE #: GTP-01/01

DEPTH (ft):

DESCRIPTION: Fine to coarse GRAVEL,
and fine to coarse sand,
some clay (GC)

MC (As tested): 2.2%

LL: 47

PL: 22

PI: 25

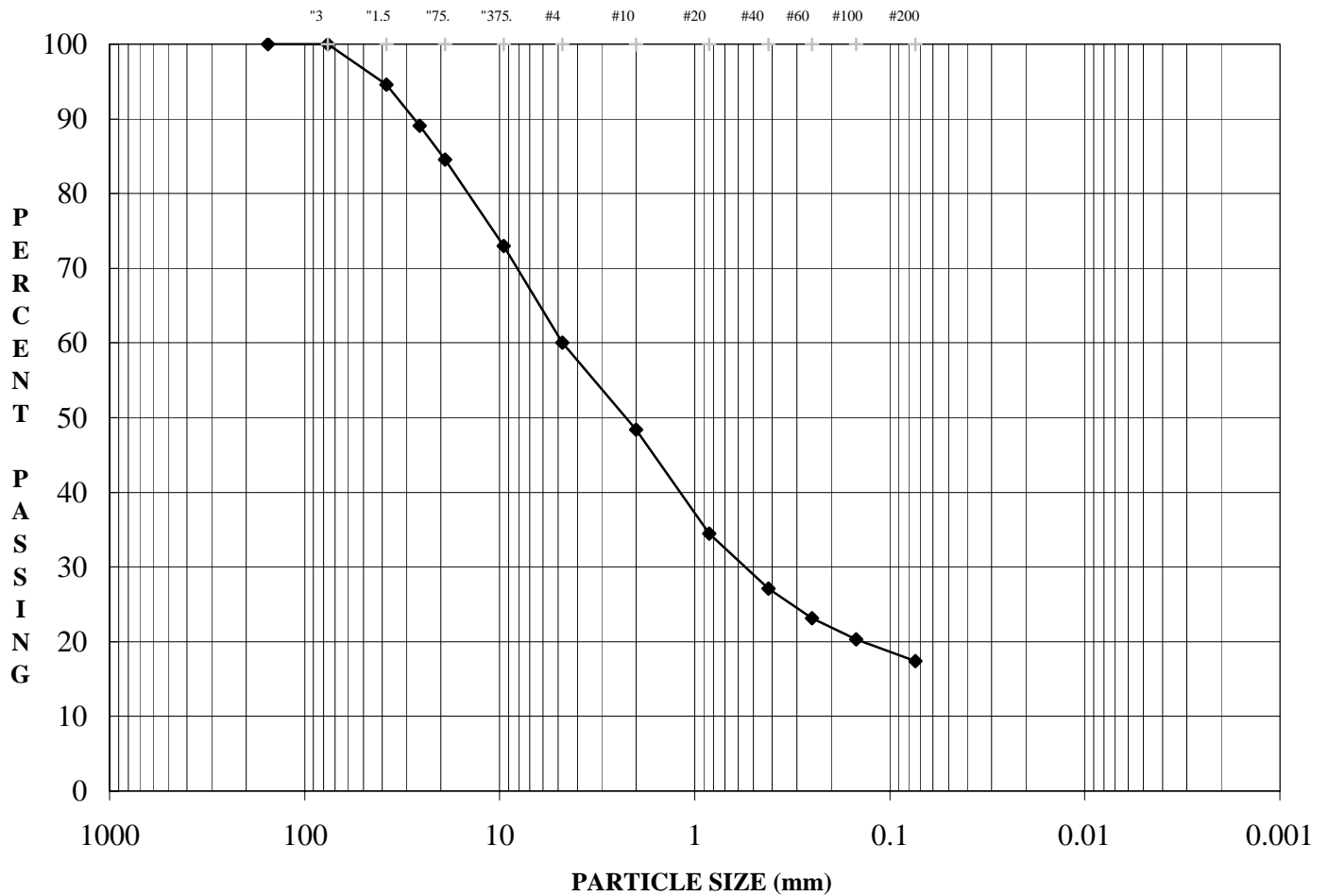
Gs: -

**TYRONE/STABILITY/
993-2546.001**

20-Jan-00
MJK
DMD

**GOLDER ASSOCIATES INC.
LAKEWOOD, COLORADO**

**PARTICLE-SIZE DISTRIBUTION ASTM D 421 AND D 422
US STANDARD SIEVE OPENING SIZES**



COBBLES	Coarse	Fine	Cor	Med	Fine	Silt or Clay Size
	GRAVEL		SAND			FINES

SAMPLE #: GTP-03/02

DEPTH (ft):

DESCRIPTION: Fine to coarse SAND,
and fine to coarse gravel,
some clay (SC)

MC (As tested): 1.0%

LL: 33

PL: 20

PI: 13

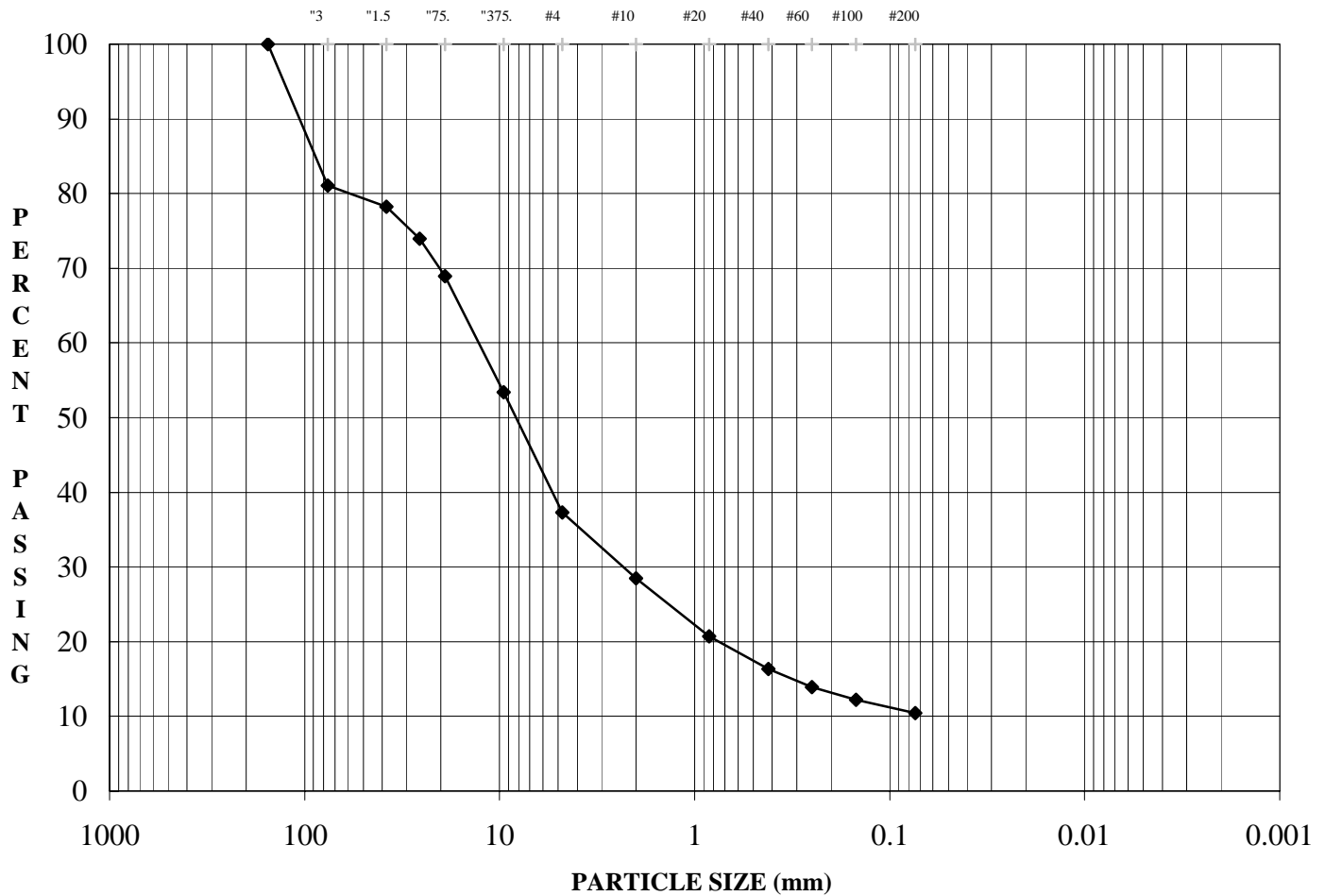
Gs: -

**TYRONE/STABILITY/
993-2546.001**

20-Jan-00
MJK
DMD

**GOLDER ASSOCIATES INC.
LAKEWOOD, COLORADO**

**PARTICLE-SIZE DISTRIBUTION ASTM D 421 AND D 422
US STANDARD SIEVE OPENING SIZES**



	Coarse	Fine	Cor	Med	Fine	Silt or Clay Size
COBBLES	GRAVEL		SAND			FINES

SAMPLE #: GTP-06/03

DEPTH (ft):

DESCRIPTION: Fine to coarse GRAVEL,
some fine to coarse sand,
little clay (GP-GC)

MC (As tested): 3.7%

LL: 42

PL: 20

PI: 22

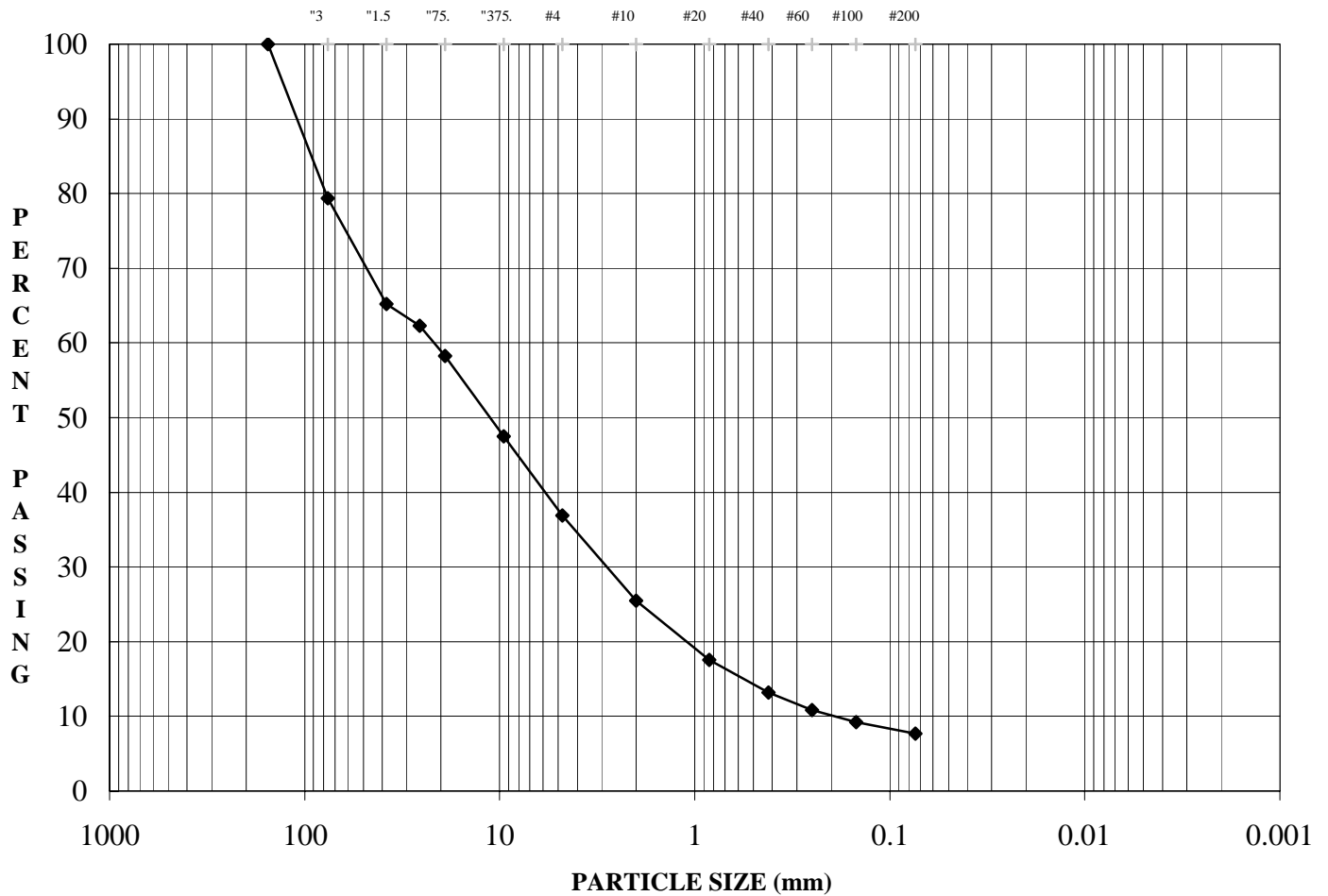
Gs: -

**TYRONE/STABILITY/
993-2546.001**

22-Jan-00
MJK
DMD

**GOLDER ASSOCIATES INC.
LAKEWOOD, COLORADO**

**PARTICLE-SIZE DISTRIBUTION ASTM D 421 AND D 422
US STANDARD SIEVE OPENING SIZES**



COBBLES	Coarse	Fine	Cor	Med	Fine	Silt or Clay Size
	GRAVEL		SAND			FINES

SAMPLE #: GTP-09/05

DEPTH (ft):

DESCRIPTION: Fine to coarse GRAVEL,
some fine to coarse sand,
little clay (GP-GC)

MC (As tested): 3.3%

LL: 39

PL: 20

PI: 19

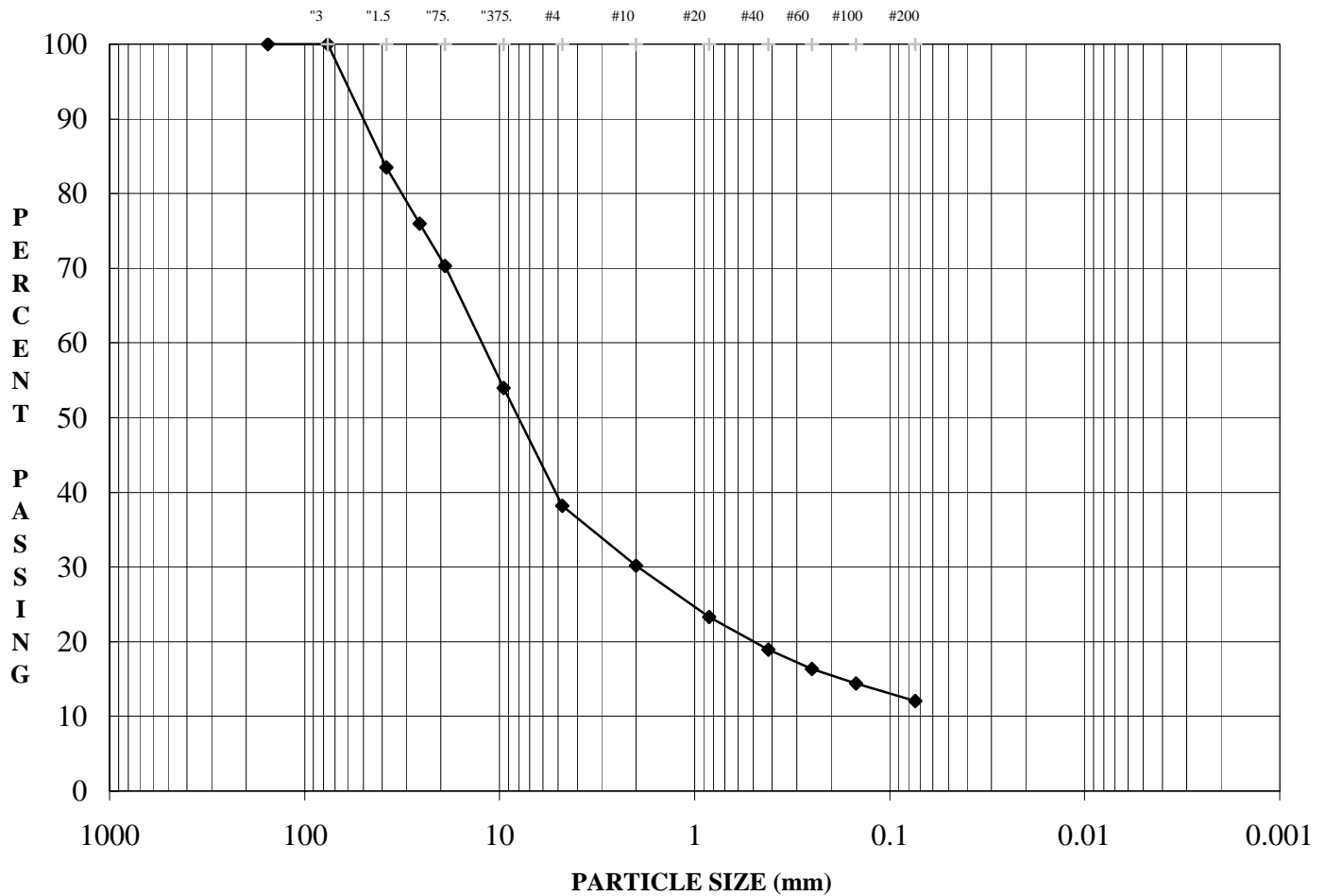
Gs: -

**TYRONE/STABILITY/
993-2546.001**

22-Jan-00
MJK
DMD

**GOLDER ASSOCIATES INC.
LAKEWOOD, COLORADO**

**PARTICLE-SIZE DISTRIBUTION ASTM D 421 AND D 422
US STANDARD SIEVE OPENING SIZES**



	Coarse	Fine	Cor	Med	Fine	Silt or Clay Size
COBBLES	GRAVEL		SAND			FINES

SAMPLE #: GTP-10/04

DEPTH (ft):

DESCRIPTION: Fine to coarse GRAVEL,
some fine to coarse sand,
some clay (GC)

MC (As tested): 3.4%

LL: 47

PL: 21

PI: 26

Gs: -

**TYRONE/STABILITY/
993-2546.001**

22-Jan-00

MJK

DMD

**GOLDER ASSOCIATES INC.
LAKEWOOD, COLORADO**

PERCENT PASSING

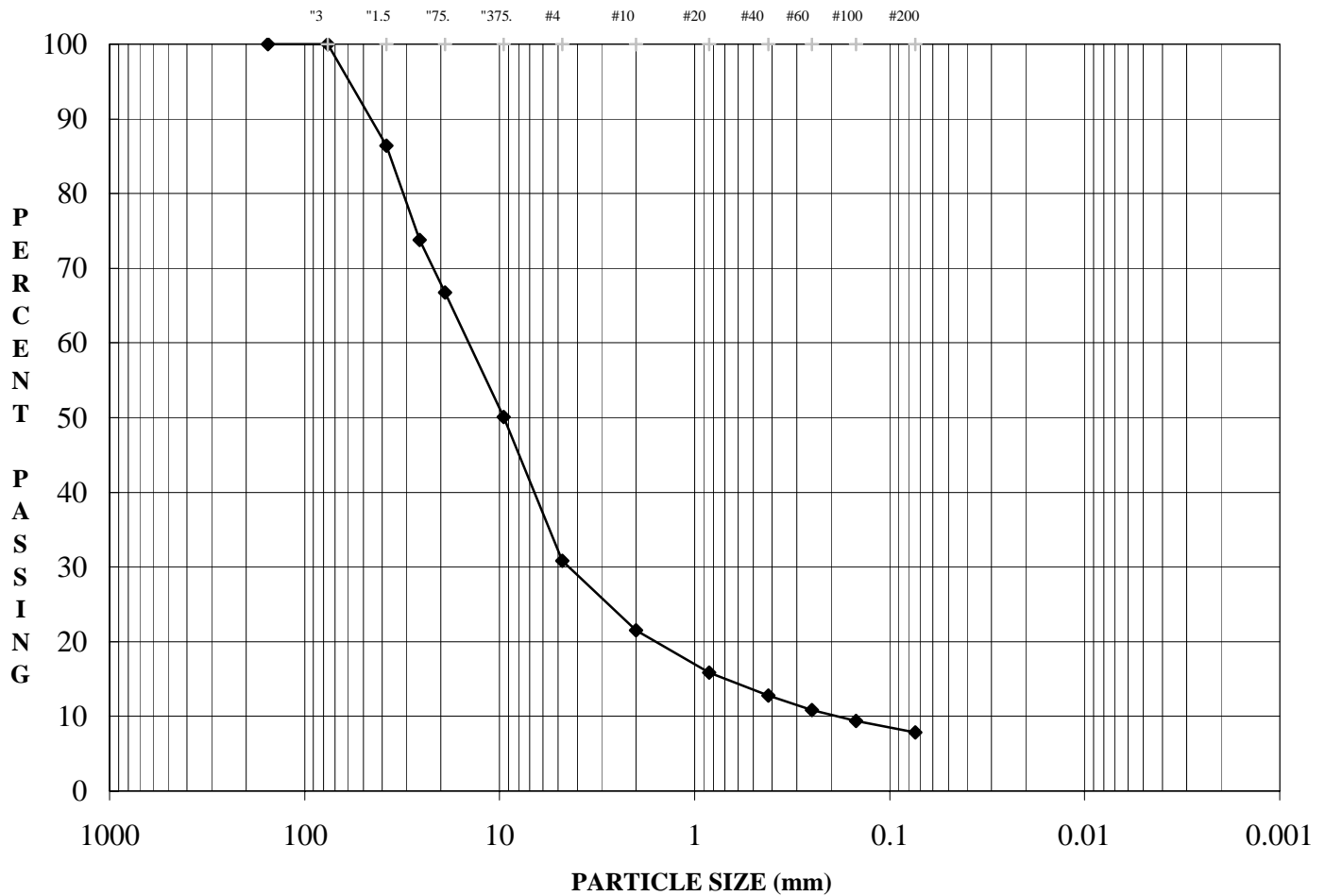
PARTICLE SIZE (mm)

Particle Size (mm)	Percent Passing (%)
1000	100
250	100
150	87
100	81
75	74
60	61
40	49
30	37
20	26
15	20
10	16
7.5	14
60	11

22-Jan-00
MJK
DMD

GOLDER ASSOCIATES INC.
LAKEWOOD, COLORADO

**PARTICLE-SIZE DISTRIBUTION ASTM D 421 AND D 422
US STANDARD SIEVE OPENING SIZES**



	Coarse	Fine	Cor	Med	Fine	Silt or Clay Size
COBBLES	GRAVEL		SAND			FINES

SAMPLE #: GTP-14/07

DEPTH (ft):

DESCRIPTION: Fine to coarse GRAVEL,
some fine to coarse sand,
little clay (GP-GC)

MC (As tested): 4.5%

LL: 28

PL: 15

PI: 13

Gs: -

**TYRONE/STABILITY/
993-2546.001**

22-Jan-00
MJK
DMD

**GOLDER ASSOCIATES INC.
LAKEWOOD, COLORADO**

APPENDIX C-2

**2001 SOIL SUMMARY AND
GRAIN-SIZE ANALYSIS**

TABLE C-2.1
SUMMARY OF SOIL DATA

Sample Type	Sample Number	USCS Soil Classification	Atterberg Limits			Grain-size Distribution		
			LL	PL	PI	% Finer 3/4"	% Finer #4	% Finer #200
bucket	TYTP01-1	GC	38	17	21	77	48	13
	TYTP01-2	GW-GC	30	14	16	78	48	10
	TYTP01-3	GP-GC	36	18	18	75	38	9
	TYTP01-4	GC	40	17	23	76	53	15
	TYTP01-5	GC	39	16	23	79	49	15
	TYTP01-6	GW-GC	29	16	13	70	42	8
	TYTP01-7	SC	28	15	13	81	63	15
	TYTP01-8	SC	24	16	8	85	60	15
	TYTP01-9	GC	30	18	12	80	58	24
	TYTP01-10	SC	30	18	12	96	80	17
	TYTP01-11	GC	37	17	20	85	50	13
	TYTP01-12	GC	30	16	14	77	46	14

Notes:

LL = Liquid Limit

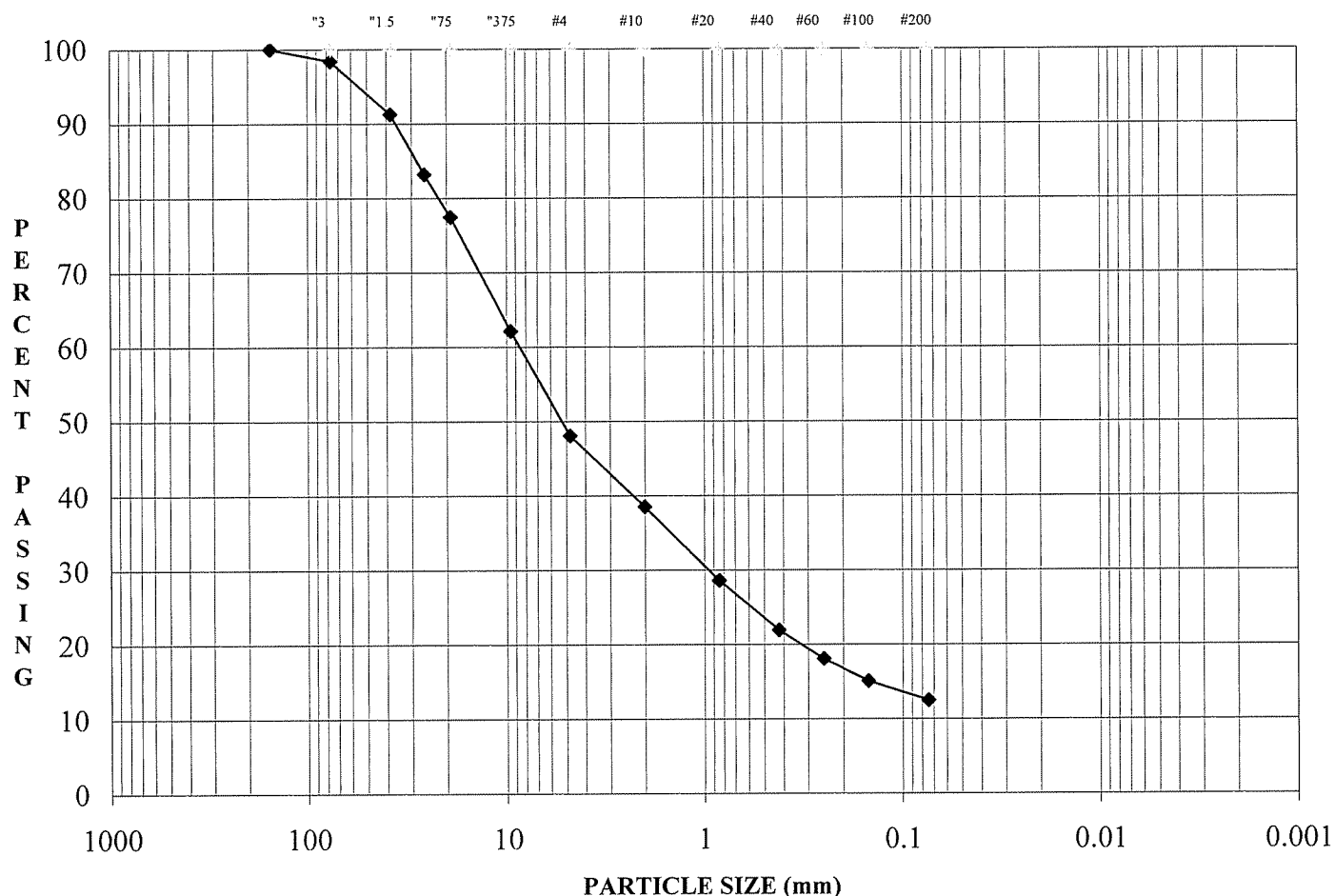
PL = Plastic Limit

PI = Plastic Index

USCS = Unified Soil Classification System

PCF = pounds per cubic foot

**PARTICLE-SIZE DISTRIBUTION ASTM D 421 AND D 422
US STANDARD SIEVE OPENING SIZES**



COBBLES	Coarse	Fine	Cor	Med	Fine	Silt or Clay Size
	GRAVEL		SAND			FINES

SAMPLE #: TYTP01-1

DEPTH (ft):

DESCRIPTION: Clayey gravel with sand (GC)

MC (As tested): 5.7%

LL: 38

PL: 17

PI: 21

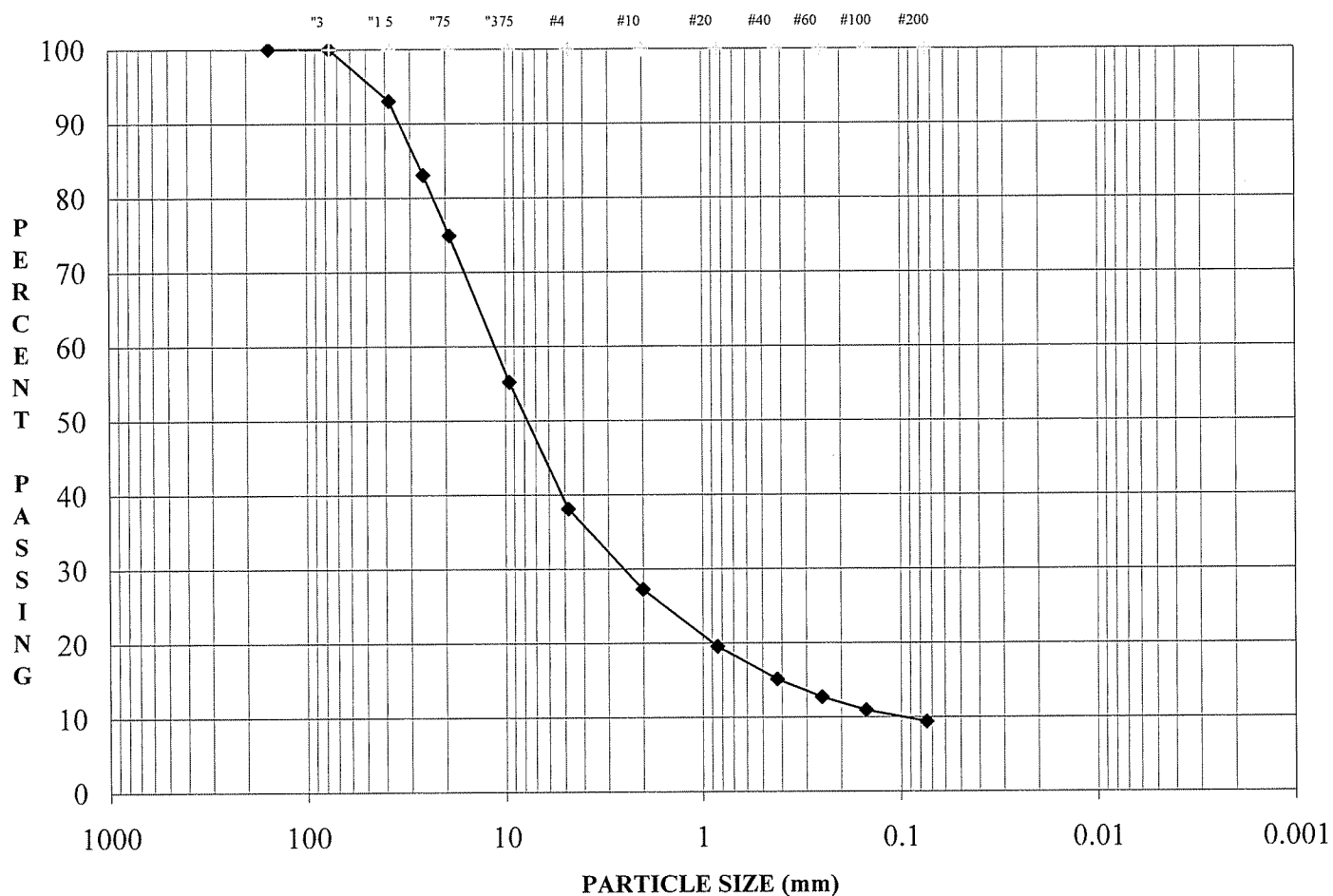
Gs: Gs

**G&K/TYRONE CLOSURE HEARING/AZ
013-1595.002**

23-Oct-01
TM
NG

**GOLDER ASSOCIATES INC.
LAKEWOOD, COLORADO**

**PARTICLE-SIZE DISTRIBUTION ASTM D 421 AND D 422
US STANDARD SIEVE OPENING SIZES**



COBBLES	Coarse	Fine	Cor	Med	Fine	Silt or Clay Size
	GRAVEL		SAND			FINES

SAMPLE #: TYTP01-3

DEPTH (ft):

DESCRIPTION: Poorly graded gravel with clay and sand (GP-GC)

MC (As tested): 7.1%

LL: 36

PL: 18

PI: 18

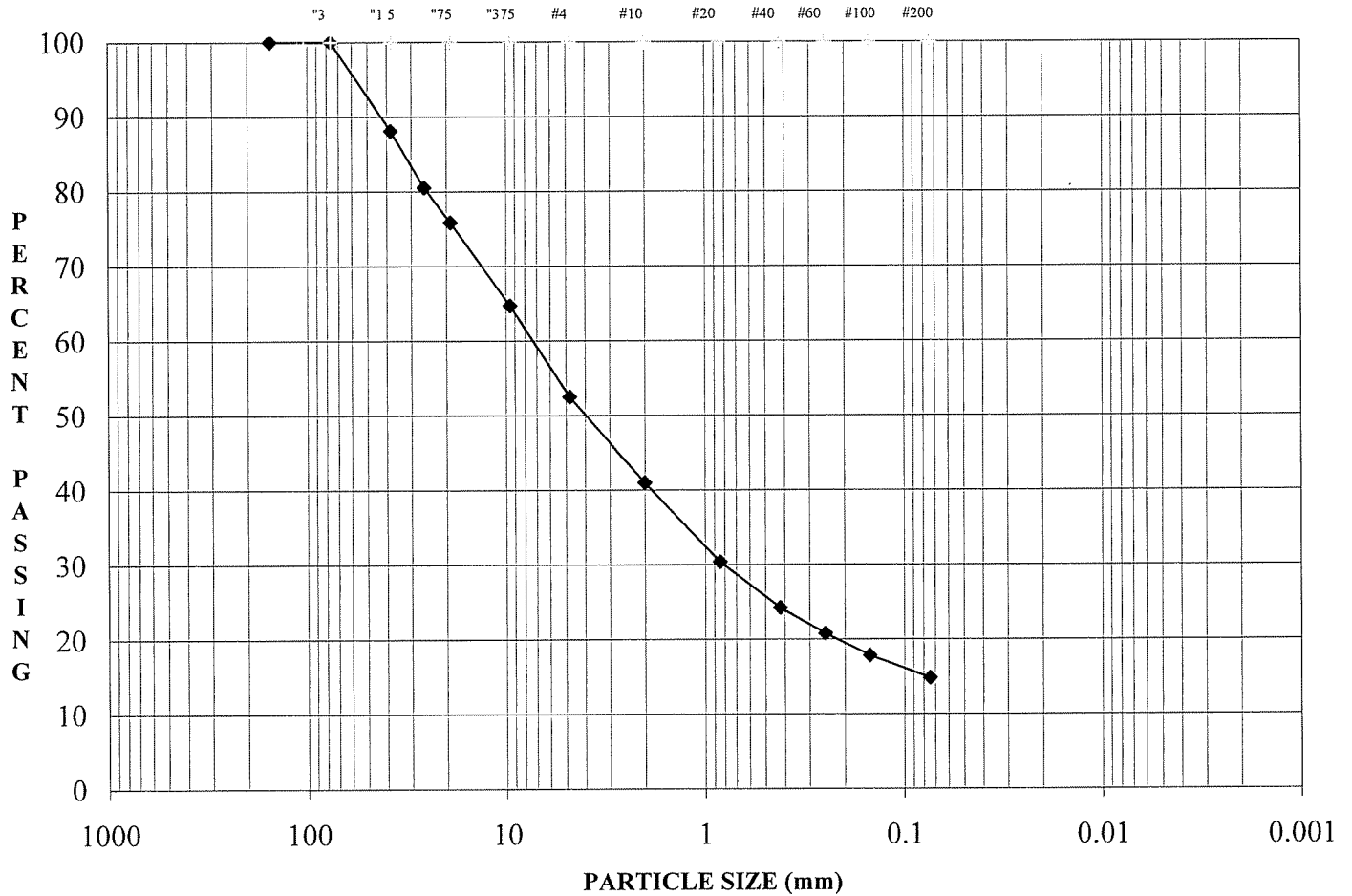
Gs: Gs

**G&K/TYRONE CLOSURE HEARING/AZ
013-1595.002**

22-Oct-01
MP
NG

**GOLDER ASSOCIATES INC.
LAKEWOOD, COLORADO**

**PARTICLE-SIZE DISTRIBUTION ASTM D 421 AND D 422
US STANDARD SIEVE OPENING SIZES**



COBBLES	Coarse	Fine	Cor	Med	Fine	Silt or Clay Size
	GRAVEL		SAND			FINES

SAMPLE #: TYTP01-4

DEPTH (ft):

DESCRIPTION: Clayey gravel with sand (GC)

MC (As tested): 3.3%

LL: 40

PL: 17

PI: 23

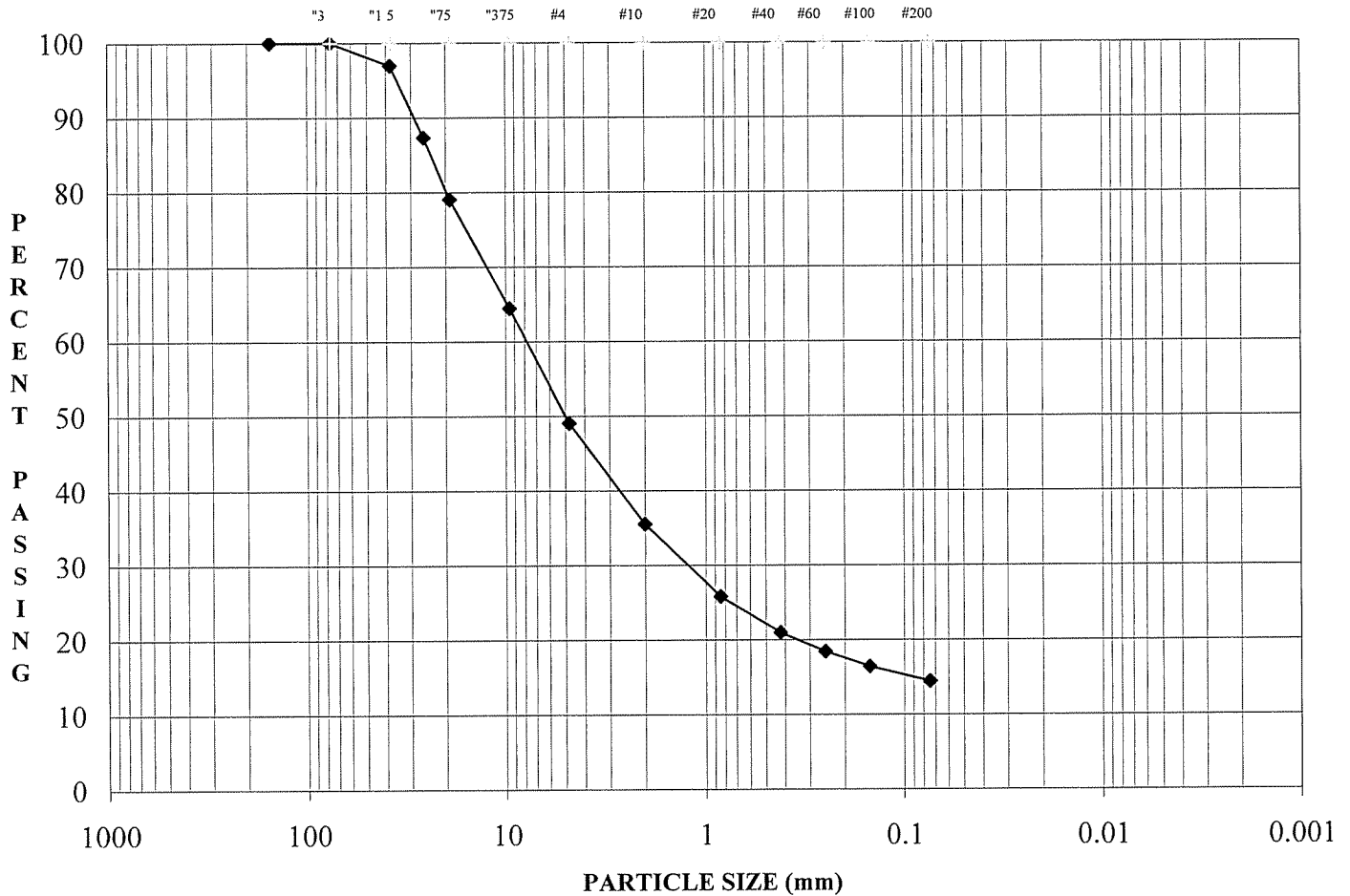
Gs: Gs

**G&K/TYRONE CLOSURE HEARING/AZ
013-1595.002**

18-Oct-01
TM
NG

**GOLDER ASSOCIATES INC.
LAKEWOOD, COLORADO**

**PARTICLE-SIZE DISTRIBUTION ASTM D 421 AND D 422
US STANDARD SIEVE OPENING SIZES**



COBBLES	Coarse	Fine	Cor	Med	Fine	Silt or Clay Size
	GRAVEL		SAND			FINES

SAMPLE #:	TYTP01-5
DEPTH (ft):	
DESCRIPTION:	Clayey gravel with sand (GC)

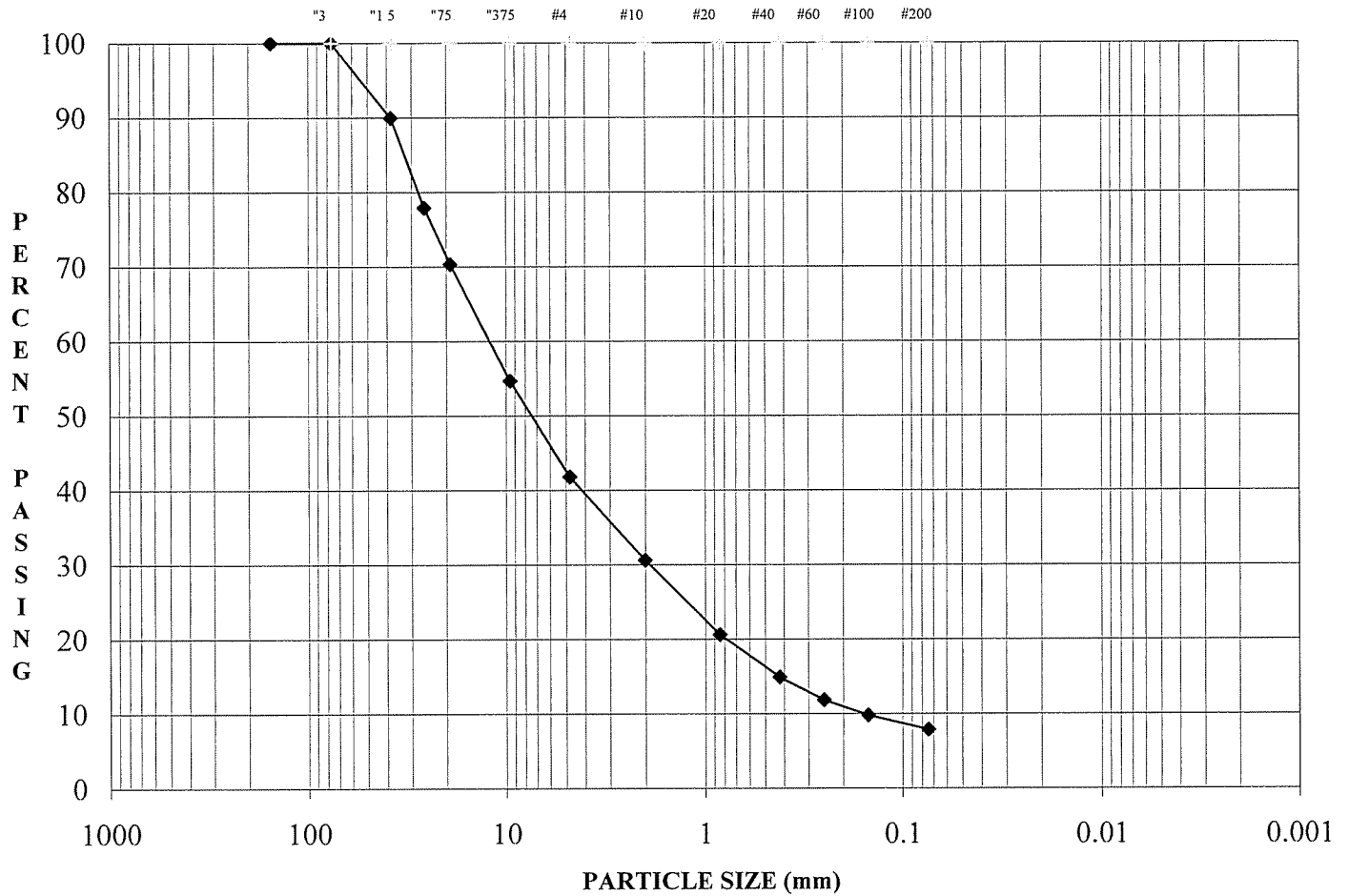
MC (As tested):	10.1%
LL:	39
PL:	16
PI:	23
Gs:	Gs

**G&K/TYRONE CLOSURE HEARING/AZ
013-1595.002**

19-Oct-01
MC
NG

**GOLDER ASSOCIATES INC.
LAKEWOOD, COLORADO**

**PARTICLE-SIZE DISTRIBUTION ASTM D 421 AND D 422
US STANDARD SIEVE OPENING SIZES**



COBBLES	Coarse	Fine	Cor	Med	Fine	Silt or Clay Size
	GRAVEL		SAND			FINES

SAMPLE #:	TYTP01-6
DEPTH (ft):	
DESCRIPTION:	Well-graded gravel with clay and sand (GW-GC)

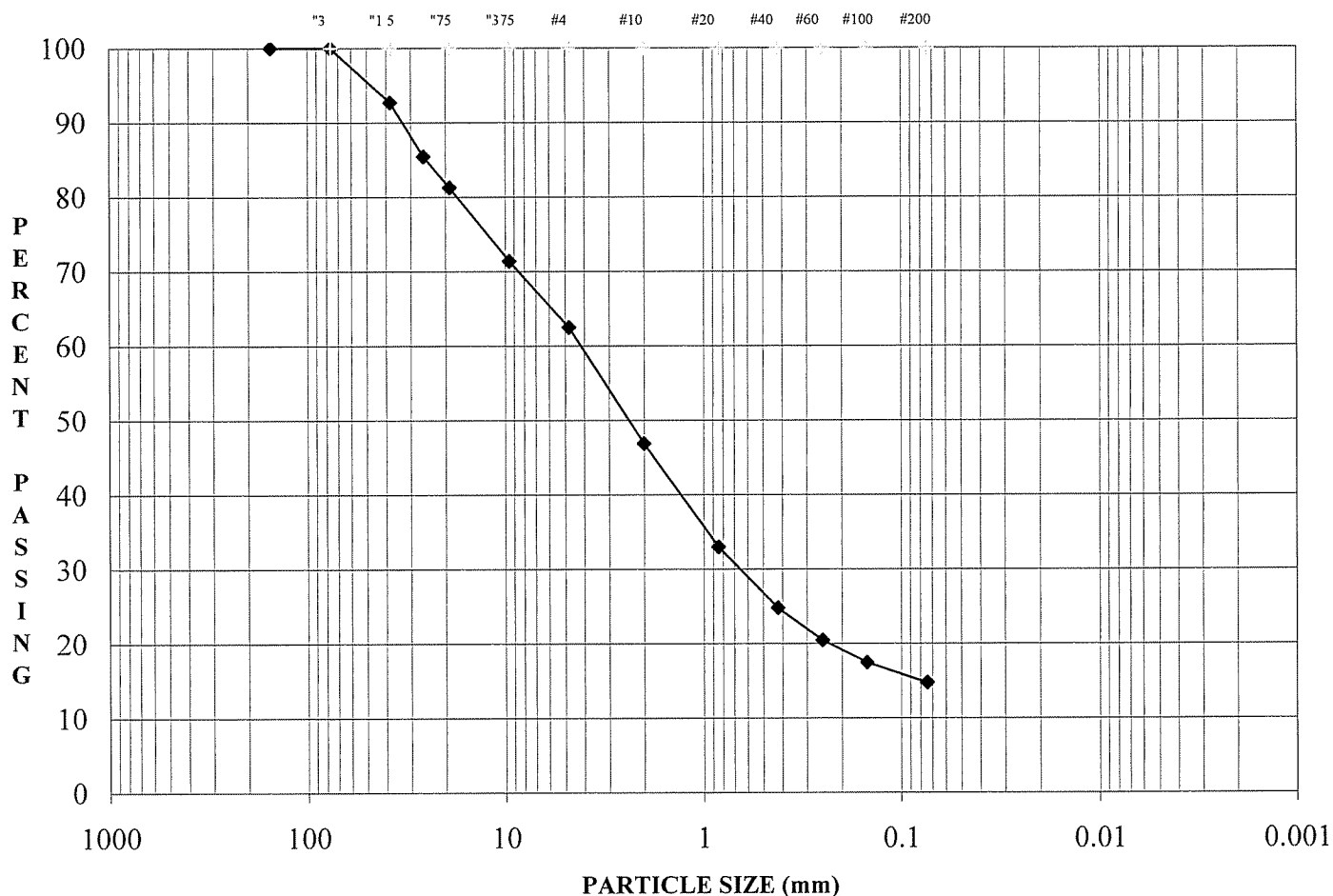
MC (As tested):	6.3%
LL:	29
PL:	16
PI:	13
Gs:	Gs

**G&K/TYRONE CLOSURE HEARING/AZ
013-1595.002**

19-Oct-01
MP
NG

**GOLDER ASSOCIATES INC.
LAKEWOOD, COLORADO**

**PARTICLE-SIZE DISTRIBUTION ASTM D 421 AND D 422
US STANDARD SIEVE OPENING SIZES**



COBBLES	Coarse	Fine	Cor	Med	Fine	Silt or Clay Size
	GRAVEL		SAND			FINES

SAMPLE #: TYTP01-7

DEPTH (ft):

DESCRIPTION: Clayey sand with gravel (SC)

MC (As tested): 7.5%

LL: 28

PL: 15

PI: 13

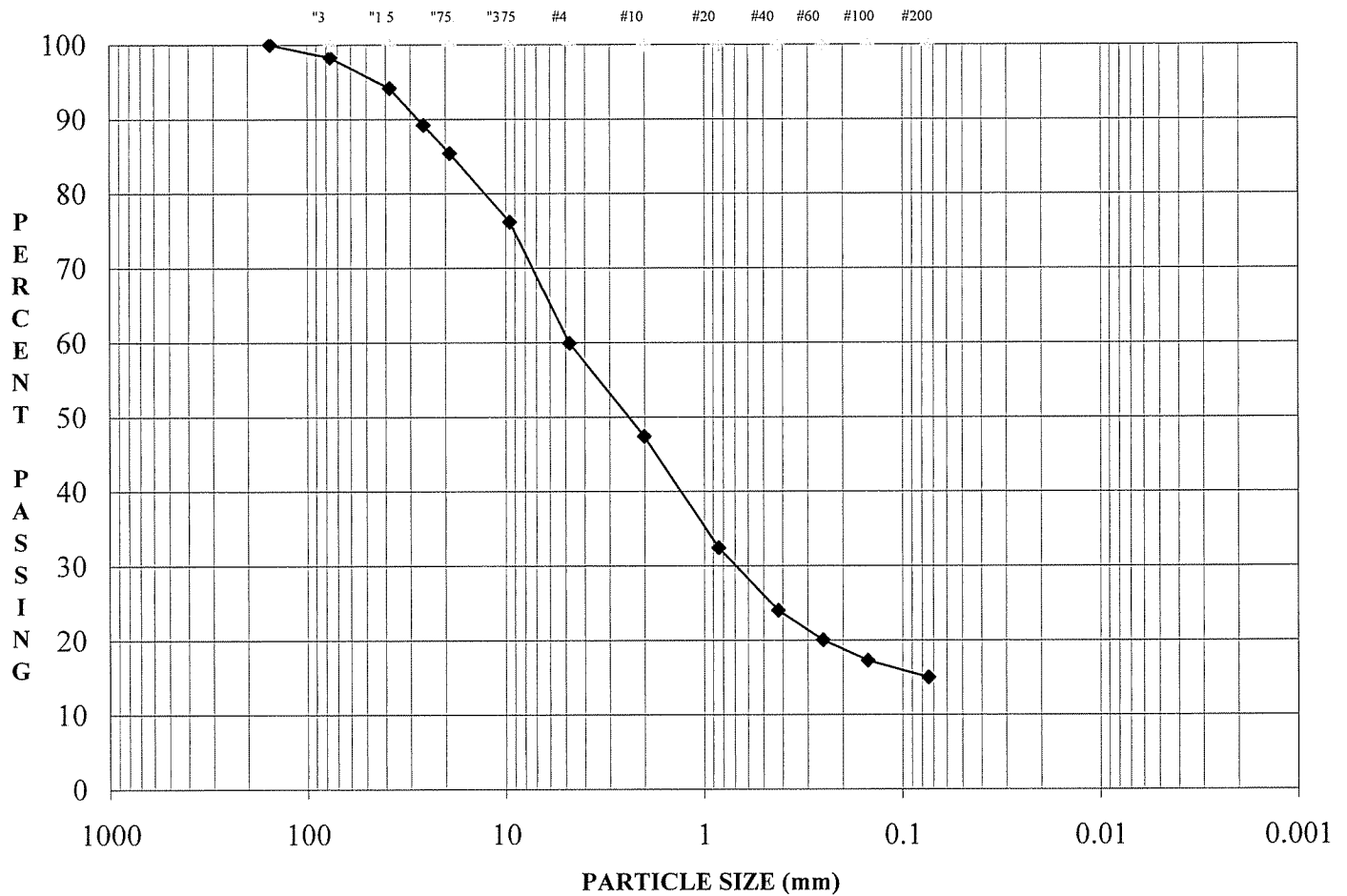
Gs: Gs

**G&K/TYRONE CLOSURE HEARING/AZ
013-1595.002**

19-Oct-01
MP
NG

**GOLDER ASSOCIATES INC.
LAKEWOOD, COLORADO**

**PARTICLE-SIZE DISTRIBUTION ASTM D 421 AND D 422
US STANDARD SIEVE OPENING SIZES**



COBBLES	Coarse	Fine	Cor	Med	Fine	Silt or Clay Size
	GRAVEL		SAND			FINES

SAMPLE #: TYTP01-8

DEPTH (ft):

DESCRIPTION: Clayey sand with gravel (SC)

MC (As tested): 5.4%

LL: 24

PL: 16

PI: 8

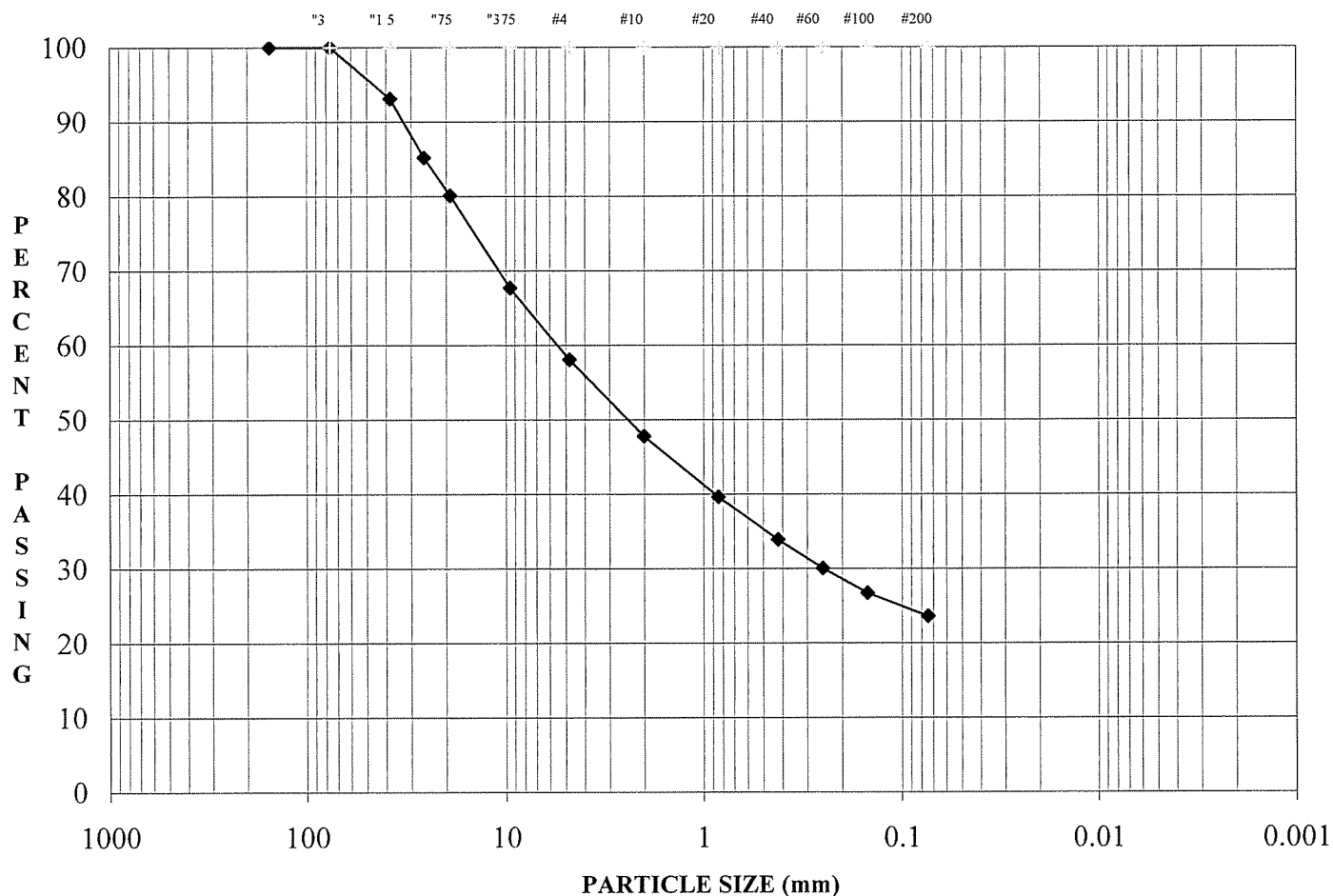
Gs: Gs

**G & K / Tyrone Closure hearing / AZ
013-1595**

24-Oct-01
MC
MB

**GOLDER ASSOCIATES INC.
LAKEWOOD, COLORADO**

**PARTICLE-SIZE DISTRIBUTION ASTM D 421 AND D 422
US STANDARD SIEVE OPENING SIZES**



COBBLES	Coarse	Fine	Cor	Med	Fine	Silt or Clay Size
	GRAVEL		SAND			FINES

SAMPLE #:	TYTP01-9
DEPTH (ft):	
DESCRIPTION:	Clayey gravel with sand (GC)

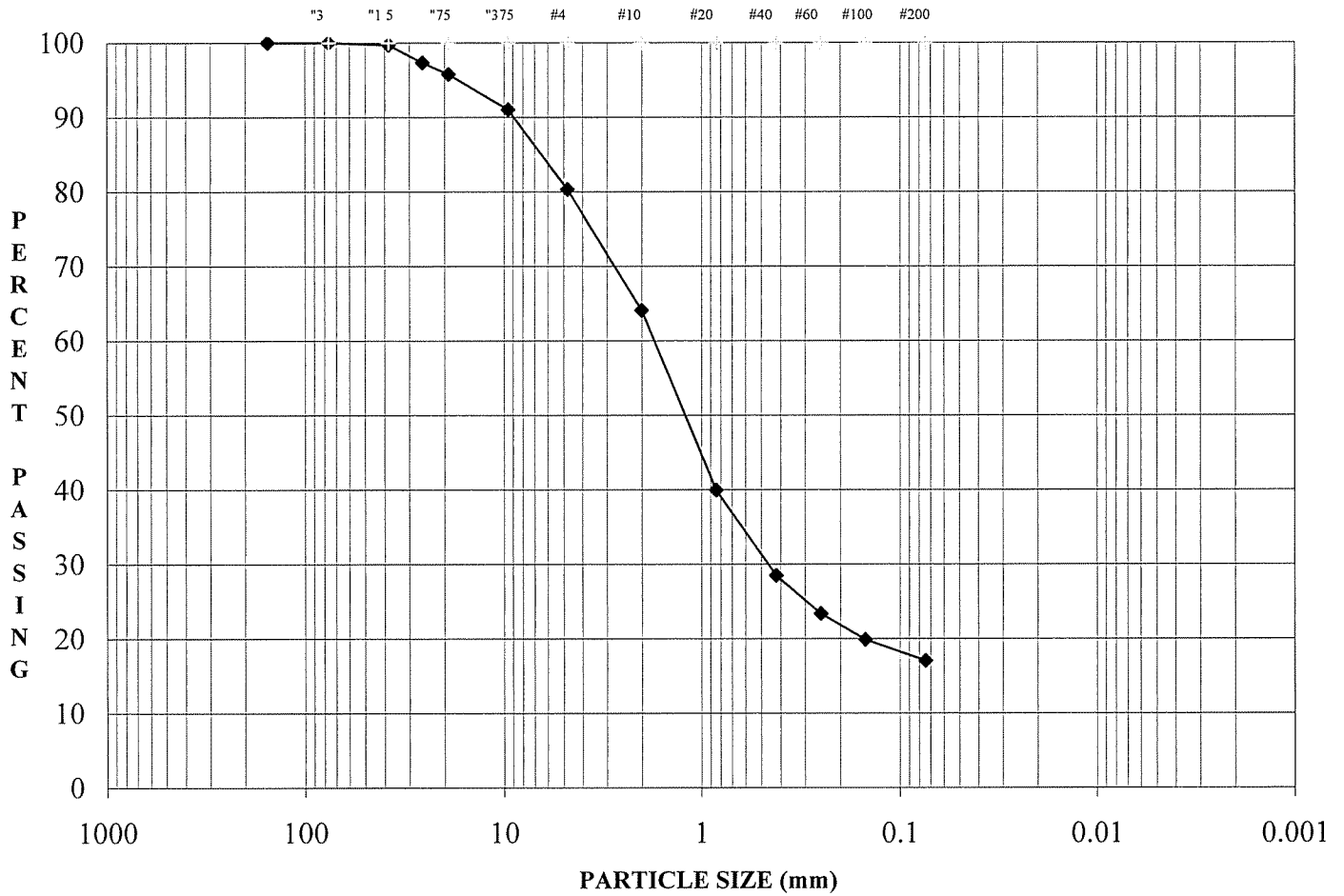
MC (As tested):	9.4%
LL:	30
PL:	18
PI:	12
Gs:	Gs

**G&K/TYRONE CLOSURE HEARING/AZ
013-1595.002**

23-Oct-01
MC
NG

**GOLDER ASSOCIATES INC.
LAKEWOOD, COLORADO**

**PARTICLE-SIZE DISTRIBUTION ASTM D 421 AND D 422
US STANDARD SIEVE OPENING SIZES**



COBBLES	Coarse	Fine	Cor	Med	Fine	Silt or Clay Size
	GRAVEL		SAND			FINES

SAMPLE #: TYTP01-10

DEPTH (ft):

DESCRIPTION: Clayey sand with gravel (SC)

MC (As tested): 3.7%

LL: 30

PL: 18

PI: 12

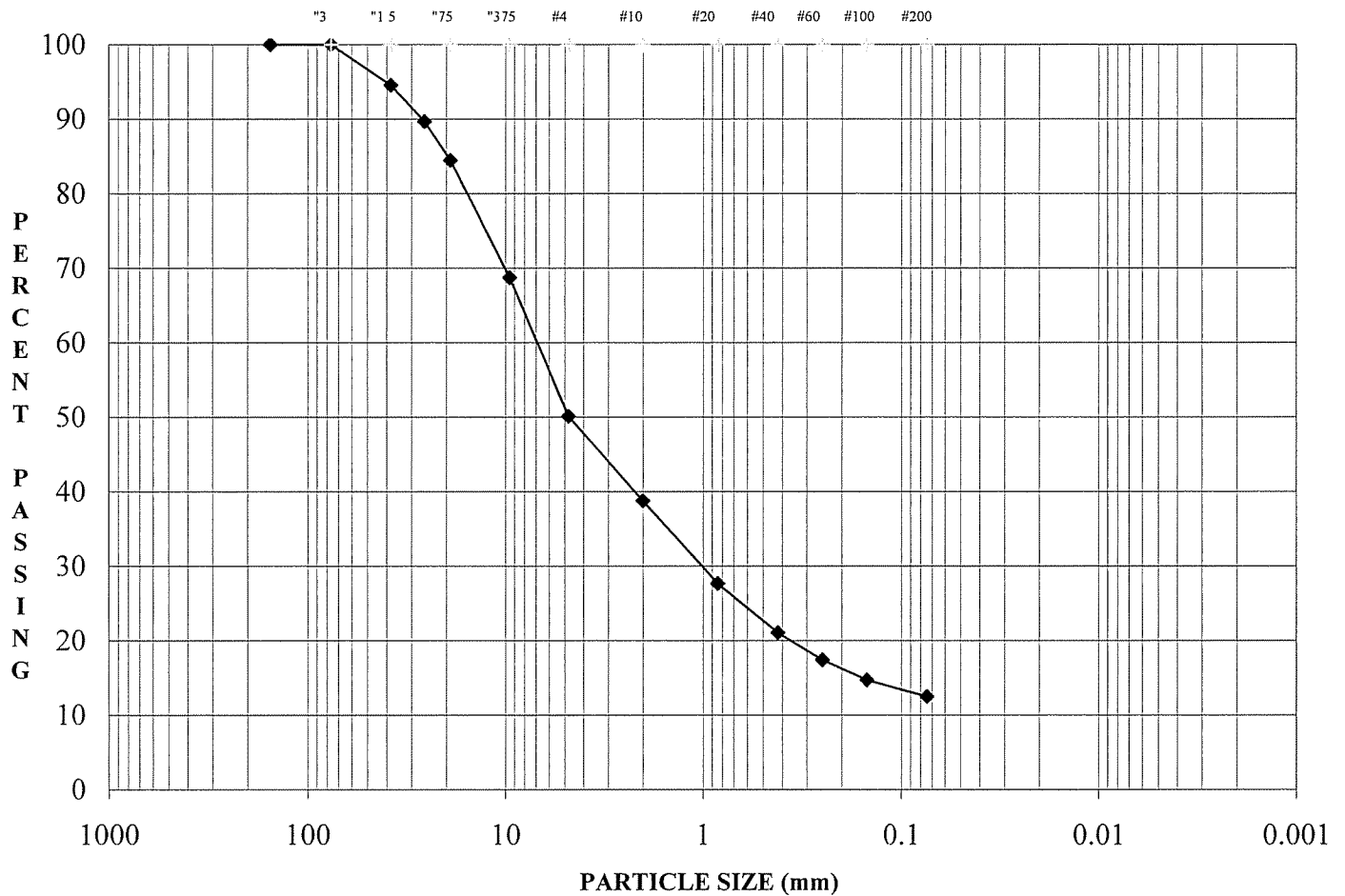
Gs: Gs

G&K/TYRONE CLOSURE HEARING/AZ
013-1595.002

22-Oct-01
MP
NG

GOLDER ASSOCIATES INC.
LAKEWOOD, COLORADO

**PARTICLE-SIZE DISTRIBUTION ASTM D 421 AND D 422
US STANDARD SIEVE OPENING SIZES**



COBBLES	Coarse	Fine	Cor	Med	Fine	Silt or Clay Size
	GRAVEL		SAND			FINES

SAMPLE #:	TYTP01-11
DEPTH (ft):	
DESCRIPTION:	Clayey gravel with sand (GC)

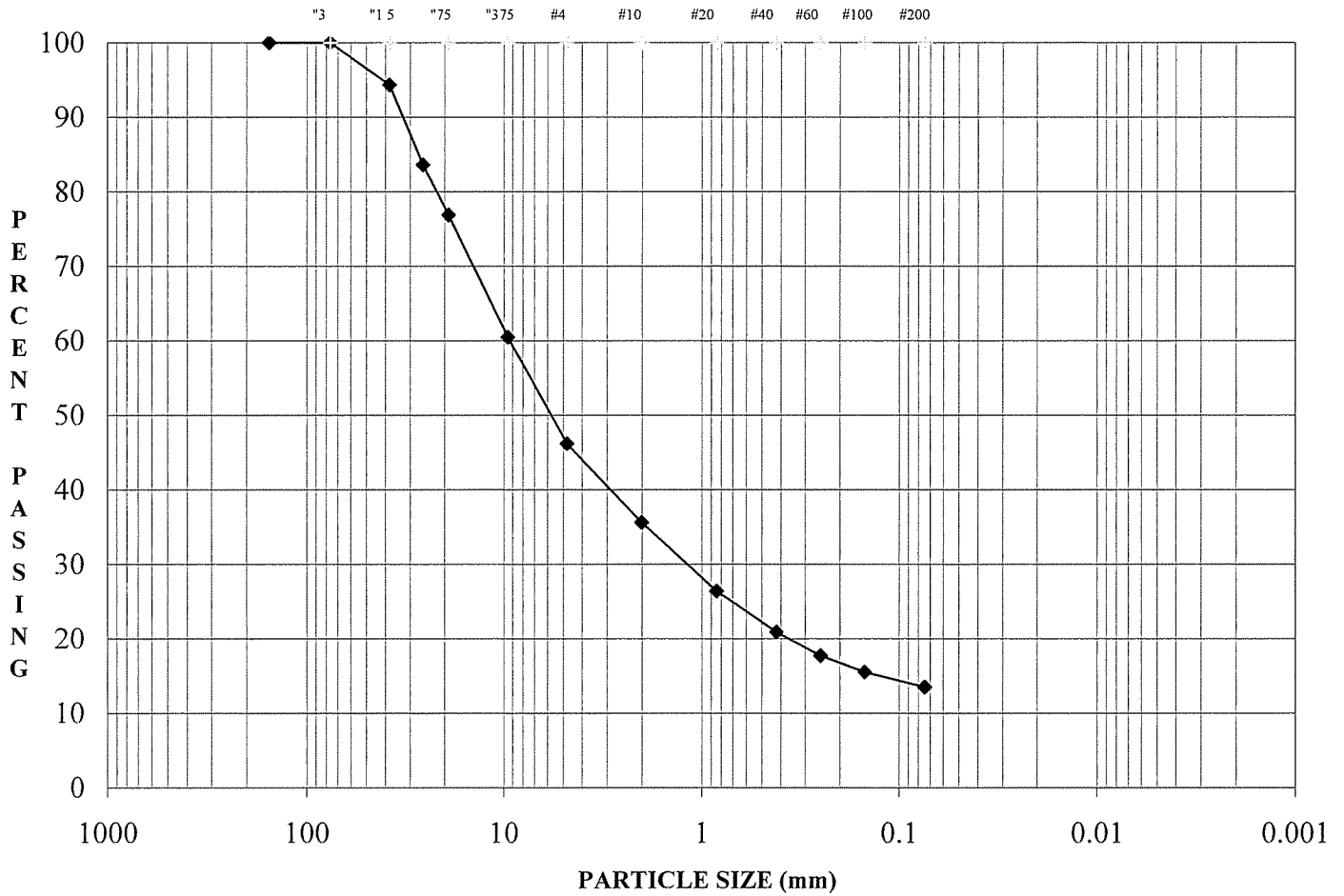
MC (As tested):	7.8%
LL:	37
PL:	17
PI:	20
Gs:	Gs

**G&K/TYRONE CLOSURE HEARING/AZ
013-1595.002**

22-Oct-01
MP
NG

**GOLDER ASSOCIATES INC.
LAKEWOOD, COLORADO**

**PARTICLE-SIZE DISTRIBUTION ASTM D 421 AND D 422
US STANDARD SIEVE OPENING SIZES**



COBBLES	Coarse	Fine	Cor	Med	Fine	Silt or Clay Size
	GRAVEL		SAND			FINES

SAMPLE #:	TYTP01-12
DEPTH (ft):	
DESCRIPTION:	Clayey gravel with sand (GC)

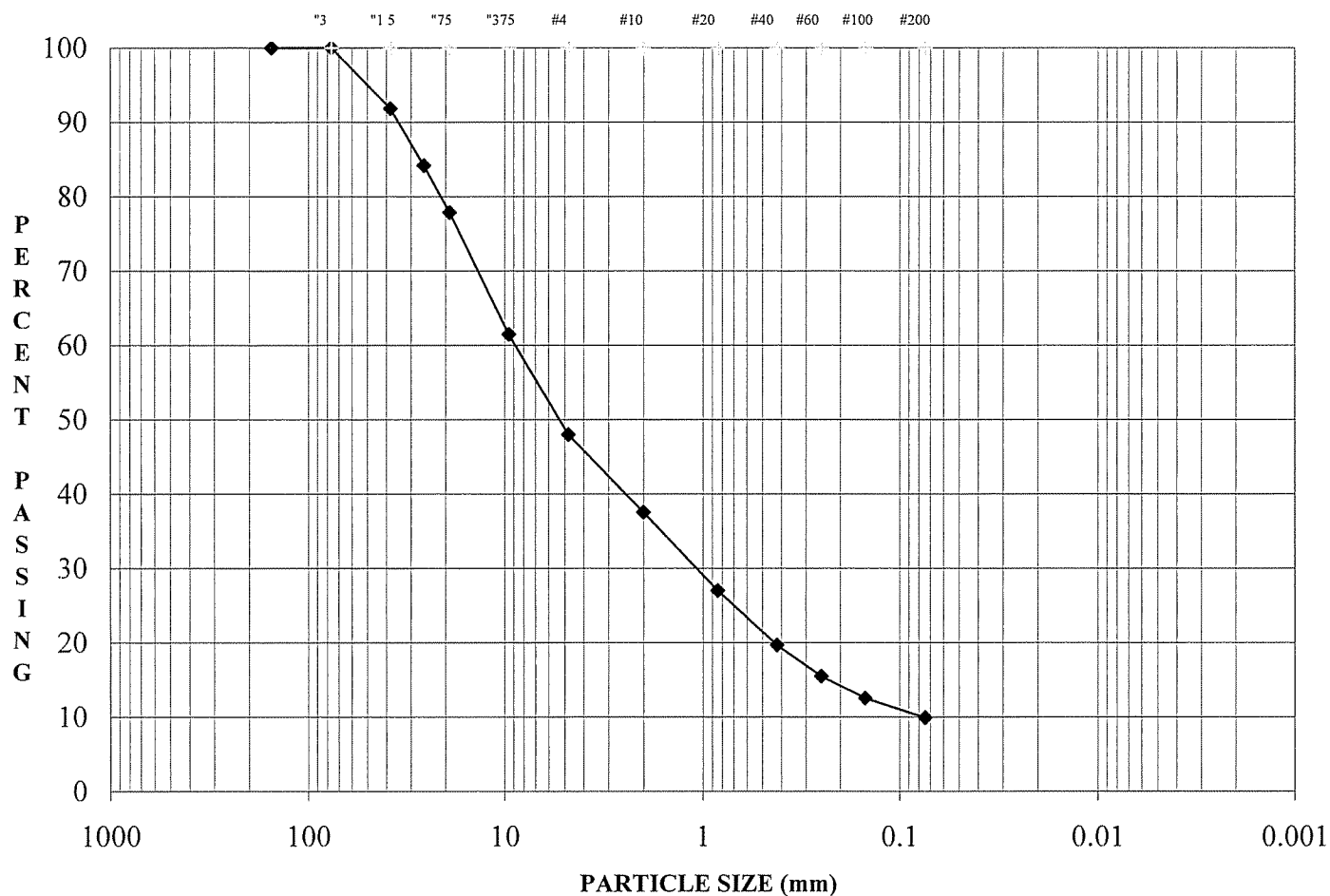
MC (As tested):	5.8%
LL:	30
PL:	16
PI:	14
Gs:	Gs

**G&K/TYRONE CLOSURE HEARING/AZ
013-1595.002**

22-Oct-01
MP
NG

**GOLDER ASSOCIATES INC.
LAKEWOOD, COLORADO**

**PARTICLE-SIZE DISTRIBUTION ASTM D 421 AND D 422
US STANDARD SIEVE OPENING SIZES**



COBBLES	Coarse	Fine	Cor	Med	Fine	Silt or Clay Size
	GRAVEL		SAND			FINES

SAMPLE #: TYTP01-2

DEPTH (ft):

DESCRIPTION: Well-graded gravel with clay and sand (GW-GC)

MC (As tested): 7.2%

LL: 30

PL: 14

PI: 16

Gs: Gs

G&K/TYRONE CLOSURE HEARING/AZ
013-1595.002

19-Oct-01
MC
NG

GOLDER ASSOCIATES INC.
LAKEWOOD, COLORADO

APPENDIX C-3

**2004 1C STOCKPILE SOIL SUMMARY
AND GRAIN-SIZE ANALYSIS**

TABLE C-3.1
SUMMARY OF SOIL DATA

Sample Type	Sample Number	Sample Depth (feet)	USCS Soil Classification	Delivered Moisture (%)	Atterberg Limits			Grain-size Distribution			Specific Gravity	Moisture/Density Relationship	
					LL	PL	PI	% Finer 3/4"	% Finer #4	% Finer #200		PCF (Dry)	Moist (%)
Bulk	GA04-TY-1-GT	--	GC	8.8	34	20	14	70	41	18	--	--	--
Bulk	GA04-TY-2-GT	--	GC	7.6	35	21	14	80	52	22	--	--	--
Bulk	GA04-TY-3-GT	--	GC	10.5	40	21	19	69	40	17	--	--	--
Bulk	GA04-TY-4-GT	--	GP-GC	1.9*	50	17	33	46	15	6	--	--	--
Bulk	GA04-TY-5-GT	--	GC	5.4	33	19	14	59	33	14	--	--	--
Bulk	GA04-TY-6-GT	--	GC	8.4	35	19	16	66	42	20	--	--	--
Bulk	GA04-TY-7-GT	--	GC	5.9	36	19	17	64	36	14	--	--	--
Bulk	GA04-TY-8-GT	--	GC	8.0	30	19	11	68	42	17	--	--	--

Notes:

* Delivered moisture content taken from Bulk sample due to no bag sample present

LL = Liquid Limit

PL = Plastic Limit

PI = Plastic Index

USCS = Unified Soil Classification System

PCF = pounds per cubic foot

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

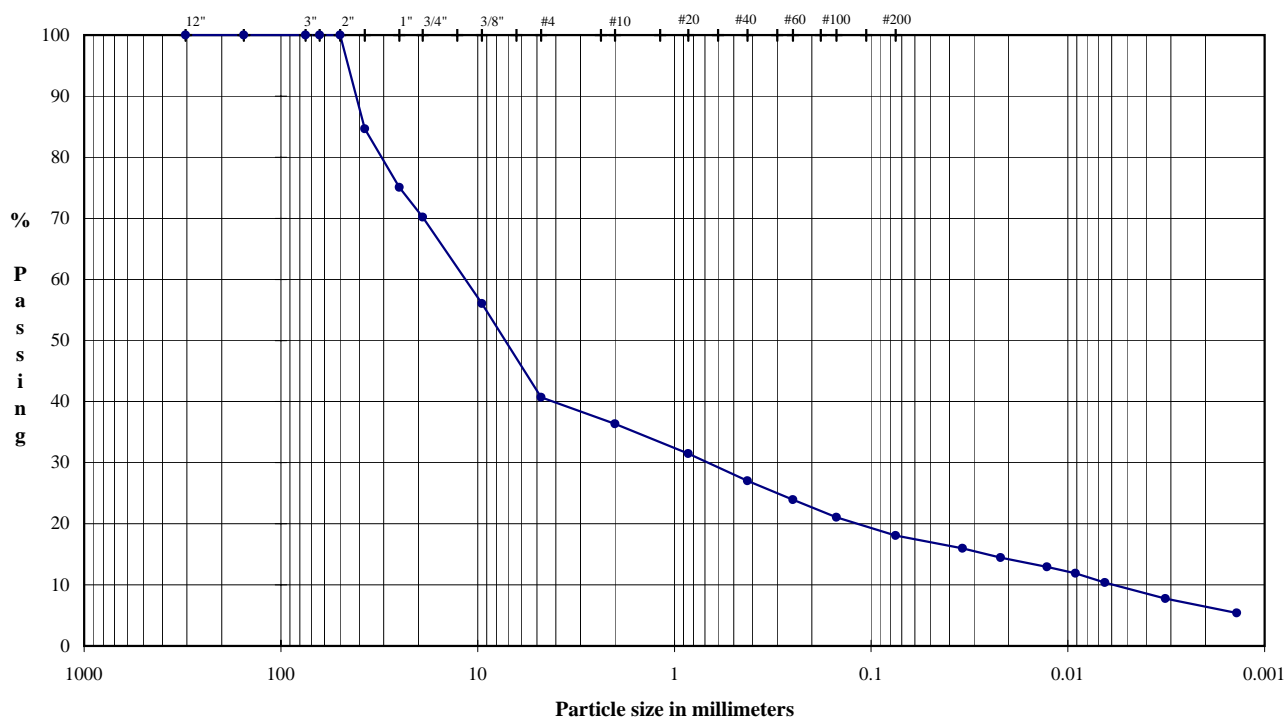
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: GA04-TY-1-GT

Depth: --

TYPE: BULK



COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

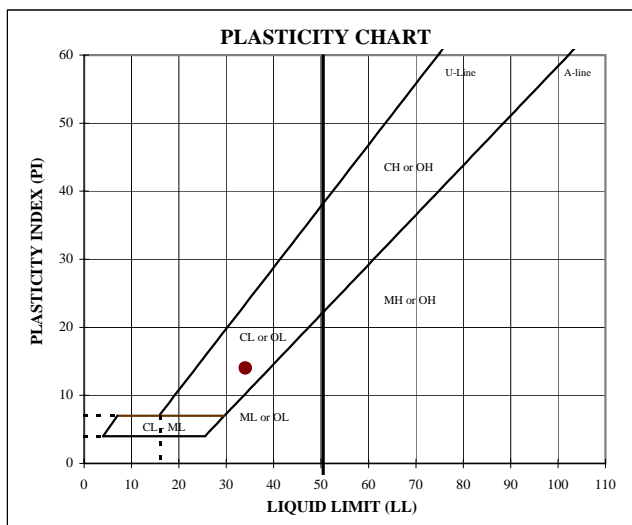
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
3.0"	75.0	100.0		
2.5"	63.5	100.0		
2.0"	50.0	100.0		
1.5"	37.5	84.7	Coarse Gravel	29.82
1.0"	25.0	75.0		
0.75"	19.0	70.2		
0.375"	9.5	56.0	Fine Gravel	29.48
#4	4.8	40.7		
#10	2.00	36.3		
#20	0.85	31.5	Coarse Sand	4.36
#40	0.43	27.1		
#60	0.25	23.9	Medium Sand	9.30
#100	0.15	21.0		
#200	0.075	18.1		
			Fine Sand	8.96

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
8.8	34	20	14	2.70

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	18.09
0.034	16.0		
0.022	14.5		
0.013	13.0		
0.0091	11.9		
0.0065	10.4		
0.0032	7.8		
0.0014	5.4		

DESCRIPTION: Pale reddish brown clayey gravel with sand

USCS: GC

TECH JR
DATE 10/4/2004
REVIEW MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

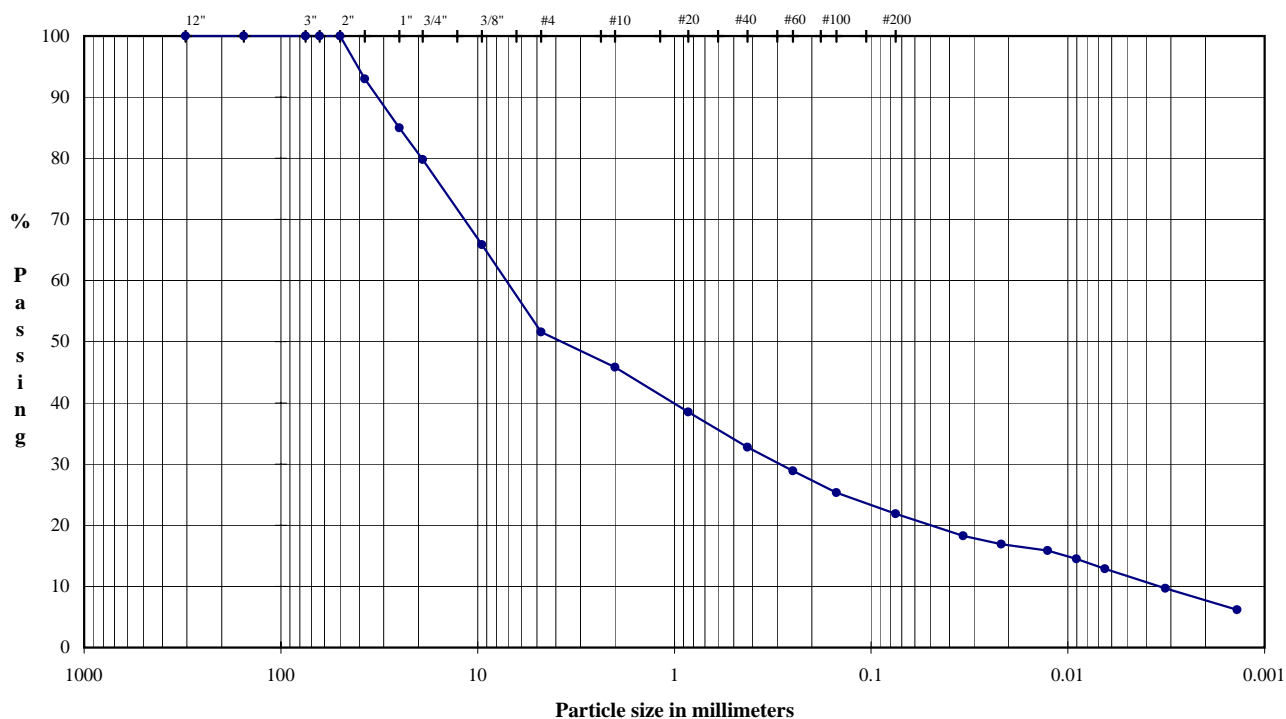
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: GA04-TY-2-GT

Depth: --

TYPE: BULK



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

% Passing

Classification

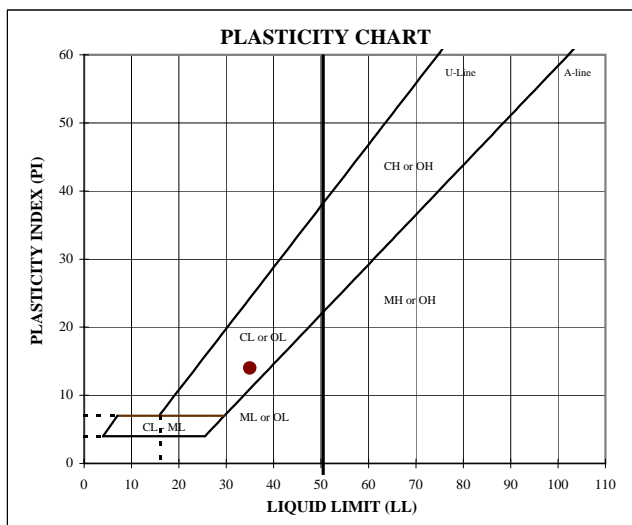
Percentage

U.S. Standard Sieves Sizes and Numbers

12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
3.0"	75.0	100.0		
2.5"	63.5	100.0		
2.0"	50.0	100.0		
1.5"	37.5	93.0	Coarse Gravel	20.18
1.0"	25.0	85.0		
0.75"	19.0	79.8		
0.375"	9.5	65.9	Fine Gravel	28.23
#4	4.8	51.6		
#10	2.00	45.8		
#20	0.85	38.5	Coarse Sand	5.77
#40	0.43	32.8	Medium Sand	13.05
#60	0.25	28.9		
#100	0.15	25.3	Fine Sand	10.91
#200	0.075	21.9		

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	21.86
0.034	18.2		
0.022	16.9		
0.013	15.8		
0.0090	14.5		
0.0065	12.9		
0.0032	9.7		
0.0014	6.2		



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
7.6	35	21	14	2.70

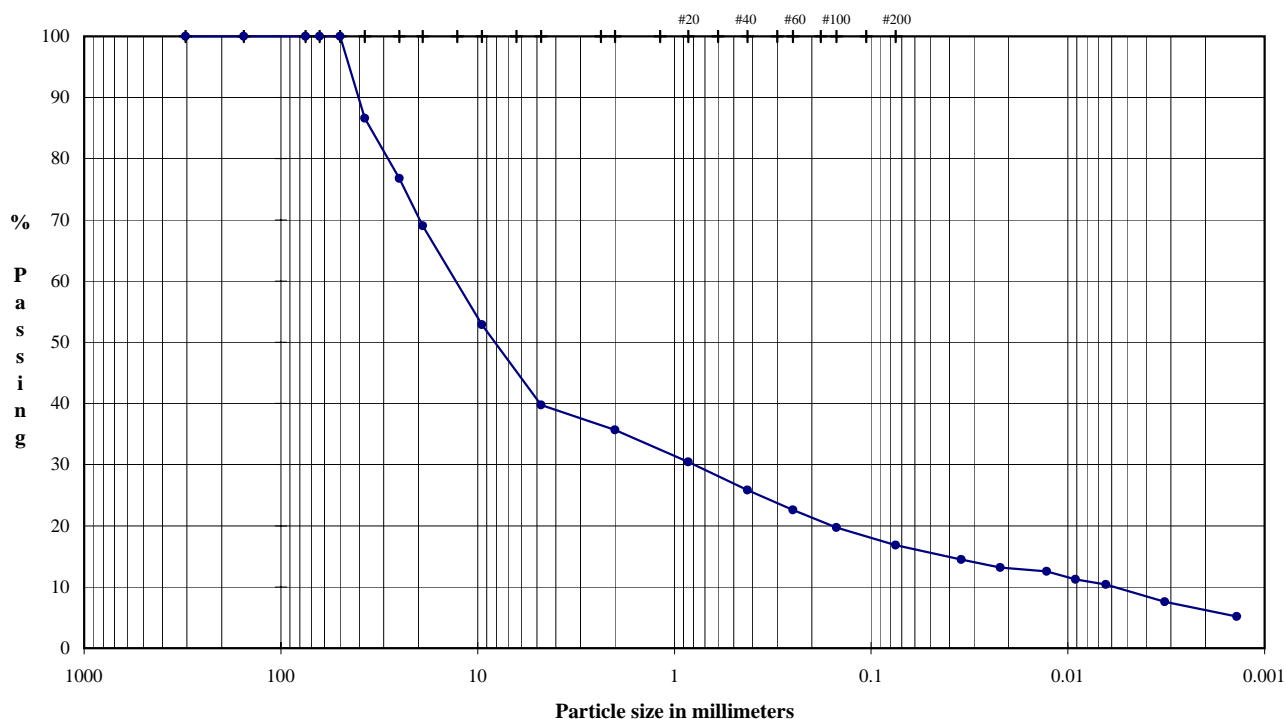
DESCRIPTION: Pale reddish brown clayey gravel with sand

USCS: GC

TECH	JR
DATE	10/4/2004
REVIEW	MB

ASTM D421, D422, D4318

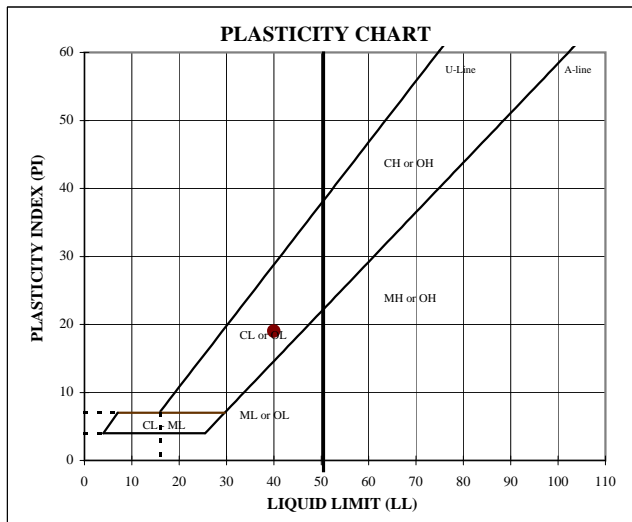
Depth: --



COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			FINES

Particle Size			Particle Size	
	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
3.0"	75.0	100.0		
2.5"	63.5	100.0	Coarse Gravel	30.96
2.0"	50.0	100.0		
1.5"	37.5	86.6		
1.0"	25.0	76.8		
0.75"	19.0	69.0		
0.375"	9.5	52.9		
#4	4.8	39.7	Fine Gravel	29.30
#10	2.00	35.7	Coarse Sand	4.08
#20	0.85	30.4	Medium Sand	9.81
#40	0.43	25.8		
#60	0.25	22.6		
#100	0.15	19.7		
#200	0.075	16.8	Fine Sand	9.02

Hydrometer Analysis	(mm)	% Finer	Fines Silt or Clay	16.83
	0.035	14.5		
	0.022	13.2		
	0.013	12.6		
	0.0091	11.3		
	0.0064	10.4		
	0.0032	7.6		
	0.0014	5.2		



M _c	LL	PL	PI	SpG (assumed)
10.5	40	21	19	2.70

USCS:	GC
-------	----

TECH	JR
DATE	10/4/2004
REVIEW	MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

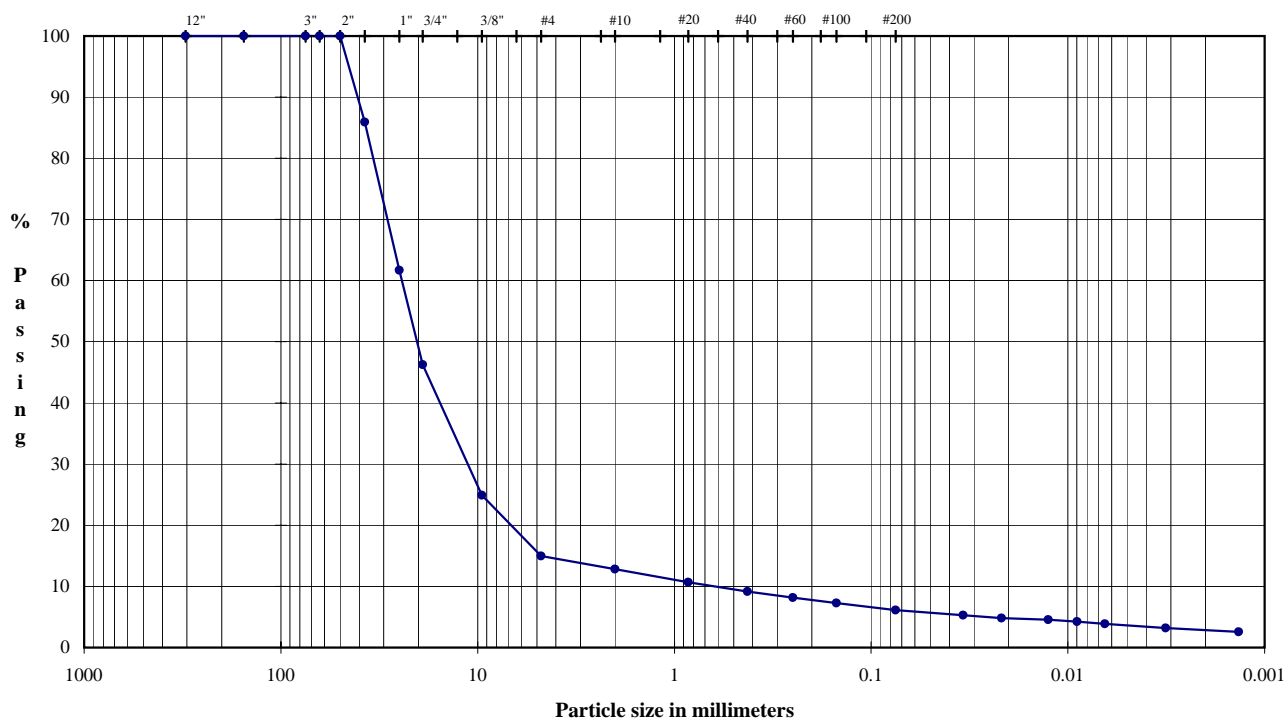
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: GA04-TY-4-GT

Depth: --

TYPE: BULK



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

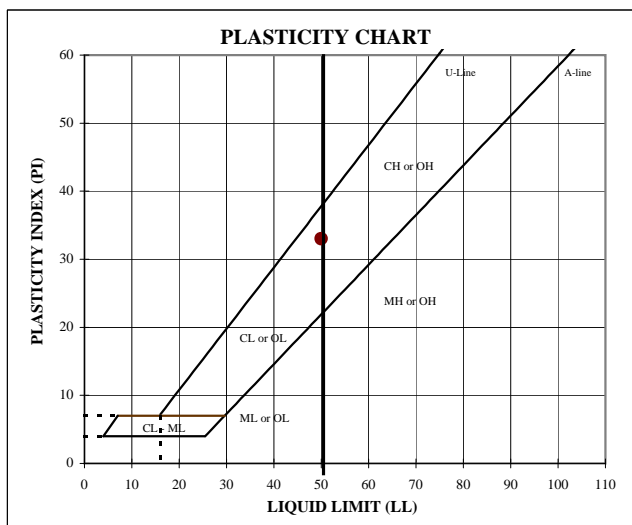
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
3.0"	75.0	100.0		
2.5"	63.5	100.0		
2.0"	50.0	100.0		
1.5"	37.5	85.9	Coarse Gravel	53.75
1.0"	25.0	61.7		
0.75"	19.0	46.2		
0.375"	9.5	24.9	Fine Gravel	31.30
#4	4.8	14.9		
#10	2.00	12.8		
#20	0.85	10.7	Coarse Sand	2.13
#40	0.43	9.2		
#60	0.25	8.2	Medium Sand	3.65
#100	0.15	7.3		
#200	0.075	6.1		
			Fine Sand	3.03

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
1.9	50	17	33	2.70

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	6.14
0.034	5.3		
0.022	4.8		
0.013	4.5		
0.0090	4.2		
0.0065	3.9		
0.0032	3.2		
0.0014	2.6		

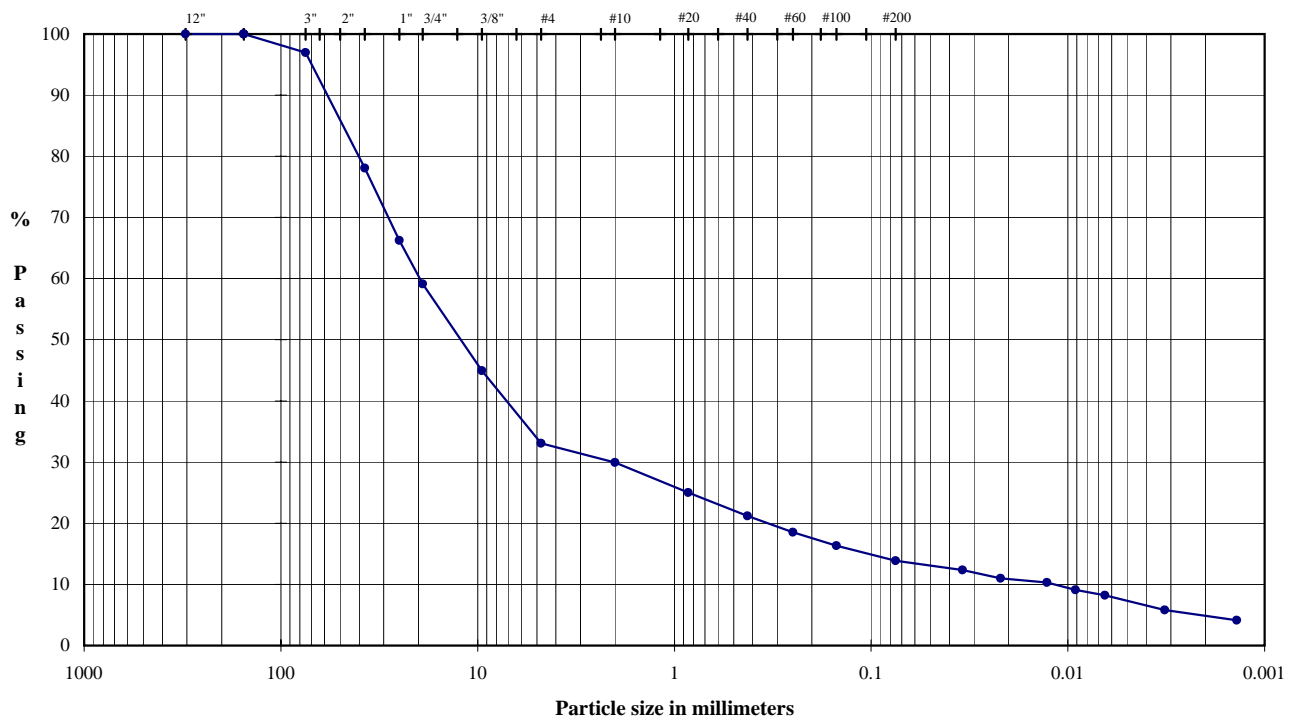
DESCRIPTION: Grayish orange poorly graded gravel with clay and sand

USCS: GP-GC

TECH JR
DATE 10/4/2004
REVIEW MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

ASTM D421, D422, D4318

PROJECT NAME: **G&K/Tyrone Stockpile Geotech/AZ**SAMPLE ID: **GA04-TY-5-GT**Depth: **--**TYPE: **BULK**

	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

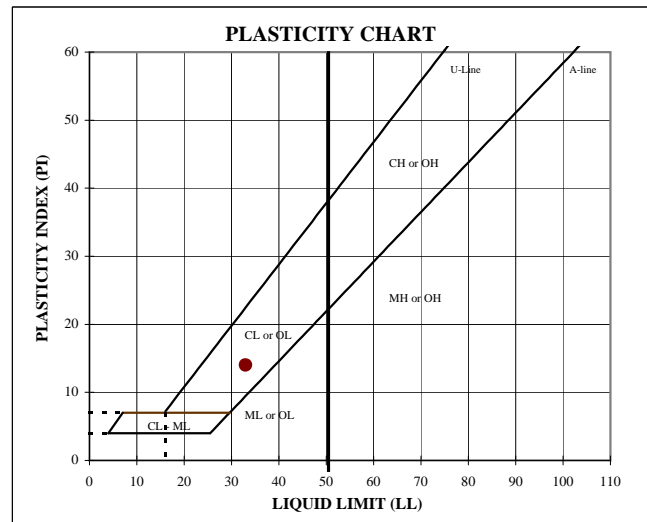
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	3.05
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	97.0	Coarse Gravel	37.83
1.5"	37.5	78.1		
1.0"	25.0	66.3		
0.75"	19.0	59.1		
0.375"	9.5	44.9	Fine Gravel	26.07
#4	4.8	33.1		
#10	2.0	29.9		
#20	0.85	25.0		
#40	0.43	21.2	Medium Sand	8.74
#60	0.25	18.5		
#100	0.15	16.3		
#200	0.075	13.9		
			Fine Sand	7.29

U.S. Standard Sieves Sizes and Numbers

**ATTERBERG LIMITS**

M _c	LL	PL	PI	SpG (assumed)
5.4	33	19	14	2.70

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	13.89
0.034	12.3		
0.022	11.0		
0.013	10.3		
0.0091	9.1		
0.0065	8.2		
0.0032	5.8		
0.0014	4.1		

DESCRIPTION: Light brown clayey gravel with sand

USCS: GC

TECH: JR
DATE: 10/4/2004
REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

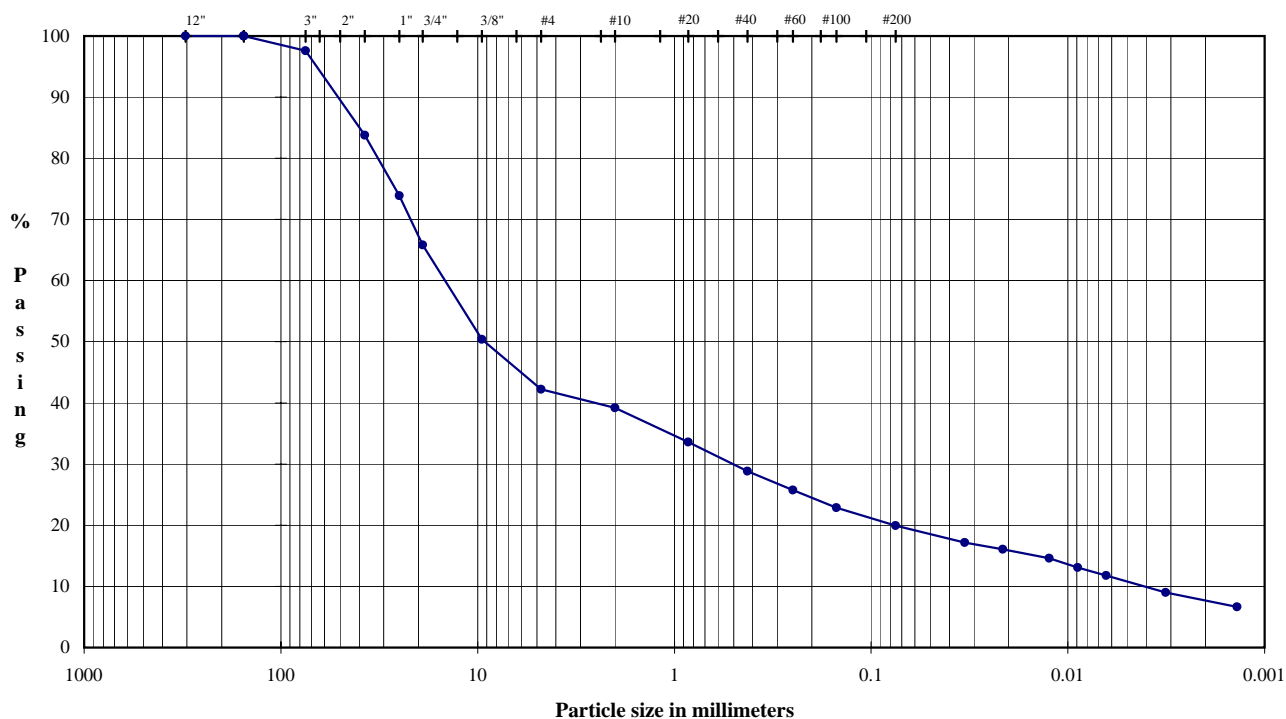
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: GA04-TY-6-GT

Depth: --

TYPE: BULK



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

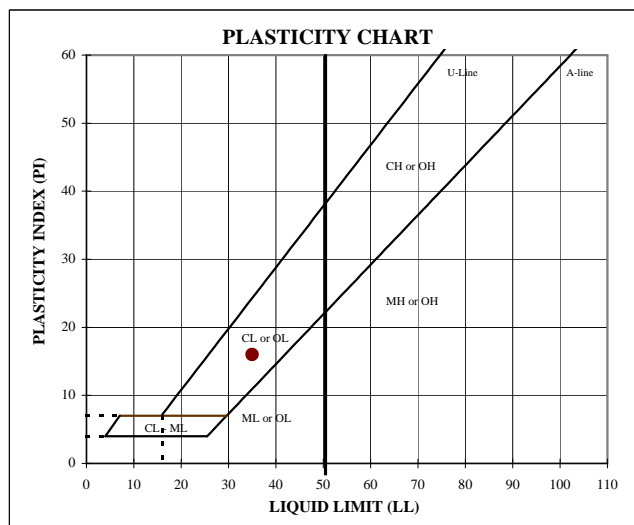
Particle Size

U.S. Standard Sieves Sizes and Numbers

	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	2.40
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	97.6	Coarse Gravel	31.80
1.5"	37.5	83.8		
1.0"	25.0	73.9		
0.75"	19.0	65.8		
0.375"	9.5	50.4	Fine Gravel	23.60
#4	4.8	42.2		
#10	2.00	39.2		
#20	0.85	33.6		
#40	0.43	28.8	Medium Sand	10.36
#60	0.25	25.8		
#100	0.15	22.9		
#200	0.075	19.9		
			Fine Sand	8.91

Hydrometer Analysis

(mm)	% Finer		
0.033	17.2	Fines Silt or Clay	19.94
0.021	16.1		
0.012	14.6		
0.0089	13.1		
0.0064	11.8		
0.0032	9.0		
0.0014	6.6		



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
8.4	35	19	16	2.70

DESCRIPTION: Moderate yellowish brown clayey gravel with sand

USCS: GC

TECH JR

DATE 10/4/2004

REVIEW MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

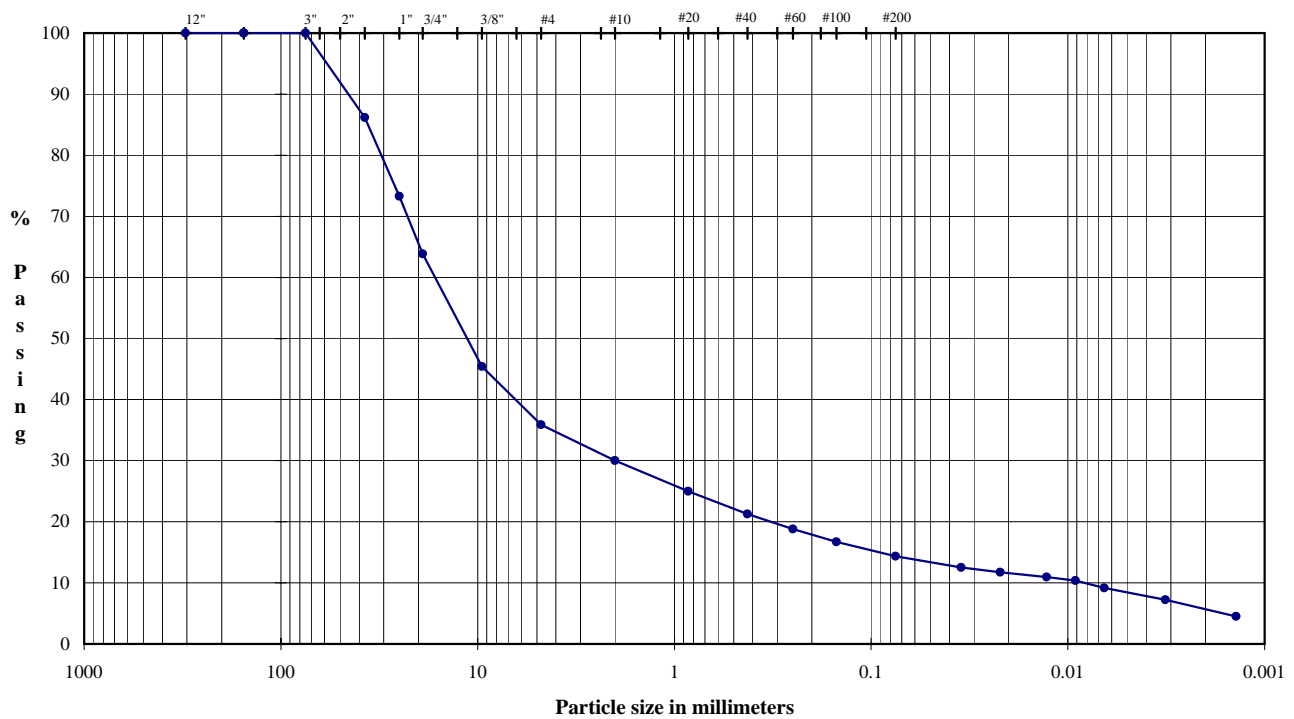
ASTM D421, D422, D4318

PROJECT NAME: **G&K/Tyrone Stockpile Geotech/AZ**

SAMPLE ID: **GA04-TY-7-GT**

Depth: **--**

TYPE: **BULK**



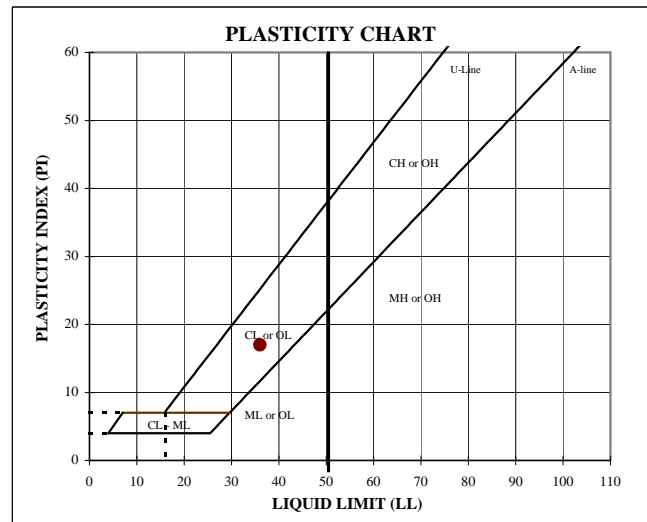
	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	36.16
1.5"	37.5	86.2		
1.0"	25.0	73.3		
0.75"	19.0	63.8		
0.375"	9.5	45.4	Fine Gravel	27.96
#4	4.8	35.9		
#10	2.00	30.0		
#20	0.85	25.0		
#40	0.43	21.3	Medium Sand	8.73
#60	0.25	18.8		
#100	0.15	16.7		
#200	0.075	14.3		
			Fine Sand	6.95

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
5.9	36	19	17	2.70

Hydrometer Analysis

(mm)	% Finer		
0.035	12.5	Fines Silt or Clay	14.33
0.022	11.7		
0.013	10.9		
0.0091	10.4		
0.0065	9.2		
0.0032	7.2		
0.0014	4.5		

DESCRIPTION: Dark yellowish orange clayey gravel with sand

USCS: GC

TECH: JR
DATE: 10/4/2004
REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

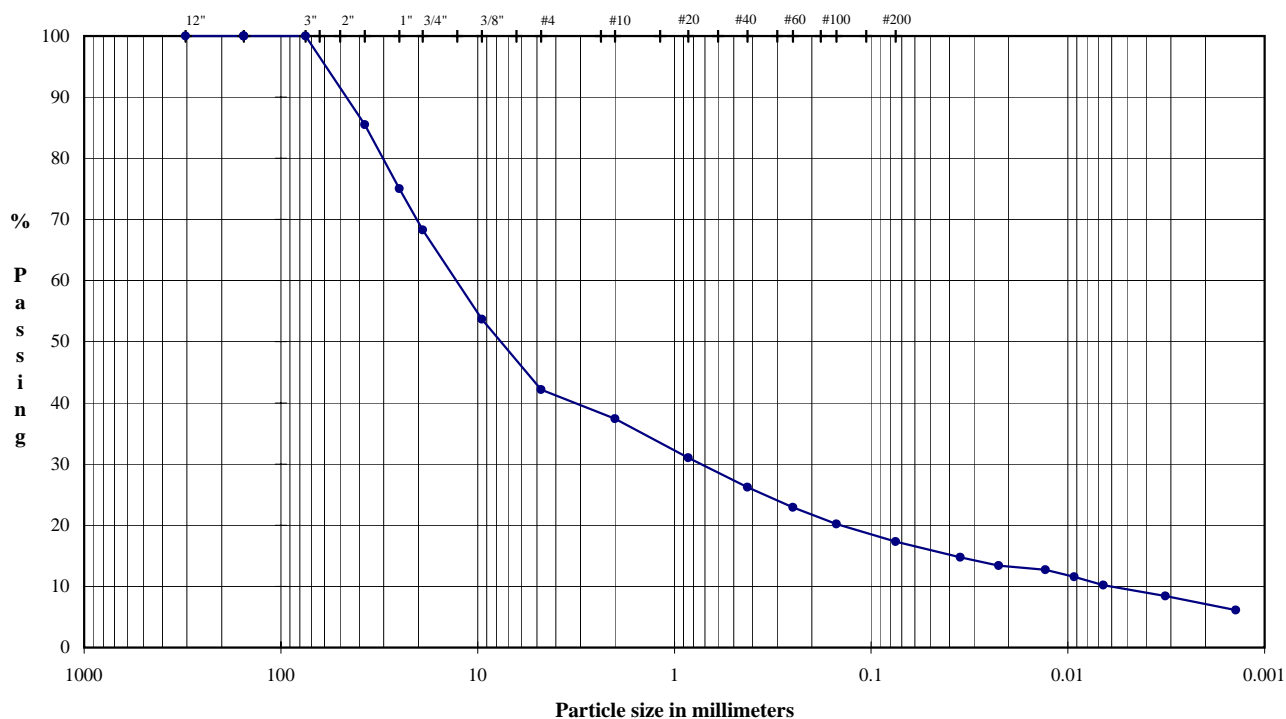
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: GA04-TY-8-GT

Depth: --

TYPE: BULK



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

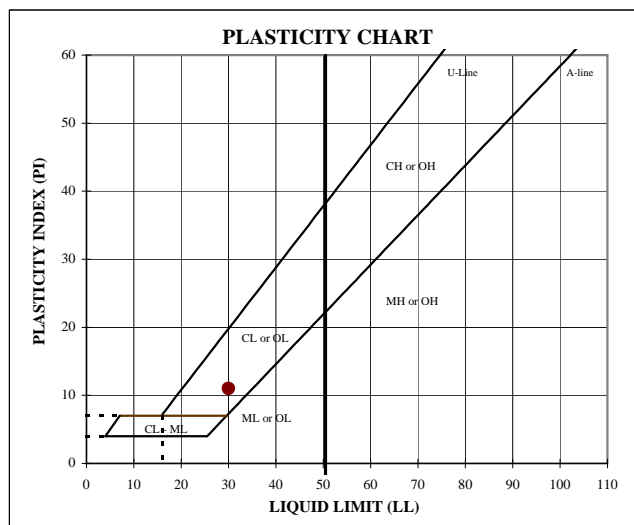
Particle Size

U.S. Standard Sieves Sizes and Numbers

	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	31.70
1.5"	37.5	85.5		
1.0"	25.0	75.0		
0.75"	19.0	68.3		
0.375"	9.5	53.7	Fine Gravel	26.15
#4	4.8	42.2		
#10	2.00	37.4		
#20	0.85	31.0		
#40	0.43	26.2	Medium Sand	11.22
#60	0.25	22.9		
#100	0.15	20.2		
#200	0.075	17.3		
			Fine Sand	8.85

Hydrometer Analysis

(mm)	% Finer		
0.035	14.8	Fines Silt or Clay	17.34
0.022	13.4		
0.013	12.7		
0.0093	11.6		
0.0066	10.2		
0.0032	8.4		
0.0014	6.1		



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
8.0	30	19	11	2.70

DESCRIPTION: Moderate yellowish brown clayey gravel with sand

USCS: GC

TECH: MKS
DATE: 10/4/2004
REVIEW: MB

APPENDIX C-4

**2004 SONIC DRILLING SOIL SUMMARY
AND GRAIN-SIZE ANALYSIS**

TABLE C-4.1
SUMMARY OF SOIL DATA

Sample Type	Sample Number	Sample Depth (feet)	USCS Soil Classification	Delivered Moisture (%)	Atterberg Limits			Grain-size Distribution			Specific Gravity	Moisture/Density Relationship Standard Proctor	
					LL	PL	PI	% Finer 3/4"	% Finer #4	% Finer #200		PCF (Dry)	Moist (%)
Bulk	GA04-TY-1-GT	--	GC	8.8	34	20	14	70	41	18	--	--	--
Bulk	GA04-TY-2-GT	--	GC	7.6	35	21	14	80	52	22	--	--	--
Bulk	GA04-TY-3-GT	--	GC	10.5	40	21	19	69	40	17	--	--	--
Bulk	GA04-TY-4-GT	--	GP-GC	1.9*	50	17	33	46	15	6	--	--	--
Bulk	GA04-TY-5-GT	--	GC	5.4	33	19	14	59	33	14	--	--	--
Bulk	GA04-TY-6-GT	--	GC	8.4	35	19	16	66	42	20	--	--	--
Bulk	GA04-TY-7-GT	--	GC	5.9	36	19	17	64	36	14	--	--	--
Bulk	GA04-TY-8-GT	--	GC	8.0	30	19	11	68	42	17	--	--	--
Pail	TSGT-1	19-21	SC	--	25	14	11	86	65	23	2.91	--	--
Pail	TSGT-1	47-48.5	GC	--	29	16	13	61	41	15	--	--	--
Pail	TSGT-1	75-77	SC-SM	--	26	19	7	81	60	17	--	--	--
Pail	TSGT-1	88-90	GC	--	28	18	10	84	58	23	--	--	--
Pail	TSGT-1	102-104	GC	--	33	20	13	80	55	21	--	--	--
Pail	TSGT-1	140-142	GC	--	29	18	11	66	46	21	--	--	--
Pail	TSGT-1	158.5-160	GC	--	34	19	15	64	41	18	--	--	--
Pail	TSGT-1	184-186	GC	--	30	20	10	72	50	17	2.75	--	--
Pail	TSGT-1	211-214	SC	--	27	16	11	93	76	32	--	--	--
Pail	TSGT-1	228-230	GC	--	23	15	8	72	52	21	--	--	--
Pail	TSGT-1	250.5-253	SC	--	27	16	11	89	69	30	--	--	--
Pail	TSGT-1	298-300	SC	--	28	14	14	95	82	38	--	--	--
Pail	TSGT-1	310-312	GC	--	29	16	13	78	55	21	--	--	--
Pail	TSGT-1	327-329	SC-SM	--	21	15	6	83	67	24	--	--	--
Pail	TSGT-1	356-358	SC	--	32	16	16	80	65	26	--	--	--
Pail	TSGT-1	388-390	SC	--	26	15	11	93	82	35	2.70	--	--
Pail	TSGT-2	18-20	GC	--	27	16	11	72	47	16	2.89	--	--
Pail	TSGT-2	42-44	GP-GC	--	22	14	8	64	37	12	--	--	--
Pail	TSGT-2	140-142	SC	--	34	20	14	86	63	18	--	--	--
Pail	TSGT-2	267-268.5	GC	--	29	19	10	66	46	12	--	--	--
Pail	TSGT-3	18-20	SC	--	28	17	11	94	66	16	2.69	--	--
Pail	TSGT-3	58-60	GC	--	29	17	12	85	56	15	--	--	--
Pail	TSGT-3	156-158	SC	--	29	15	14	94	75	28	--	--	--
Pail	TSGT-3	248-250	GC	--	28	17	11	74	56	22	2.78	--	--
Pail	TSGT-4	12-14	GC	--	34	18	16	78	55	21	2.83	--	--
Pail	TSGT-4	69-71	GC	--	36	20	16	64	34	12	--	--	--
Pail	TSGT-4	117-119	GC	--	35	20	15	68	38	14	2.73	--	--
Pail	TSGT-4	152-154	GC	--	31	18	13	88	55	18	2.66	--	--
Pail	TSGT-4	183-185	SC	--	31	19	12	94	78	24	2.71	--	--
Pail	TSGT-4	221-223	GC	--	29	17	12	78	47	16	--	--	--
Pail	TSGT-4	265-269	SM	--	NP	NP	NP	97	90	22	--	--	--

Notes:

* Delivered moisture content taken from Bulk sample due to no bag sample present

LL = Liquid Limit

PL = Plastic Limit

PI = Plastic Index

USCS = Unified Soil Classification System

PCF = pounds per cubic foot

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

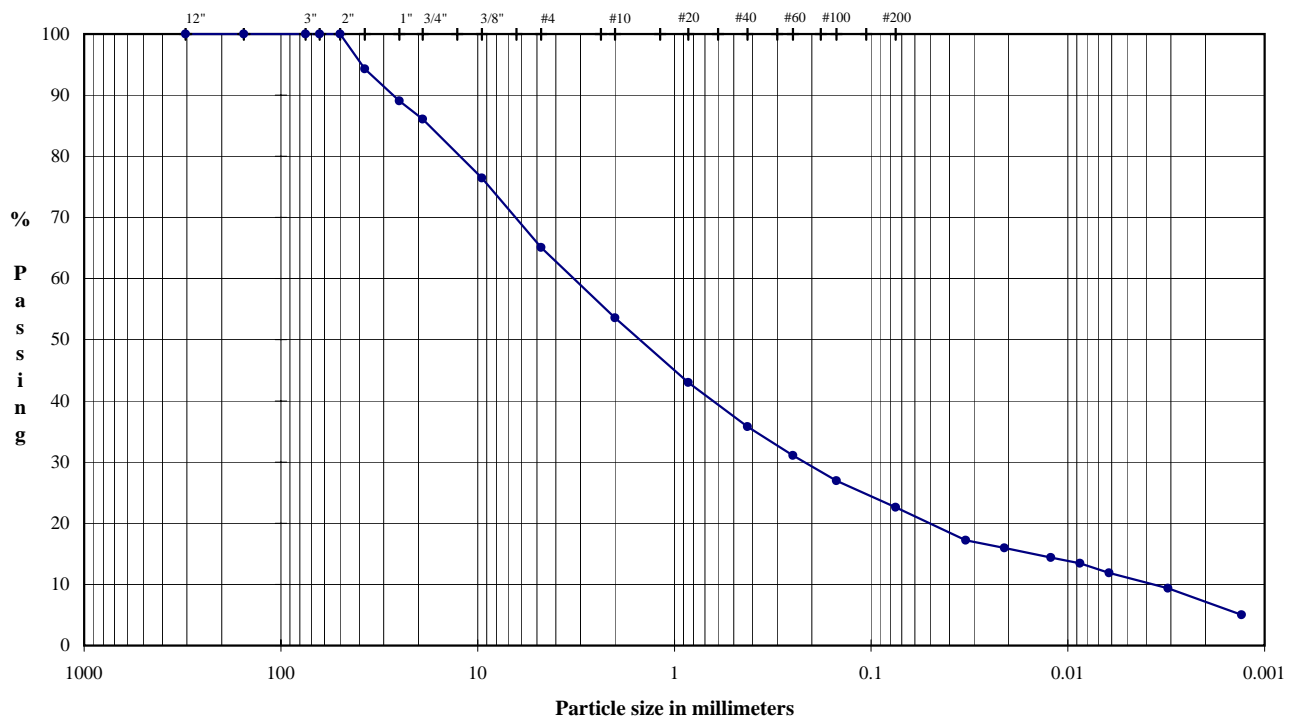
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-01

Depth (ft): 19-21

TYPE: Pail



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

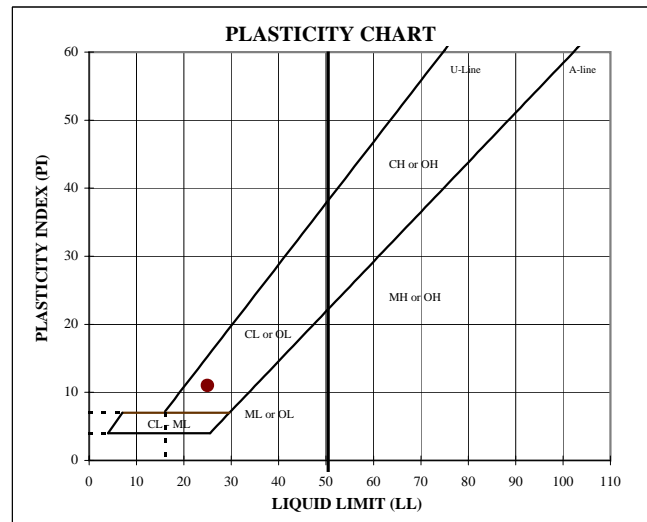
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
3.0"	75.0	100.0		
2.5"	63.5	100.0		
2.0"	50.0	100.0		
1.5"	37.5	94.3	Coarse Gravel	13.90
1.0"	25.0	89.1		
0.75"	19.0	86.1		
0.375"	9.5	76.5	Fine Gravel	21.03
#4	4.8	65.1		
#10	2.0	53.6		
#20	0.85	43.0	Coarse Sand	11.49
#40	0.43	35.8		
#60	0.25	31.1	Medium Sand	17.79
#100	0.15	26.9		
#200	0.075	22.6		
			Fine Sand	13.19

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
--	25	14	11	2.91

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	22.59
0.033	17.2		
0.021	16.0		
0.012	14.4		
0.0087	13.5		
0.0062	11.9		
0.0031	9.4		
0.0013	5.0		

DESCRIPTION: Pale yellow clayey sand with gravel

USCS: SC

 TECH: MKS
 DATE: 6/15/2005
 REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

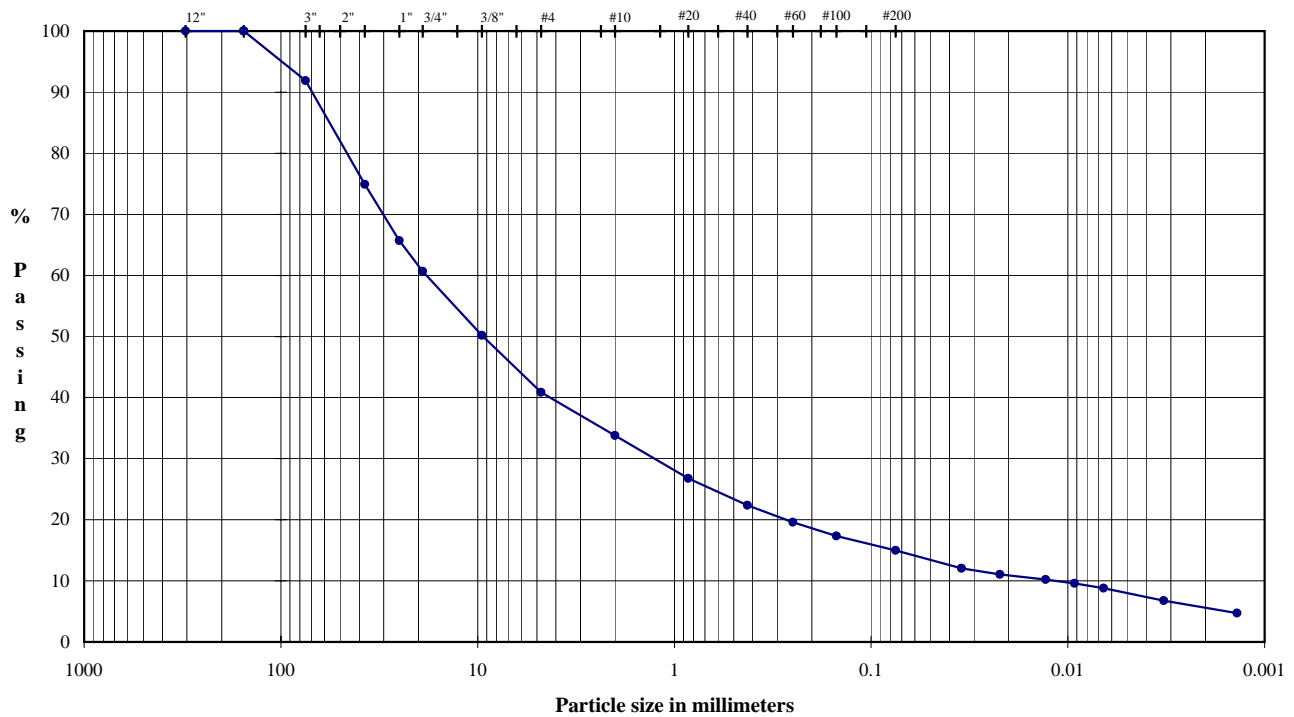
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-01

Depth (ft): 47-48.5

TYPE: Pail



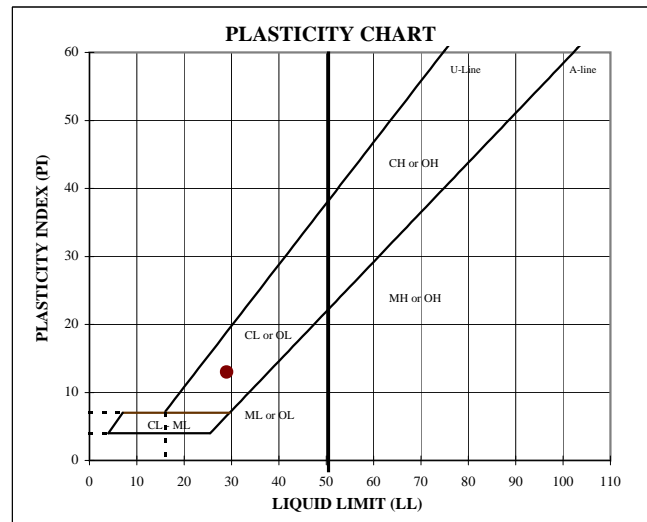
	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	8.09
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	91.9	Coarse Gravel	31.23
1.5"	37.5	74.9		
1.0"	25.0	65.7		
0.75"	19.0	60.7		
0.375"	9.5	50.2	Fine Gravel	19.83
#4	4.8	40.8		
#10	2.00	33.8		
#20	0.85	26.8		
#40	0.43	22.4	Medium Sand	11.40
#60	0.25	19.6		
#100	0.15	17.4		
#200	0.075	15.0		
			Fine Sand	7.39

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
--	29	16	13	2.70

Hydrometer Analysis

(mm)	% Finer		
0.035	12.1	Fines Silt or Clay	14.99
0.022	11.0		
0.013	10.2		
0.0092	9.6		
0.0066	8.8		
0.0033	6.7		
0.0014	4.7		

DESCRIPTION: Reddish yellow clayey gravel with sand

USCS: GC

 TECH: MKS
 DATE: 6/15/2005
 REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

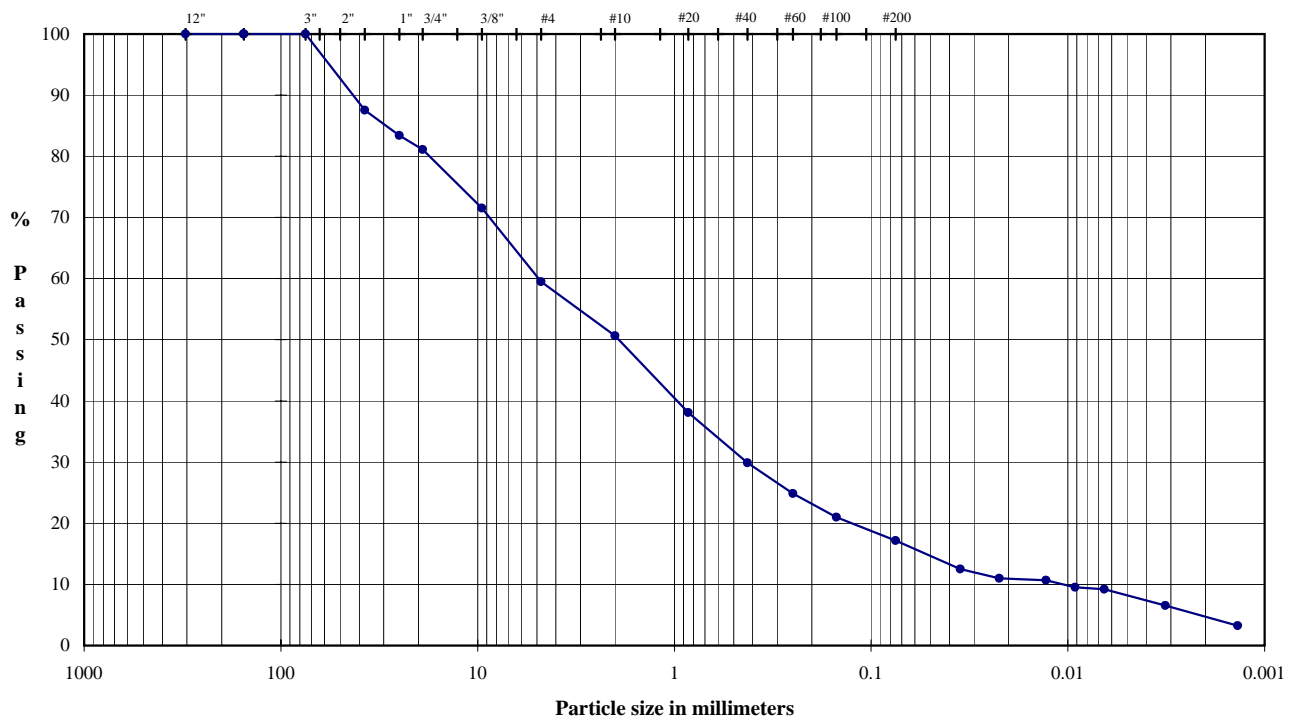
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-01

Depth (ft): 75-77

TYPE: Pail



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

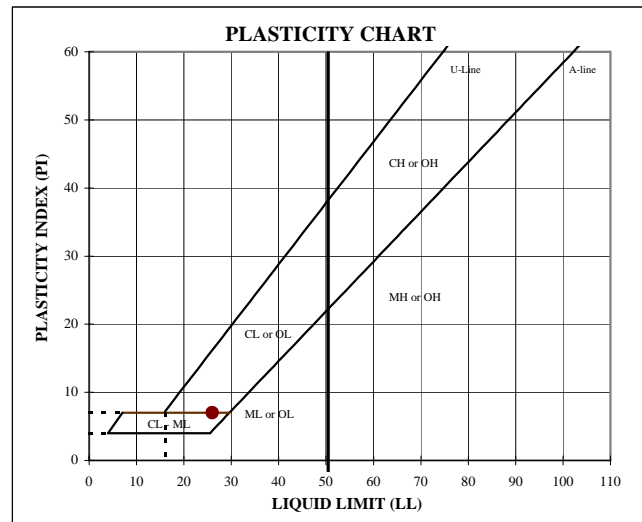
Particle Size

U.S. Standard Sieves Sizes and Numbers

	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	18.91
1.5"	37.5	87.6		
1.0"	25.0	83.4		
0.75"	19.0	81.1		
0.375"	9.5	71.5	Fine Gravel	21.57
#4	4.8	59.5		
#10	2.0	50.7		
#20	0.85	38.1		
#40	0.43	29.9	Medium Sand	20.78
#60	0.25	24.8		
#100	0.15	21.0	Fine Sand	12.68
#200	0.075	17.2		

Hydrometer Analysis

(mm)	% Finer		
0.035	12.5	Fines	17.19
0.022	11.0		
0.013	10.7		
0.0092	9.5		
0.0065	9.2	Silt or Clay	17.19
0.0032	6.5		
0.0014	3.3		



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
--	26	19	7	2.70

DESCRIPTION: Pale red silty, clayey sand with gravel

USCS: SC-SM

TECH DS
DATE 6/17/2005
REVIEW MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

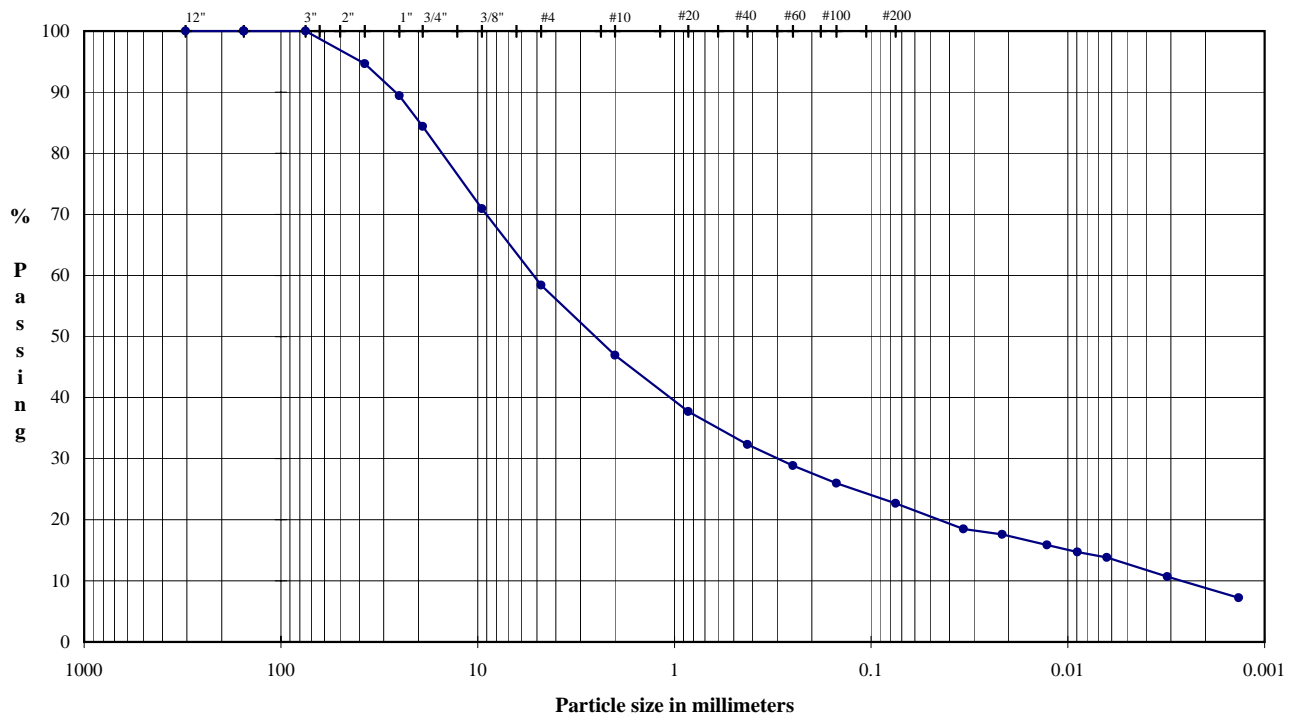
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-01

Depth (ft): 88-90

TYPE: Pail



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

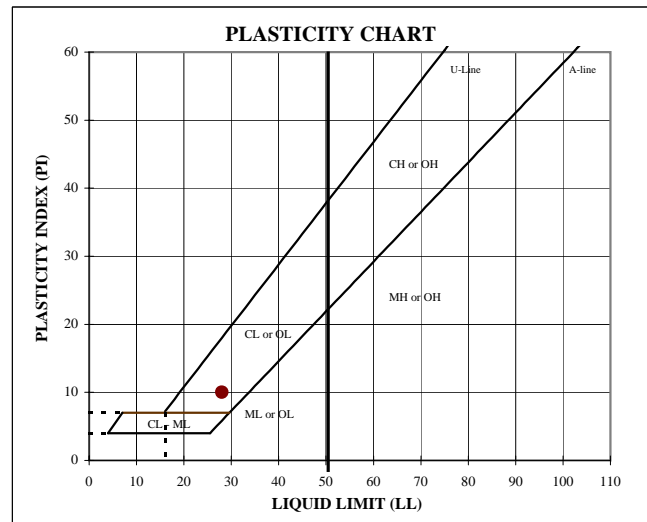
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	15.63
1.5"	37.5	94.6		
1.0"	25.0	89.4		
0.75"	19.0	84.4		
0.375"	9.5	71.0	Fine Gravel	25.95
#4	4.8	58.4		
#10	2.0	46.9		
#20	0.85	37.7		
#40	0.43	32.3	Medium Sand	14.60
#60	0.25	28.9		
#100	0.15	26.0		
#200	0.075	22.7		
			Fine Sand	9.63

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
--	28	18	10	2.70

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	22.70
0.034	18.5		
0.022	17.6		
0.013	15.9		
0.0090	14.7		
0.0063	13.8		
0.0031	10.7		
0.0014	7.2		

DESCRIPTION: Light brown clayey gravel with sand

USCS: GC

TECH DS

DATE 6/15/2005

REVIEW MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

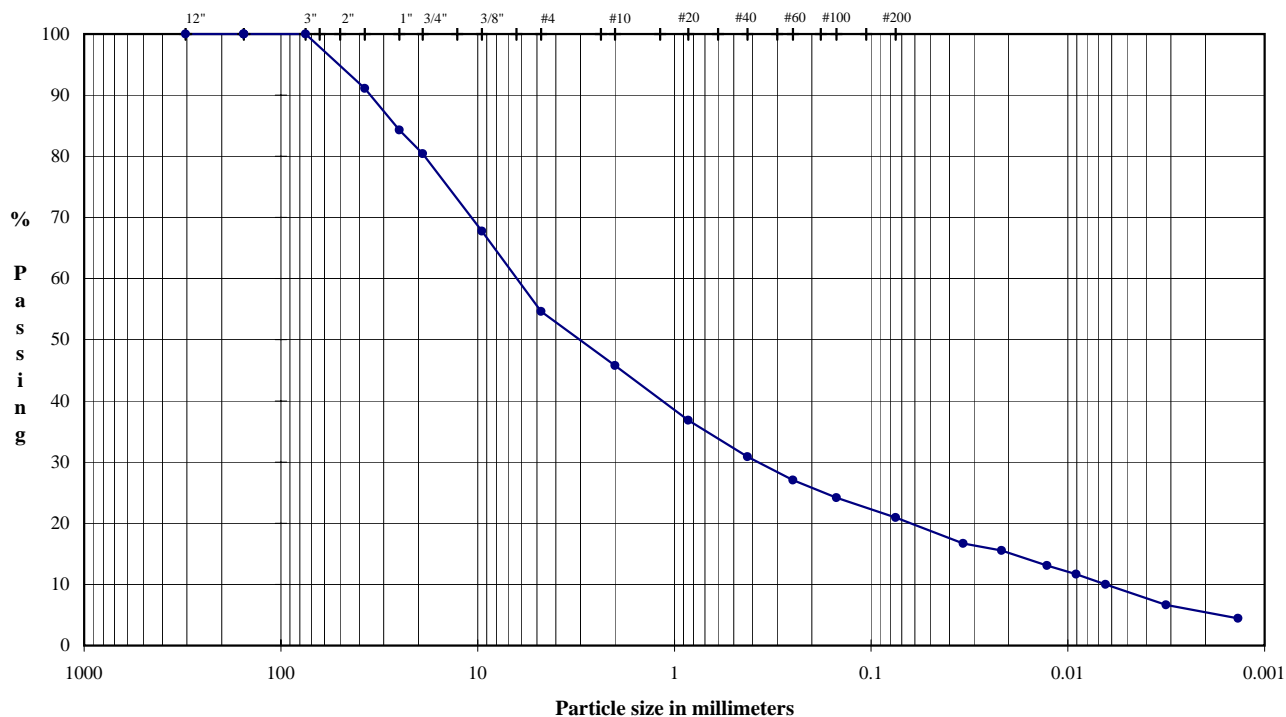
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-01

Depth (ft): 102-104

TYPE: Pail



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

% Passing

Classification

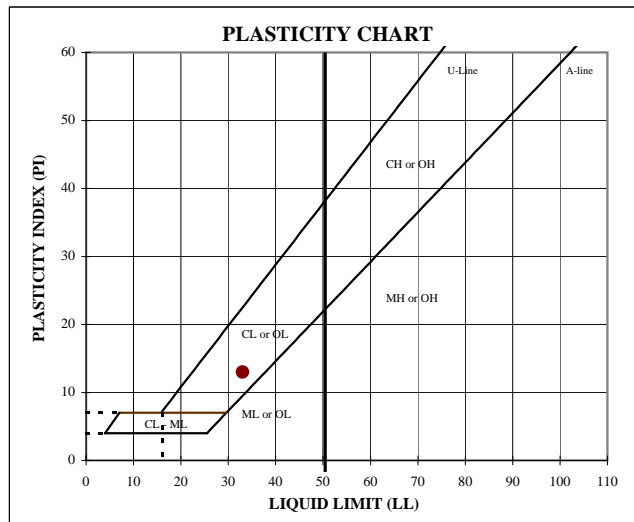
Percentage

U.S. Standard Sieves Sizes and Numbers

12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	19.55
1.5"	37.5	91.1		
1.0"	25.0	84.3		
0.75"	19.0	80.4		
0.375"	9.5	67.8	Fine Gravel	25.81
#4	4.8	54.6		
#10	2.0	45.8		
#20	0.85	36.8		
#40	0.43	30.9	Medium Sand	14.94
#60	0.25	27.1		
#100	0.15	24.2		
#200	0.075	21.0		
			Fine Sand	9.90

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	20.95
0.034	16.7		
0.022	15.6		
0.013	13.1		
0.0091	11.7		
0.0064	10.0		
0.0032	6.7		
0.0014	4.4		



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
--	33	20	13	2.70

DESCRIPTION: Weak red clayey gravel with sand

USCS: GC

TECH: MKS/DS

DATE: 6/17/2005

REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

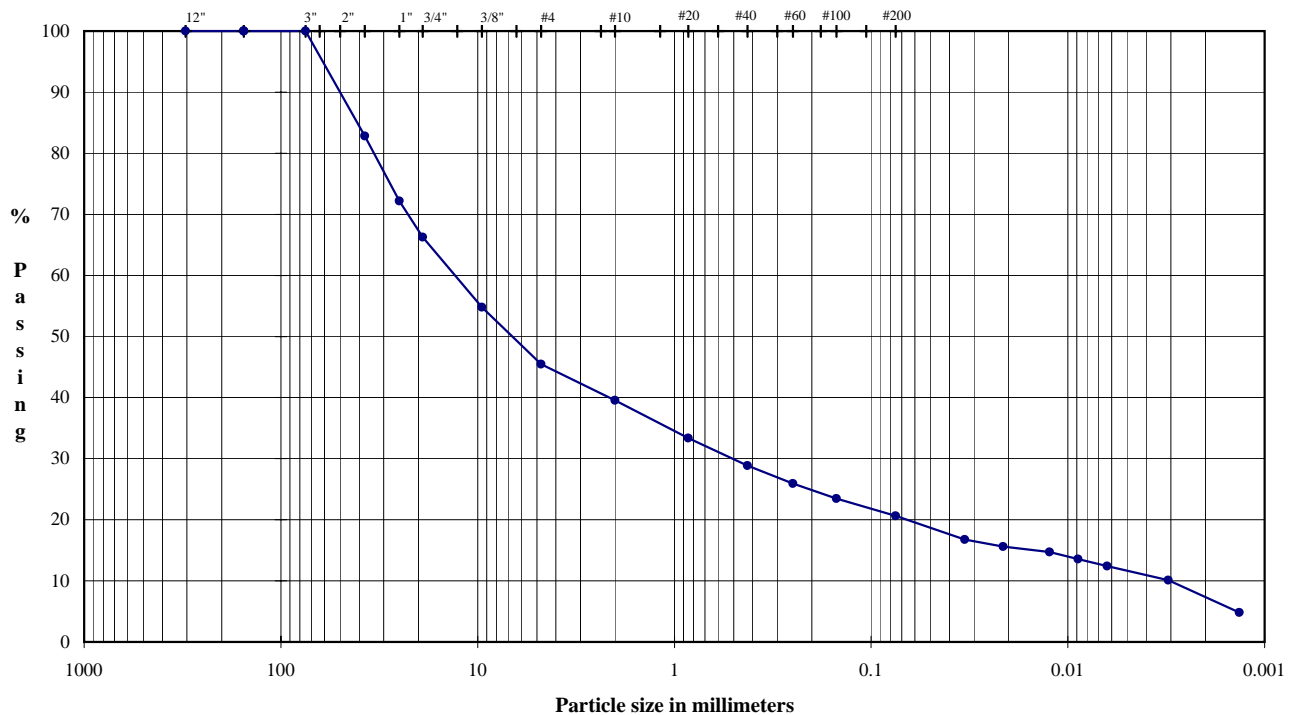
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-01

Depth (ft): 140-142

TYPE: Pail



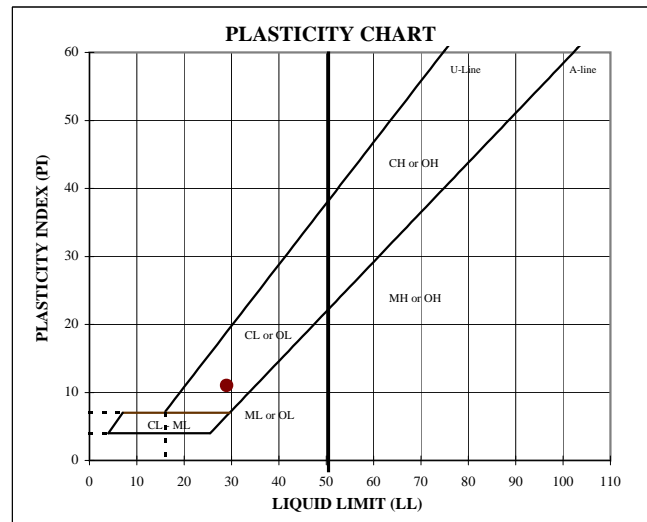
	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	33.72
1.5"	37.5	82.8		
1.0"	25.0	72.2		
0.75"	19.0	66.3		
0.375"	9.5	54.8	Fine Gravel	20.80
#4	4.8	45.5		
#10	2.00	39.6		
#20	0.85	33.3		
#40	0.43	28.9	Medium Sand	10.67
#60	0.25	25.9		
#100	0.15	23.5		
#200	0.075	20.6		
			Fine Sand	8.24

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
--	29	18	11	2.70

Hydrometer Analysis

(mm)	% Finer		
0.033	16.8	Fines Silt or Clay	20.64
0.021	15.6		
0.012	14.7		
0.0089	13.6		
0.0063	12.4		
0.0031	10.1		
0.0013	4.8		

DESCRIPTION: Pale yellow clayey gravel with sand

USCS: GC

 TECH DS
 DATE 6/17/2005
 REVIEW MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

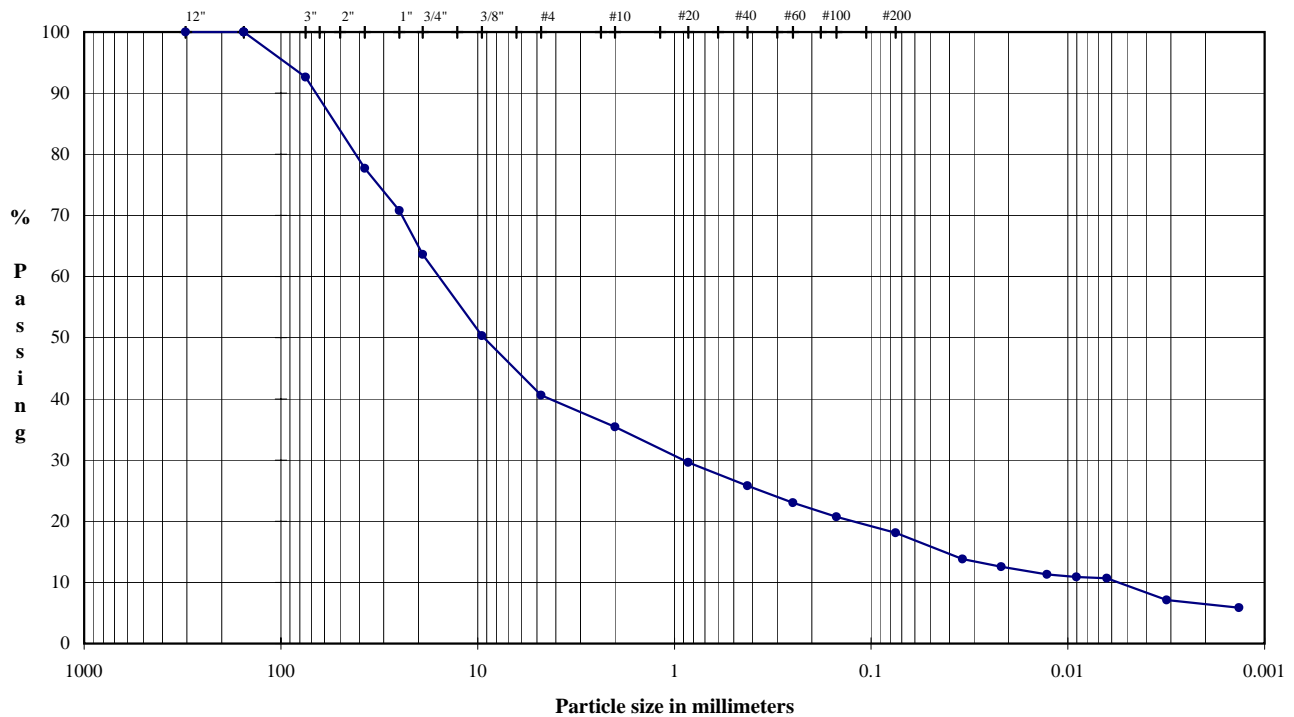
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-01

Depth (ft): 158.5-160

TYPE: Pail



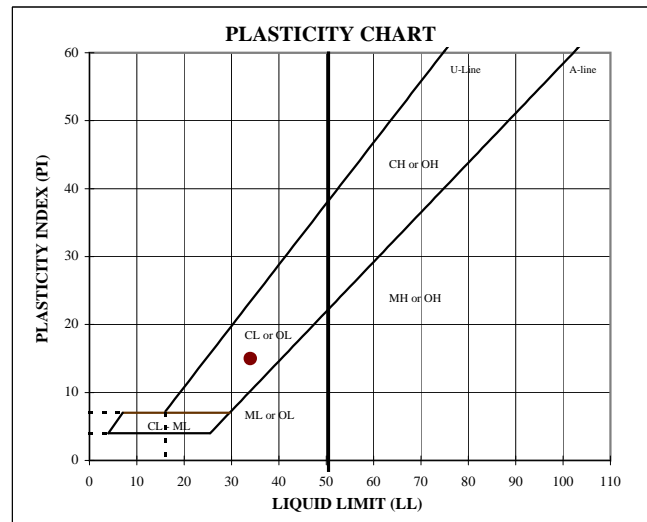
COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			FINES

Particle Size

Particle Size

	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	7.39
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0	Coarse Gravel	28.99
3.0"	75.0	92.6		
1.5"	37.5	77.7		
1.0"	25.0	70.8		
0.75"	19.0	63.6	Fine Gravel	23.01
0.375"	9.5	50.3		
#4	4.8	40.6		
#10	2.00	35.4	Coarse Sand	5.19
#20	0.85	29.6	Medium Sand	9.63
#40	0.43	25.8		
#60	0.25	23.0	Fine Sand	7.69
#100	0.15	20.7		
#200	0.075	18.1		

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
--	34	19	15	2.70

Hydrometer Analysis

(mm)	% Finer		
0.034	13.8	Fines Silt or Clay	18.10
0.022	12.5		
0.013	11.3		
0.0090	10.9		
0.0063	10.7		
0.0031	7.1		
0.0013	5.9		

DESCRIPTION: Pale olive clayey gravel with sand

USCS: GC

TECH: MKS
DATE: 6/17/2005
REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

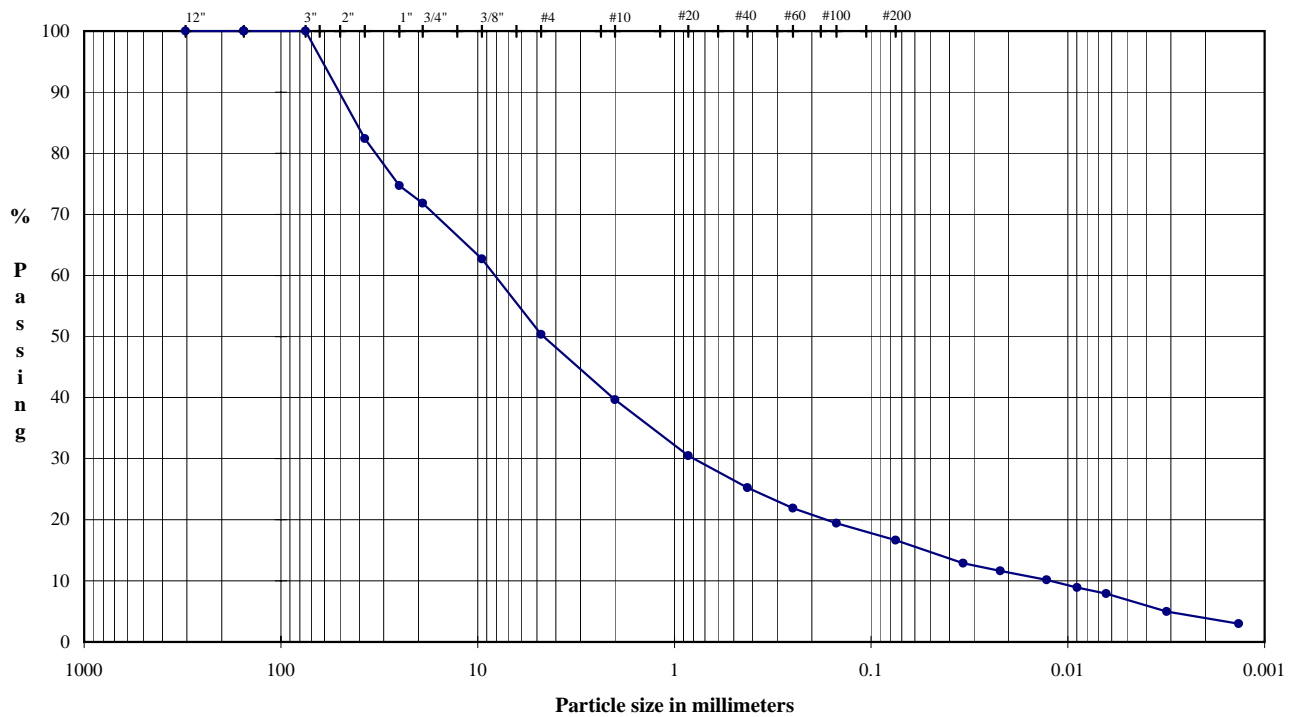
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-01

Depth (ft): 184-186

TYPE: Pail



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

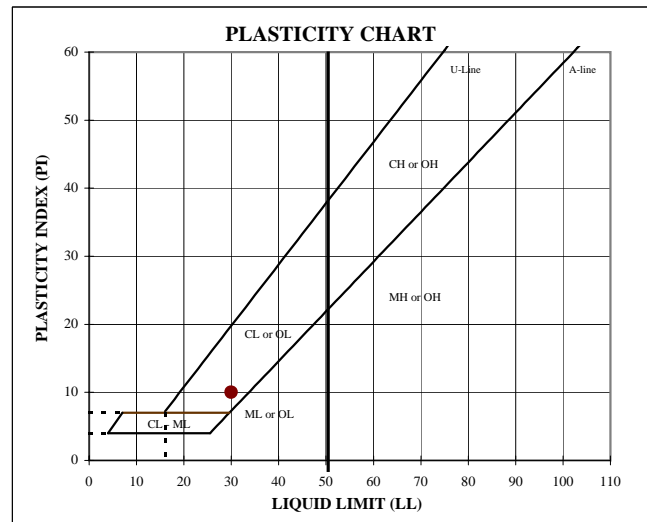
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	28.18
1.5"	37.5	82.4		
1.0"	25.0	74.7		
0.75"	19.0	71.8		
0.375"	9.5	62.7	Fine Gravel	21.50
#4	4.8	50.3		
#10	2.0	39.6		
#20	0.85	30.5		
#40	0.43	25.3	Medium Sand	14.39
#60	0.25	21.9		
#100	0.15	19.5		
#200	0.075	16.7		
			Fine Sand	8.57

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
--	30	20	10	2.75

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	16.68
0.034	12.9		
0.022	11.6		
0.013	10.2		
0.0090	8.9		
0.0064	7.9		
0.0031	5.0		
0.0014	3.0		

DESCRIPTION: Pale red clayey gravel with sand

USCS: GC

TECH: MKS
DATE: 6/17/2005
REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

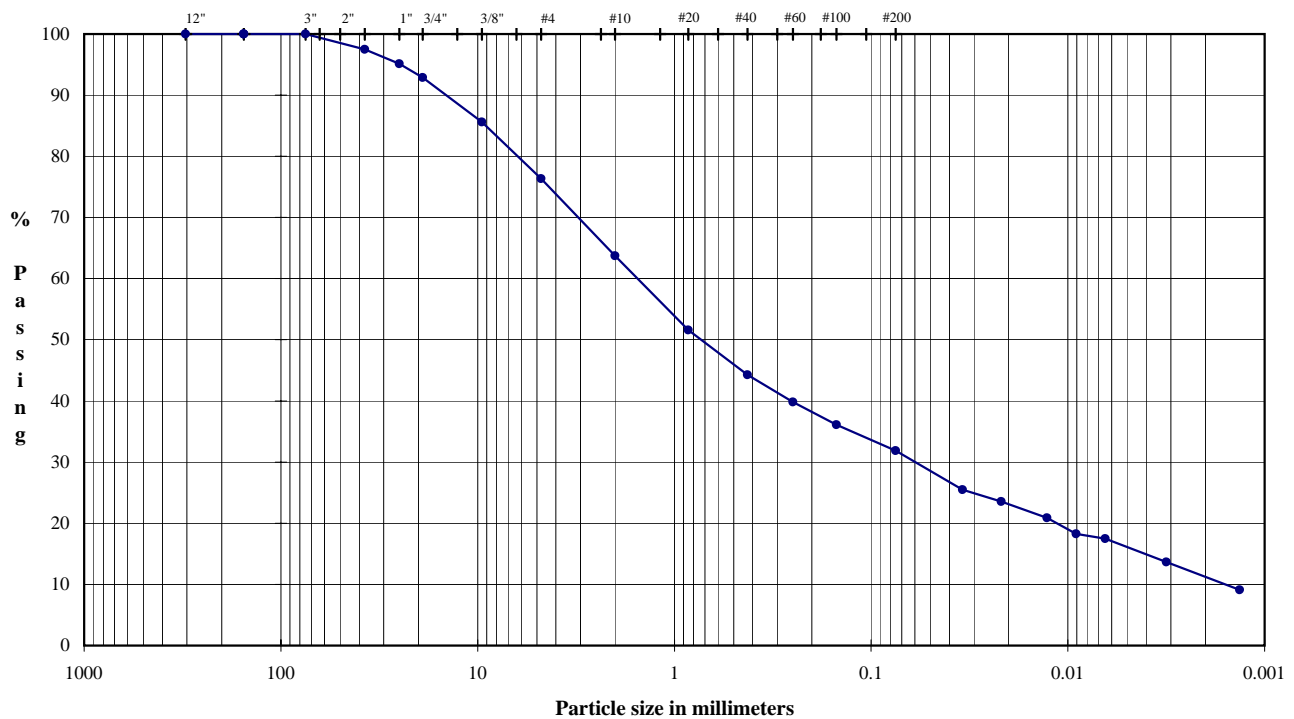
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-01

Depth (ft): 211-214

TYPE: Pail



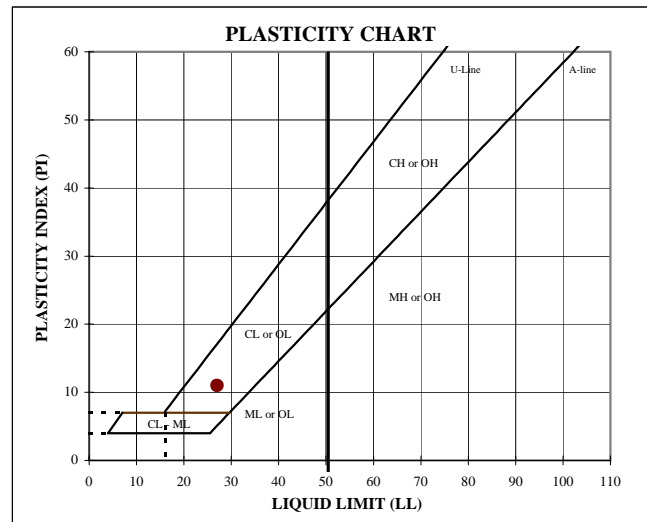
	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	7.12
1.5"	37.5	97.5		
1.0"	25.0	95.1		
0.75"	19.0	92.9		
0.375"	9.5	85.6	Fine Gravel	16.51
#4	4.8	76.4		
#10	2.00	63.7		
#20	0.85	51.6		
#40	0.43	44.3	Medium Sand	19.45
#60	0.25	39.8		
#100	0.15	36.1		
#200	0.075	31.9		
			Fine Sand	12.42

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
--	27	16	11	2.70

Hydrometer Analysis

(mm)	% Finer		
0.034	25.5	Fines Silt or Clay	31.86
0.022	23.6		
0.013	20.9		
0.0091	18.2		
0.0065	17.5		
0.0032	13.7		
0.0013	9.1		

DESCRIPTION: Light brown clayey sand with gravel

USCS: SC

 TECH DS
 DATE 6/20/2005
 REVIEW MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

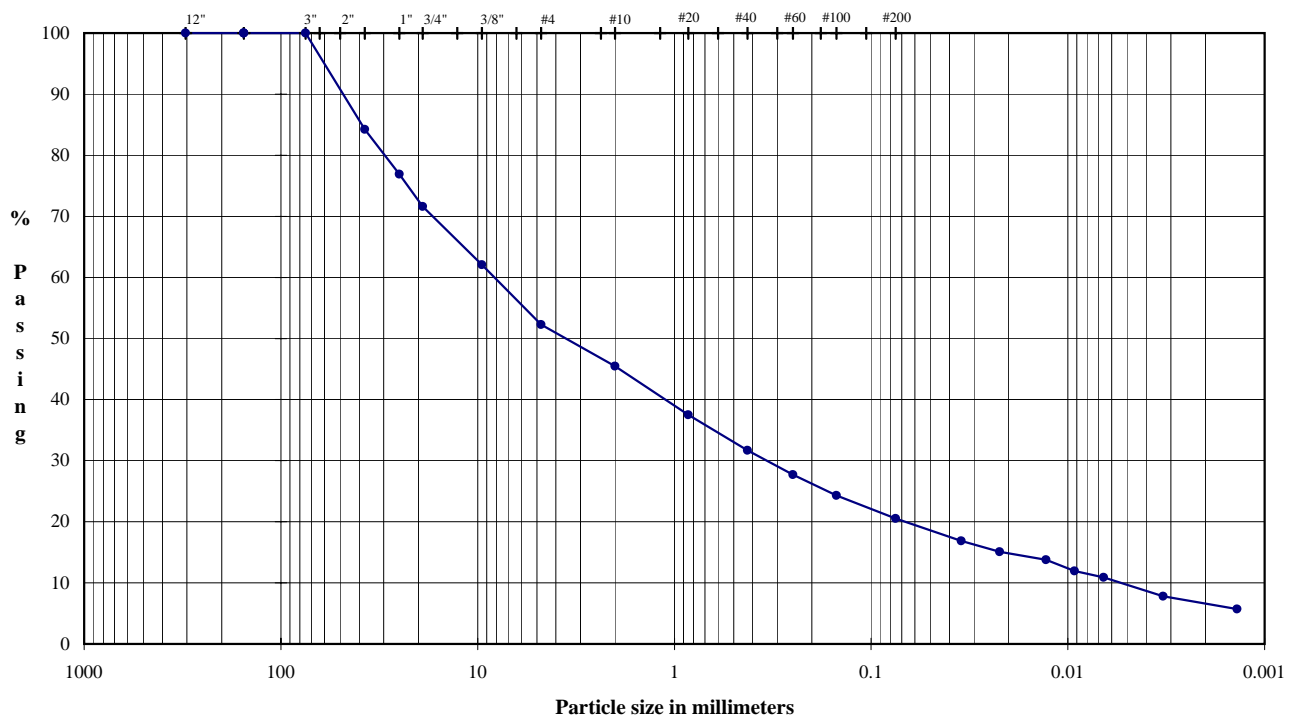
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-01

Depth (ft): 228-230

TYPE: Pail



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

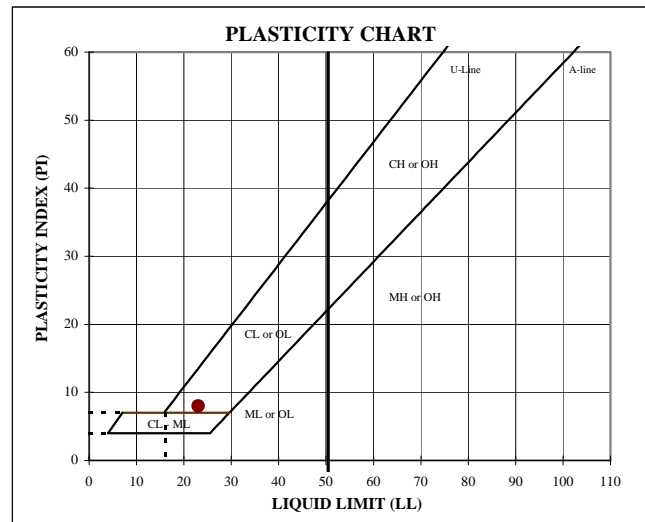
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0	Coarse Gravel	28.41
3.0"	75.0	100.0		
1.5"	37.5	84.2		
1.0"	25.0	76.9		
0.75"	19.0	71.6	Fine Gravel	19.32
0.375"	9.5	62.1		
#4	4.8	52.3		
#10	2.0	45.5	Coarse Sand	6.78
#20	0.85	37.5	Medium Sand	13.78
#40	0.43	31.7		
#60	0.25	27.7	Fine Sand	11.19
#100	0.15	24.3		
#200	0.075	20.5		

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
--	23	15	8	2.70

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	20.52
0.035	16.9		
0.022	15.1		
0.013	13.8		
0.0092	11.9		
0.0066	10.9		
0.0033	7.8		
0.0014	5.7		

DESCRIPTION: Pinkish grey clayey gravel with sand

USCS: GC

TECH: MKS
DATE: 6/15/2005
REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

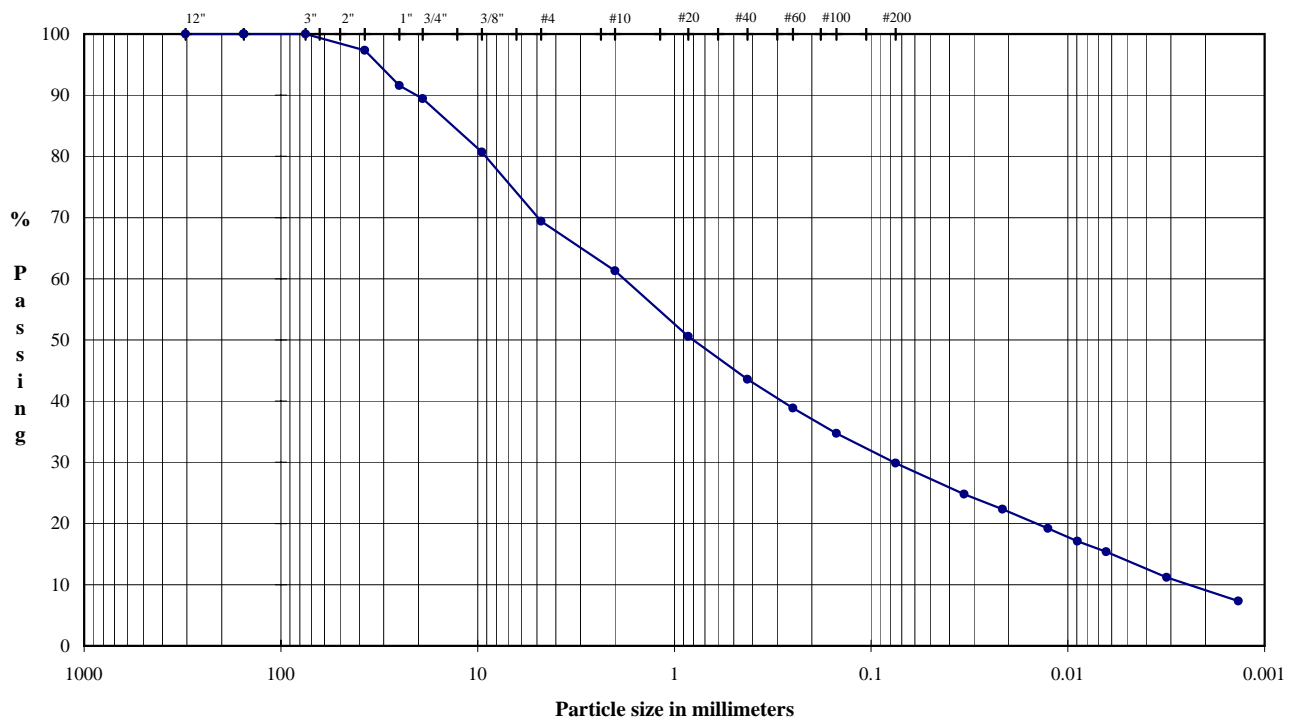
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-01

Depth (ft): 250.5-253

TYPE: Pail



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

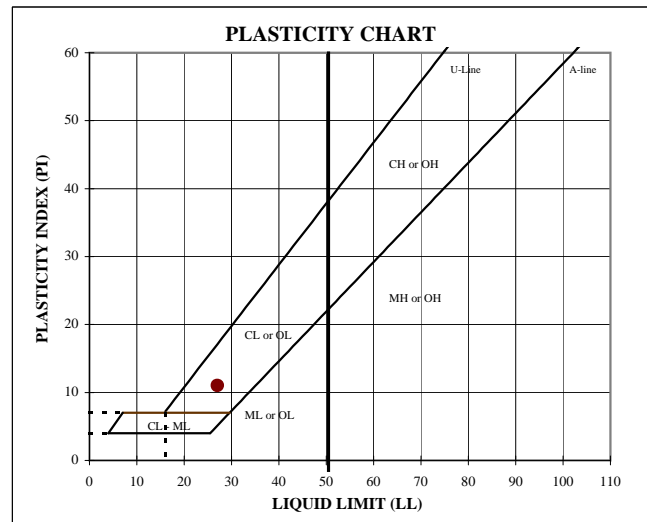
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	10.59
1.5"	37.5	97.3		
1.0"	25.0	91.6		
0.75"	19.0	89.4		
0.375"	9.5	80.7	Fine Gravel	20.02
#4	4.8	69.4		
#10	2.0	61.3		
#20	0.85	50.6		
#40	0.43	43.6	Medium Sand	17.75
#60	0.25	38.9		
#100	0.15	34.7		
#200	0.075	29.9		
			Fine Sand	13.68

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
--	27	16	11	2.70

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	29.89
0.034	24.8		
0.021	22.3		
0.013	19.2		
0.0090	17.1		
0.0064	15.4		
0.0031	11.2		
0.0014	7.3		

DESCRIPTION: Weak red clayey sand with gravel

USCS: SC

TECH	DS
DATE	6/17/2005
REVIEW	MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

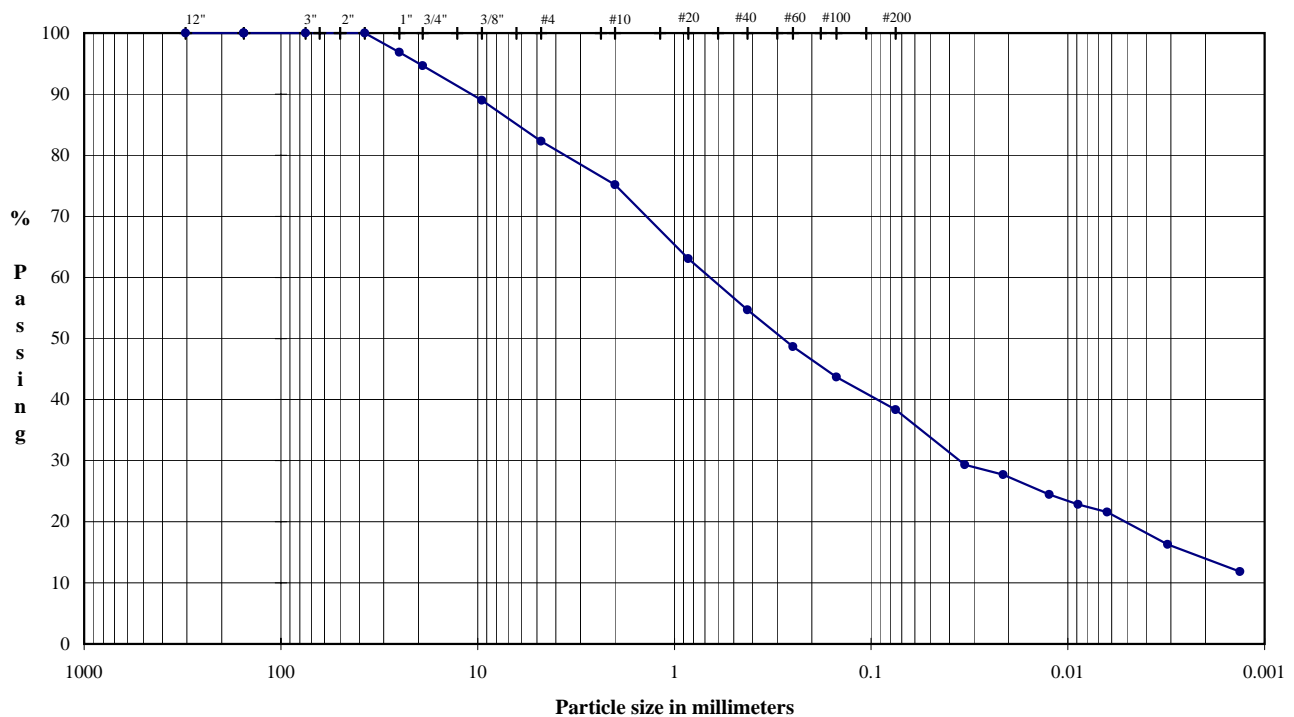
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-01

Depth (ft): 298-300

TYPE: Pail



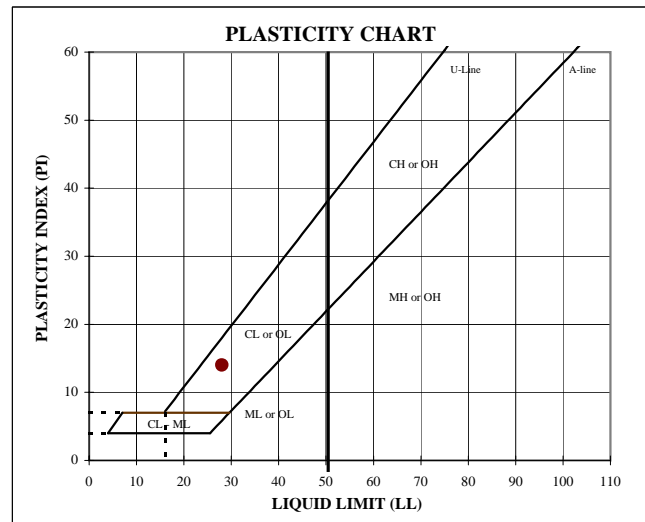
	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	5.35
1.5"	37.5	100.0		
1.0"	25.0	96.9		
0.75"	19.0	94.6		
0.375"	9.5	89.0	Fine Gravel	12.34
#4	4.8	82.3		
#10	2.00	75.2	Coarse Sand	7.13
#20	0.85	63.1	Medium Sand	20.50
#40	0.43	54.7		
#60	0.25	48.7	Fine Sand	16.35
#100	0.15	43.7		
#200	0.075	38.3		

U.S. Standard Sieves Sizes and Numbers



Hydrometer Analysis

(mm)	% Finer		
0.033	29.3	Fines Silt or Clay	38.33
0.021	27.7		
0.012	24.4		
0.0089	22.8		
0.0063	21.6		
0.0031	16.3		
0.0013	11.8		

ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
--	28	14	14	2.70

DESCRIPTION: Light brown clayey sand with gravel

USCS: SC

TECH DS
DATE 6/17/2005
REVIEW MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

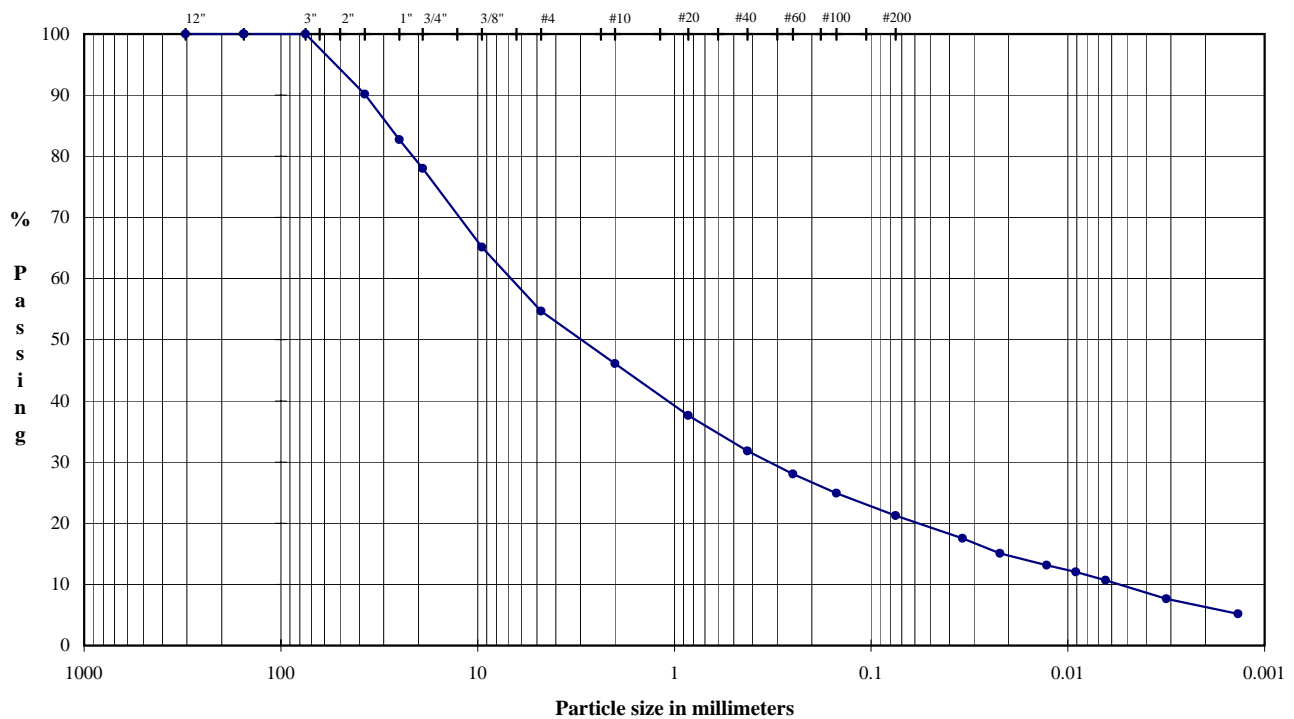
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-01

Depth (ft): 310-312

TYPE: Pail



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

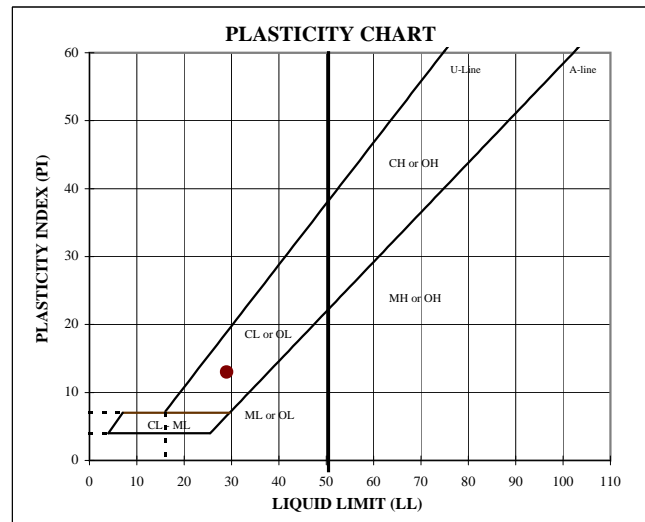
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	21.96
1.5"	37.5	90.1		
1.0"	25.0	82.7		
0.75"	19.0	78.0		
0.375"	9.5	65.1	Fine Gravel	23.35
#4	4.8	54.7		
#10	2.00	46.1		
#20	0.85	37.6		
#40	0.43	31.8	Medium Sand	14.30
#60	0.25	28.0		
#100	0.15	24.9		
#200	0.075	21.3		
			Fine Sand	10.55

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
--	29	16	13	2.70

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	21.26
0.034	17.5		
0.022	15.1		
0.013	13.1		
0.0091	12.0		
0.0064	10.7		
0.0032	7.7		
0.0014	5.2		

DESCRIPTION: Pale yellow gravelly clay with sand

USCS: GC

TECH: MKS
DATE: 6/17/2005
REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

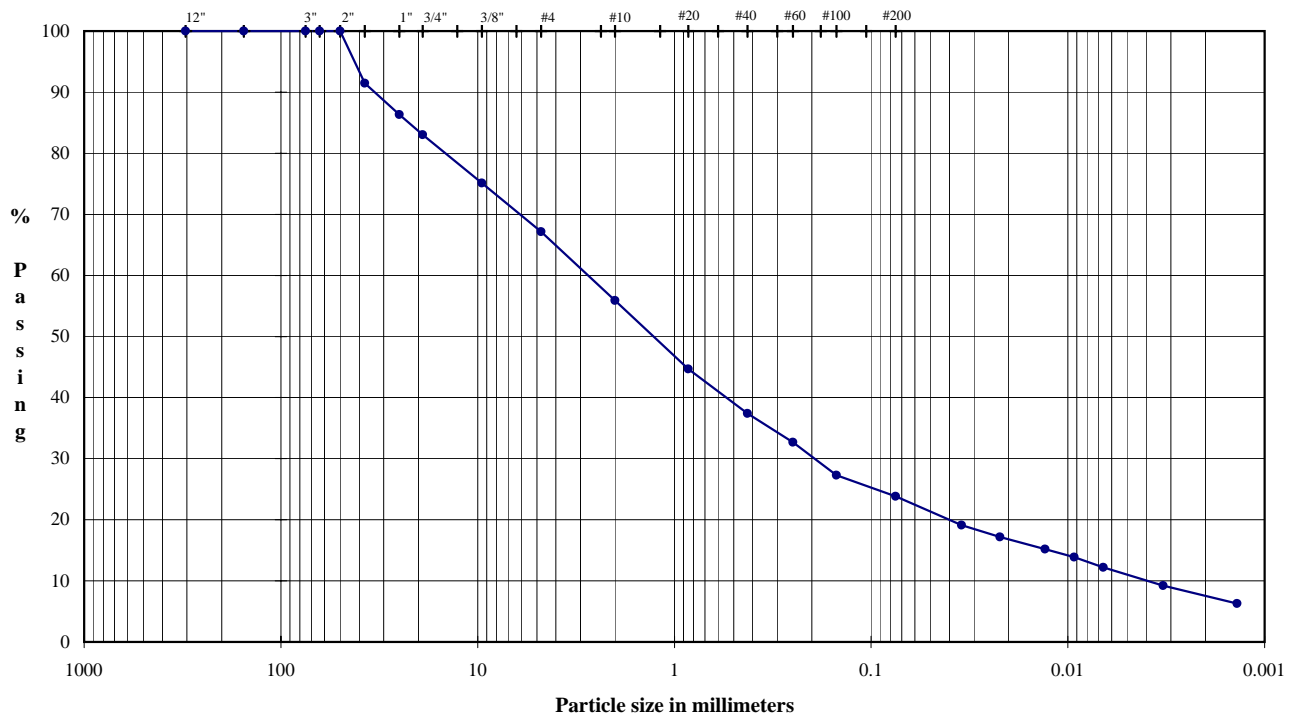
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-01

Depth (ft): 327-329

TYPE: Pail



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

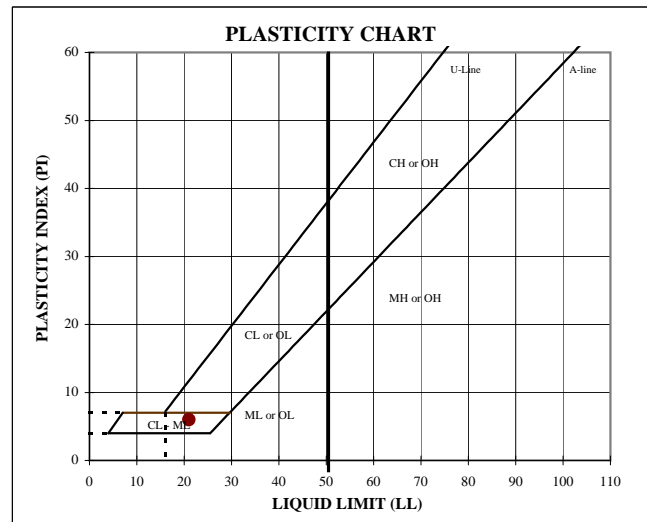
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
3.0"	75.0	100.0		
2.5"	63.5	100.0		
2.0"	50.0	100.0		
1.5"	37.5	91.4	Coarse Gravel	16.97
1.0"	25.0	86.3		
0.75"	19.0	83.0		
0.375"	9.5	75.1	Fine Gravel	15.88
#4	4.8	67.1		
#10	2.0	55.9		
#20	0.85	44.7	Coarse Sand	11.28
#40	0.43	37.4	Medium Sand	18.47
#60	0.25	32.7		
#100	0.15	27.3	Fine Sand	13.57
#200	0.075	23.8		

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
--	21	15	6	2.70

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	23.82
0.035	19.1		
0.022	17.2		
0.013	15.2		
0.0093	13.9		
0.0066	12.2		
0.0033	9.2		
0.0014	6.3		

DESCRIPTION: Light greenish grey silty, clayey sand with gravel

USCS: SC-SM

TECH: MKS
DATE: 6/15/2005
REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

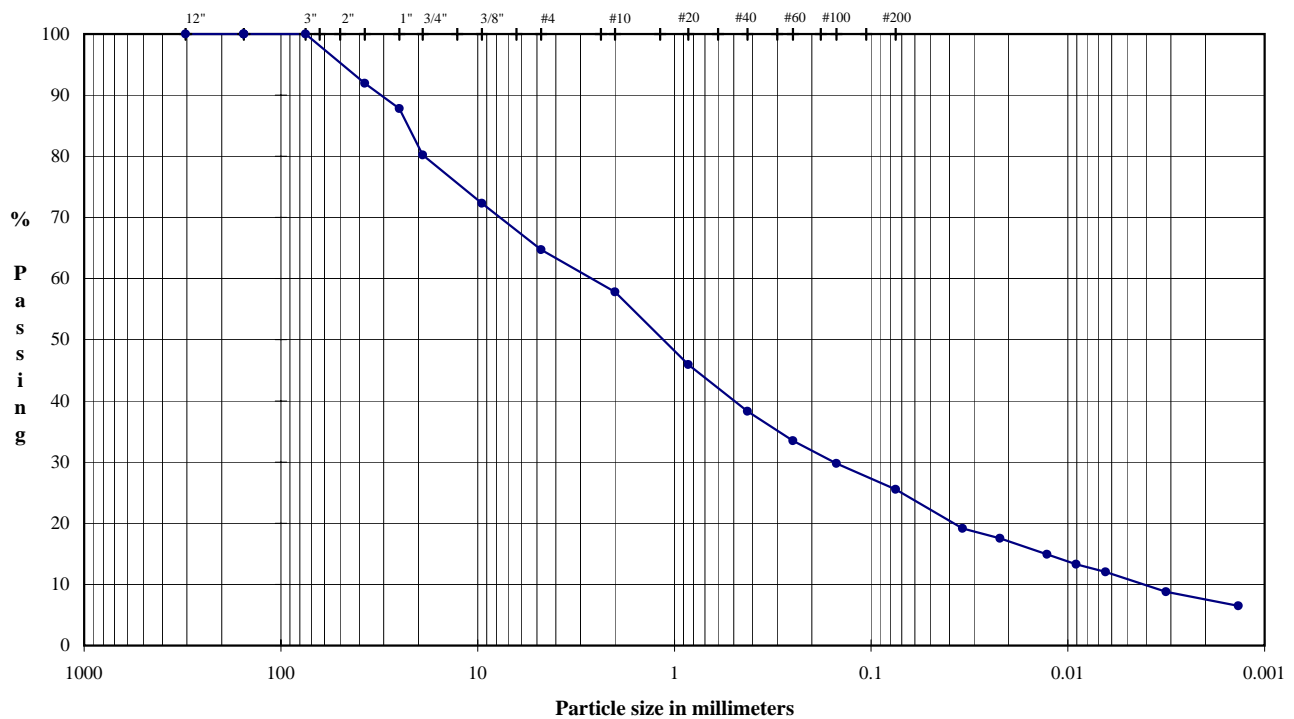
ASTM D421, D422, D4318

PROJECT NAME: **G&K/Tyrone Stockpile Geotech/AZ**

SAMPLE ID: **TSGT-01**

Depth (ft): **356-358**

TYPE: **Pail**



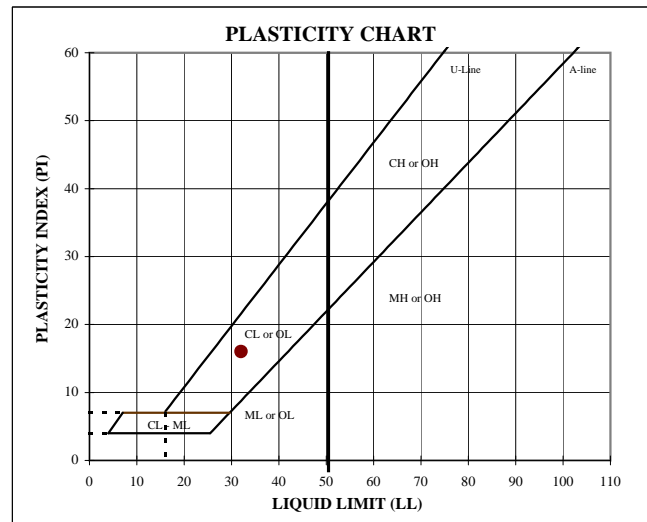
	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	19.76
1.5"	37.5	91.9		
1.0"	25.0	87.8		
0.75"	19.0	80.2		
0.375"	9.5	72.3	Fine Gravel	15.50
#4	4.8	64.7		
#10	2.00	57.8	Coarse Sand	6.93
#20	0.85	45.9	Medium Sand	19.48
#40	0.43	38.3		
#60	0.25	33.5	Fine Sand	12.82
#100	0.15	29.7		
#200	0.075	25.5		

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
--	32	16	16	2.70

Hydrometer Analysis

(mm)	% Finer		
0.034	19.2	Fines Silt or Clay	25.51
0.022	17.5		
0.013	14.9		
0.0091	13.3		
0.0064	12.0		
0.0032	8.8		
0.0014	6.5		

DESCRIPTION: Pinkish white clayey sand with gravel

USCS: SC

TECH: MKS
DATE: 6/17/2005
REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

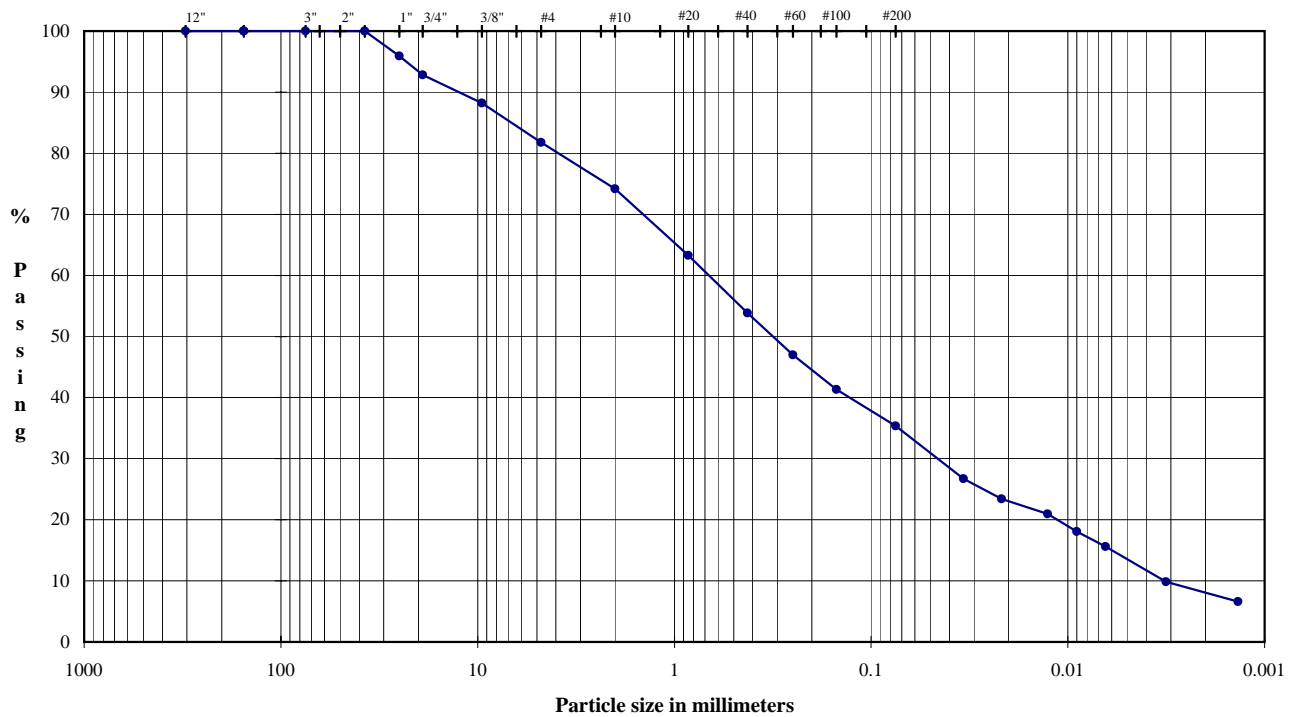
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-01

Depth (ft): 388-390

TYPE: Pail



COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

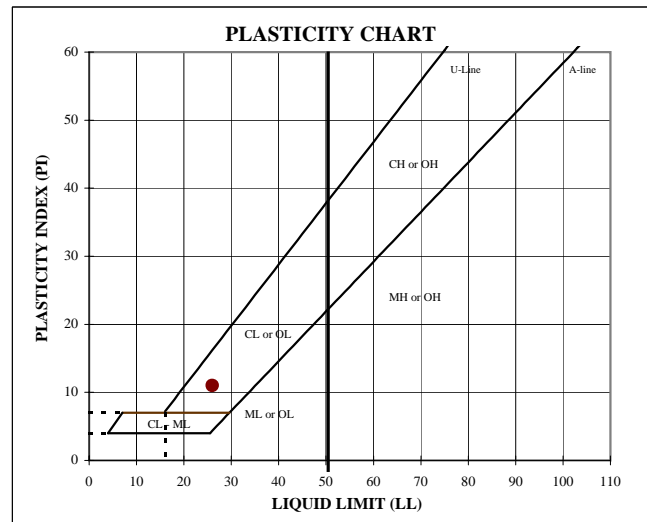
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	7.16
1.5"	37.5	100.0		
1.0"	25.0	95.9		
0.75"	19.0	92.8		
0.375"	9.5	88.2	Fine Gravel	11.06
#4	4.8	81.8		
#10	2.00	74.2		
#20	0.85	63.3		
#40	0.43	53.9	Medium Sand	20.33
#60	0.25	47.0		
#100	0.15	41.3		
#200	0.075	35.3		
			Fine Sand	18.52

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
--	26	15	11	2.70

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	35.34
0.034	26.7		
0.022	23.4		
0.013	21.0		
0.0090	18.1		
0.0064	15.6		
0.0032	9.9		
0.0014	6.6		

DESCRIPTION: Pale red clayey sand with gravel

USCS: SC

 TECH: MKS
 DATE: 6/15/2005
 REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

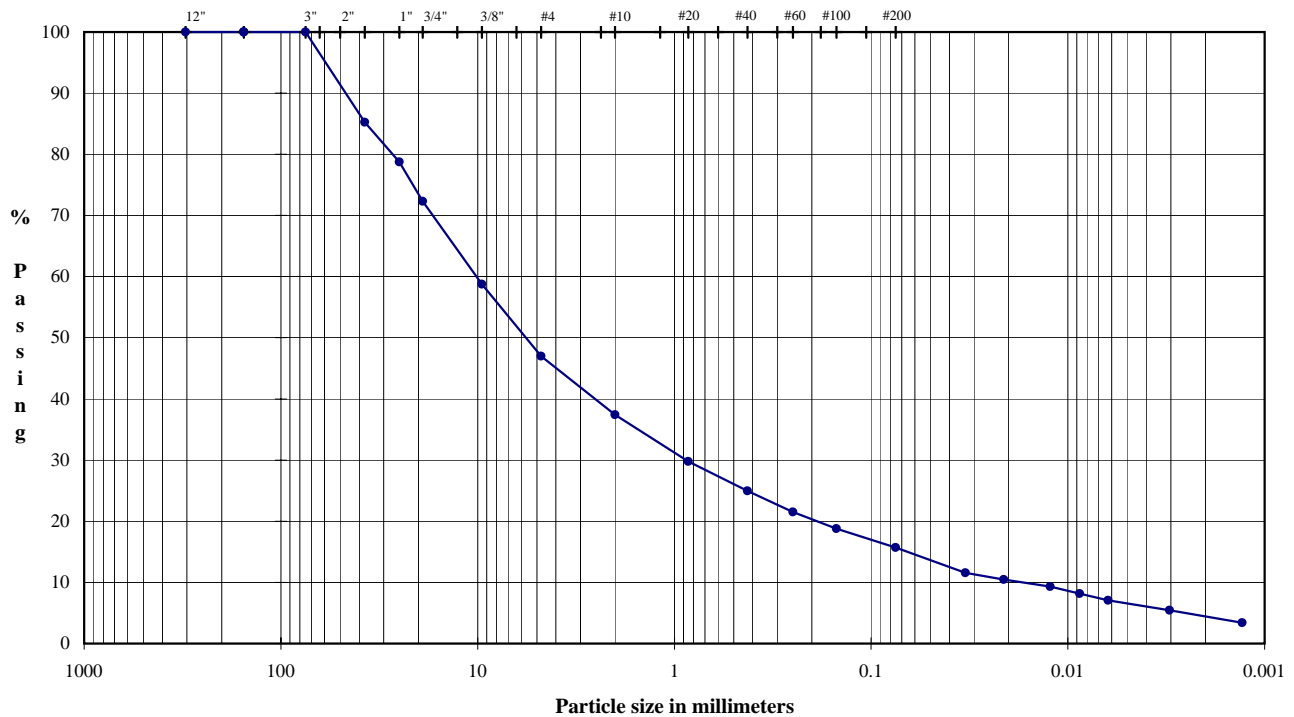
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-02

Depth (ft): 18-20

TYPE: Pail



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

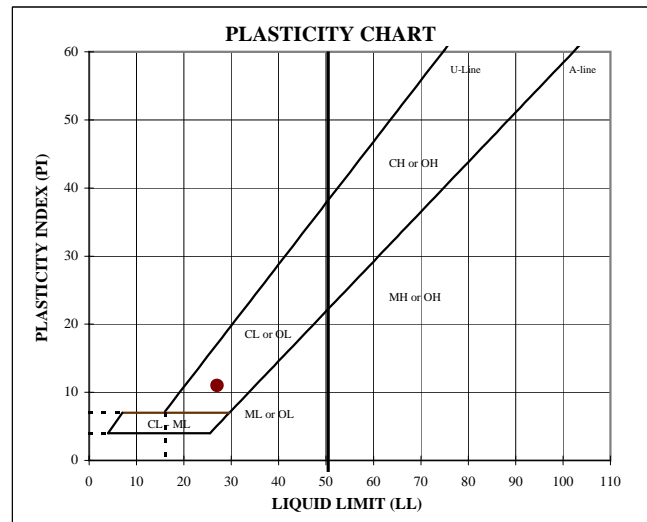
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	27.70
1.5"	37.5	85.2		
1.0"	25.0	78.7		
0.75"	19.0	72.3		
0.375"	9.5	58.7	Fine Gravel	25.29
#4	4.8	47.0		
#10	2.00	37.4		
#20	0.85	29.8		
#40	0.43	25.0	Medium Sand	12.47
#60	0.25	21.5		
#100	0.15	18.8		
#200	0.075	15.7		
			Fine Sand	9.27

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
--	27	16	11	2.89

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	15.69
0.033	11.6		
0.021	10.5		
0.012	9.3		
0.0087	8.2		
0.0062	7.0		
0.0030	5.5		
0.0013	3.4		

DESCRIPTION: Pale yellow clayey gravel with sand

USCS: GC

TECH DS
DATE 6/21/2005
REVIEW MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

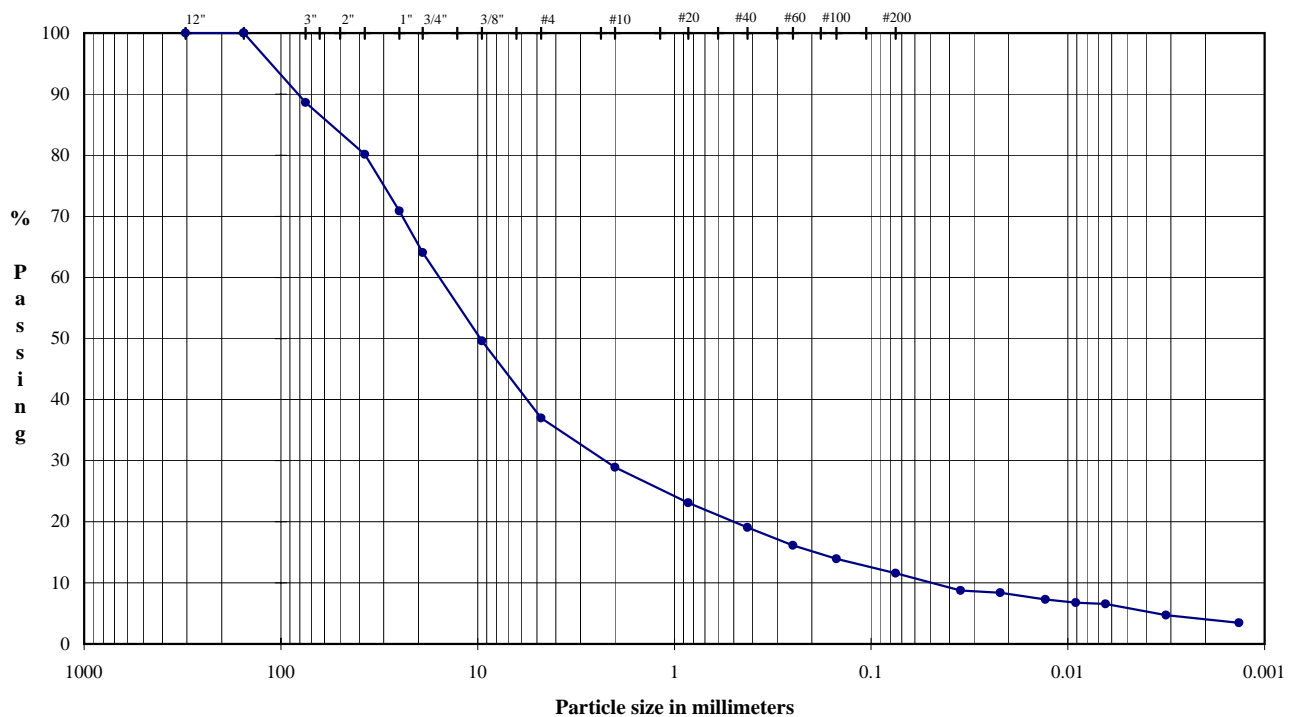
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-02

Depth (ft): 40-42

TYPE: Pail



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

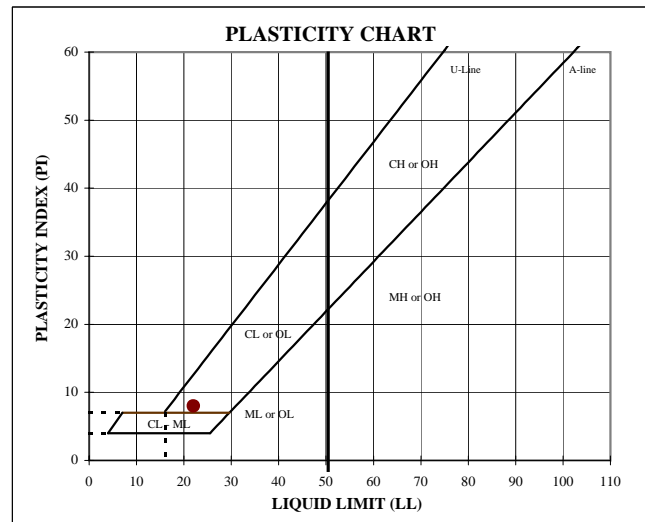
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	11.39
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	88.6	Coarse Gravel	24.55
1.5"	37.5	80.1		
1.0"	25.0	70.9		
0.75"	19.0	64.1		
0.375"	9.5	49.6	Fine Gravel	27.07
#4	4.8	37.0		
#10	2.0	28.9		
#20	0.85	23.1		
#40	0.43	19.1	Medium Sand	9.88
#60	0.25	16.1		
#100	0.15	13.9		
#200	0.075	11.6		
			Fine Sand	7.49

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
--	22	14	8	2.70

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	11.57
0.035	8.7		
0.022	8.4		
0.013	7.3		
0.0091	6.7		
0.0064	6.6		
0.0032	4.7		
0.0014	3.5		

DESCRIPTION: Light gray poorly graded gravel with clay and sand

USCS: GP-GC

TECH DS
DATE 6/20/2005
REVIEW MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

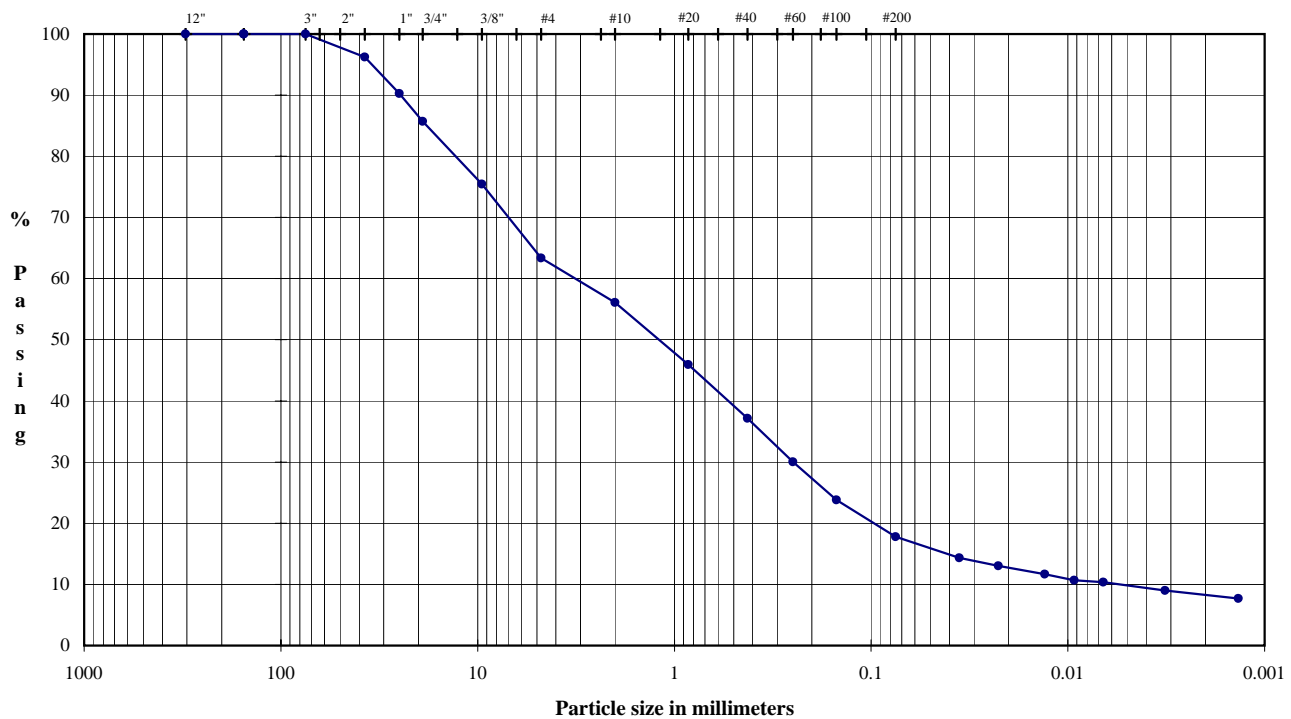
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-02

Depth (ft): 140-142

TYPE: Pail



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

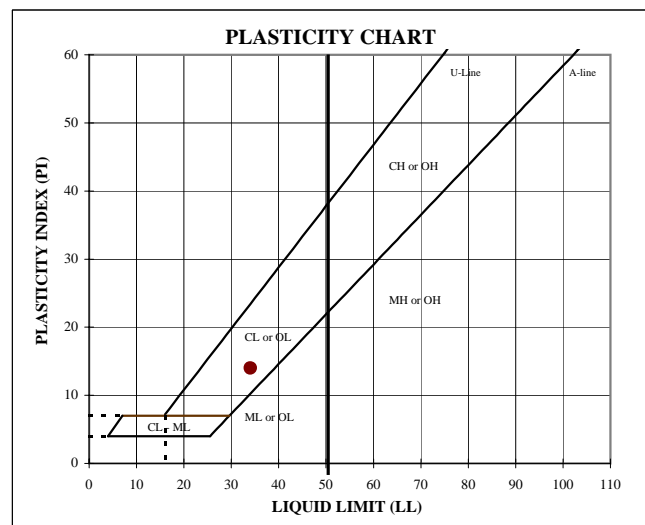
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	14.27
1.5"	37.5	96.2		
1.0"	25.0	90.3		
0.75"	19.0	85.7		
0.375"	9.5	75.5	Fine Gravel	22.34
#4	4.8	63.4		
#10	2.00	56.1		
#20	0.85	46.0		
#40	0.43	37.2	Medium Sand	18.94
#60	0.25	30.0		
#100	0.15	23.8		
#200	0.075	17.8		
			Fine Sand	19.39

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
--	34	20	14	2.70

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	17.77
0.036	14.4		
0.023	13.0		
0.013	11.7		
0.0093	10.7		
0.0066	10.3		
0.0032	9.0		
0.0014	7.7		

DESCRIPTION: Ligth brown clayey sand with gravel

USCS: SC

TECH DS
DATE 6/24/2005
REVIEW MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

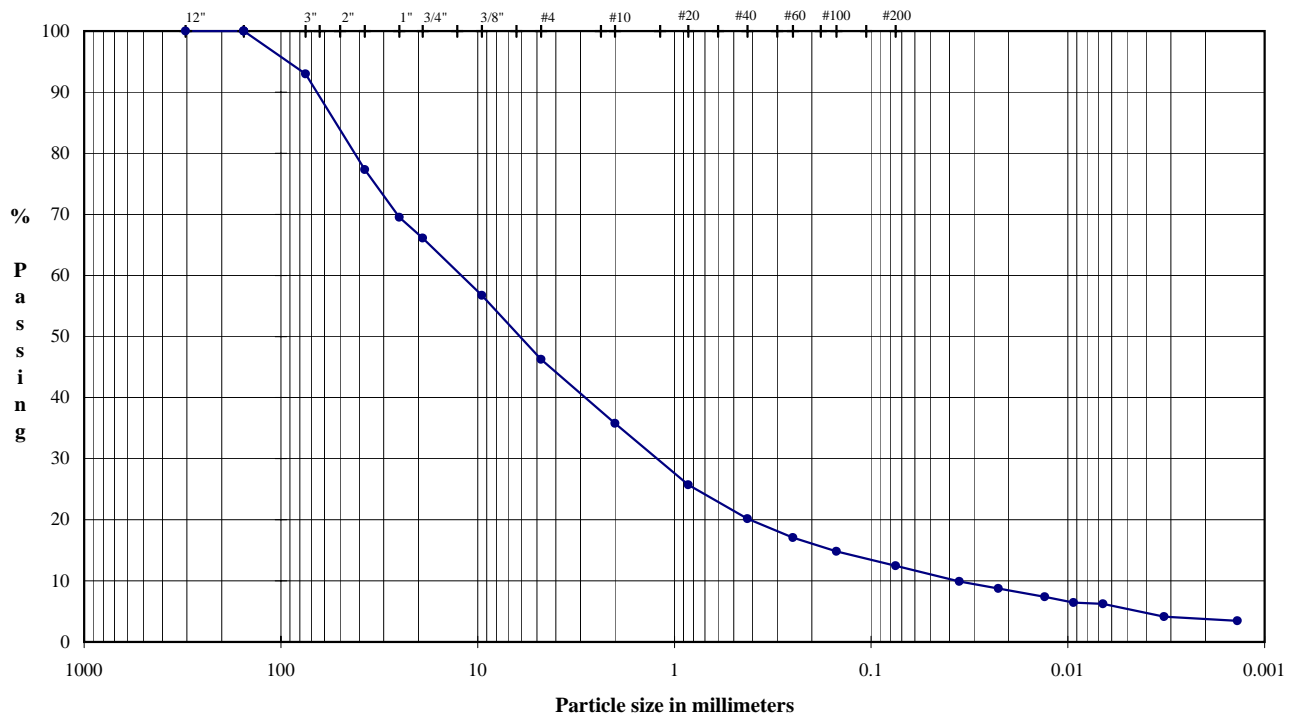
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-02

Depth (ft): 267-268.5

TYPE: Pail



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

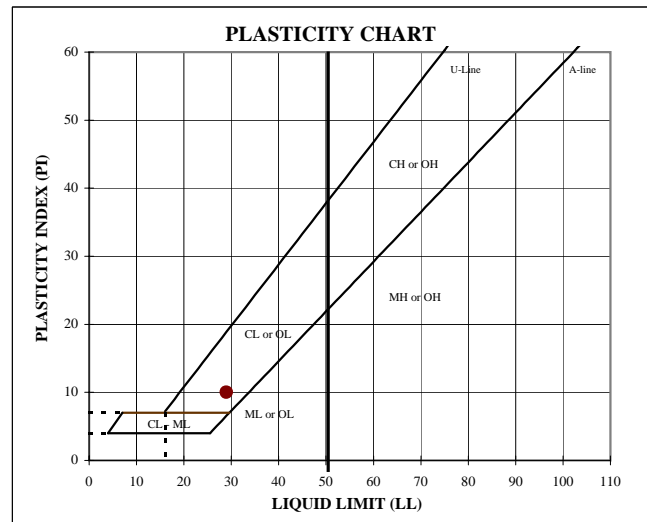
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	7.03
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	93.0	Coarse Gravel	26.84
1.5"	37.5	77.3		
1.0"	25.0	69.5		
0.75"	19.0	66.1		
0.375"	9.5	56.7	Fine Gravel	19.85
#4	4.8	46.3		
#10	2.00	35.8		
#20	0.85	25.7		
#40	0.43	20.1	Medium Sand	15.62
#60	0.25	17.1		
#100	0.15	14.8		
#200	0.075	12.4		
			Fine Sand	7.70

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
--	29	19	10	2.70

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	12.45
0.036	9.9		
0.023	8.8		
0.013	7.4		
0.0093	6.5		
0.0066	6.2		
0.0032	4.2		
0.0014	3.5		

DESCRIPTION: Pink clayey gravel with sand

USCS: GC

TECH DS
DATE 6/20/2005
REVIEW MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

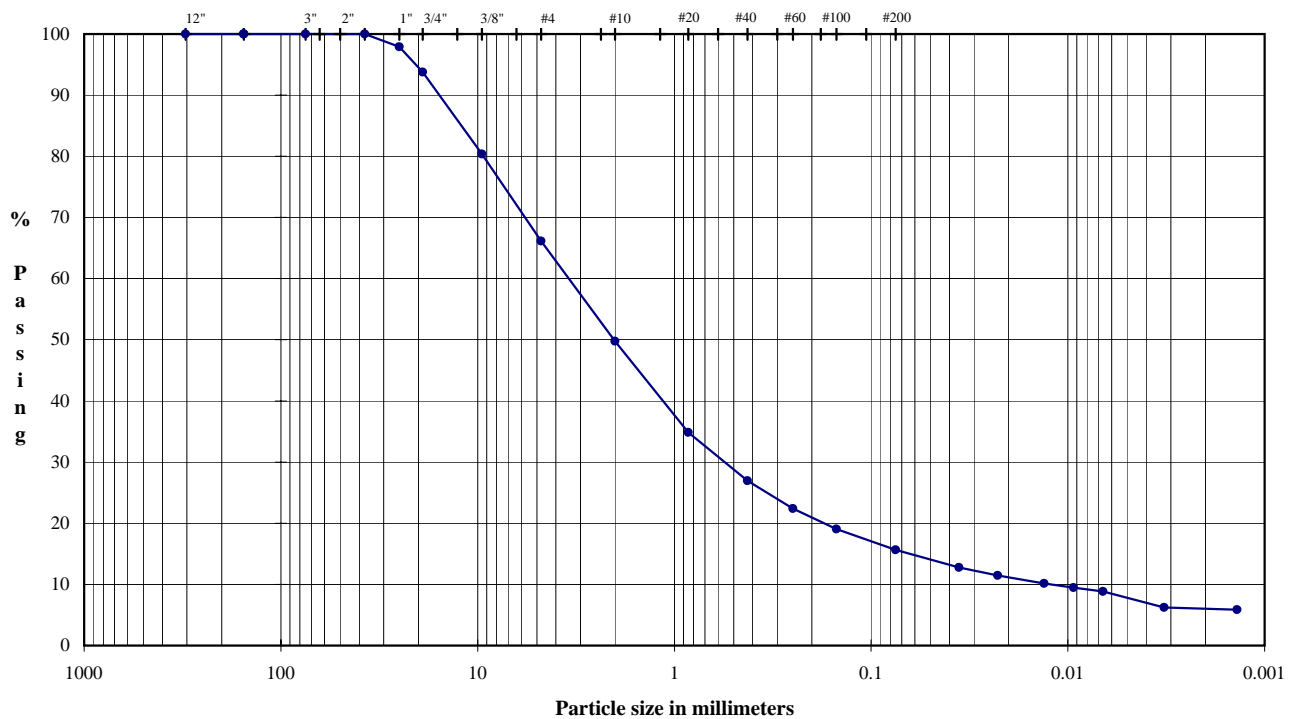
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-03

Depth (ft): 18-20

TYPE: Pail



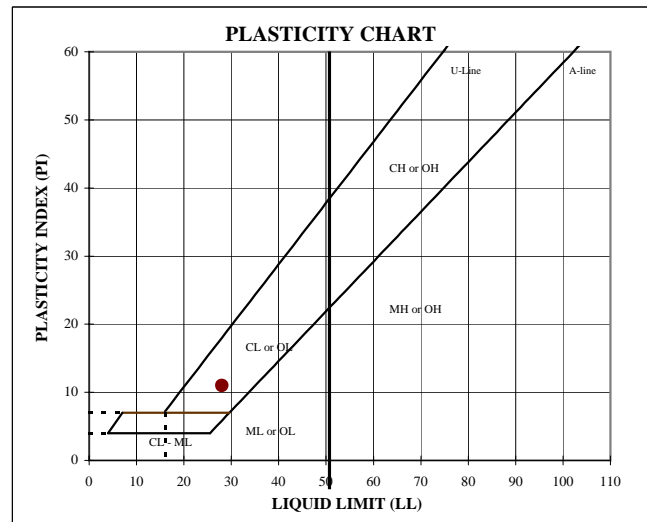
	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	6.20
1.5"	37.5	100.0		
1.0"	25.0	97.9		
0.75"	19.0	93.8		
0.375"	9.5	80.4	Fine Gravel	27.66
#4	4.8	66.1		
#10	2.00	49.8	Coarse Sand	16.38
#20	0.85	34.9	Medium Sand	22.81
#40	0.43	26.9		
#60	0.25	22.4	Fine Sand	11.30
#100	0.15	19.1		
#200	0.075	15.7		

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
--	28	17	11	2.69

Hydrometer Analysis

(mm)	% Finer		
0.036	12.7	Fines Silt or Clay	15.65
0.023	11.4		
0.013	10.1		
0.0093	9.5		
0.0066	8.8		
0.0032	6.2		
0.0014	5.9		

DESCRIPTION: Light brown clay sand with gravel

USCS: SC

TECH: MGC
DATE: 6/27/2005
REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

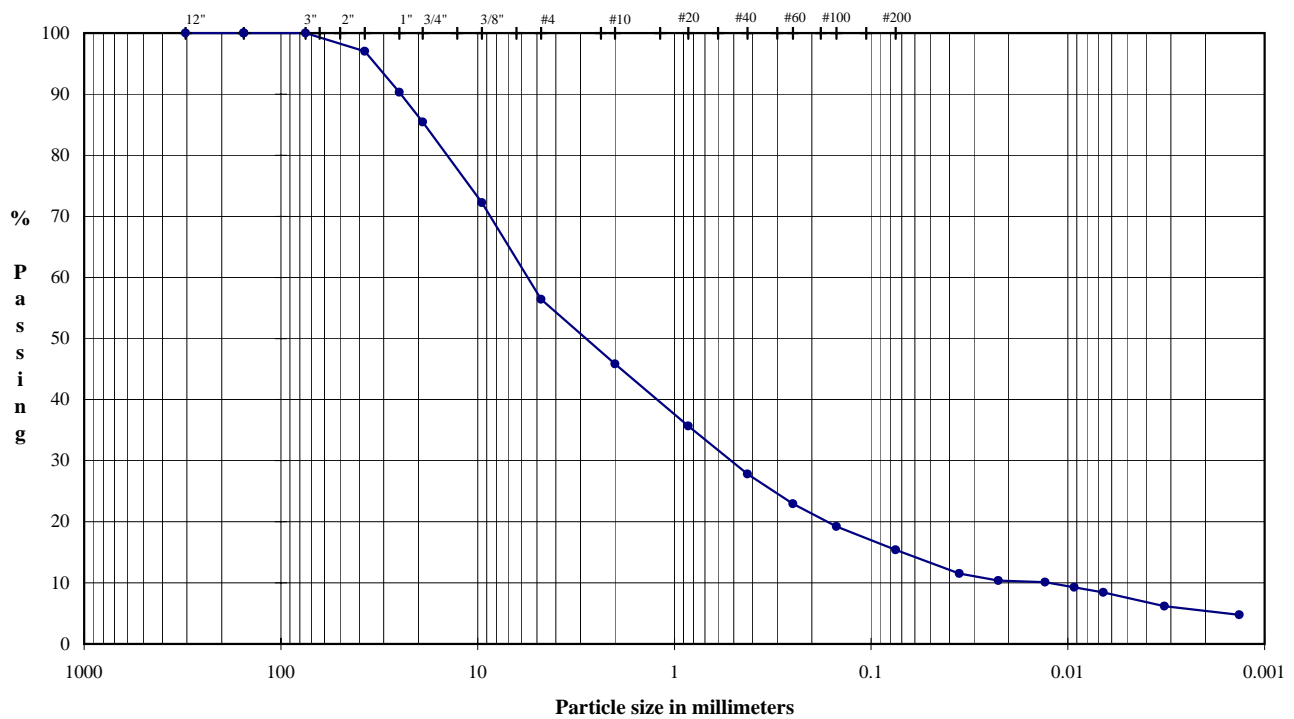
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-03

Depth (ft): 58-60

TYPE: Pail



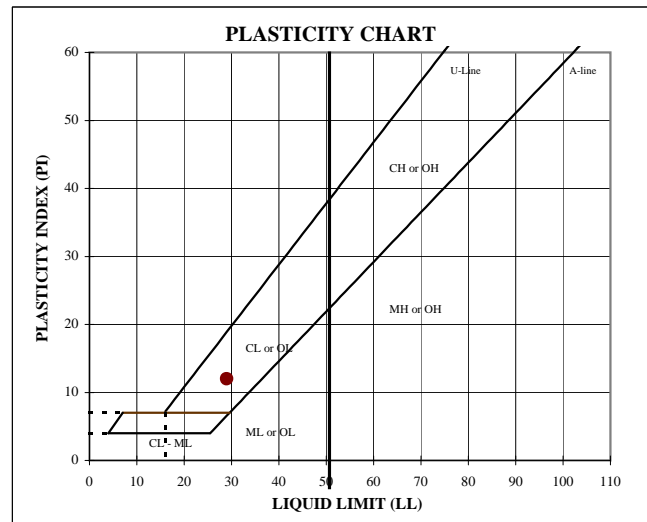
	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	14.55
1.5"	37.5	97.0		
1.0"	25.0	90.3		
0.75"	19.0	85.4		
0.375"	9.5	72.3	Fine Gravel	29.04
#4	4.8	56.4		
#10	2.00	45.8	Coarse Sand	10.57
#20	0.85	35.6	Medium Sand	18.01
#40	0.43	27.8		
#60	0.25	22.9	Fine Sand	12.41
#100	0.15	19.2		
#200	0.075	15.4		

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
--	29	17	12	2.70

Hydrometer Analysis

(mm)	% Finer		
0.036	11.5	Fines Silt or Clay	15.41
0.023	10.4		
0.013	10.1		
0.0093	9.3		
0.0066	8.4		
0.0032	6.2		
0.0013	4.8		

DESCRIPTION: Light yellowish brown clayey gravel with sand

USCS: GC

TECH: MGC
DATE: 6/27/2005
REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

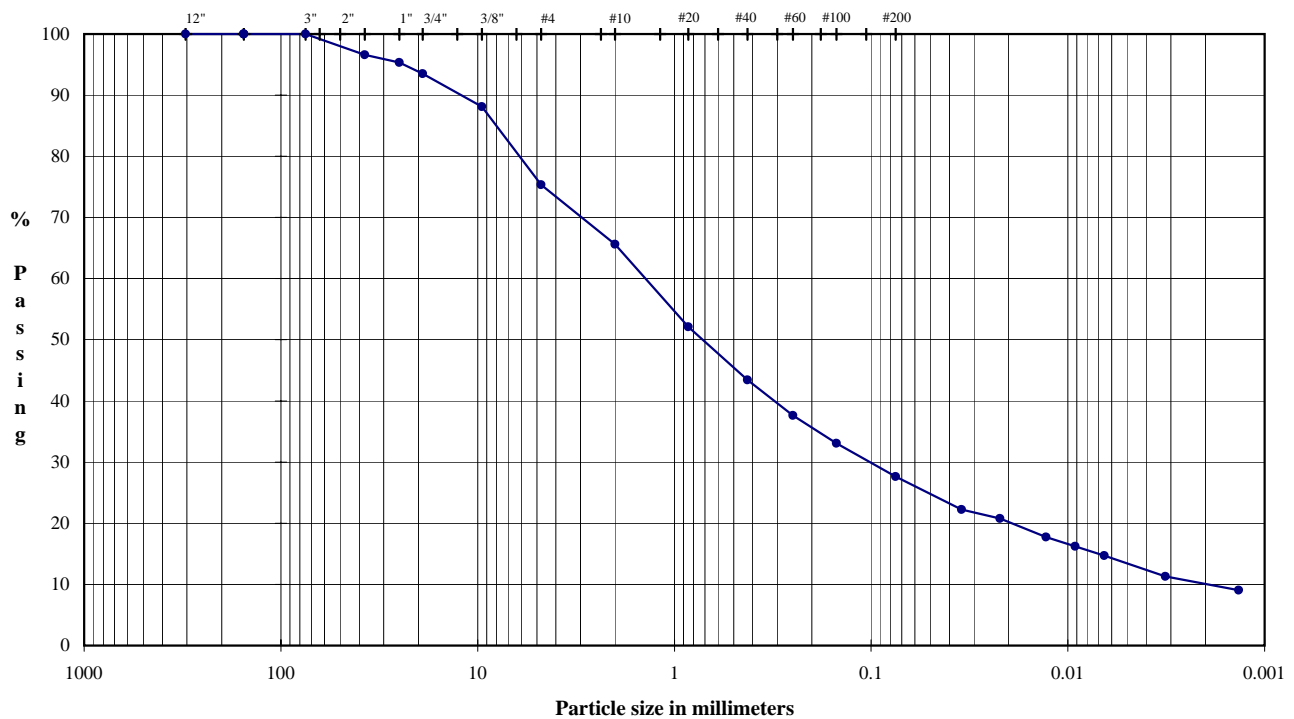
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-03

Depth (ft): 156-158

TYPE: Pail



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

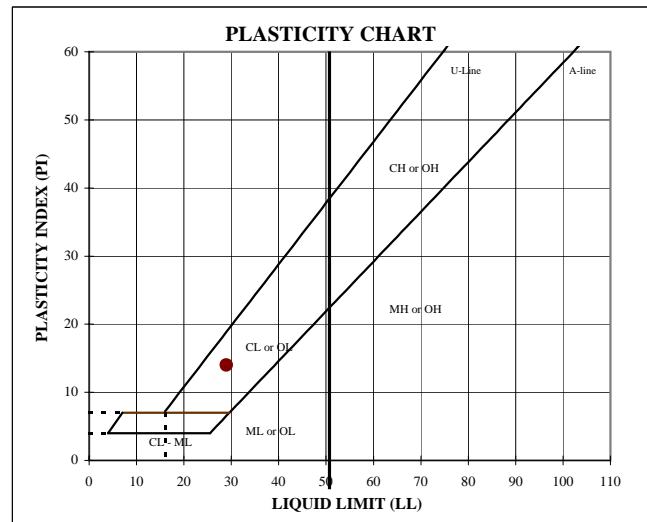
Particle Size

U.S. Standard Sieves Sizes and Numbers

	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	6.47
1.5"	37.5	96.6		
1.0"	25.0	95.3		
0.75"	19.0	93.5		
0.375"	9.5	88.1	Fine Gravel	18.18
#4	4.8	75.3		
#10	2.00	65.6	Coarse Sand	9.71
#20	0.85	52.1	Medium Sand	22.18
#40	0.43	43.5		
#60	0.25	37.6	Fine Sand	15.84
#100	0.15	33.1		
#200	0.075	27.6		

Hydrometer Analysis

(mm)	% Finer		
0.035	22.3	Fines Silt or Clay	27.62
0.022	20.8		
0.013	17.7		
0.0092	16.2		
0.0065	14.7		
0.0032	11.3		
0.0014	9.1		



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
--	29	15	14	2.70

DESCRIPTION: Light brown clayey sand with gravel

USCS: SC

TECH	DS
DATE	6/24/2005
REVIEW	MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

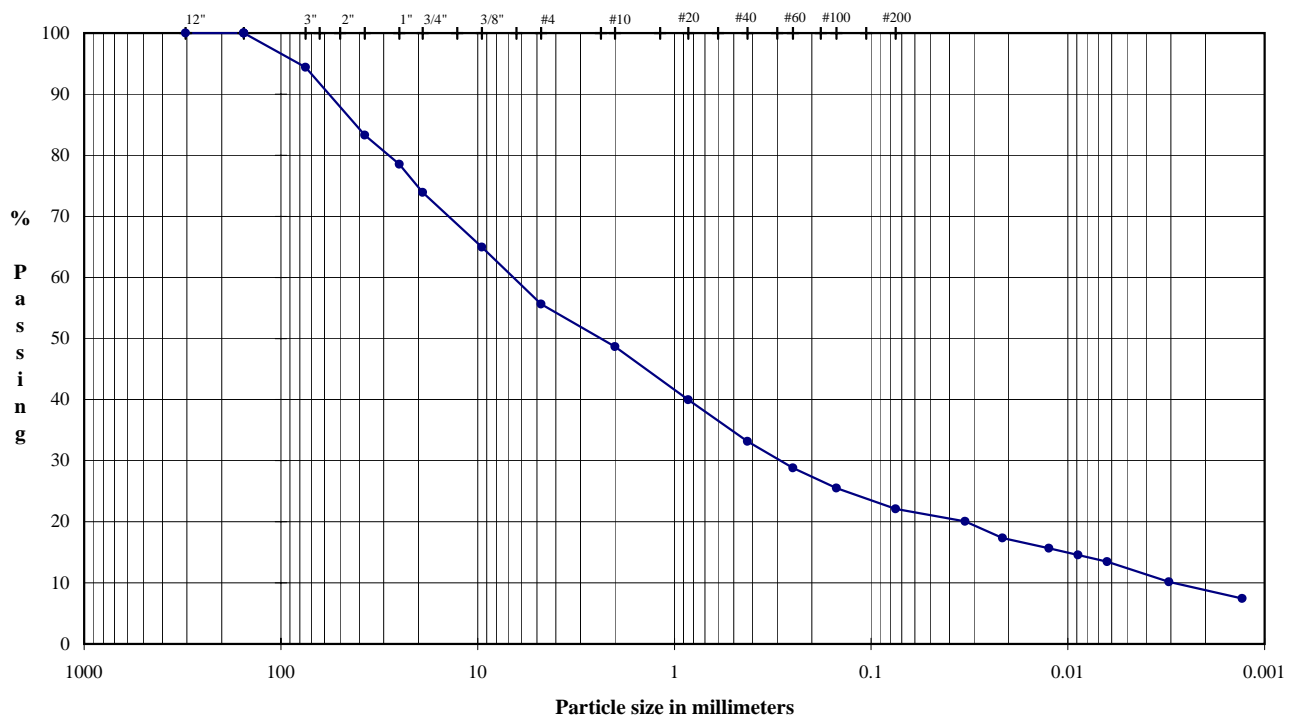
ASTM D421, D422, D4318

PROJECT NAME: **G&K/Tyrone Stockpile Geotech/AZ**

SAMPLE ID: **TSGT-03**

Depth (ft): **248-250**

TYPE: **Pail**



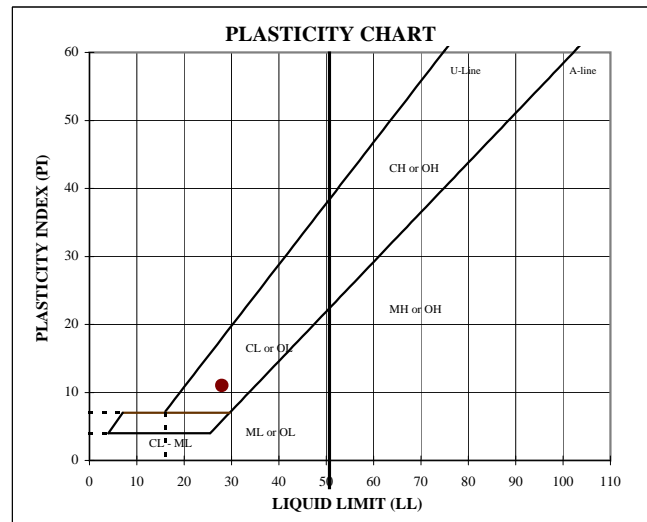
	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	5.58
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	94.4	Coarse Gravel	20.50
1.5"	37.5	83.3		
1.0"	25.0	78.5		
0.75"	19.0	73.9		
0.375"	9.5	64.9	Fine Gravel	18.28
#4	4.8	55.6		
#10	2.0	48.7	Coarse Sand	6.98
#20	0.85	40.0	Medium Sand	15.51
#40	0.43	33.1		
#60	0.25	28.8	Fine Sand	11.05
#100	0.15	25.5		
#200	0.075	22.1		

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
--	28	17	11	2.78

Hydrometer Analysis

(mm)	% Finer		
0.033	20.1	Fines Silt or Clay	22.10
0.021	17.3		
0.012	15.7		
0.0089	14.6		
0.0063	13.5		
0.0031	10.2		
0.0013	7.4		

DESCRIPTION: Pale pink clayey gravel with sand

USCS: GC

TECH: MGC
DATE: 6/27/2005
REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

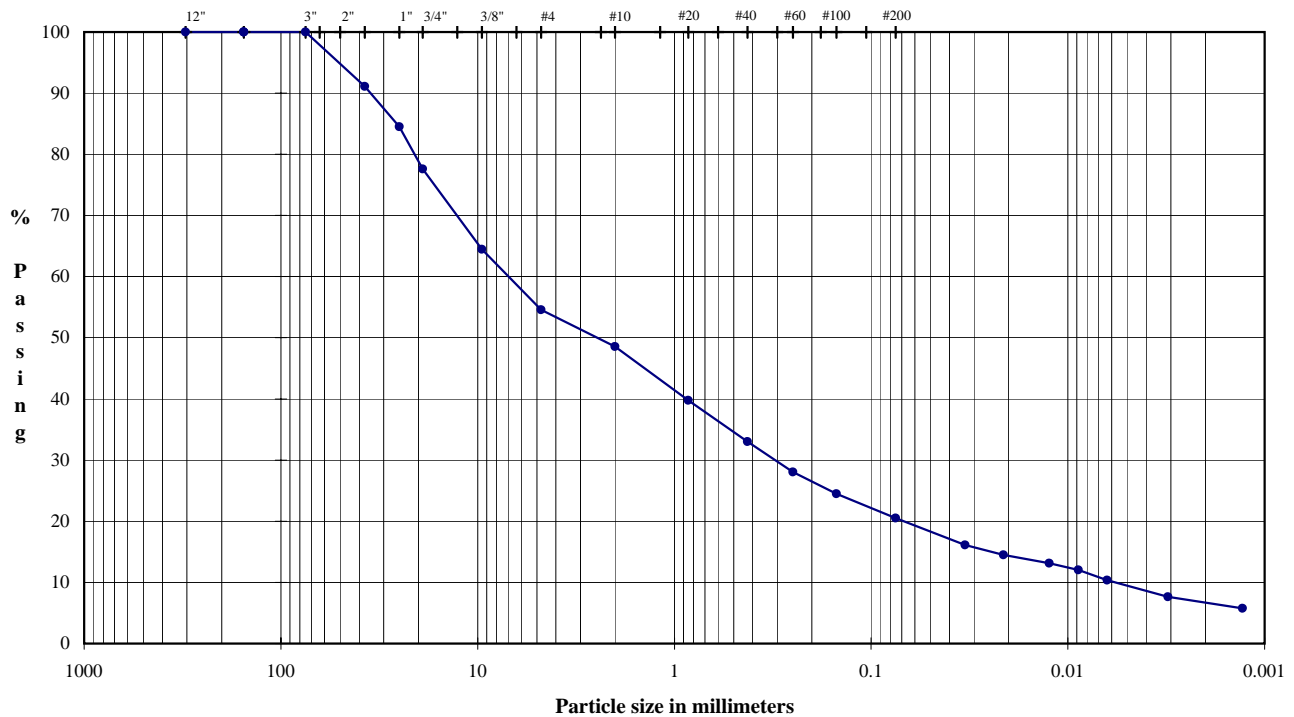
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-04

Depth (ft): 12-14

TYPE: Pail



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

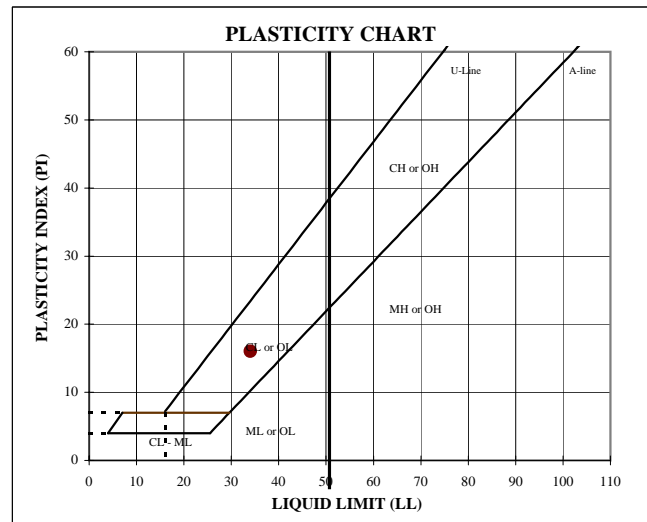
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	22.39
1.5"	37.5	91.1		
1.0"	25.0	84.5		
0.75"	19.0	77.6		
0.375"	9.5	64.5	Fine Gravel	23.04
#4	4.8	54.6		
#10	2.00	48.6		
#20	0.85	39.8		
#40	0.43	33.0	Medium Sand	15.55
#60	0.25	28.1		
#100	0.15	24.5		
#200	0.075	20.5		

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
--	34	18	16	2.83

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	20.53
0.033	16.1		
0.021	14.5		
0.012	13.1		
0.0089	12.0		
0.0063	10.4		
0.0031	7.7		
0.0013	5.7		

DESCRIPTION: Pale yellow clayey gravel with sand

USCS: GC

 TECH: MGC
 DATE: 6/29/2005
 REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

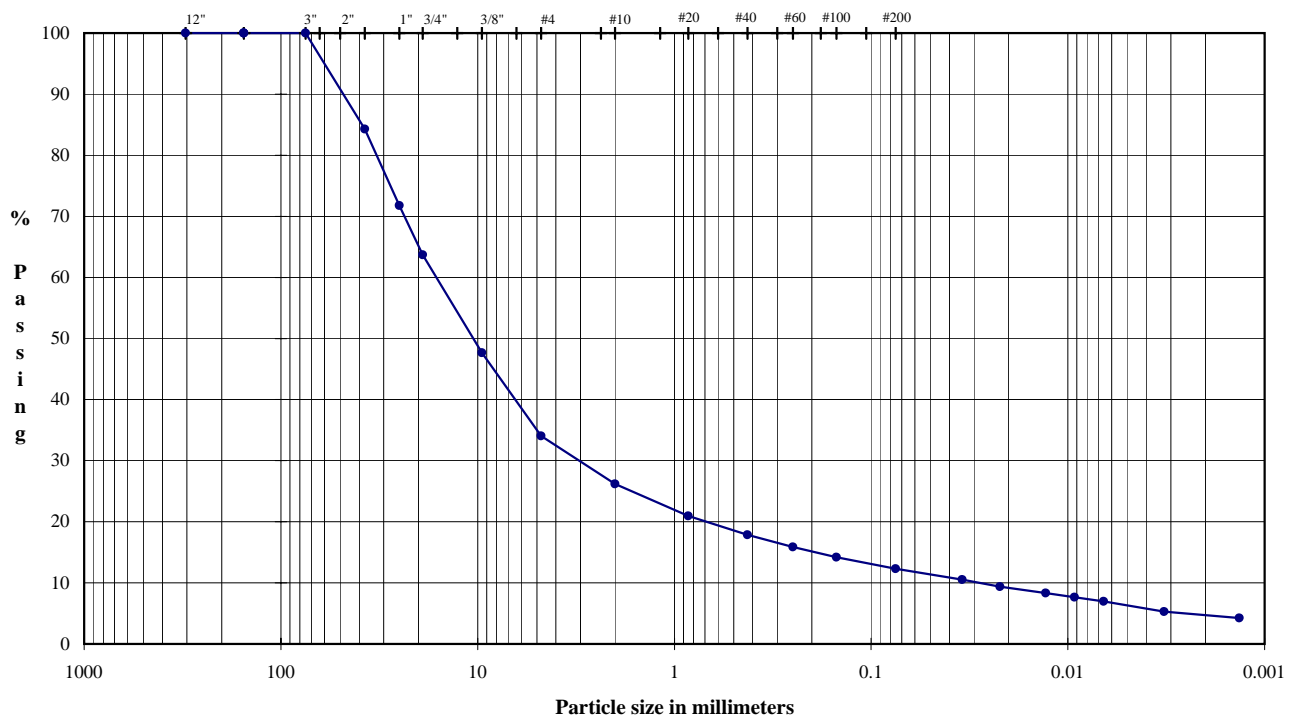
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-04

Depth (ft): 69-71

TYPE: Pail



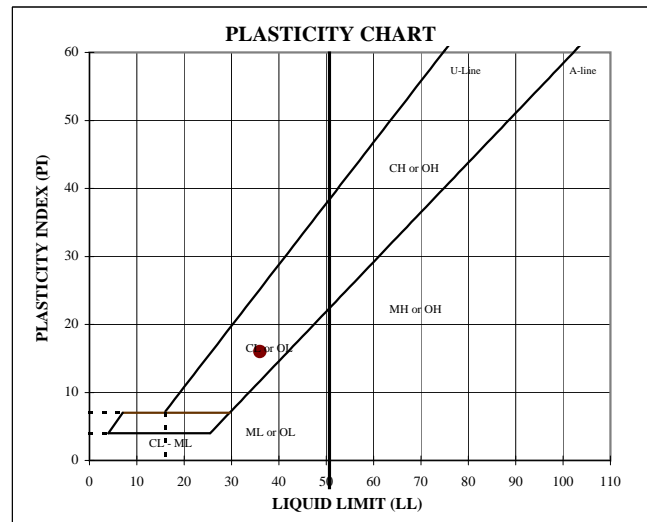
	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	36.30
1.5"	37.5	84.3		
1.0"	25.0	71.8		
0.75"	19.0	63.7		
0.375"	9.5	47.7	Fine Gravel	29.63
#4	4.8	34.1		
#10	2.00	26.2		
#20	0.85	21.0		
#40	0.43	17.9	Medium Sand	8.31
#60	0.25	15.9		
#100	0.15	14.2	Fine Sand	5.56
#200	0.075	12.3		

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
--	36	20	16	2.70

Hydrometer Analysis

(mm)	% Finer		
0.034	10.5	Fines Silt or Clay	12.29
0.022	9.4		
0.013	8.3		
0.0092	7.7		
0.0066	7.0		
0.0032	5.3		
0.0013	4.3		

DESCRIPTION: Pink clayey gravel with sand

USCS: GC

 TECH DS
 DATE 6/28/2005
 REVIEW MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

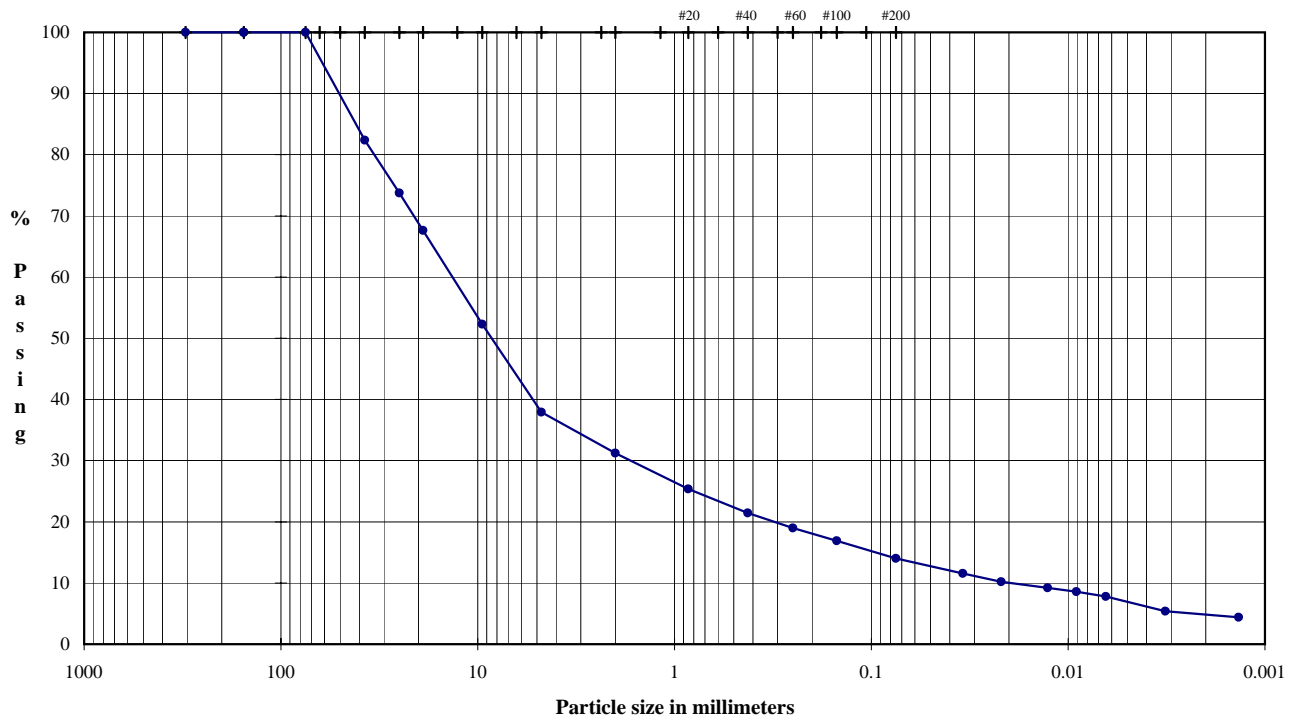
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-04

Depth (ft): 117-119

TYPE: Pail



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

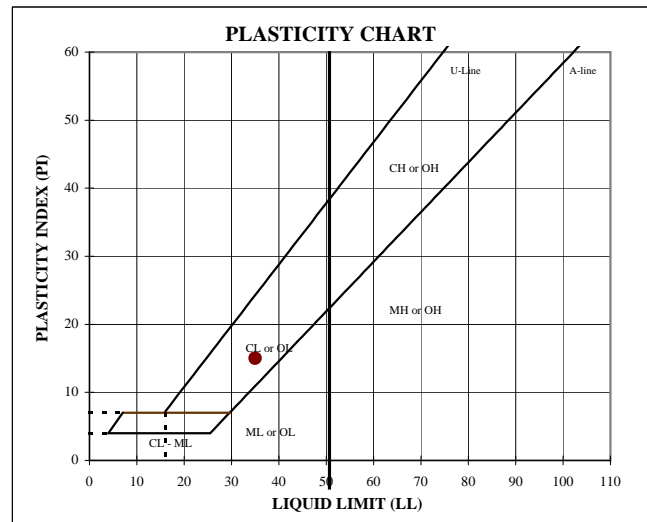
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	32.35
1.5"	37.5	82.4		
1.0"	25.0	73.8		
0.75"	19.0	67.6		
0.375"	9.5	52.3	Fine Gravel	29.70
#4	4.8	37.9		
#10	2.00	31.2		
#20	0.85	25.3		
#40	0.43	21.5	Medium Sand	9.75
#60	0.25	19.0		
#100	0.15	16.9		
#200	0.075	14.0		
			Fine Sand	7.45

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
--	35	20	15	2.73

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	14.02
0.034	11.6		
0.022	10.2		
0.013	9.2		
0.0091	8.6		
0.0064	7.8		
0.0032	5.4		
0.0014	4.4		

DESCRIPTION: Yellow clayey gravel with sand

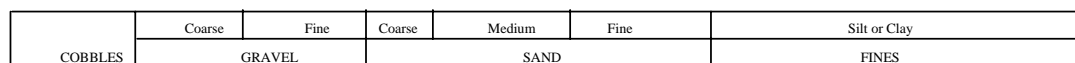
USCS: GC

 TECH: MGC
 DATE: 6/27/2005
 REVIEW: MB

ASTM D421, D422, D4318

Depth (ft): **152-154**

TYPE: **Pail**



PLASTICITY CHART

The chart plots Plasticity Index (PI) on the Y-axis (0 to 60) against Liquid Limit (LL) on the X-axis (0 to 110). Key lines and regions are labeled:

- U-Line** and **A-Line** are the upper boundary lines.
- CH or OH** is the region above the U-Line.
- MH or OH** is the region between the U-Line and A-Line.
- CL or OH** is the region between the A-Line and the U-Line.
- ML or OL** is the region below the A-Line.
- CL^L, ML^L** is the region below the U-Line and above the A-Line.

A red dot is plotted at approximately (32, 13), which falls within the **CL or OH** region.

ATTERBERG LIMITS				
M _c	LL	PL	PI	SpG
--	31	18	13	2.66

USCS:	GC
-------	----

TECH	MGC
DATE	6/27/2005
REVIEW	MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

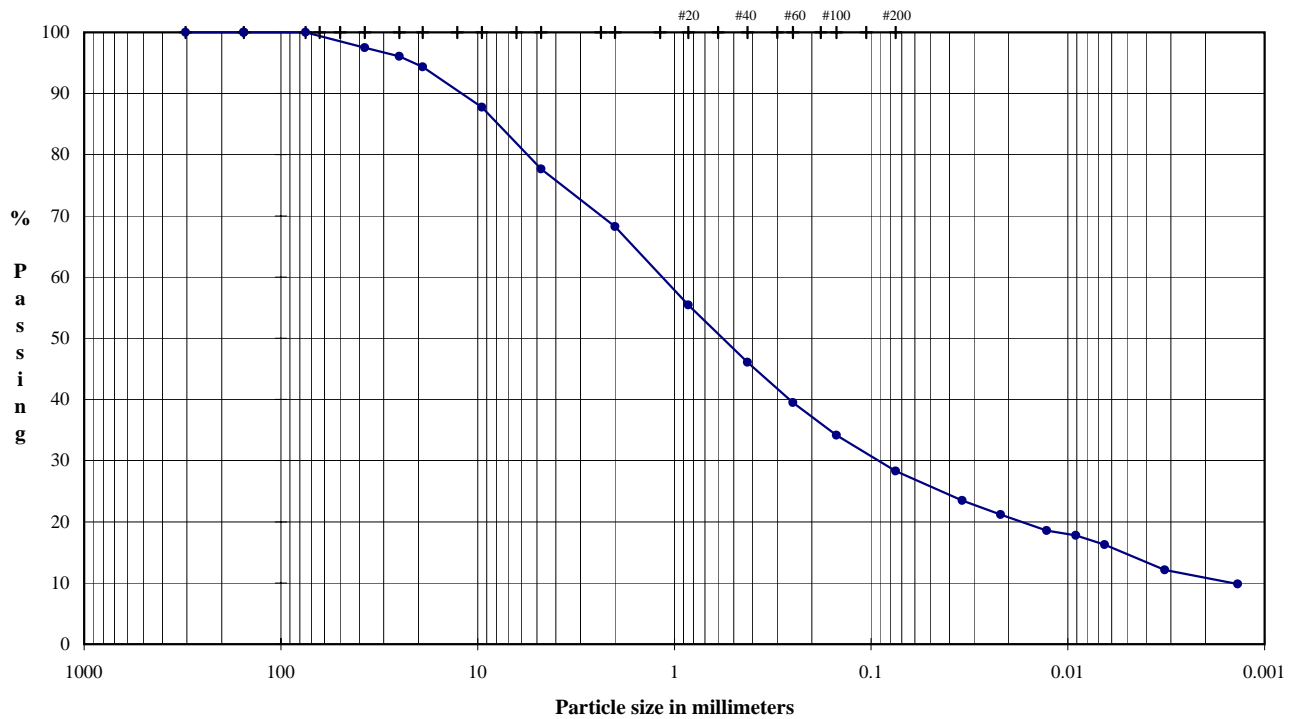
ASTM D421, D422, D4318

PROJECT NAME: **G&K/Tyrone Stockpile Geotech/AZ**

SAMPLE ID: **TSGT-04**

Depth (ft): **183-185**

TYPE: **Pail**



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

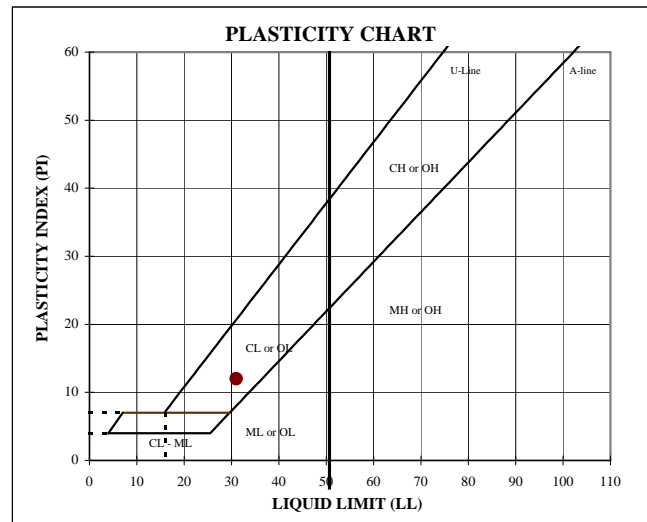
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0		
1.5"	37.5	97.5	Coarse Gravel	5.66
1.0"	25.0	96.1		
0.75"	19.0	94.3		
0.375"	9.5	87.8	Fine Gravel	16.69
#4	4.8	77.7		
#10	2.00	68.2		
#20	0.85	55.5	Coarse Sand	9.42
#40	0.43	46.1	Medium Sand	22.17
#60	0.25	39.5		
#100	0.15	34.2	Fine Sand	17.74
#200	0.075	28.3		

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
--	31	19	12	2.71

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	28.32
0.034	23.5		
0.022	21.2		
0.013	18.6		
0.0091	17.8		
0.0065	16.3		
0.0032	12.1		
0.0014	9.8		

DESCRIPTION: Pink clayey sand with gravel

USCS: SC

TECH: MGC
DATE: 6/27/2005
REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

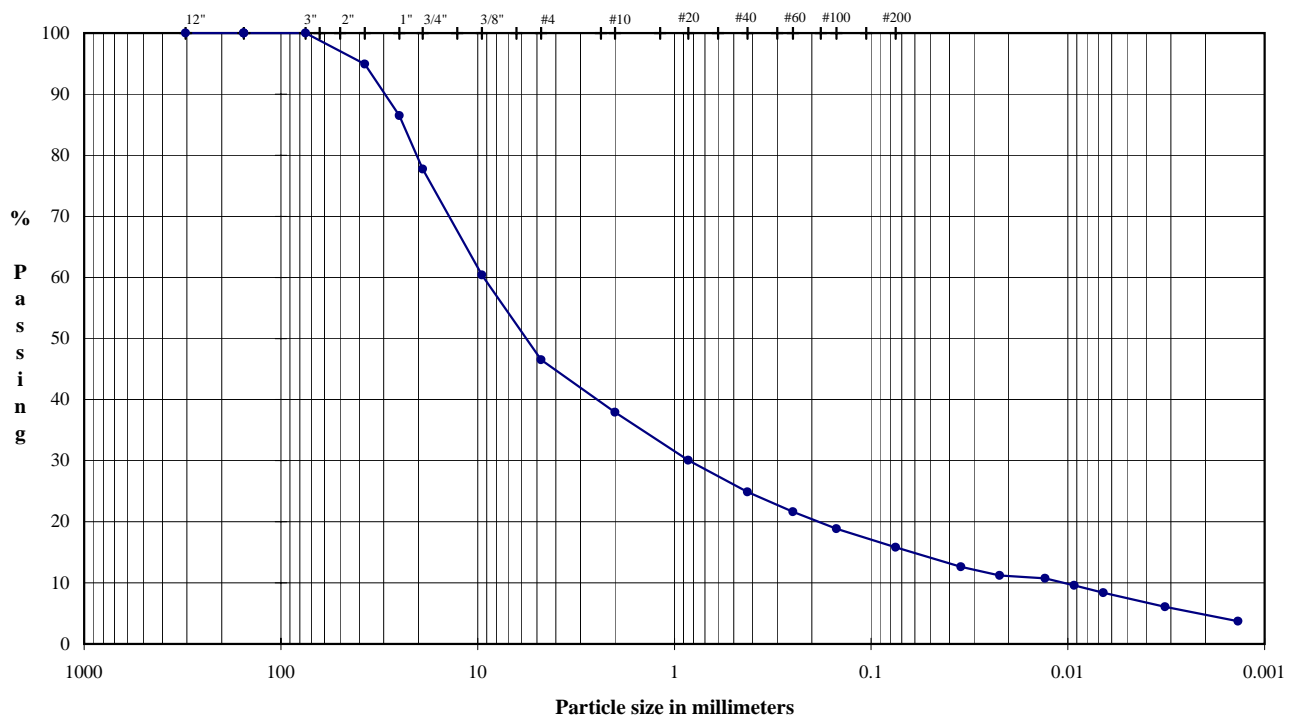
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: TSGT-04

Depth (ft): 221-223

TYPE: Pail



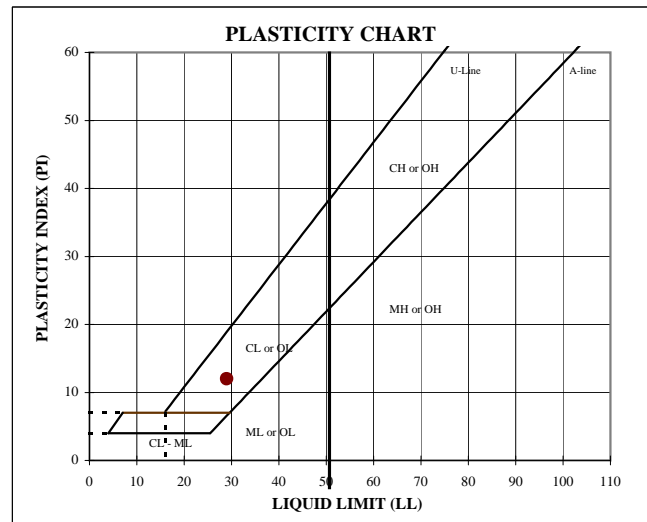
	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	22.29
1.5"	37.5	94.9		
1.0"	25.0	86.5		
0.75"	19.0	77.7		
0.375"	9.5	60.4	Fine Gravel	31.17
#4	4.8	46.5		
#10	2.00	37.9	Coarse Sand	8.61
#20	0.85	30.1	Medium Sand	13.03
#40	0.43	24.9		
#60	0.25	21.6	Fine Sand	9.08
#100	0.15	18.8		
#200	0.075	15.8		

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
--	29	17	12	2.70

Hydrometer Analysis

(mm)	% Finer		
0.035	12.6	Fines Silt or Clay	15.83
0.022	11.2		
0.013	10.7		
0.0093	9.6		
0.0066	8.4		
0.0032	6.1		
0.0014	3.7		

DESCRIPTION: Very pale brown clayey gravel with sand

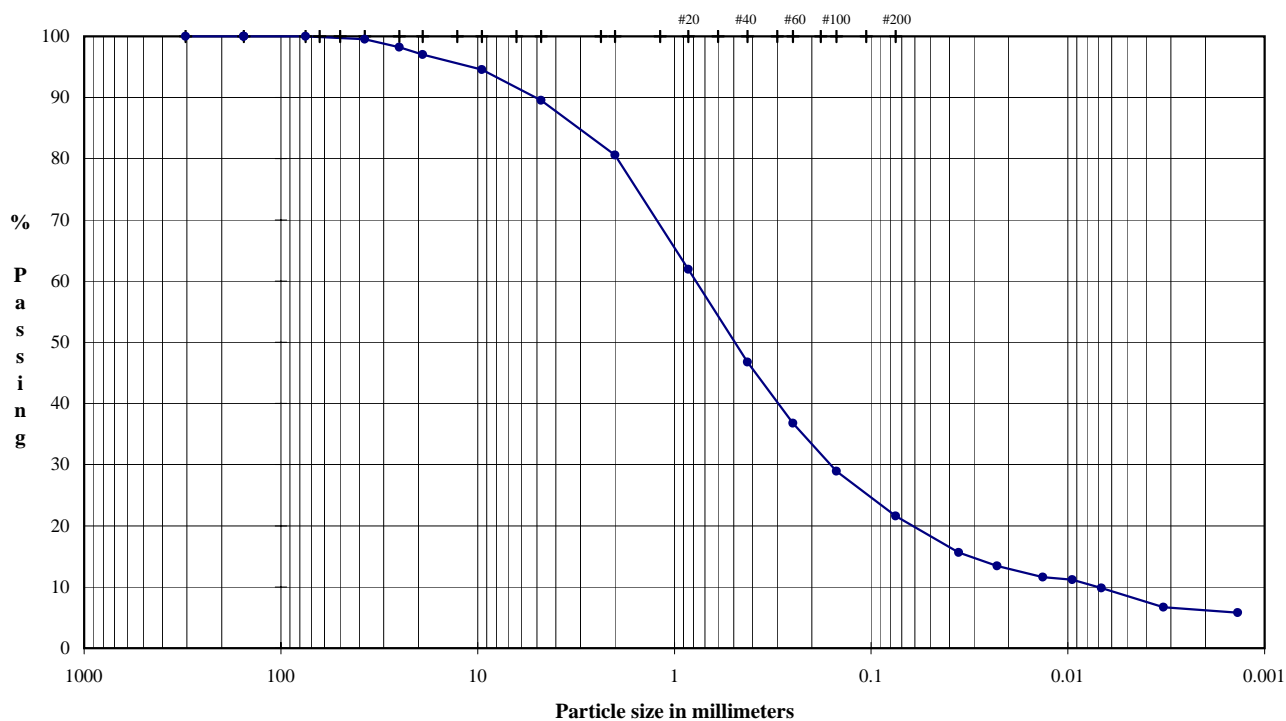
USCS: GC

TECH: MGC
DATE: 6/29/2005
REVIEW: MB

ASTM D421, D422, D4318

Depth (ft): **265-269**

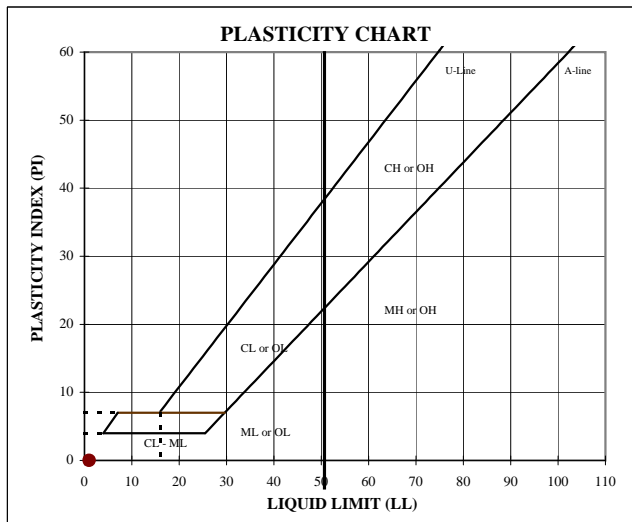
TYPE: **Pail**



COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			FINES

Particle Size			Particle Size	
	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	0.00
6.0"	154.2	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0	Coarse Gravel	2.97
3.0"	75.0	100.0		
1.5"	37.5	99.5		
1.0"	25.0	98.2		
0.75"	19.0	97.0		
0.375"	9.5	94.6		
#4	4.8	89.6	Fine Gravel	7.48
#10	2.00	80.6	Coarse Sand	8.97
#20	0.85	61.9	Medium Sand	33.83
#40	0.43	46.8		
#60	0.25	36.8		
#100	0.15	28.9		
#200	0.075	21.6	Fine Sand	25.15

Hydrometer Analysis	(mm)	% Finer	Fines Silt or Clay	21.60
	0.036	15.7		
	0.023	13.4		
	0.013	11.6		
	0.0095	11.2		
	0.0068	9.8		
	0.0033	6.7		
	0.0014	5.8		



M _c	LL	PL	PI	SpG (assumed)
--	NP	NP	NP	2.70

USCS:	SM
-------	----

TECH	DS
DATE	6/28/2005
REVIEW	MB

APPENDIX C-5

**2005 SOIL SUMMARY AND
GRAIN-SIZE ANALYSIS**

TABLE C-5.1
SUMMARY OF SOIL DATA

Sample Type	Sample Number	Sample Depth (feet)	USCS Soil Classification	Delivered Moisture (%)	Atterberg Limits			Grain-size Distribution			Specific Gravity	Moisture/Density Relationship Standard Proctor		Additional Tests Comments (see Notes)
					LL	PL	PI	% Finer 3/4"	% Finer #4	% Finer #200		PCF (Dry)	Moist (%)	
Bag	T3	11.0-12.5	GC	10.6	32	17	15	71	41	16	2.66	--	--	
Bag	T3	31.0-32.5	GC	22.5	34	18	16	69	56	28	2.76	--	--	
Bag	T3	51.0-52.5	GC	11.8	31	19	12	71	55	21	2.84	--	--	
Bag	T3	71.0-72.5	SC	12.8	33	20	13	91	76	35	2.87	--	--	
Bag	T3	76.5-77.5	SC-SM	4.3	23	16	7	88	67	28	2.85	--	--	Bag labeled T5@76.5-77.5
Bag	T3	91.0-92.5	GC	8.6	31	18	13	79	62	26	2.74	--	--	
Bag	T4	49.0-50.0	GC	6.8	31	17	14	77	55	22	2.80	--	--	
Bag	T4	69.0-70.0	SC	6.9	31	16	15	86	63	25	2.81	--	--	
Bag	T4	89.0-90.0	SC	9.5	36	20	16	86	66	30	2.93	--	--	
Bag	T4	109.0-110.0	GC	12.0	35	20	15	74	55	24	2.79	--	--	
Bag	T4	113.0-114.0	SC	17.7	36	19	17	97	76	33	2.86	--	--	
Bag	T4	129.0-130.0	GC	9.3	34	23	11	72	49	24	2.91	--	--	
Bag	T5	16.5-17.5	GM	5.9	NP	NP	NP	69	49	15	2.78	--	--	
Bag	T5	36.5-37.5	GC	10.2	30	16	14	56	37	13	2.81	--	--	
Bag	T5	56.5-57.5	GC	8.7	34	18	16	58	38	13	2.71	--	--	
Bag	T5	96.5-97.5	GC	12.6	37	21	16	71	55	20	2.75	--	--	
Bag	T5	116.5-117.5	GC	8.2	33	19	14	84	62	27	2.81	--	--	
Bag	T5	136.5-137.5	GC	9.6	33	18	15	67	47	19	2.80	--	--	
Bag	T5	164.0-165.0	GC	14.0	35	17	18	75	53	20	2.85	--	--	

Notes:

LL = Liquid Limit

PL = Plastic Limit

PI = Plastic Index

USCS = Unified Soil Classification System

PCF = pounds per cubic foot

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

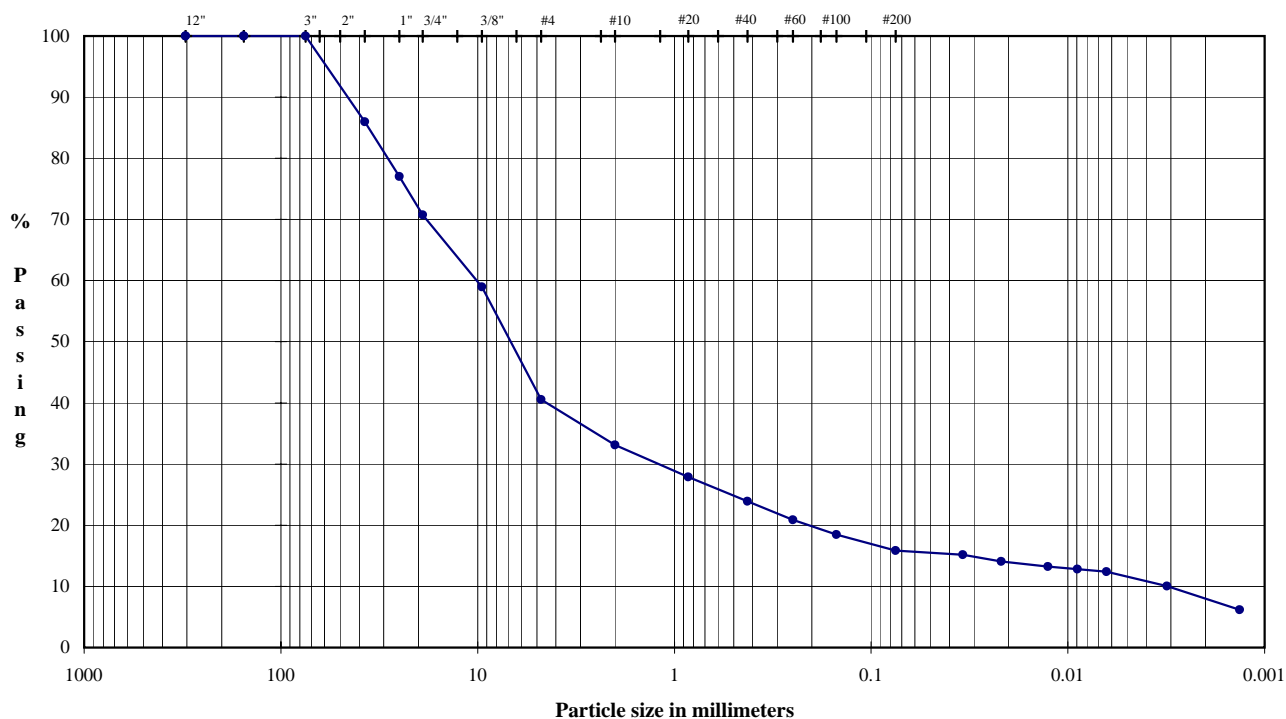
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: T-3

Depth: 11.0-12.5

TYPE: Bag



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

% Passing

Classification

Percentage

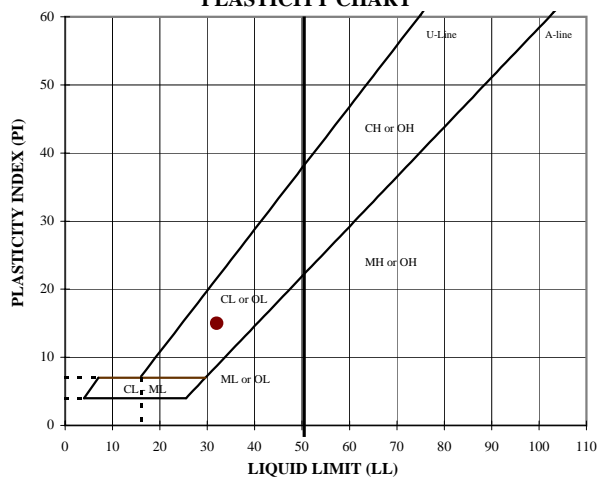
U.S. Standard Sieves Sizes and Numbers

12.0"	304.8	100.0	Cobbles	0.00
12.0"	304.8	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	29.23
1.5"	37.5	86.0		
1.0"	25.0	77.0		
0.75"	19.0	70.8		
0.375"	9.5	59.0	Fine Gravel	30.22
#4	4.8	40.5		
#10	2.00	33.1		
#20	0.85	27.9		
#40	0.43	23.9	Medium Sand	9.19
#60	0.25	20.9		
#100	0.15	18.5		
#200	0.075	15.8		
			Fine Sand	8.07

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	15.85
0.034	15.2		
0.022	14.1		
0.013	13.2		
0.0089	12.8		
0.0064	12.4		
0.0031	10.0		
0.0013	6.2		

PLASTICITY CHART



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
10.6	32	17	15	2.66

DESCRIPTION: Weak red clayey gravel with sand

USCS: GC

TECH: SW
DATE: 9/29/2005
REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

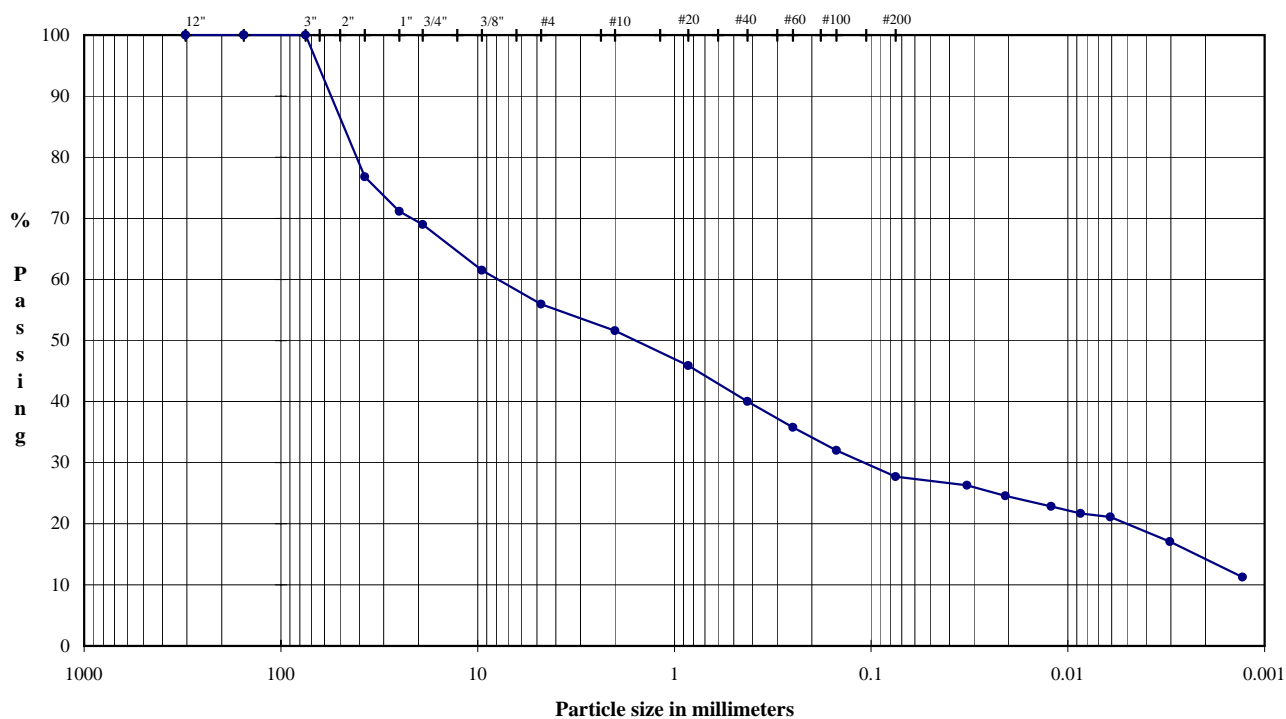
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: T-3

Depth: 31.0-32.5

TYPE: Bag



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

% Passing

Classification

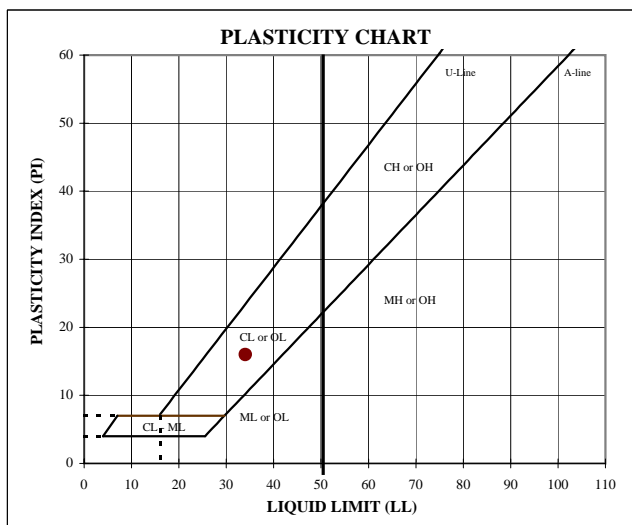
Percentage

U.S. Standard Sieves Sizes and Numbers

12.0"	304.8	100.0	Cobbles	0.00
12.0"	304.8	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	31.01
1.5"	37.5	76.8		
1.0"	25.0	71.1		
0.75"	19.0	69.0		
0.375"	9.5	61.5	Fine Gravel	13.04
#4	4.8	55.9		
#10	2.00	51.6		
#20	0.85	45.9		
#40	0.43	40.0	Medium Sand	11.57
#60	0.25	35.8		
#100	0.15	32.0		
#200	0.075	27.7		
			Fine Sand	12.32

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	27.72
0.033	26.3		
0.021	24.6		
0.012	22.8		
0.0086	21.7		
0.0061	21.1		
0.0030	17.1		
0.0013	11.3		



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
22.5	34	18	16	2.76

DESCRIPTION: Weak red clayey gravel with sand

USCS: GC

TECH	MKS
DATE	10/11/2005
REVIEW	MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

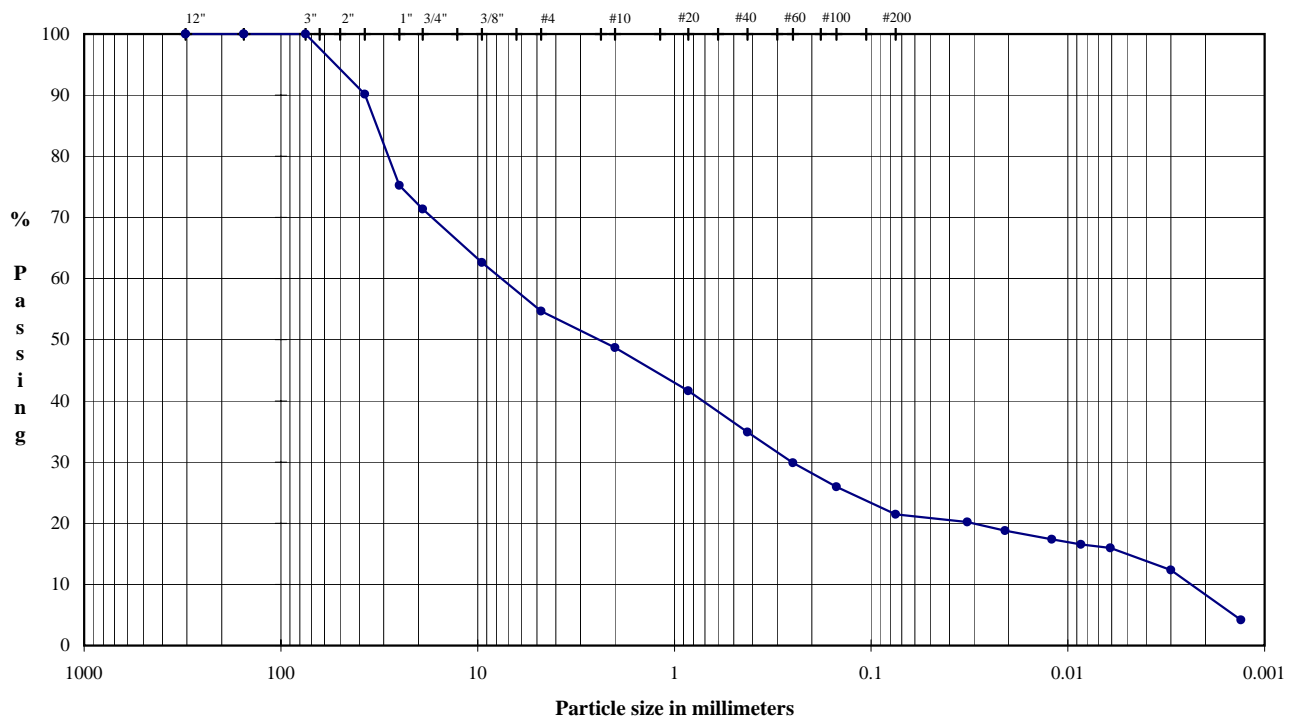
ASTM D421, D422, D4318

PROJECT NAME: **G&K/Tyrone Stockpile Geotech/AZ**

SAMPLE ID: **T-3**

Depth: **51.0-52.5**

TYPE: **Bag**



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

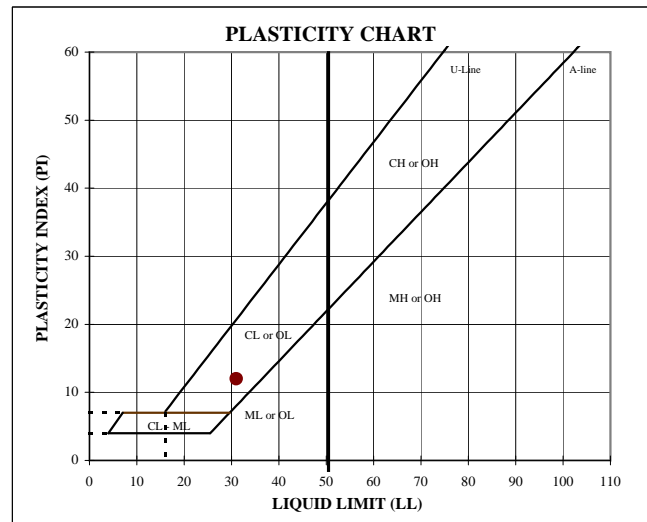
Particle Size

U.S. Standard Sieves Sizes and Numbers

	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	0.00
12.0"	304.8	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0		
1.5"	37.5	90.2	Coarse Gravel	28.64
1.0"	25.0	75.2		
0.75"	19.0	71.4		
0.375"	9.5	62.6		
#4	4.8	54.7	Fine Gravel	16.68
#10	2.00	48.7	Coarse Sand	5.94
#20	0.85	41.6		
#40	0.43	34.9	Medium Sand	13.84
#60	0.25	29.9		
#100	0.15	25.9	Fine Sand	13.45
#200	0.075	21.4		

Hydrometer Analysis

(mm)	% Finer		
0.032	20.2	Fines Silt or Clay	21.44
0.021	18.8		
0.012	17.4		
0.0086	16.5		
0.0061	16.0		
0.0030	12.3		
0.0013	4.2		



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
11.8	31	19	12	2.84

DESCRIPTION: Light reddish brown clayey gravel with sand

USCS: GC

TECH: MKS
DATE: 10/11/2005
REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

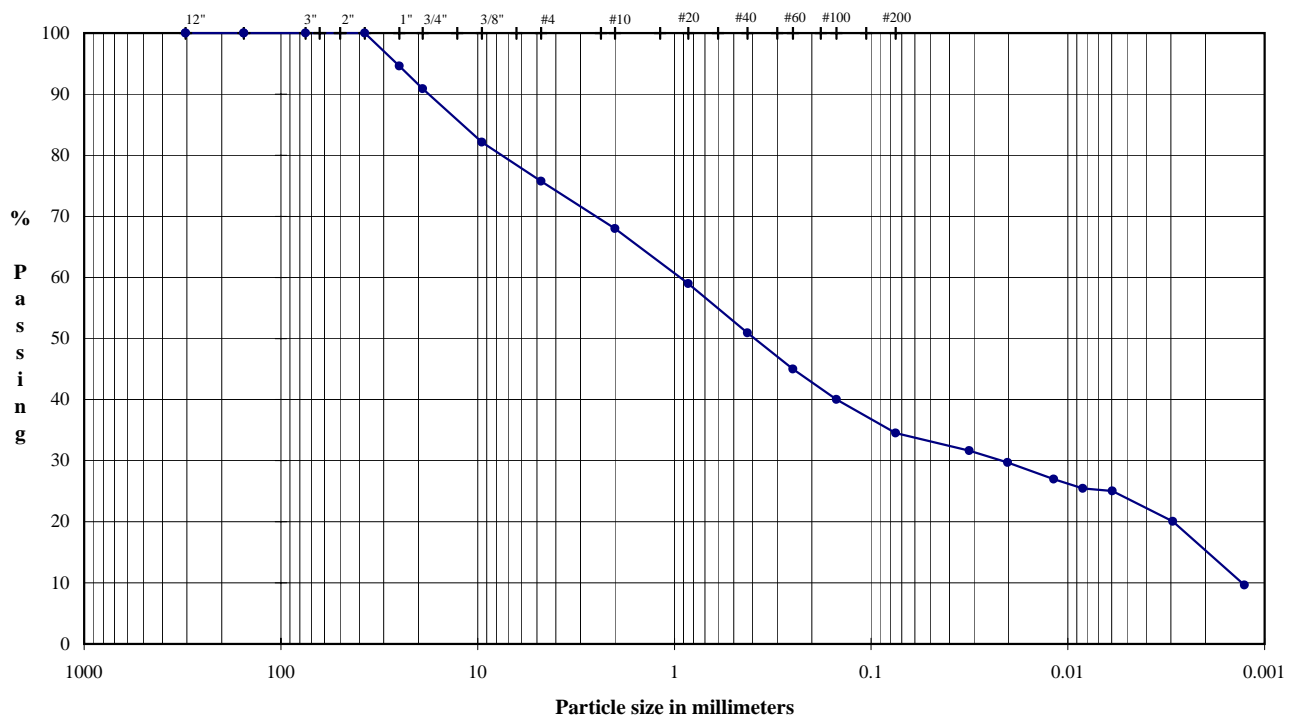
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: T-3

Depth: 71.0-72.5

TYPE: Bag



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

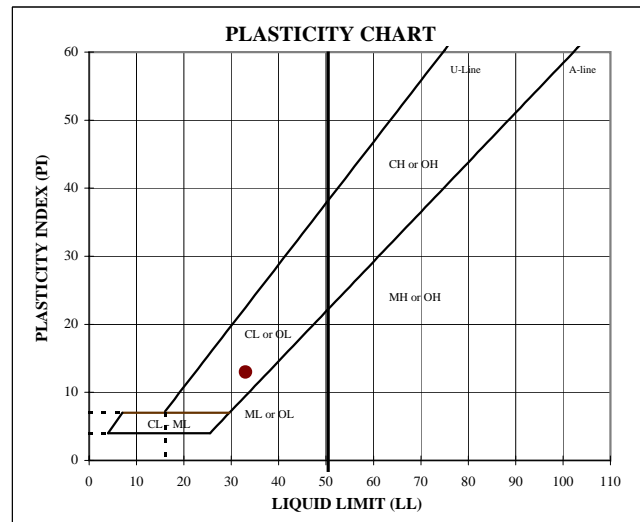
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	0.00
12.0"	304.8	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0		
1.5"	37.5	100.0	Coarse Gravel	9.09
1.0"	25.0	94.6		
0.75"	19.0	90.9		
0.375"	9.5	82.1	Fine Gravel	15.16
#4	4.8	75.7		
#10	2.0	68.0		
#20	0.85	59.0	Coarse Sand	7.73
#40	0.43	50.9		
#60	0.25	45.0	Medium Sand	17.09
#100	0.15	40.0		
#200	0.075	34.5		

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
12.8	33	20	13	2.87

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	34.52
0.032	31.6		
0.020	29.7		
0.012	27.0		
0.0084	25.4		
0.0059	25.1		
0.0029	20.0		
0.0013	9.6		

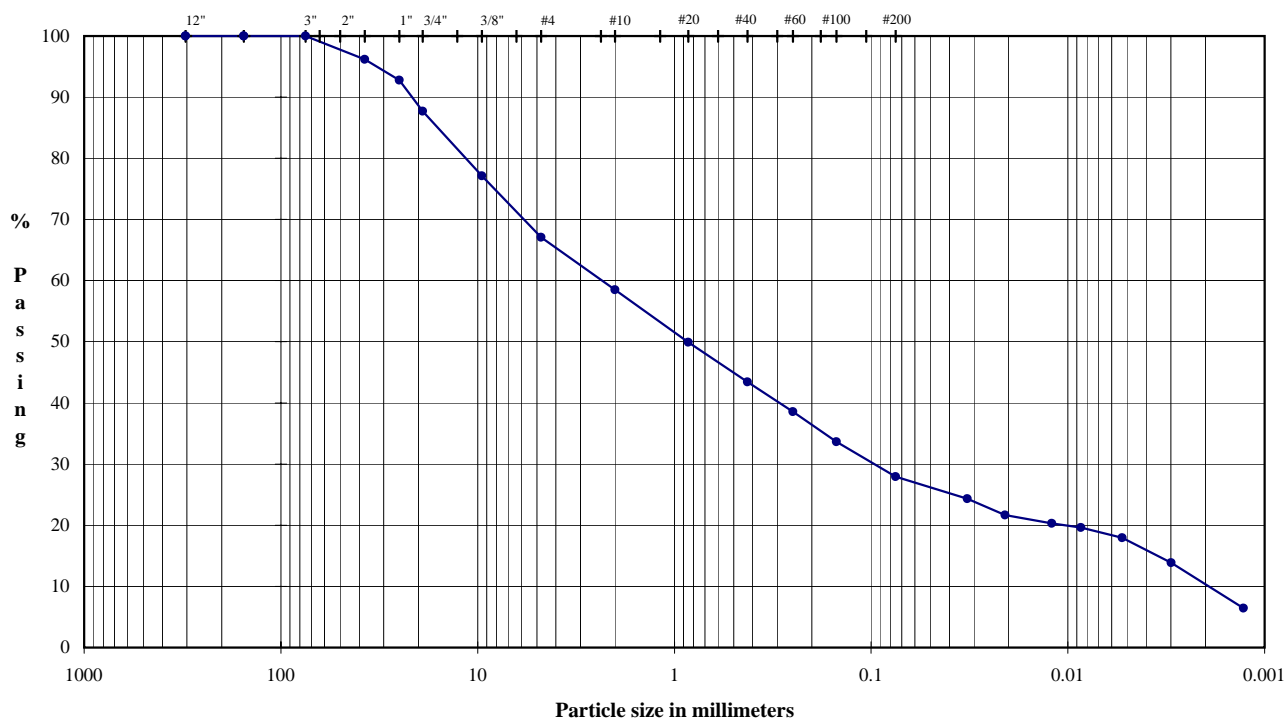
DESCRIPTION: Red clayey sand with gravel

USCS: SC

TECH: SW
DATE: 9/29/2005
REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

ASTM D421, D422, D4318

PROJECT NAME: **G&K/Tyrone Stockpile Geotech/AZ**SAMPLE ID: **T-3**Depth: **76.5-77.5**TYPE: **Bag**

	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

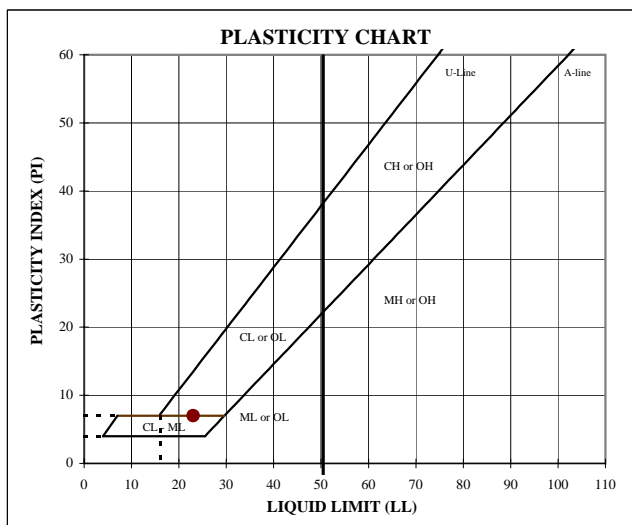
% Passing

Classification

Percentage

U.S. Standard Sieves Sizes and Numbers

12.0"	304.8	100.0	Cobbles	0.00
12.0"	304.8	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	12.28
1.5"	37.5	96.2		
1.0"	25.0	92.8		
0.75"	19.0	87.7		
0.375"	9.5	77.1	Fine Gravel	20.61
#4	4.8	67.1		
#10	2.00	58.5		
#20	0.85	49.9		
#40	0.43	43.4	Medium Sand	15.05
#60	0.25	38.6		
#100	0.15	33.6		
#200	0.075	27.9		
			Fine Sand	15.52



Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	27.92
0.032	24.4		
0.021	21.6		
0.012	20.3		
0.0086	19.6		
0.0053	17.9		
0.0030	13.9		
0.0013	6.4		

ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
4.3	23	16	7	2.85

DESCRIPTION: Light gray silty, clayey sand with gravel

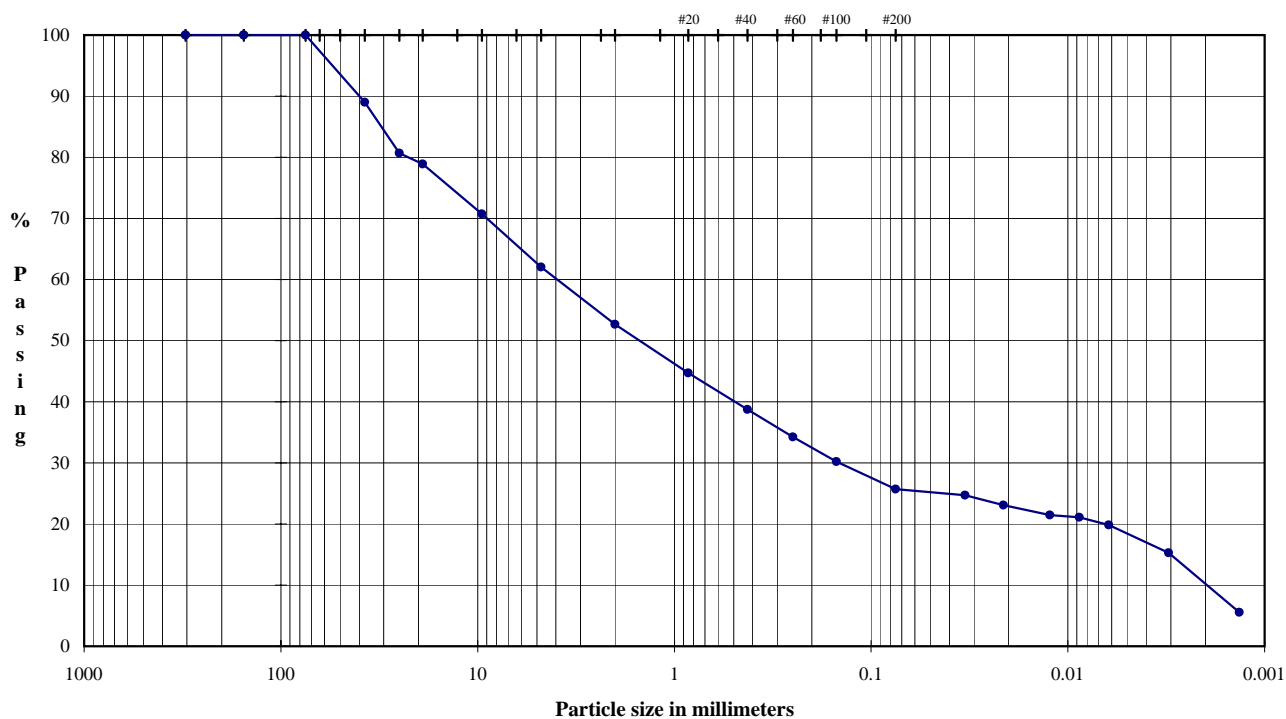
USCS: SC-SM

TECH	MKS
DATE	10/11/2005
REVIEW	0

ASTM D421, D422, D4318

Depth: **91.0-92.5**

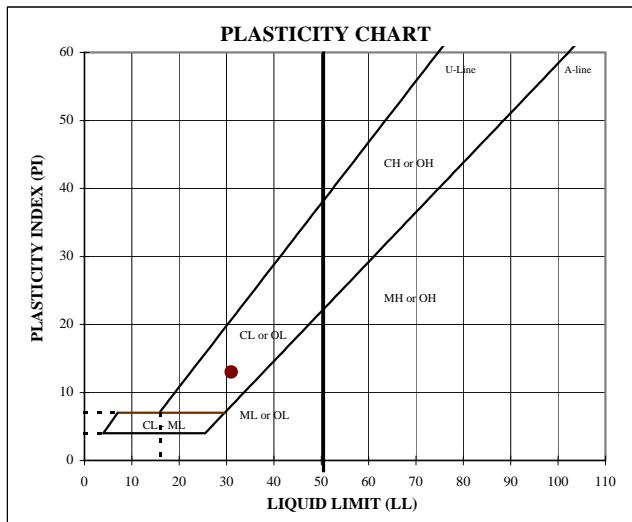
TYPE: **Bag**



COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			FINES

Particle Size			Particle Size	
	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	0.00
12.0"	304.8	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0	Coarse Gravel	21.09
3.0"	75.0	100.0		
1.5"	37.5	89.0		
1.0"	25.0	80.7		
0.75"	19.0	78.9		
0.375"	9.5	70.7		
#4	4.8	62.1	Fine Gravel	16.85
#10	2.00	52.7	Coarse Sand	9.40
#20	0.85	44.7	Medium Sand	13.93
#40	0.43	38.7		
#60	0.25	34.2		
#100	0.15	30.2		
#200	0.075	25.7	Fine Sand	13.04

Hydrometer Analysis	(mm)	% Finer	Fines Silt or Clay	25.69
	0.033	24.7		
	0.021	23.1		
	0.012	21.5		
	0.0087	21.1		
	0.0062	19.8		
	0.0031	15.3		
	0.0013	5.5		



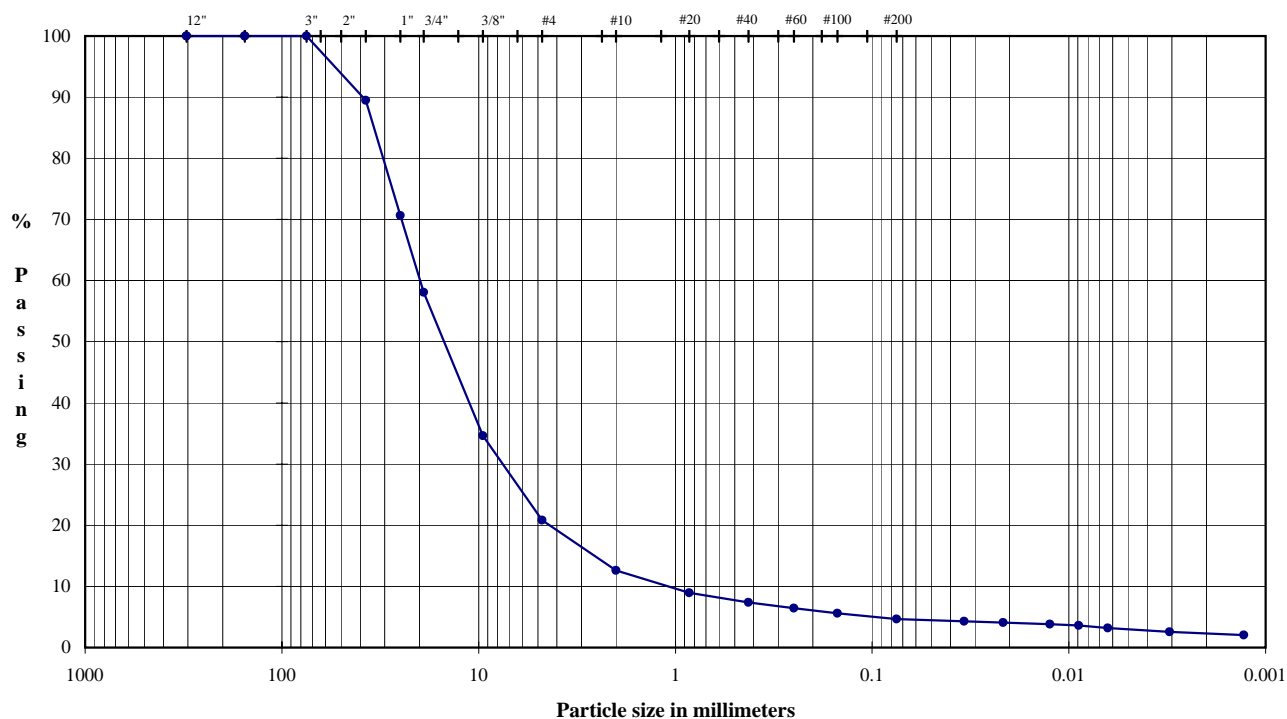
M _c	LL	PL	PI	SpG
8.6	31	18	13	2.74

USCS:	GC
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TECH	MKS
DATE	10/12/2005
REVIEW	MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

ASTM D421, D422, D4318

PROJECT NAME: **G&K/Tyrone Stockpile Geotech/AZ**SAMPLE ID: **T-4**Depth: **9.0-10.0**TYPE: **Bag**

	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

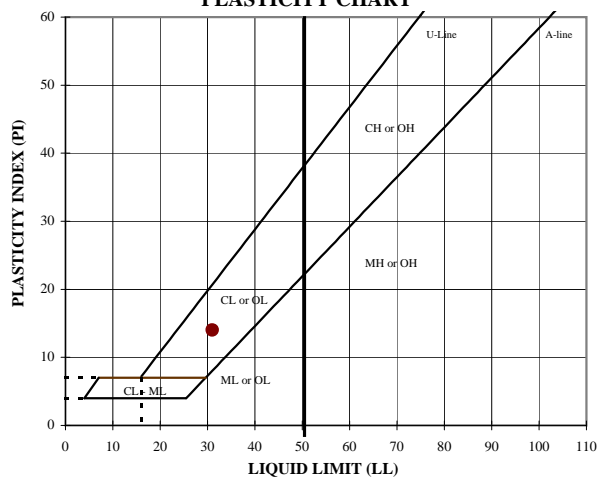
U.S. Standard Sieves Sizes and Numbers

	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	0.00
12.0"	304.8	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	41.92
1.5"	37.5	89.5		
1.0"	25.0	70.6		
0.75"	19.0	58.1		
0.375"	9.5	34.6	Fine Gravel	37.27
#4	4.8	20.8		
#10	2.00	12.6		
#20	0.85	8.9		
#40	0.43	7.4	Medium Sand	5.22
#60	0.25	6.4		
#100	0.15	5.6	Fine Sand	2.69
#200	0.075	4.7		

Hydrometer Analysis

(mm)	% Finer		
0.034	4.3	Fines Silt or Clay	4.68
0.022	4.1		
0.012	3.8		
0.0089	3.6		
0.0063	3.2		
0.0031	2.6		
0.0013	2.1		

PLASTICITY CHART



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
4.9	31	17	14	2.83

DESCRIPTION: Pale yellow well-graded gravel with sand

USCS: GW

TECH: MKS

DATE: 10/12/2005

REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

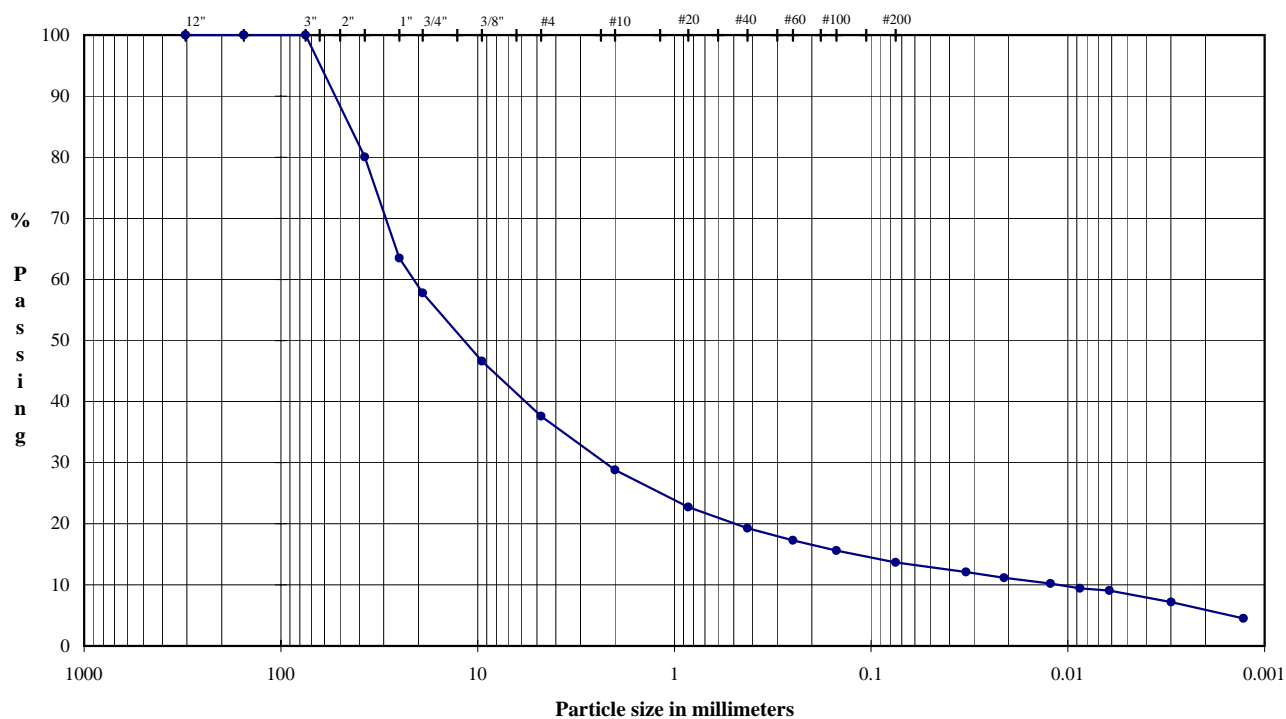
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: T-4

Depth: 29.0-30.0

TYPE: Bag



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

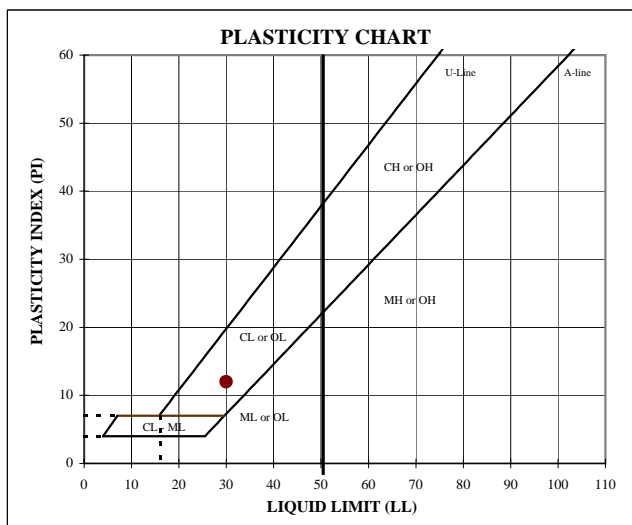
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	0.00
12.0"	304.8	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	42.23
1.5"	37.5	80.0		
1.0"	25.0	63.5		
0.75"	19.0	57.8		
0.375"	9.5	46.6	Fine Gravel	20.17
#4	4.8	37.6		
#10	2.00	28.8		
#20	0.85	22.7		
#40	0.43	19.3	Medium Sand	9.52
#60	0.25	17.3		
#100	0.15	15.6		
#200	0.075	13.7		

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
6.9	30	18	12	2.84

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	13.70
0.033	12.1		
0.021	11.1		
0.012	10.2		
0.0087	9.4		
0.0061	9.1		
0.0030	7.2		
0.0013	4.5		

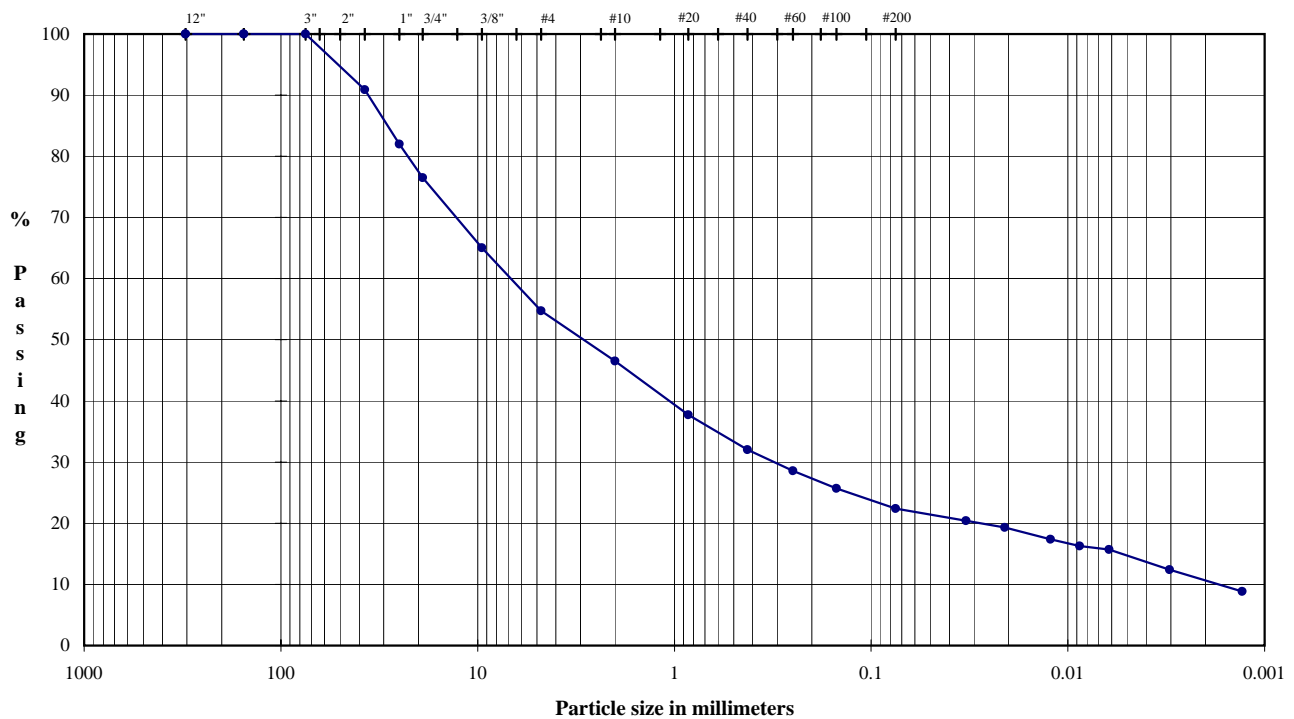
DESCRIPTION: Light gray clayey gravel with sand

USCS: GC

TECH: SW
DATE: 9/29/2005
REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

ASTM D421, D422, D4318

PROJECT NAME: **G&K/Tyrone Stockpile Geotech/AZ**SAMPLE ID: **T-4**Depth: **49.0 - 50.0**TYPE: **Bag**

	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

% Passing

Classification

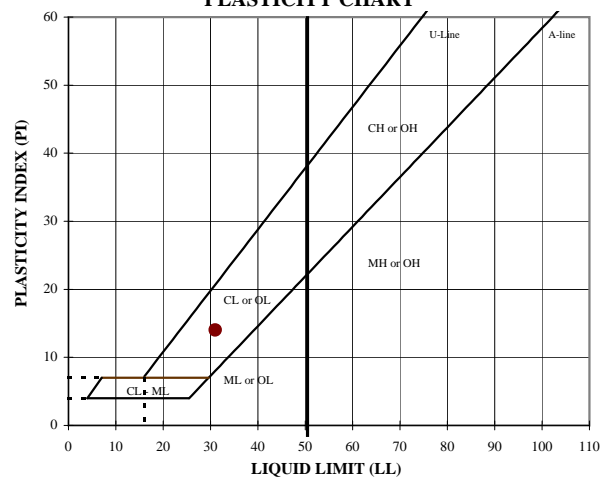
Percentage

12.0"	304.8	100.0	Cobbles	0.00
12.0"	304.8	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	23.50
1.5"	37.5	90.9		
1.0"	25.0	82.0		
0.75"	19.0	76.5		
0.375"	9.5	65.1	Fine Gravel	21.75
#4	4.8	54.7		
#10	2.00	46.5		
#20	0.85	37.7		
#40	0.43	32.0	Medium Sand	14.49
#60	0.25	28.6		
#100	0.15	25.7		
#200	0.075	22.4		
			Fines Silt or Clay	22.40

U.S. Standard Sieves Sizes and Numbers

Hydrometer Analysis

(mm)	% Finer		
0.033	20.4	Fines Silt or Clay	22.40
0.021	19.3		
0.012	17.4		
0.0087	16.3		
0.0062	15.7		
0.0030	12.4		
0.0013	8.8		

PLASTICITY CHART**ATTERBERG LIMITS**

M _c	LL	PL	PI	SpG
6.8	31	17	14	2.80

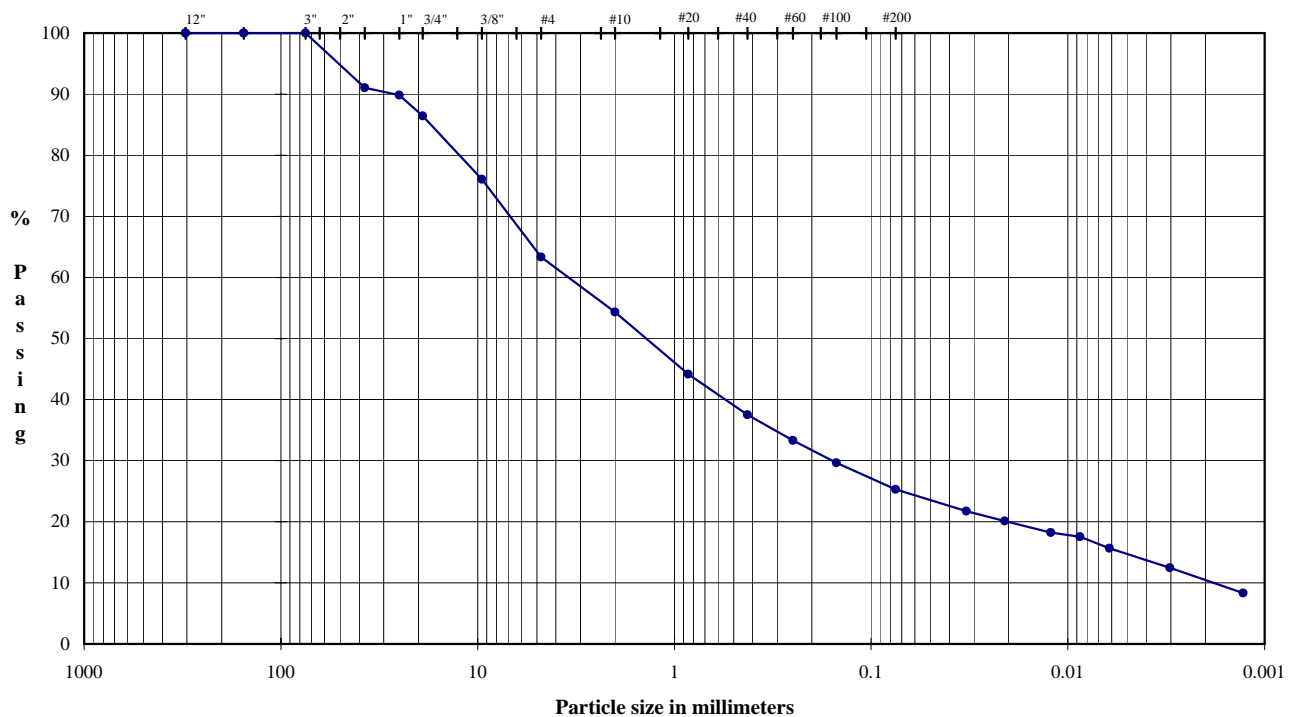
DESCRIPTION: Light gray clayey gravel with sand

USCS: GC

TECH: MKS
DATE: 10/12/2005
REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

ASTM D421, D422, D4318

PROJECT NAME: **G&K/Tyrone Stockpile Geotech/AZ**SAMPLE ID: **T-4**Depth: **69.0-70.0**TYPE: **Bag**

	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

% Passing

Classification

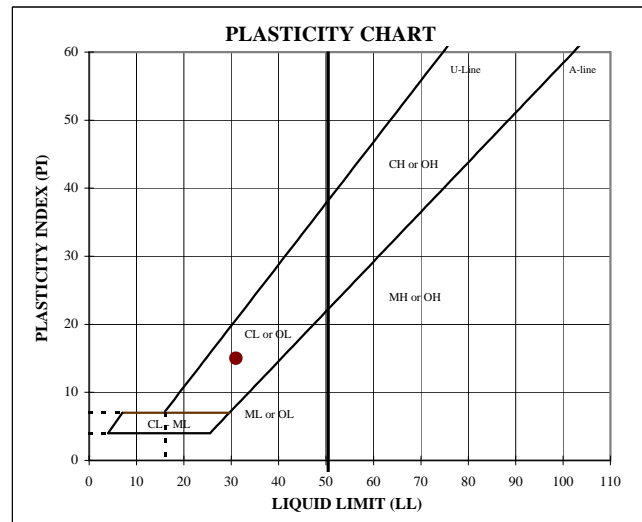
Percentage

U.S. Standard Sieves Sizes and Numbers

12.0"	304.8	100.0	Cobbles	0.00
12.0"	304.8	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0	Coarse Gravel	13.55
3.0"	75.0	91.1		
1.5"	37.5	89.8		
1.0"	25.0	86.4		
0.75"	19.0	76.0		
0.375"	9.5	63.3	Fine Gravel	23.13
#4	4.8	54.3		
#10	2.0	44.2	Coarse Sand	9.00
#20	0.85	37.5		
#40	0.43	33.3	Medium Sand	16.83
#60	0.25	29.6		
#100	0.15	25.3		
#200	0.075	25.3	Fine Sand	12.17

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	25.32
0.033	21.7		
0.021	20.1		
0.012	18.2		
0.0087	17.6		
0.0062	15.7		
0.0030	12.5		
0.0013	8.3		



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
6.9	31	16	15	2.81

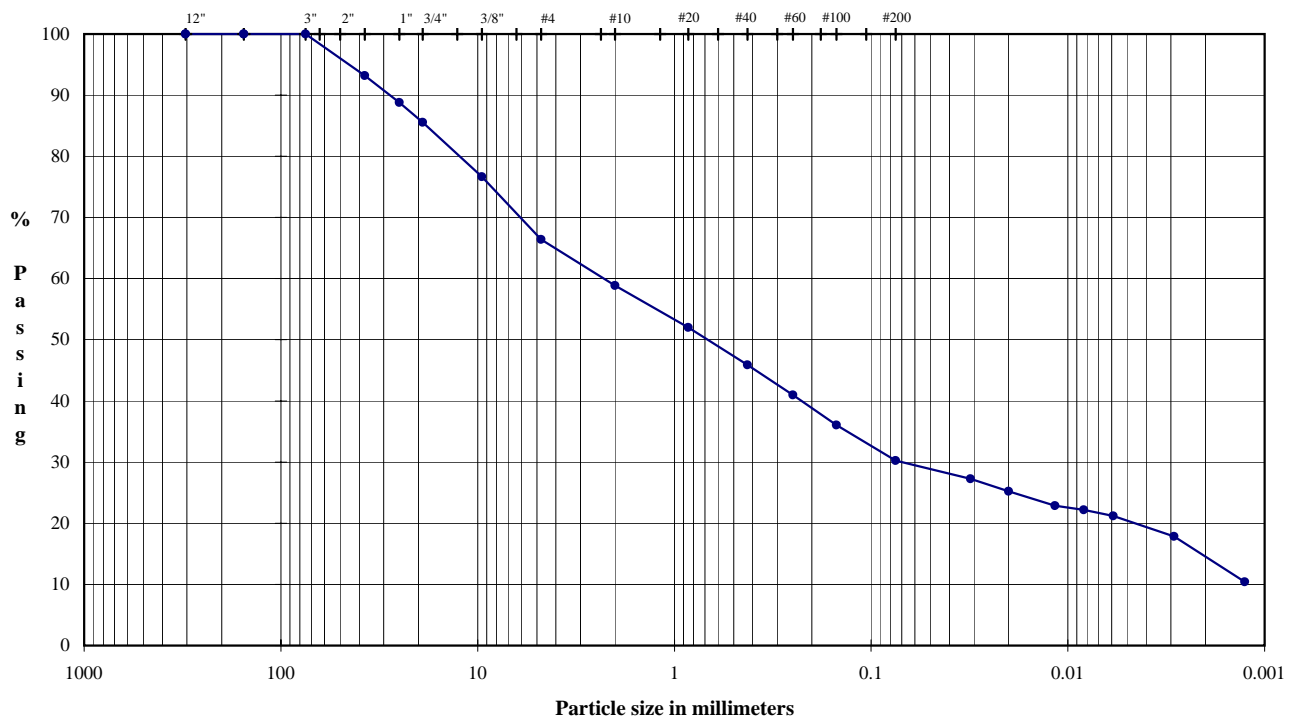
DESCRIPTION: Weak red clayey sand with gravel

USCS: SC

TECH	SW
DATE	9/29/2005
REVIEW	MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

ASTM D421, D422, D4318

PROJECT NAME: **G&K/Tyrone Stockpile Geotech/AZ**SAMPLE ID: **T-4**Depth: **89.0-90.0**TYPE: **Bag**

	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

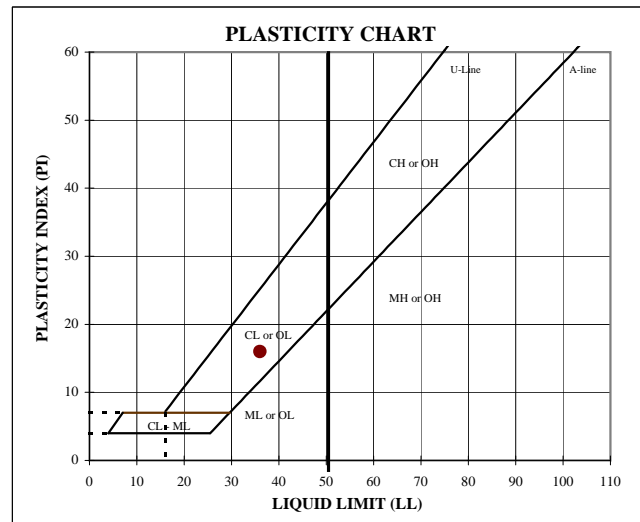
Particle Size

U.S. Standard Sieves Sizes and Numbers

	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	0.00
12.0"	304.8	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0		
1.5"	37.5	93.2	Coarse Gravel	14.46
1.0"	25.0	88.8		
0.75"	19.0	85.5		
0.375"	9.5	76.7	Fine Gravel	19.13
#4	4.8	66.4		
#10	2.00	58.9		
#20	0.85	52.0	Coarse Sand	7.52
#40	0.43	45.9	Medium Sand	12.97
#60	0.25	41.0		
#100	0.15	36.1	Fine Sand	15.66
#200	0.075	30.3		

Hydrometer Analysis

(mm)	% Finer		
0.031	27.2	Fines Silt or Clay	30.25
0.020	25.2		
0.012	22.9		
0.0083	22.2		
0.0059	21.2		
0.0029	17.8		
0.0013	10.4		

**ATTERBERG LIMITS**

M _c	LL	PL	PI	SpG
9.5	36	20	16	2.93

DESCRIPTION: Light gray clayey sand with gravel

USCS: SC

TECH	MKS
DATE	10/11/2005
REVIEW	0

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

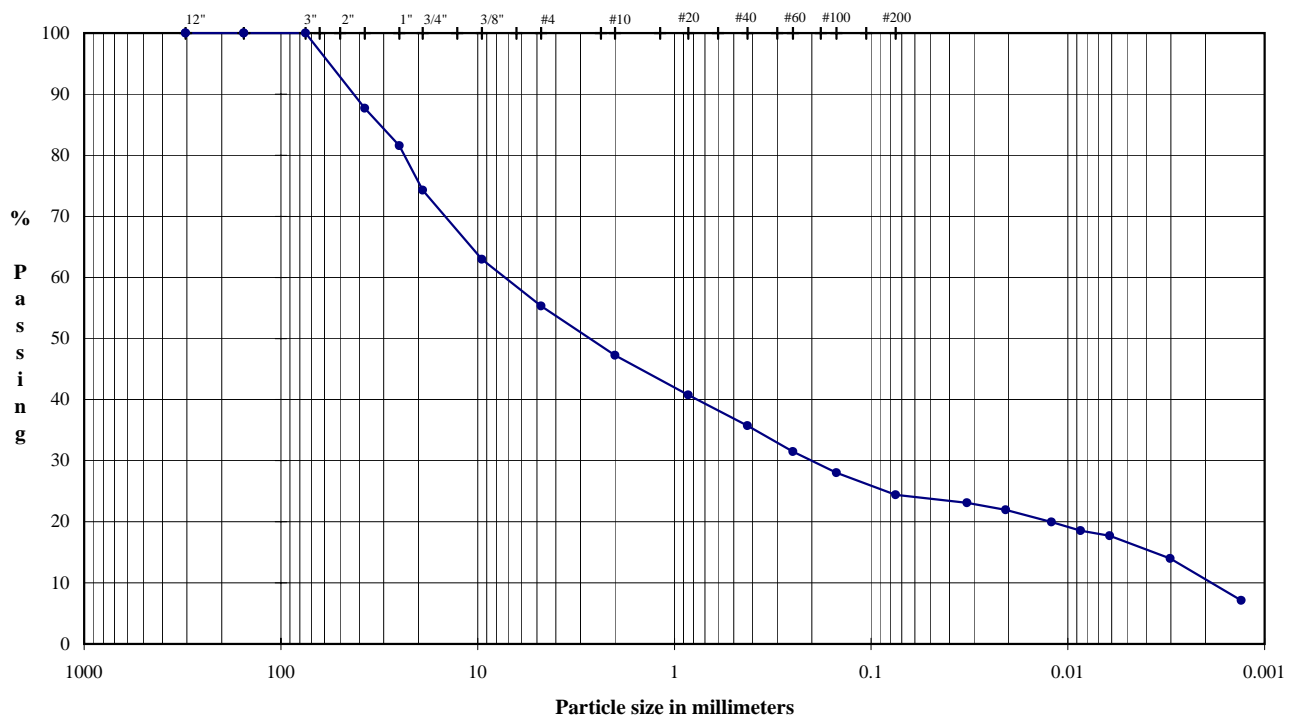
ASTM D421, D422, D4318

PROJECT NAME: **G&K/Tyrone Stockpile Geotech/AZ**

SAMPLE ID: **T-4**

Depth: **109-110**

TYPE: **Bag**



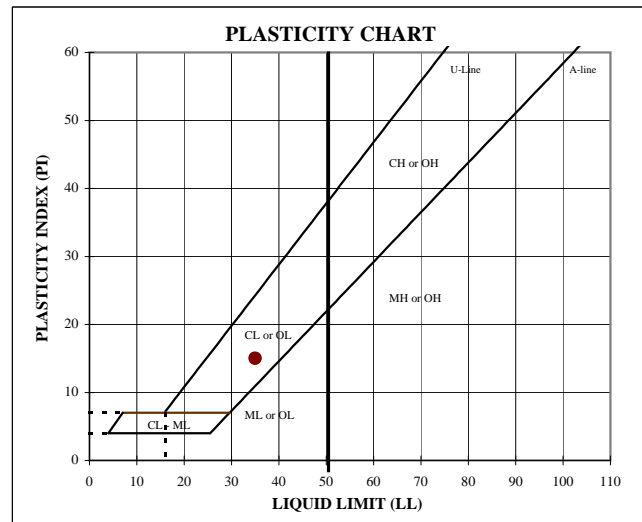
	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

U.S. Standard Sieves Sizes and Numbers

	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	0.00
12.0"	304.8	100.0		
6.0"	154.2	100.0	Coarse Gravel	25.72
6.0"	154.2	100.0		
3.0"	75.0	100.0		
1.5"	37.5	87.7		
1.0"	25.0	81.5	Fine Gravel	18.96
0.75"	19.0	74.3		
0.375"	9.5	63.0		
#4	4.8	55.3	Coarse Sand	8.06
#10	2.0	47.3		
#20	0.85	40.8	Medium Sand	11.55
#40	0.43	35.7		
#60	0.25	31.5	Fine Sand	11.31
#100	0.15	28.0		
#200	0.075	24.4		



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
12.0	35	20	15	2.79

Hydrometer Analysis

(mm)	% Finer		
0.033	23.1	Fines Silt or Clay	24.40
0.021	22.0		
0.012	20.0		
0.0086	18.5		
0.0061	17.7		
0.0030	14.0		
0.0013	7.1		

DESCRIPTION: Pale yellow clayey gravel with sand

USCS: GC

TECH: MKS
DATE: 10/12/2005
REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

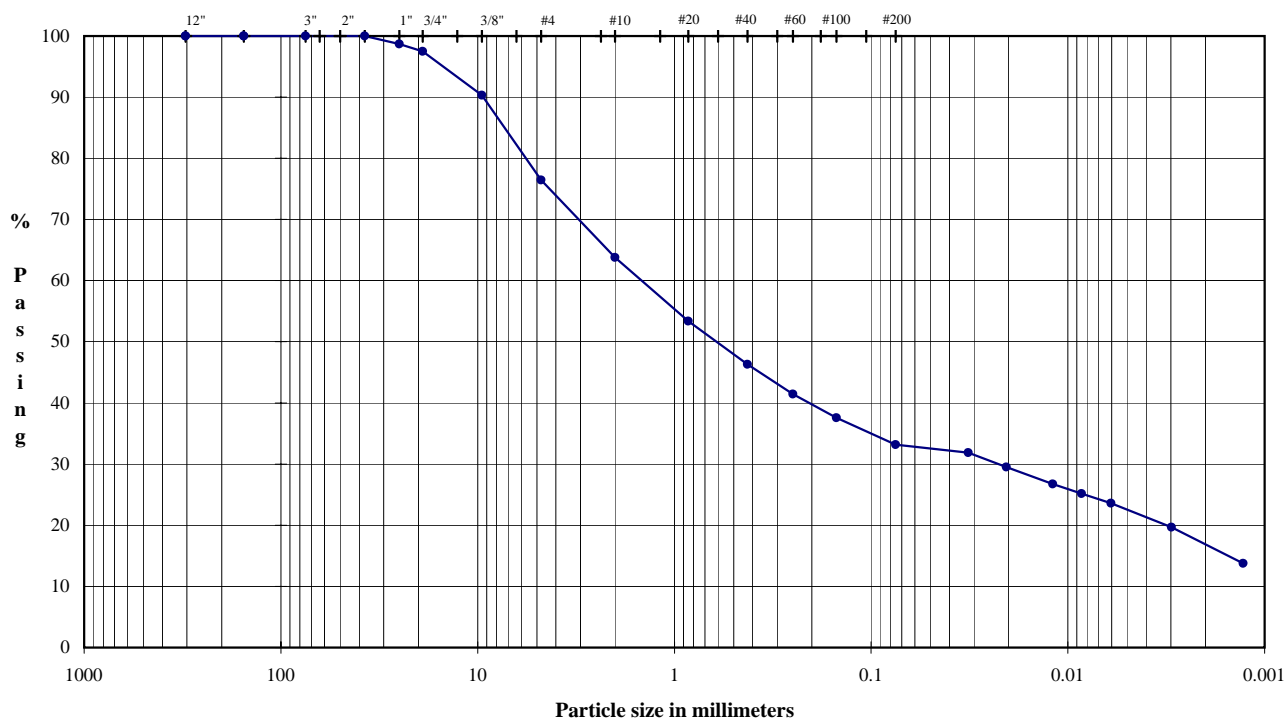
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: T-4

Depth: 113-114

TYPE: Bag



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

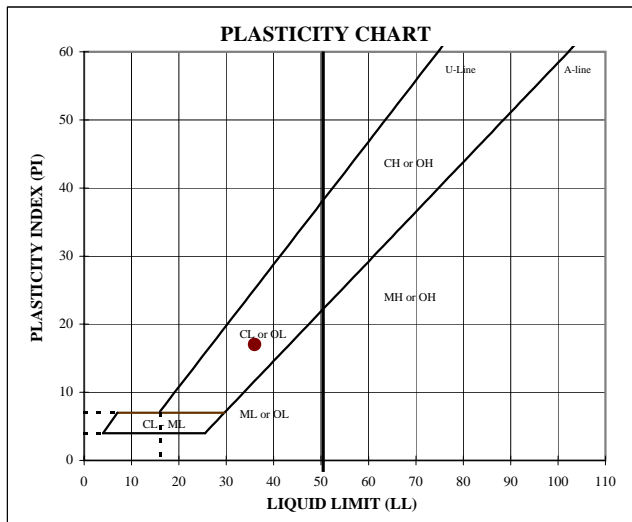
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	0.00
12.0"	304.8	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	2.52
1.5"	37.5	100.0		
1.0"	25.0	98.7		
0.75"	19.0	97.5		
0.375"	9.5	90.3	Fine Gravel	21.04
#4	4.8	76.4		
#10	2.00	63.8		
#20	0.85	53.4		
#40	0.43	46.3	Medium Sand	17.48
#60	0.25	41.4		
#100	0.15	37.6		
#200	0.075	33.2		
			Fine Sand	13.11

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
17.7	36	19	17	2.86

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	33.18
0.032	31.9		
0.021	29.5		
0.012	26.7		
0.0085	25.2		
0.0060	23.6		
0.0030	19.7		
0.0013	13.8		

DESCRIPTION: Light yellowish brown clayey sand with gravel

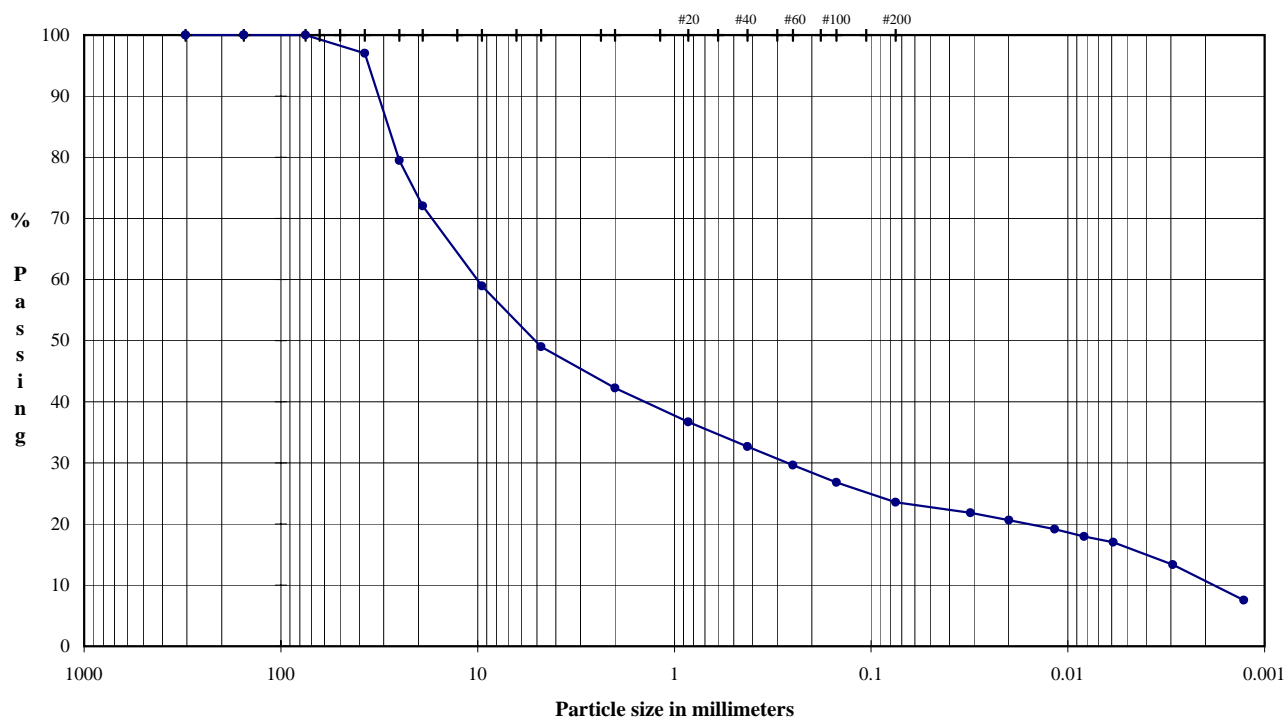
USCS: SC

TECH: MKS
DATE: 10/14/2005
REVIEW: MB

ASTM D421, D422, D4318

Depth: **129-130**

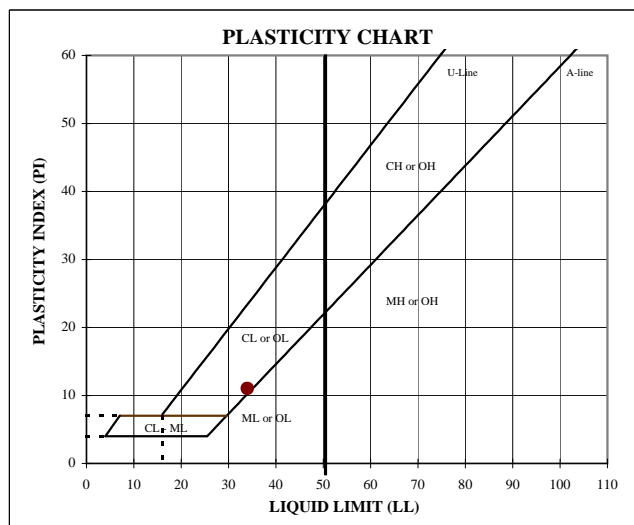
TYPE: **Bag**



COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			FINES

Particle Size			Particle Size	
	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	0.00
12.0"	304.8	100.0		
6.0"	154.2	100.0	Coarse Gravel	27.96
6.0"	154.2	100.0		
3.0"	75.0	100.0		
1.5"	37.5	97.0		
1.0"	25.0	79.5		
0.75"	19.0	72.0		
0.375"	9.5	58.9	Fine Gravel	23.06
#4	4.8	49.0		
#10	2.00	42.3	Coarse Sand	6.73
#20	0.85	36.7	Medium Sand	9.60
#40	0.43	32.7		
#60	0.25	29.7		
#100	0.15	26.8		
#200	0.075	23.6	Fine Sand	9.09

Hydrometer Analysis	(mm)	% Finer	Fines Silt or Clay	23.57
	0.031	21.8		
	0.020	20.6		
	0.012	19.2		
	0.0083	18.0		
	0.0059	17.0		
	0.0029	13.4		
	0.0013	7.5		



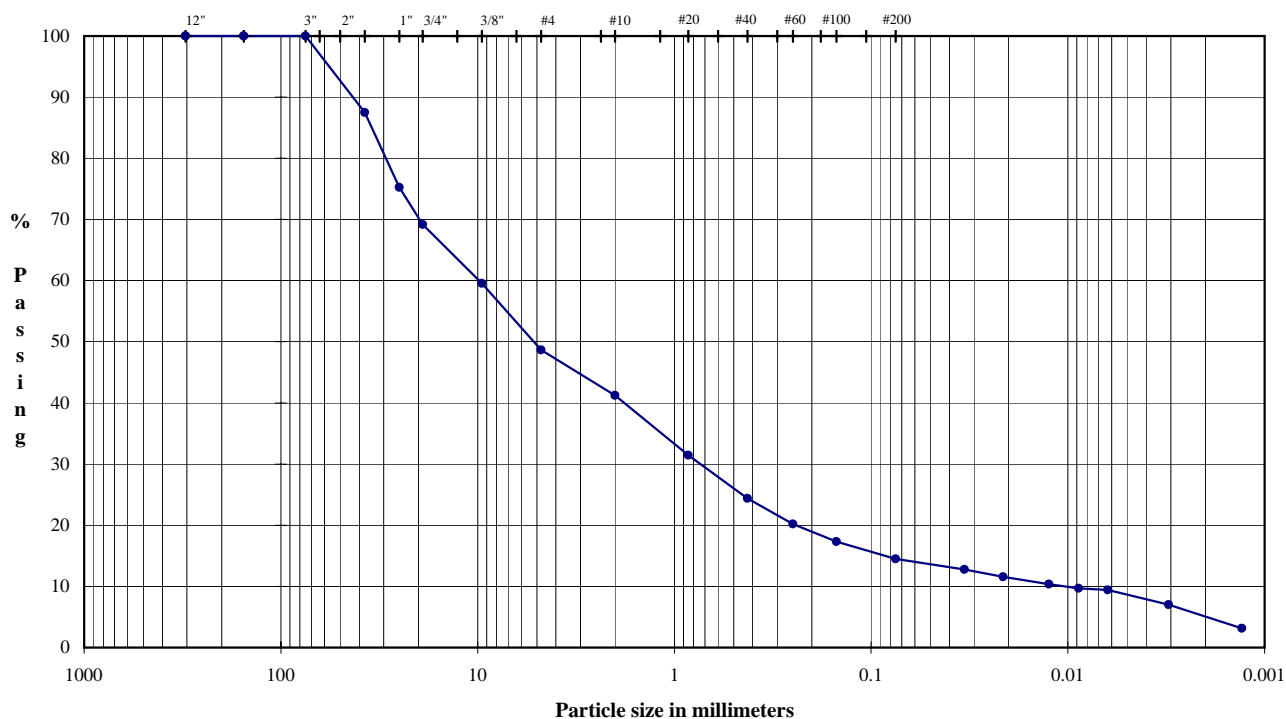
M _c	LL	PL	PI	SpG
9.3	34	23	11	2.91

USCS:	GC
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TECH	MKS
DATE	10/14/2005
REVIEW	MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

ASTM D421, D422, D4318

PROJECT NAME: **G&K/Tyrone Stockpile Geotech/AZ**SAMPLE ID: **T-5**Depth: **16.5-17.5**TYPE: **Bag**

	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

% Passing

Classification

Percentage

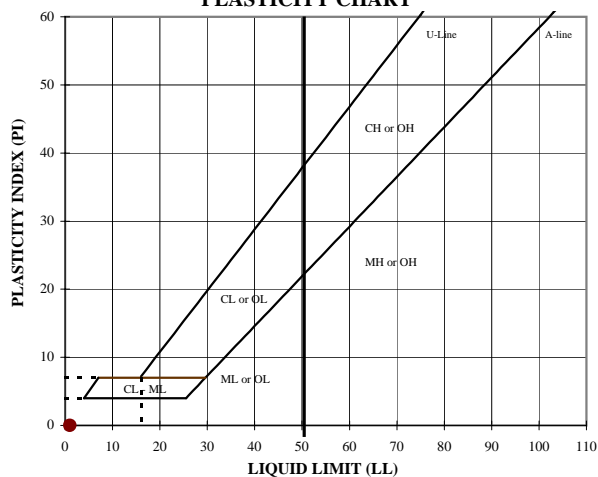
U.S. Standard Sieves Sizes and Numbers

12.0"	304.8	100.0	Cobbles	0.00
12.0"	304.8	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	30.84
1.5"	37.5	87.5		
1.0"	25.0	75.2		
0.75"	19.0	69.2		
0.375"	9.5	59.6	Fine Gravel	20.49
#4	4.8	48.7		
#10	2.00	41.2		
#20	0.85	31.4		
#40	0.43	24.4	Medium Sand	16.88
#60	0.25	20.2		
#100	0.15	17.3		
#200	0.075	14.5		
			Fine Sand	9.87

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	14.50
0.034	12.8		
0.021	11.6		
0.012	10.4		
0.0088	9.7		
0.0063	9.4		
0.0031	7.0		
0.0013	3.1		

PLASTICITY CHART



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
5.9	NP	NP	NP	2.78

DESCRIPTION: Red silty gravel with sand

USCS: GM

TECH: SW

DATE: 9/29/2005

REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

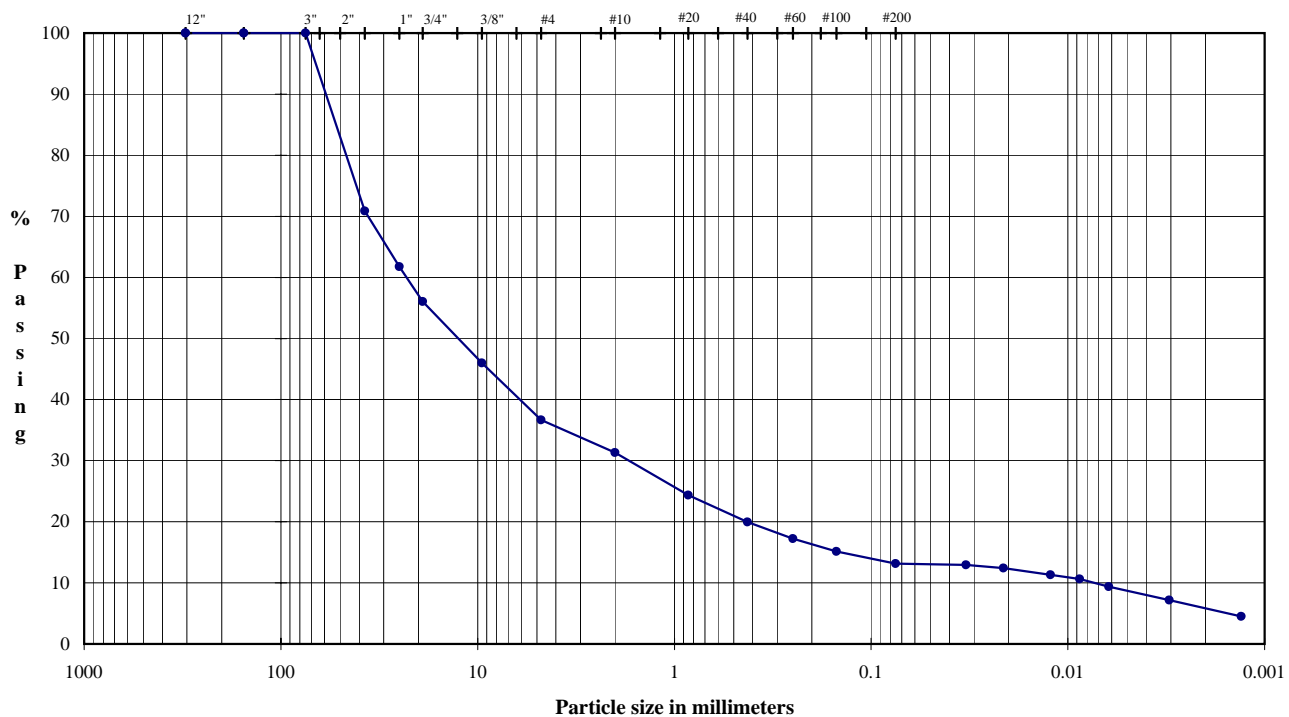
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: T-5

Depth: 36.5-37.5

TYPE: Bag



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

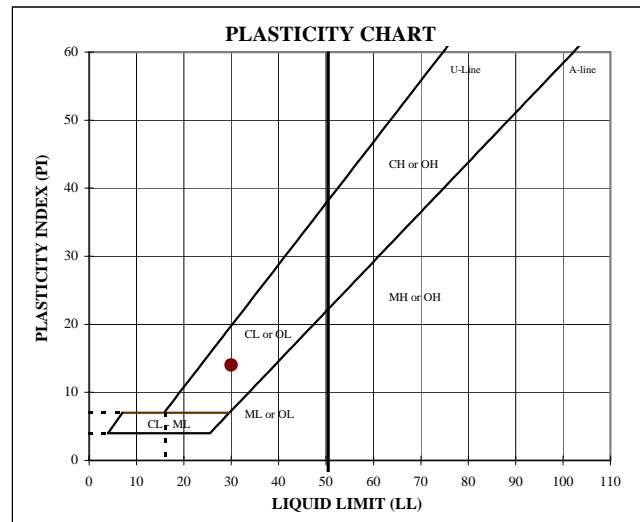
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	0.00
12.0"	304.8	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	43.94
1.5"	37.5	70.9		
1.0"	25.0	61.8		
0.75"	19.0	56.1		
0.375"	9.5	46.0	Fine Gravel	19.40
#4	4.8	36.7		
#10	2.00	31.3		
#20	0.85	24.4		
#40	0.43	20.0	Medium Sand	11.36
#60	0.25	17.2		
#100	0.15	15.1		
#200	0.075	13.1		
			Fine Sand	6.81

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
10.2	30	16	14	2.81

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	13.15
0.033	12.9		
0.021	12.4		
0.012	11.3		
0.0087	10.6		
0.0062	9.4		
0.0031	7.2		
0.0013	4.5		

DESCRIPTION: Very pale brown clayey gravel with sand

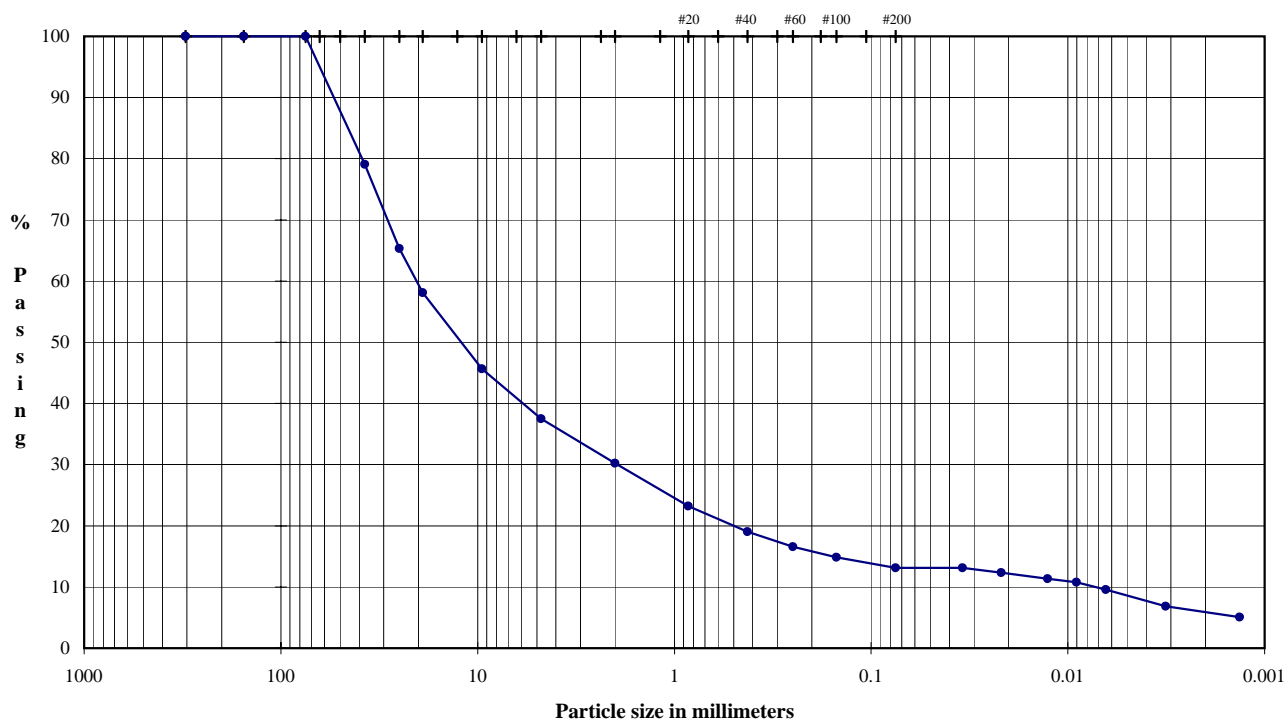
USCS: GC

TECH: MKS
DATE: 10/14/2005
REVIEW: MB

ASTM D421, D422, D4318

Depth: **56.5-57.5**

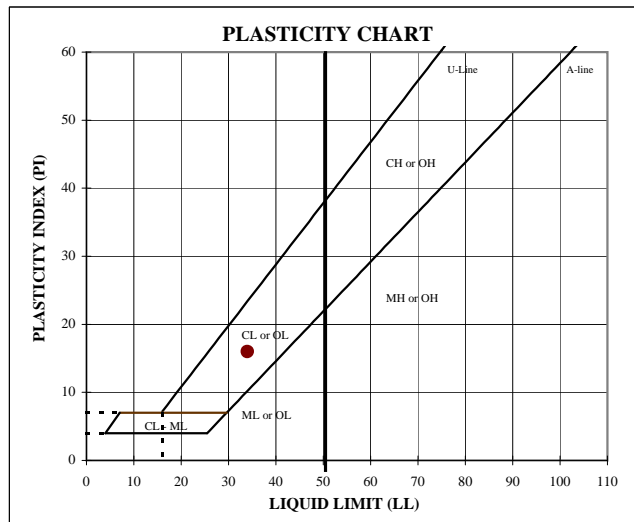
TYPE: **Bag**



COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			FINES

Particle Size			Particle Size	
	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	0.00
12.0"	304.8	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0	Coarse Gravel	41.87
3.0"	75.0	100.0		
1.5"	37.5	79.1		
1.0"	25.0	65.3		
0.75"	19.0	58.1		
0.375"	9.5	45.7		
#4	4.8	37.5	Fine Gravel	20.62
#10	2.00	30.2	Coarse Sand	7.30
#20	0.85	23.2	Medium Sand	11.15
#40	0.43	19.1		
#60	0.25	16.6		
#100	0.15	14.9		
#200	0.075	13.2	Fine Sand	5.91

Hydrometer Analysis	(mm)	% Finer	Fines Silt or Clay	13.15
	0.034	13.1		
	0.022	12.3		
	0.013	11.4		
	0.0090	10.8		
	0.0064	9.6		
	0.0032	6.9		
	0.0013	5.1		



M _c	LL	PL	PI	SpG
8.7	34	18	16	2.71

USCS:	GC
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TECH	MKS
DATE	10/11/2005
REVIEW	MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

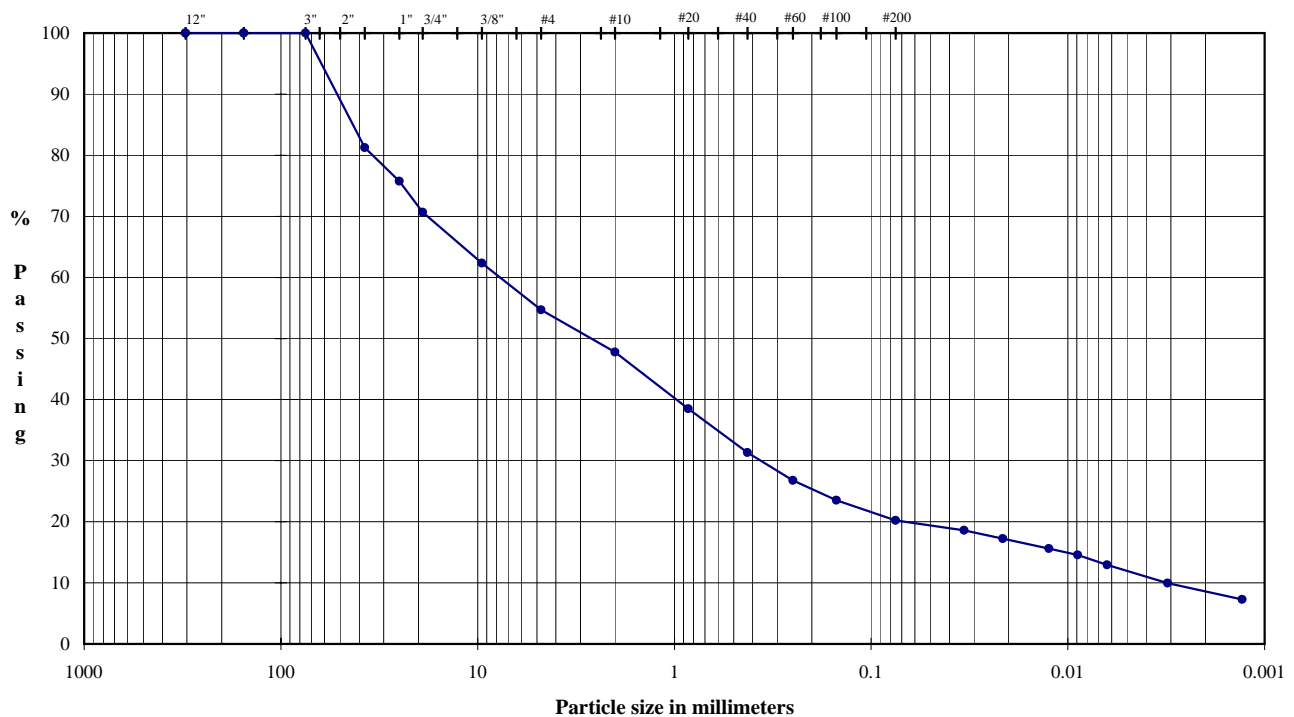
ASTM D421, D422, D4318

PROJECT NAME: **G&K/Tyrone Stockpile Geotech/AZ**

SAMPLE ID: **T-5**

Depth: **96.5-97.5**

TYPE: **Bag**



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

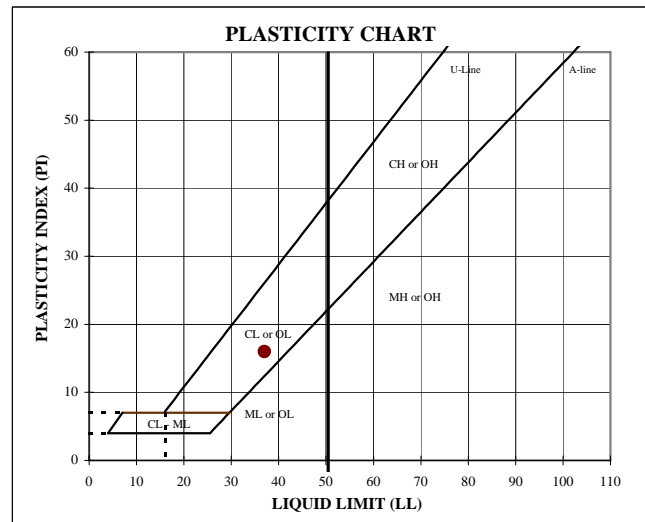
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	0.00
12.0"	304.8	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	29.33
1.5"	37.5	81.3		
1.0"	25.0	75.8		
0.75"	19.0	70.7		
0.375"	9.5	62.3	Fine Gravel	16.00
#4	4.8	54.7		
#10	2.00	47.8		
#20	0.85	38.5		
#40	0.43	31.3	Medium Sand	16.44
#60	0.25	26.8		
#100	0.15	23.5		
#200	0.075	20.2		
			Fine Sand	11.11

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
12.6	37	21	16	2.75

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	20.23
0.034	18.6		
0.021	17.3		
0.012	15.6		
0.0089	14.6		
0.0063	12.9		
0.0031	10.0		
0.0013	7.3		

DESCRIPTION: Pinkish gray clayey gravel with sand

USCS: GC

TECH: MKS
DATE: 10/5/2005
REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

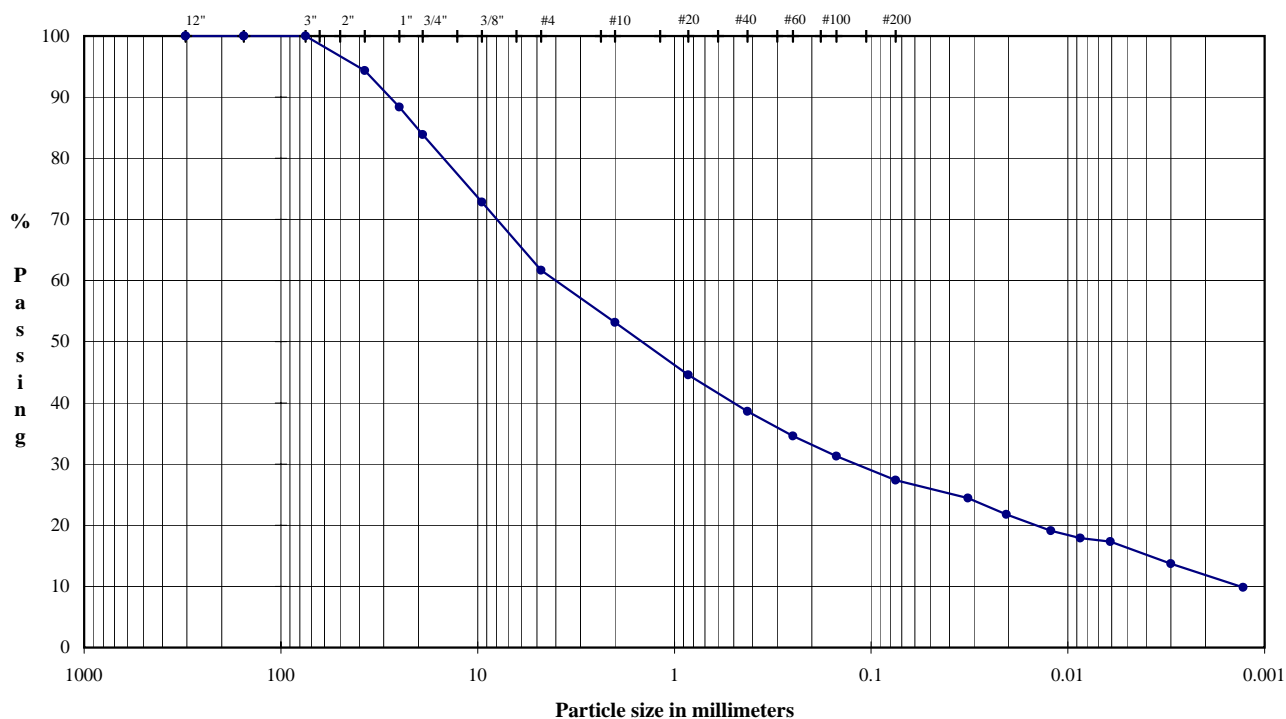
ASTM D421, D422, D4318

PROJECT NAME: **G&K/Tyrone Stockpile Geotech/AZ**

SAMPLE ID: **T-5**

Depth: **116.5-117.5**

TYPE: **Bag**



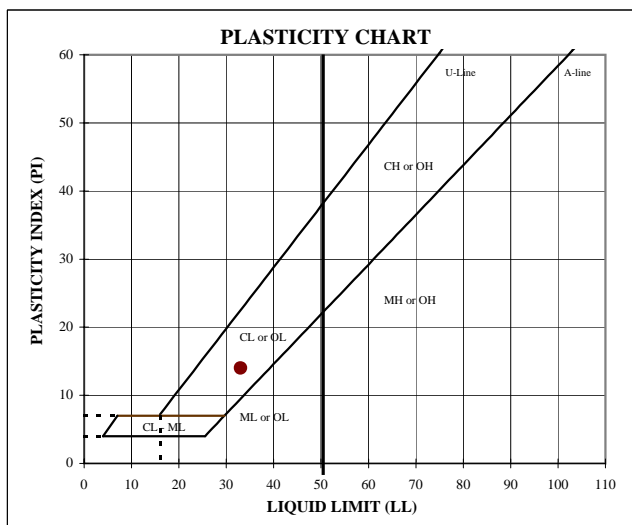
	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

	(mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	Cobbles	0.00
12.0"	304.8	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	16.13
1.5"	37.5	94.4		
1.0"	25.0	88.4		
0.75"	19.0	83.9		
0.375"	9.5	72.9	Fine Gravel	22.15
#4	4.8	61.7		
#10	2.00	53.2		
#20	0.85	44.6		
#40	0.43	38.6	Medium Sand	14.56
#60	0.25	34.6		
#100	0.15	31.3		
#200	0.075	27.4		
			Fine Sand	11.23

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
8.2	33	19	14	2.81

Hydrometer Analysis

(mm)	% Finer		
0.032	24.5	Fines Silt or Clay	27.39
0.021	21.8		
0.012	19.1		
0.0087	17.9		
0.0061	17.3		
0.0030	13.7		
0.0013	9.8		

DESCRIPTION: Light bluish gray clayey gravel with sand

USCS: GC

TECH: MKS
DATE: 10/4/2005
REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

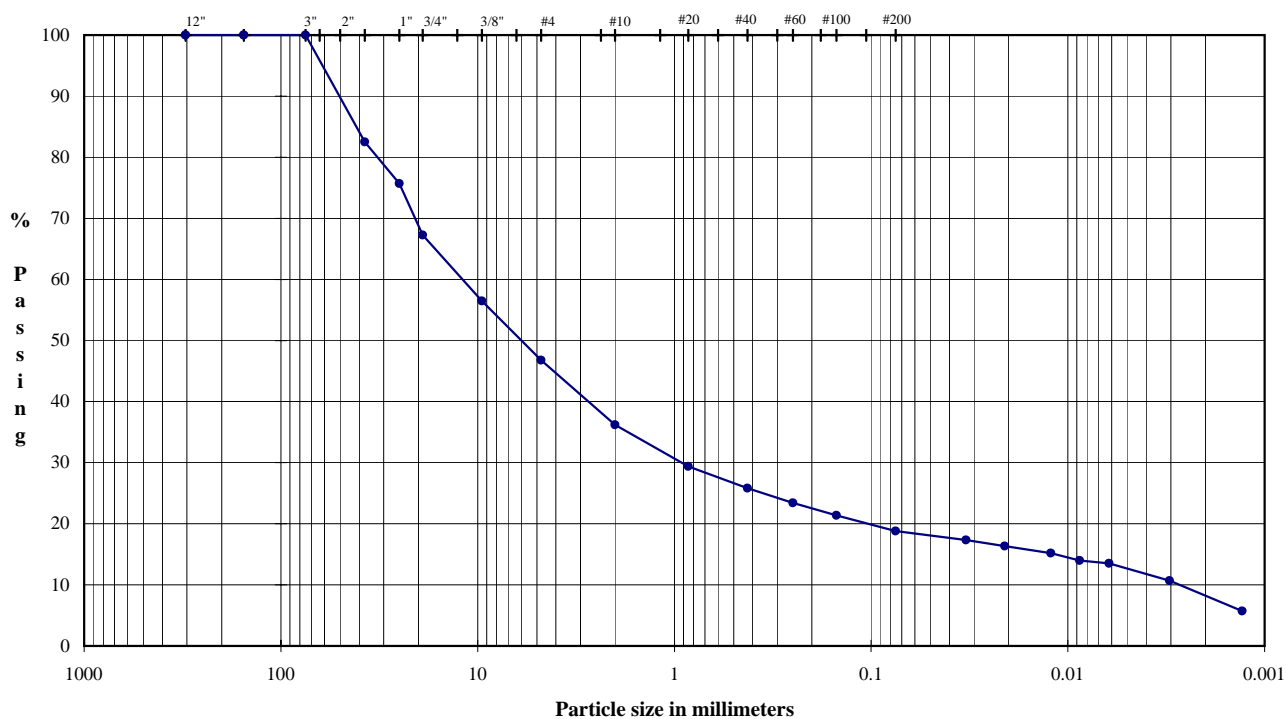
ASTM D421, D422, D4318

PROJECT NAME: G&K/Tyrone Stockpile Geotech/AZ

SAMPLE ID: T-5

Depth: 136.5-137.5

TYPE: Bag



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

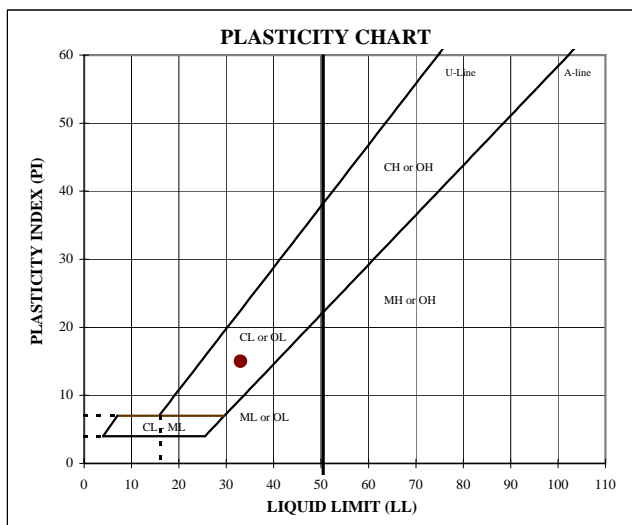
% Passing

Classification

Percentage

12.0"	304.8	100.0	Cobbles	0.00
12.0"	304.8	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0		
1.5"	37.5	82.5	Coarse Gravel	32.77
1.0"	25.0	75.7		
0.75"	19.0	67.2		
0.375"	9.5	56.5	Fine Gravel	20.48
#4	4.8	46.8		
#10	2.0	36.2		
#20	0.85	29.4	Coarse Sand	10.54
#40	0.43	25.8		
#60	0.25	23.4	Medium Sand	10.39
#100	0.15	21.4		
#200	0.075	18.8		

U.S. Standard Sieves Sizes and Numbers



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
9.6	33	18	15	2.80

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	18.79
0.033	17.3		
0.021	16.4		
0.012	15.2		
0.0087	14.0		
0.0062	13.5		
0.0030	10.7		
0.0013	5.7		

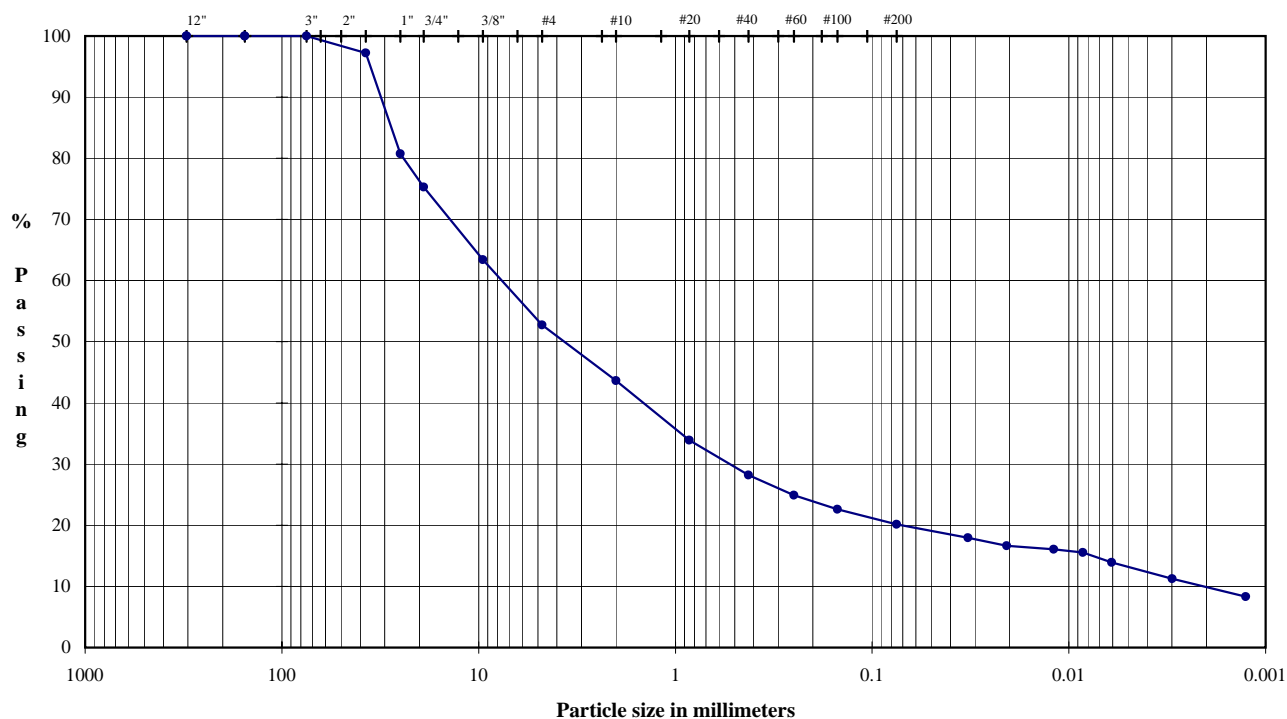
DESCRIPTION: Very pale brown clayey gravel with sand

USCS: GC

TECH: MKS
DATE: 10/12/2005
REVIEW: MB

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

ASTM D421, D422, D4318

PROJECT NAME: **G&K/Tyrone Stockpile Geotech/AZ**SAMPLE ID: **T-5**Depth: **164.0-165.0**TYPE: **Bag**

	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

Particle Size

Particle Size

(mm)

% Passing

Classification

Percentage

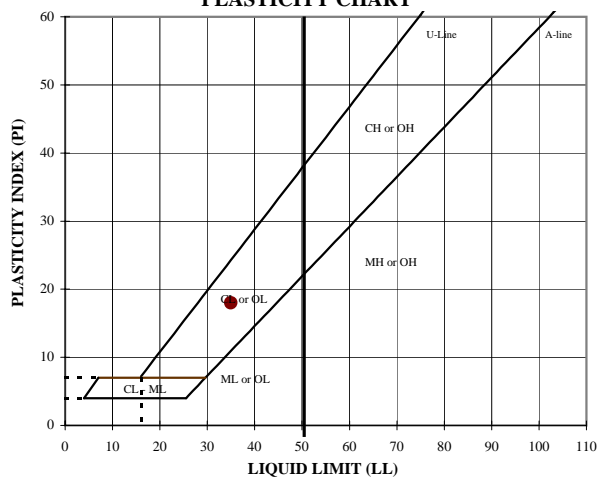
U.S. Standard Sieves Sizes and Numbers

12.0"	304.8	100.0	Cobbles	0.00
12.0"	304.8	100.0		
6.0"	154.2	100.0		
6.0"	154.2	100.0		
3.0"	75.0	100.0	Coarse Gravel	24.72
1.5"	37.5	97.2		
1.0"	25.0	80.7		
0.75"	19.0	75.3		
0.375"	9.5	63.4	Fine Gravel	22.53
#4	4.8	52.8		
#10	2.00	43.7		
#20	0.85	33.9		
#40	0.43	28.2	Medium Sand	15.46
#60	0.25	24.9		
#100	0.15	22.6		
#200	0.075	20.2		
			Fine Sand	8.04

Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	20.15
0.033	18.0		
0.021	16.6		
0.012	16.1		
0.0085	15.5		
0.0061	13.9		
0.0030	11.3		
0.0013	8.3		

PLASTICITY CHART



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG
14.0	35	17	18	2.85

DESCRIPTION: Weak red clayey gravel with sand

USCS: GC

TECH: SW

DATE: 9/29/2005

REVIEW: MB

APPENDIX D

LABORATORY SHEAR STRENGTH TESTS

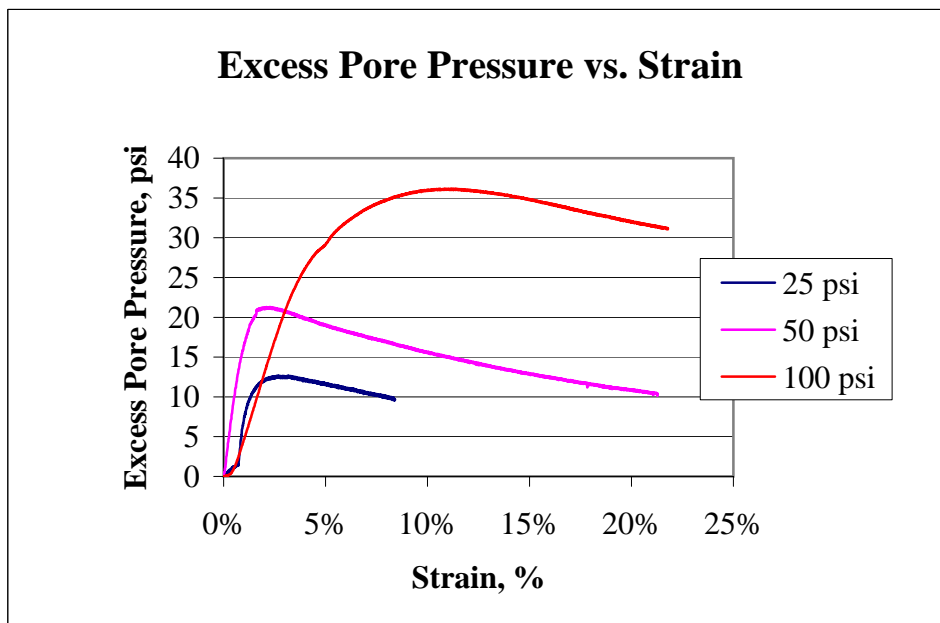
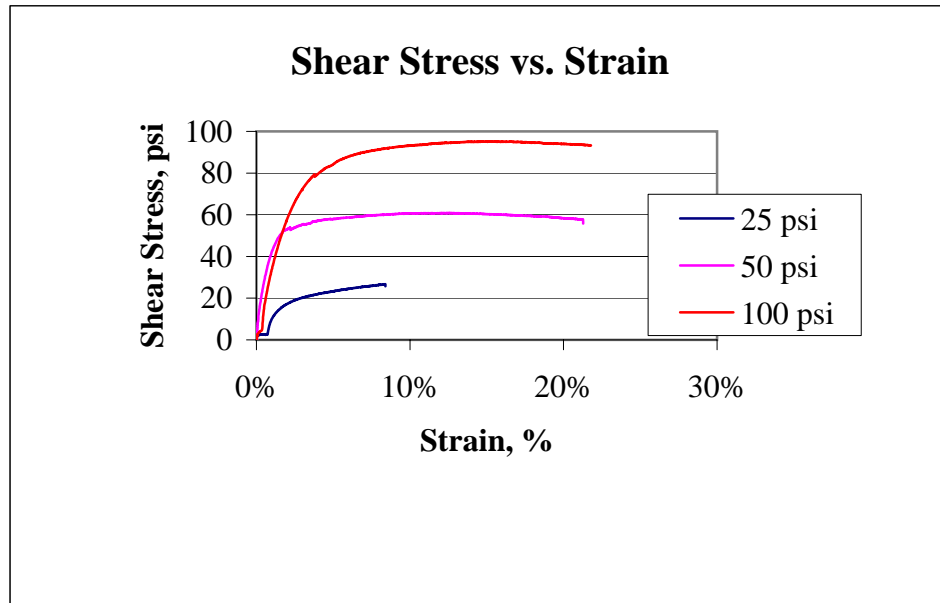
APPENDIX D-1

2000 TRIAXIAL SHEAR TESTS

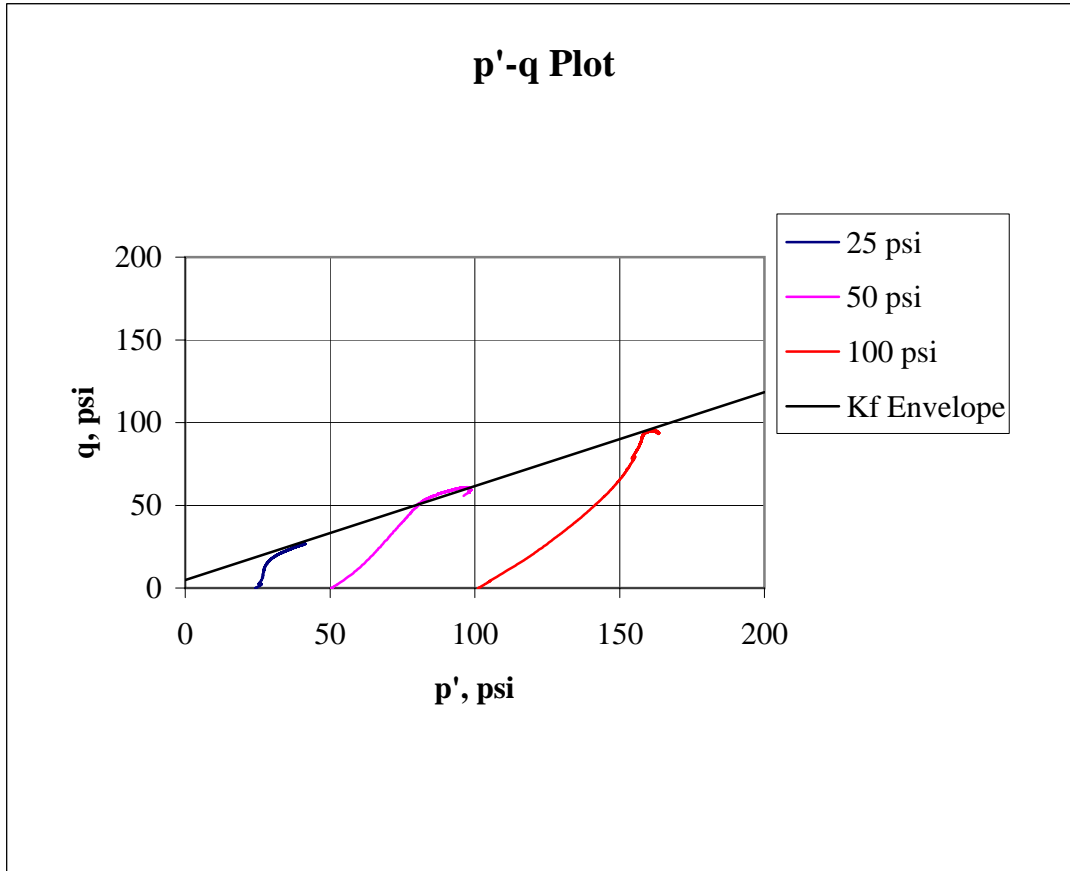
Sample # =	Bag BD		Sample # =	Bag BD		Sample # =	Bag BD	
Point # =	1		Point # =	2		Point # =	3	
	Initial			Initial			Initial	
Length =	20.35	cm	Length =	20.35	cm	Length =	20.30	cm
Diameter =	10.10	cm	Diameter =	10.10	cm	Diameter =	10.10	cm
Wet Weight =	3407.40	g	Wet Weight =	3407.40	g	Wet Weight =	3402.70	g
Area =	80.1	sq.cm	Area =	80.1	sq.cm	Area =	80.1	sq.cm
Sample Area =	12.42	sq. in.	Sample Area =	12.42	sq. in.	Sample Area =	12.42	sq. in.
Volume =	1630.4	cc	Volume =	1630.4	cc	Volume =	1626.4	cc
Moisture Content =	5.3%		Moisture Content =	5.3%		Moisture Content =	5.2%	
Specific Gravity =	2.7		Specific Gravity =	2.7		Specific Gravity =	2.7	
Dry Weight of Solids =	3235.90	g	Dry Weight of Solids =	3235.90	g	Dry Weight of Solids =	3234.51	g
Wet Density =	2.09	g/cc	Wet Density =	2.09	g/cc	Wet Density =	2.09	g/cc
Dry Density =	1.98	g/cc	Dry Density =	1.98	g/cc	Dry Density =	1.99	g/cc
Wet Density =	130.4	pcf	Wet Density =	130.4	pcf	Wet Density =	130.6	pcf
Dry Density =	123.8	pcf	Dry Density =	123.8	pcf	Dry Density =	124.1	pcf
Cell Pressure =	100	psi	Cell Pressure =	100	psi	Cell Pressure =	110	psi
Back Pressure =	75	psi	Back Pressure =	50	psi	Back Pressure =	10	psi
Confining Pressure =	25	psi	Confining Pressure =	50	psi	Confining Pressure =	100	psi

Notes:

<div>Golder Associates, Inc. Denver, Colorado</div>			<div>Title:</div> <div>TRIAXIAL SHEAR TEST REPORT SAMPLE DATA AND CALCULATIONS</div>			
<div>Job Short Title:</div> <div>PD/TYRONE/NM</div>						
<div>Sample No.</div> <div>GTP3-2</div>	<div>Depth</div> <div>n/a</div>	<div>Reviewed:</div>	<div>Date:</div> <div>Feb., 2000</div>	<div>Job Number:</div> <div>993-2546</div>		<div>Figure:</div> <div>1</div>



Golder Associates, Inc. Denver, Colorado			Title: CU TRIAXIAL SHEAR DATA STRESS AND Δ PORE PRESSURE PLOTS			
Job Short Title: PD/TYRONE/NM						
Sample No. GTP3-2	Depth n/a	Reviewed:	Date: Feb., 2000	Job Number: 993-2546	Figure:	2

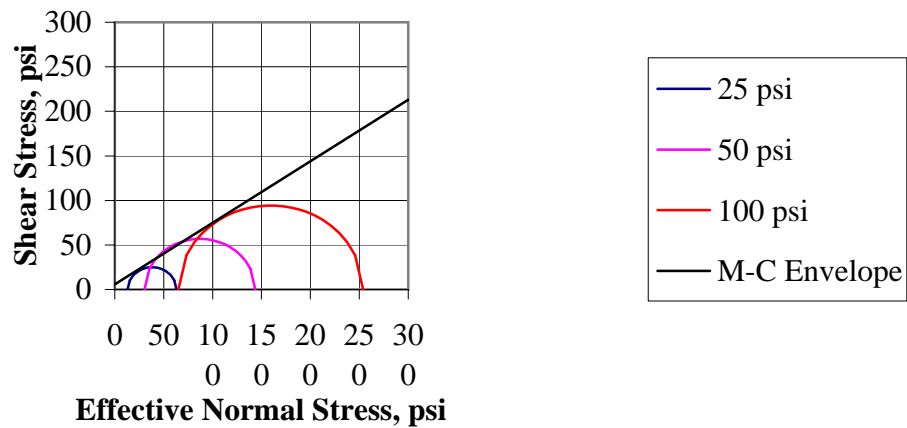


Stress Path Parameters

$\psi' = 29.6$ degrees
 $a' = 4.7$ psi

Golder Associates, Inc. Denver, Colorado			Title: CU TRIAXIAL SHEAR DATA STRESS PATH PLOT		
Job Short Title: PD/TYRONE/NM					
Sample No. GTP3-2	Depth n/a	Reviewed:	Date: Feb., 2000	Job Number: 993-2546	Figure: 3

Mohr Circle Diagram Effective Stress Parameters



Mohr-Coulomb Parameters

$$\phi' = 34.6 \text{ degrees}$$

$$c' = 5.8 \text{ psi}$$

Golder Associates, Inc. Denver, Colorado			Title: CU TRIAXIAL SHEAR DATA MOHR CIRCLE DIAGRAM		
Job Short Title: PD/TYRONE/NM					
Sample No. GTP3-2	Depth n/a	Reviewed:	Date: Feb., 2000	Job Number: 993-2546	Figure: 4

Sample # = GTP06/03

Point # = 1

Initial

Length = 20.40 cm
Diameter = 10.10 cm
Wet Weight = 3510.40 g
Area = 80.1 sq.cm
Sample Area = 12.42 sq. in.

Volume = 1634.4 cc
Moisture Content = 7.0%
Specific Gravity = 2.65
Dry Weight of Solids = 3281.67 g
Wet Density = 2.15 g/cc
Dry Density = 2.01 g/cc
Wet Density = 134.0 pcf
Dry Density = 125.3 pcf

Cell Pressure = 100 psi
Back Pressure = 75 psi
Confining Pressure = 25 psi

Notes:

Sample # = GTP06/03

Point # = 2

Initial

Length = 20.40 cm
Diameter = 10.10 cm
Wet Weight = 3510.40 g
Area = 80.1 sq.cm
Sample Area = 12.42 sq. in.

Volume = 1634.4 cc
Moisture Content = 7.0%
Specific Gravity = 2.65
Dry Weight of Solids = 3281.67 g
Wet Density = 2.15 g/cc
Dry Density = 2.01 g/cc
Wet Density = 134.0 pcf
Dry Density = 125.3 pcf

Cell Pressure = 100 psi
Back Pressure = 50 psi
Confining Pressure = 50 psi

Sample # = GTP06/03

Point # = 3

Initial

Length = 20.40 cm
Diameter = 10.10 cm
Wet Weight = 3510.00 g
Area = 80.1 sq.cm
Sample Area = 12.42 sq. in.

Volume = 1634.4 cc
Moisture Content = 7.0%
Specific Gravity = 2.65
Dry Weight of Solids = 3281.29 g
Wet Density = 2.15 g/cc
Dry Density = 2.01 g/cc
Wet Density = 134.0 pcf
Dry Density = 125.3 pcf

Cell Pressure = 110 psi
Back Pressure = 10 psi
Confining Pressure = 100 psi

Golder Associates, Inc.
Denver, Colorado

Job Short Title:

Phelps Dodge/Tyrone

Title:

TRIAXIAL SHEAR TEST REPORT
SAMPLE DATA AND CALCULATIONS

Sample No.

GTP06/03

Depth

-

Reviewed:

Date:

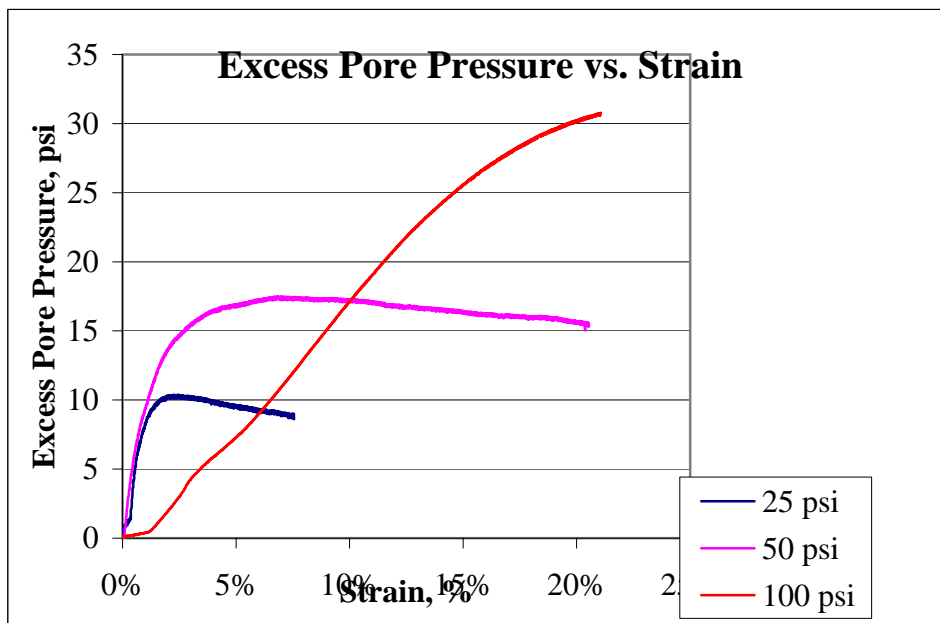
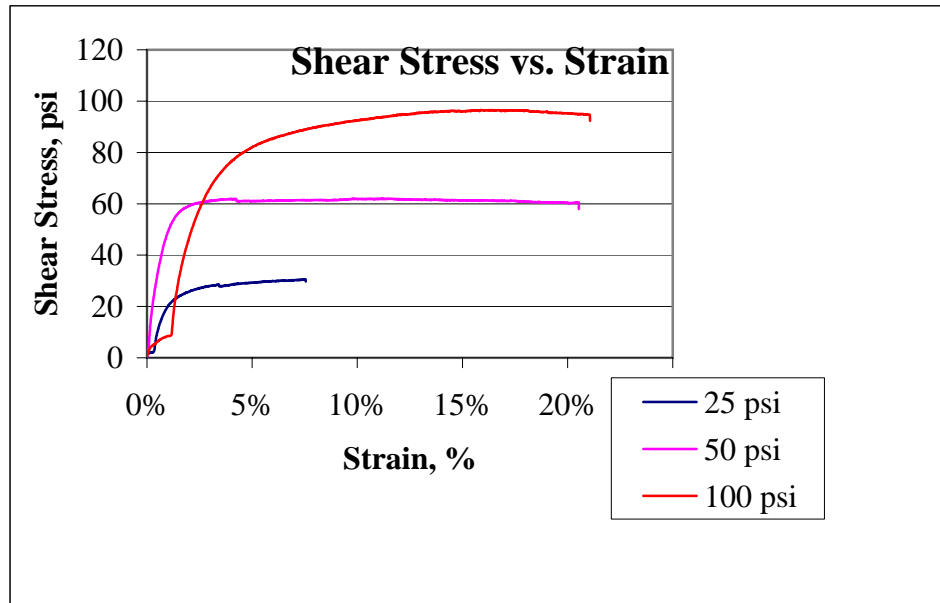
Feb. 2000

Job Number:

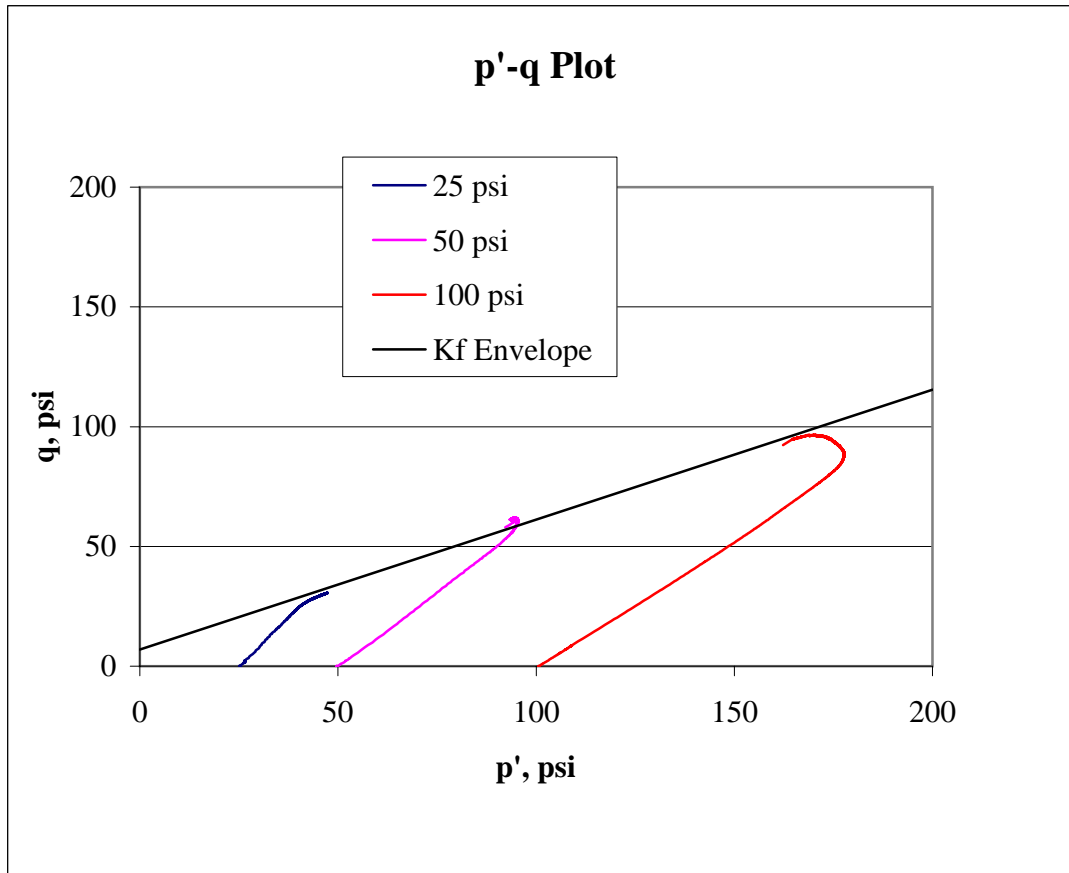
993-2546

Figure:

1



Golder Associates, Inc. Denver, Colorado			Title: CU TRIAXIAL SHEAR DATA STRESS AND Δ PORE PRESSURE PLOTS			
Job Short Title: Phelps Dodge/Tyrone						
Sample No. GTP06/03	Depth -	Reviewed:	Date: Feb. 2000	Job Number: 993-2546	Figure:	2

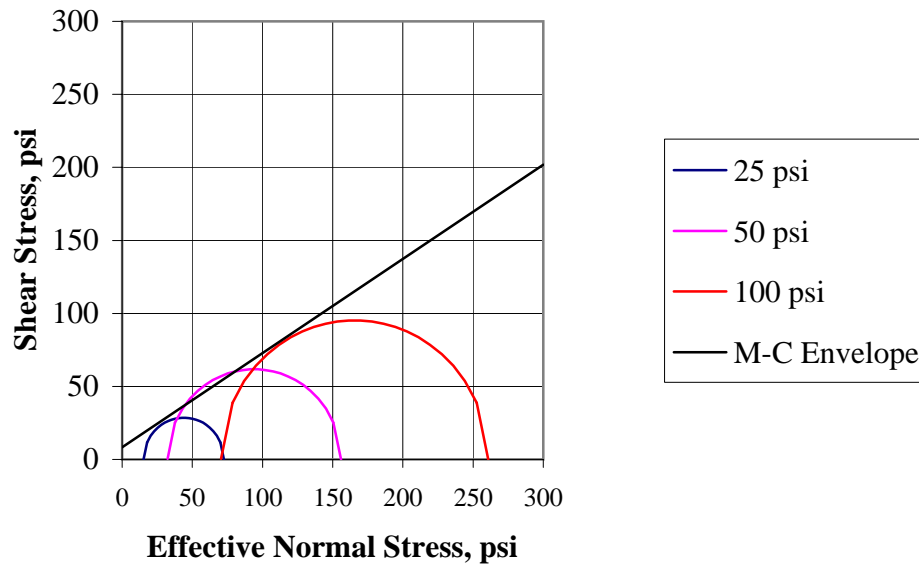


Stress Path Parameters

$\psi' = 28.5$ degrees
 $a' = 7.0$ psi

Golder Associates, Inc. Denver, Colorado			Title: CU TRIAXIAL SHEAR DATA STRESS PATH PLOT		
Job Short Title: Phelps Dodge/Tyrone					
Sample No.	Depth	Reviewed:	Date:	Job Number:	Figure:
GTP06/03	-		Feb. 2000	993-2546	3

Mohr Circle Diagram Effective Stress Parameters



Mohr-Coulomb Parameters

$\phi' = 32.8$ degrees
 $c' = 8.3$ psi

Golder Associates, Inc. Denver, Colorado			Title: CU TRIAXIAL SHEAR DATA MOHR CIRCLE DIAGRAM		
Job Short Title: Phelps Dodge/Tyrone					
Sample No. GTP06/03	Depth -	Reviewed:	Date: Feb. 2000	Job Number: 993-2546	Figure: 4

APPENDIX D-2

2001 TRIAXIAL SHEAR TESTS

Sample # = TYTP 01-2
Point # = 1

Initial
Length = 20.35 cm
Diameter = 10.14 cm
Wet Weight = 3386.30 g
Area = 80.8 sq.cm
Sample Area = 12.52 sq. in.

Volume = 1643.3 cc
Moisture Content = 5.8%
Specific Gravity = NA
Dry Weight of Solids = 3200.66 g
Wet Density = 2.06 g/cc
Dry Density = 1.95 g/cc
Wet Density = 128.6 pcf
Dry Density = 121.5 pcf

Cell Pressure = 100 psi
Back Pressure = 75 psi
Confining Pressure = 25 psi

Sample # = TYTP 01-2
Point # = 2

Initial
Length = 20.35 cm
Diameter = 10.14 cm
Wet Weight = 3386.30 g
Area = 80.8 sq.cm
Sample Area = 12.52 sq. in.

Volume = 1643.3 cc
Moisture Content = 5.8%
Specific Gravity = NA
Dry Weight of Solids = 3200.66 g
Wet Density = 2.06 g/cc
Dry Density = 1.95 g/cc
Wet Density = 128.6 pcf
Dry Density = 121.5 pcf

Cell Pressure = 100 psi
Back Pressure = 50 psi
Confining Pressure = 50 psi

Sample # = TYTP 01-2
Point # = 3

Initial
Length = 20.35 cm
Diameter = 10.14 cm
Wet Weight = 3386.30 g
Area = 80.8 sq.cm
Sample Area = 12.52 sq. in.

Volume = 1643.3 cc
Moisture Content = 5.8%
Specific Gravity = NA
Dry Weight of Solids = 3200.66 g
Wet Density = 2.06 g/cc
Dry Density = 1.95 g/cc
Wet Density = 128.6 pcf
Dry Density = 121.5 pcf

Cell Pressure = 120 psi
Back Pressure = 20 psi
Confining Pressure = 100 psi

- Notes: 1. Staged test at 25, 50 and 100 psi confining pressure
2. Sample remolded to 122.6 lb/ft³ and 6% moisture content

Golder Associates, Inc.
Denver, Colorado

Job Short Title:

G&K/TYRONE CLOSURE HEARING/AZ

Title:

TRIAXIAL SHEAR TEST REPORT
SAMPLE DATA AND CALCULATIONS

Sample No.

TYTP 01-2

Reviewed:

GE

Date:

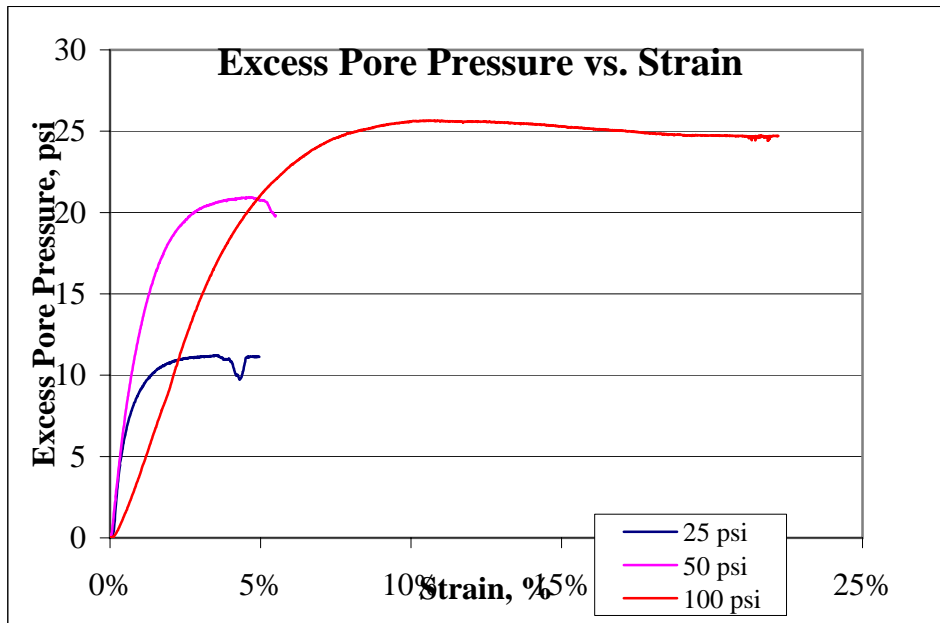
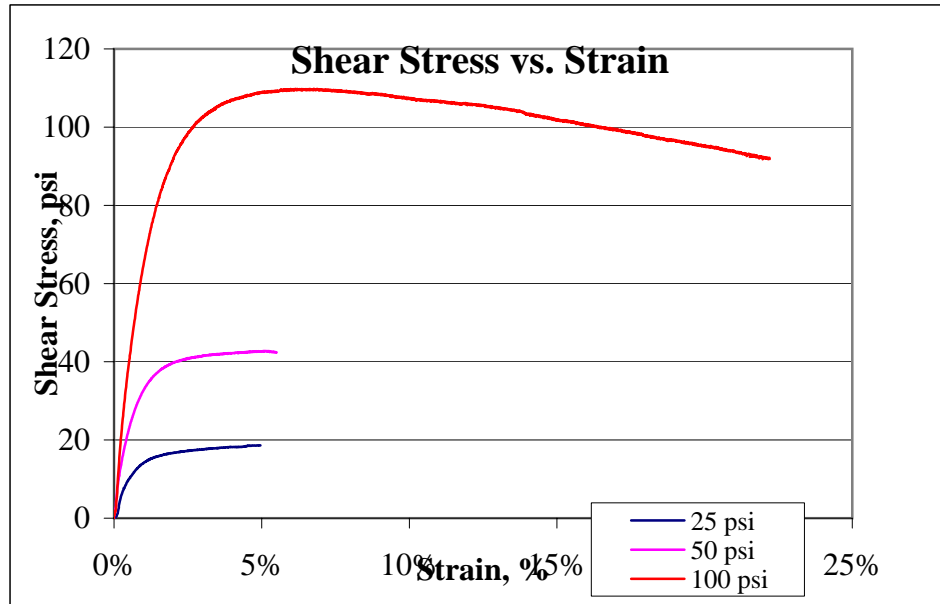
11/16/2001

Job Number:

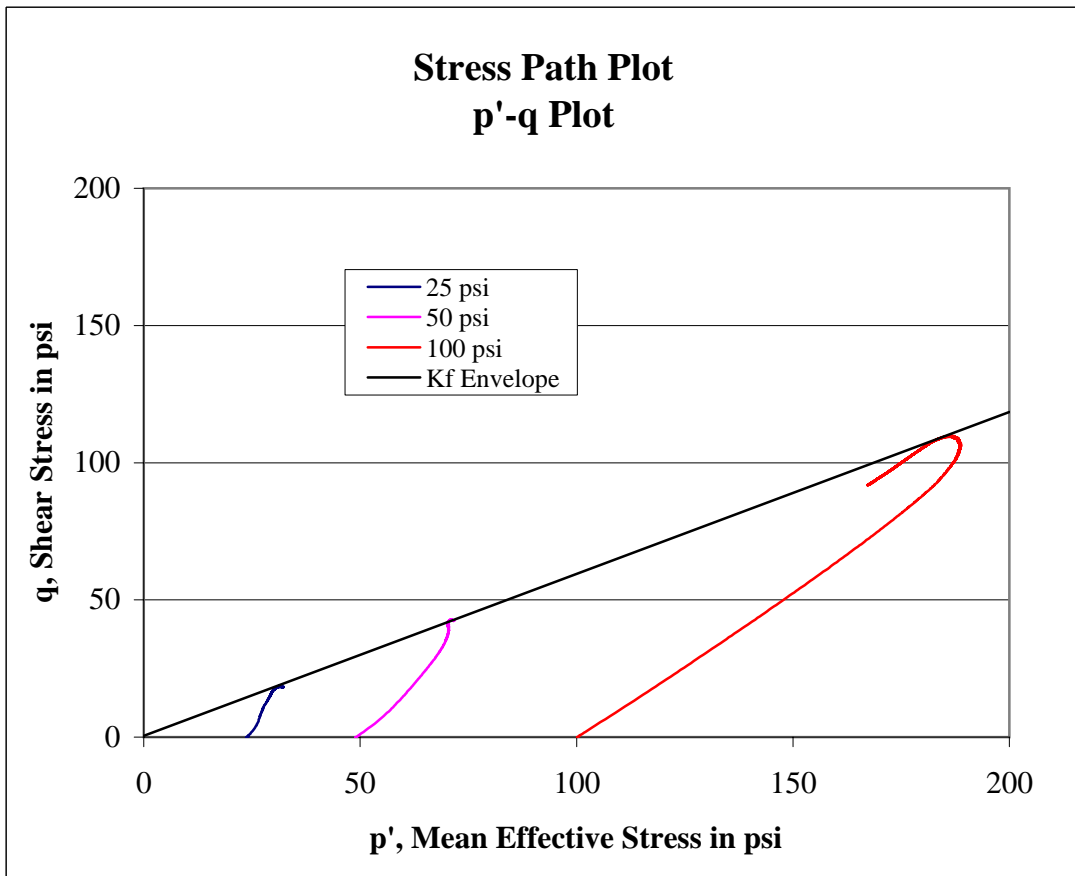
013-1595

Figure:

1



Golder Associates, Inc. Denver, Colorado		Title: CU TRIAXIAL SHEAR DATA STRESS AND Δ PORE PRESSURE PLOTS			
Job Short Title: G&K/TYRONE CLOSURE HEARING/AZ					
Sample No. TYTP 01-2	Reviewed: GE	Date: 11/16/01	Job Number: 013-1595	Figure:	2

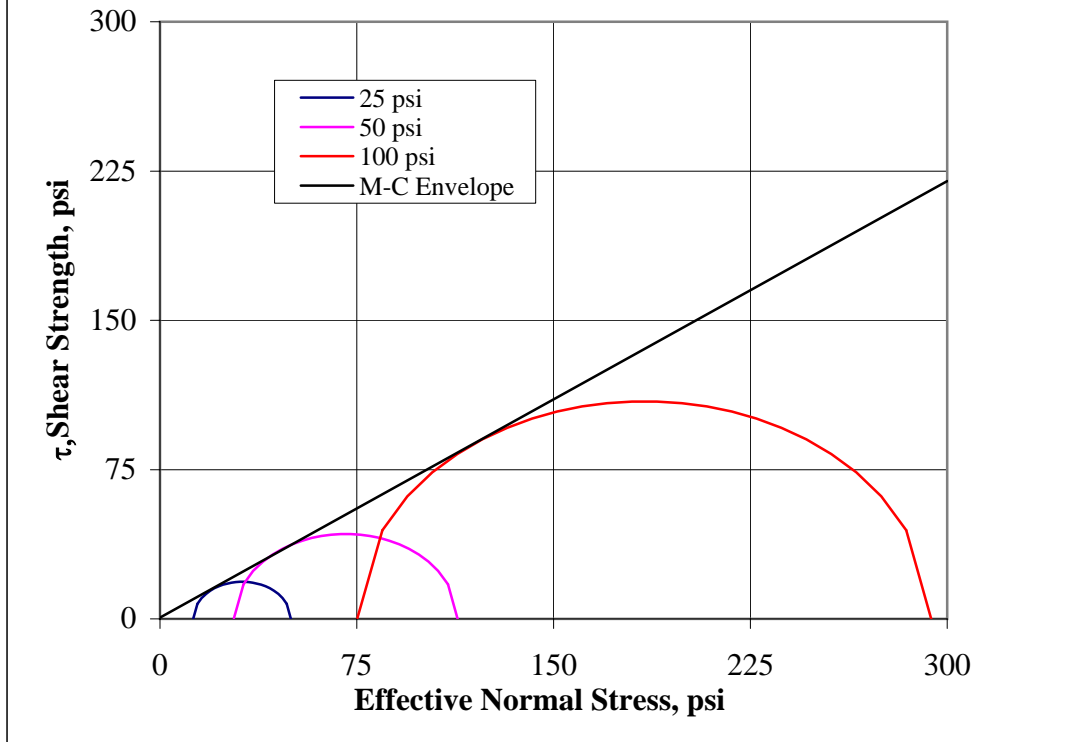


Stress Path Parameters

$\psi' = 30.5$ degrees
 $a' = 0.5$ psi

Golder Associates, Inc. Denver, Colorado		Title: CU TRIAXIAL SHEAR DATA STRESS PATH PLOT		
Job Short Title: G&K/TYRONE CLOSURE HEARING/AZ				
Sample No. TYTP 01-2	Reviewed: GE	Date: 11/16/01	Job Number: 013-1595	Figure: 3

Mohr Circle Diagram Effective Stress Parameters



Mohr-Coulomb Parameters

$\phi' = 36.2$ degrees
 $c' = 0.6$ psi

Golder Associates, Inc. Denver, Colorado		Title: CU TRIAXIAL SHEAR DATA MOHR CIRCLE DIAGRAM		
Job Short Title: G&K/TYRONE CLOSURE HEARING/AZ				
Sample No. TYTP 01-2	Reviewed: GE	Date: 11/16/01	Job Number: 013-1595	Figure: 4

Sample # = TYTP 01-04
Point # = 1

Initial
Length = 20.55 cm
Diameter = 10.11 cm
Wet Weight = 3362.40 g
Area = 80.3 sq.cm
Sample Area = 12.44 sq. in.

Volume = 1649.6 cc
Moisture Content = 5.8%
Specific Gravity = NA
Dry Weight of Solids = 3178.07 g
Wet Density = 2.04 g/cc
Dry Density = 1.93 g/cc
Wet Density = 127.2 pcf
Dry Density = 120.2 pcf

Cell Pressure = 100 psi
Back Pressure = 75 psi
Confining Pressure = 25 psi

Sample # = TYTP 01-04
Point # = 2

Initial
Length = 20.55 cm
Diameter = 10.11 cm
Wet Weight = 3362.40 g
Area = 80.3 sq.cm
Sample Area = 12.44 sq. in.

Volume = 1649.6 cc
Moisture Content = 5.8%
Specific Gravity = NA
Dry Weight of Solids = 3178.07 g
Wet Density = 2.04 g/cc
Dry Density = 1.93 g/cc
Wet Density = 127.2 pcf
Dry Density = 120.2 pcf

Cell Pressure = 100 psi
Back Pressure = 50 psi
Confining Pressure = 50 psi

Sample # = TYTP 01-04
Point # = 3

Initial
Length = 20.55 cm
Diameter = 10.11 cm
Wet Weight = 3362.40 g
Area = 80.3 sq.cm
Sample Area = 12.44 sq. in.

Volume = 1649.6 cc
Moisture Content = 5.8%
Specific Gravity = NA
Dry Weight of Solids = 3178.07 g
Wet Density = 2.04 g/cc
Dry Density = 1.93 g/cc
Wet Density = 127.2 pcf
Dry Density = 120.2 pcf

Cell Pressure = 120 psi
Back Pressure = 20 psi
Confining Pressure = 100 psi

Notes: 1. Sample remolded to 122 lb/ft³ and 6% mc

Golder Associates, Inc.
Denver, Colorado

Job Short Title:

G&K/TYRONE CLOSURE HEARING/AZ

Title:

TRIAXIAL SHEAR TEST REPORT
SAMPLE DATA AND CALCULATIONS

Sample No.

TYTP 01-04

Reviewed:

GE

Date:

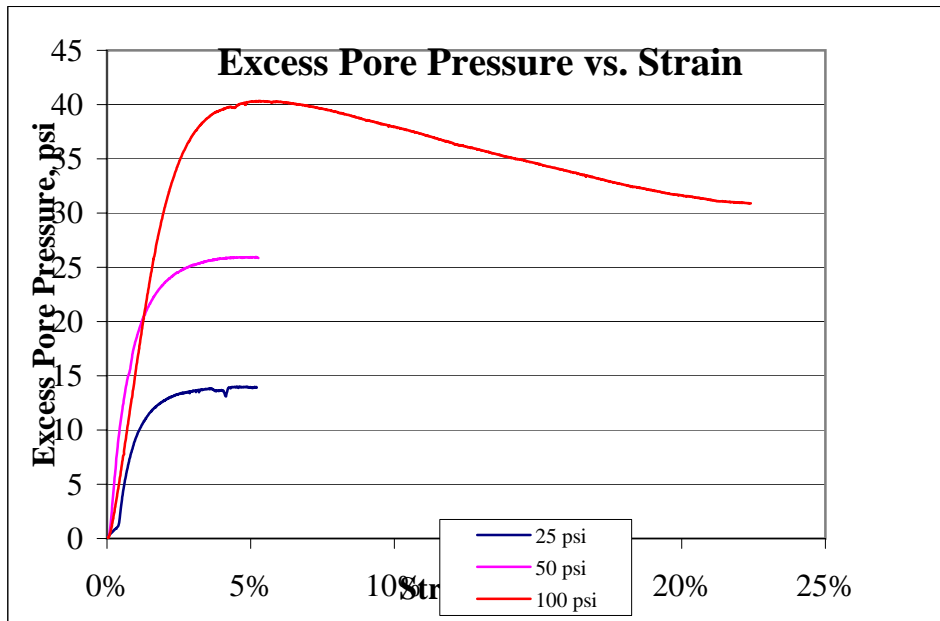
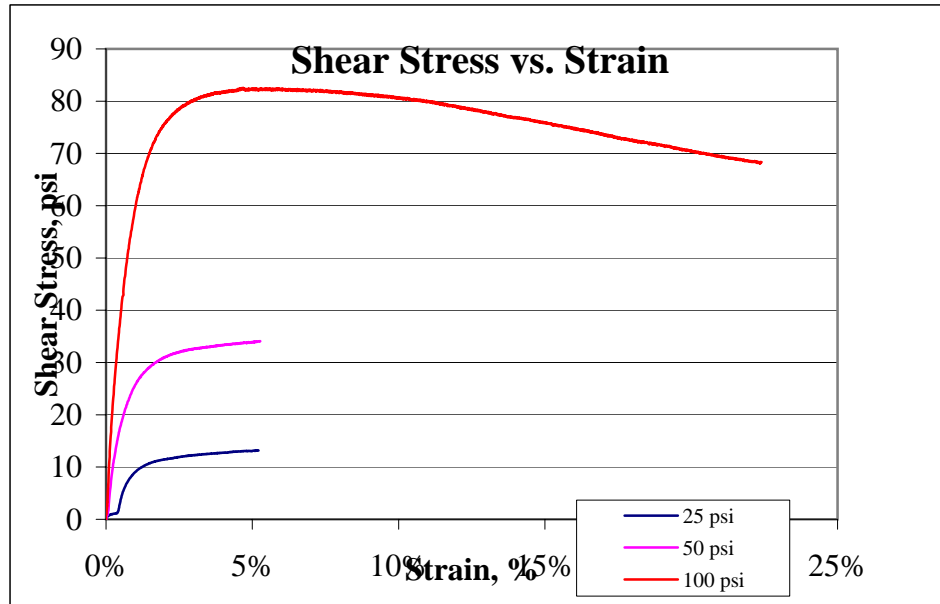
11/30/2001

Job Number:

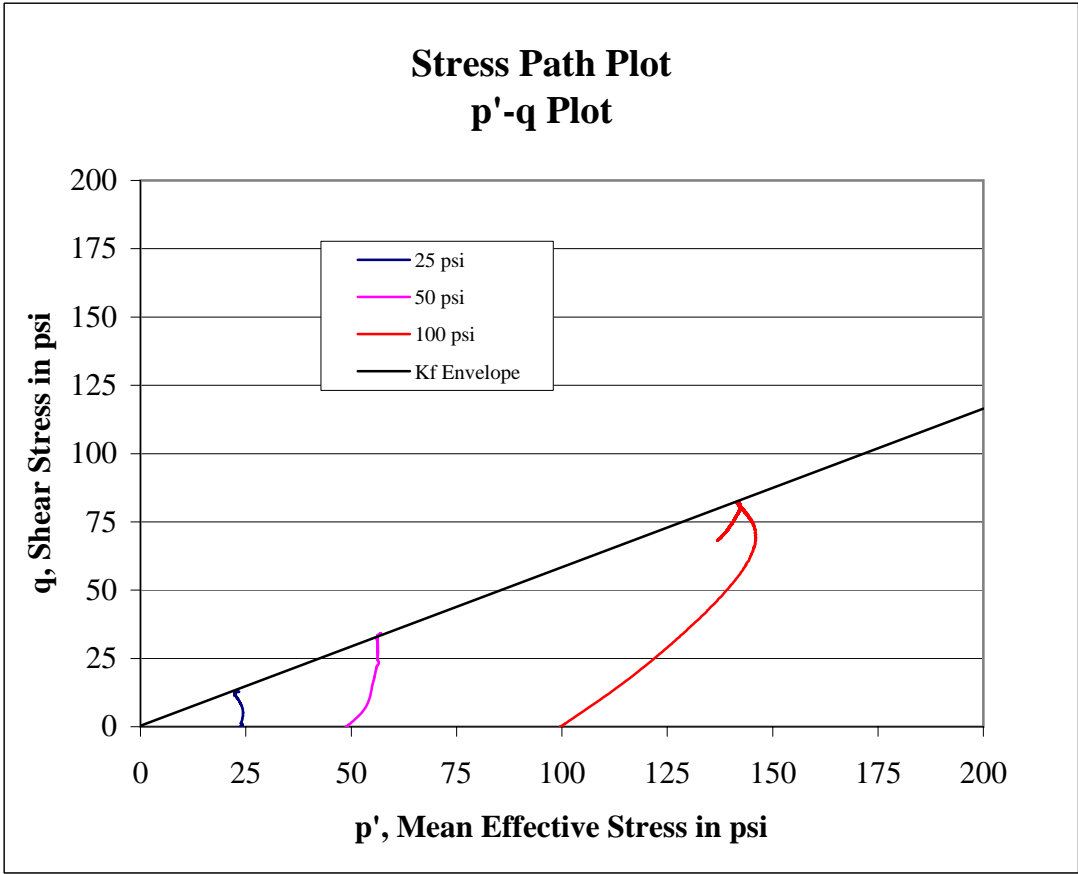
013-1595

Figure:

1



Golder Associates, Inc. Denver, Colorado		Title: CU TRIAXIAL SHEAR DATA STRESS AND Δ PORE PRESSURE PLOTS		
Job Short Title: G&K/TYRONE CLOSURE HEARING/AZ				
Sample No. TYTP 01-04	Reviewed: GE	Date: 11/30/01	Job Number: 013-1595	Figure: 2

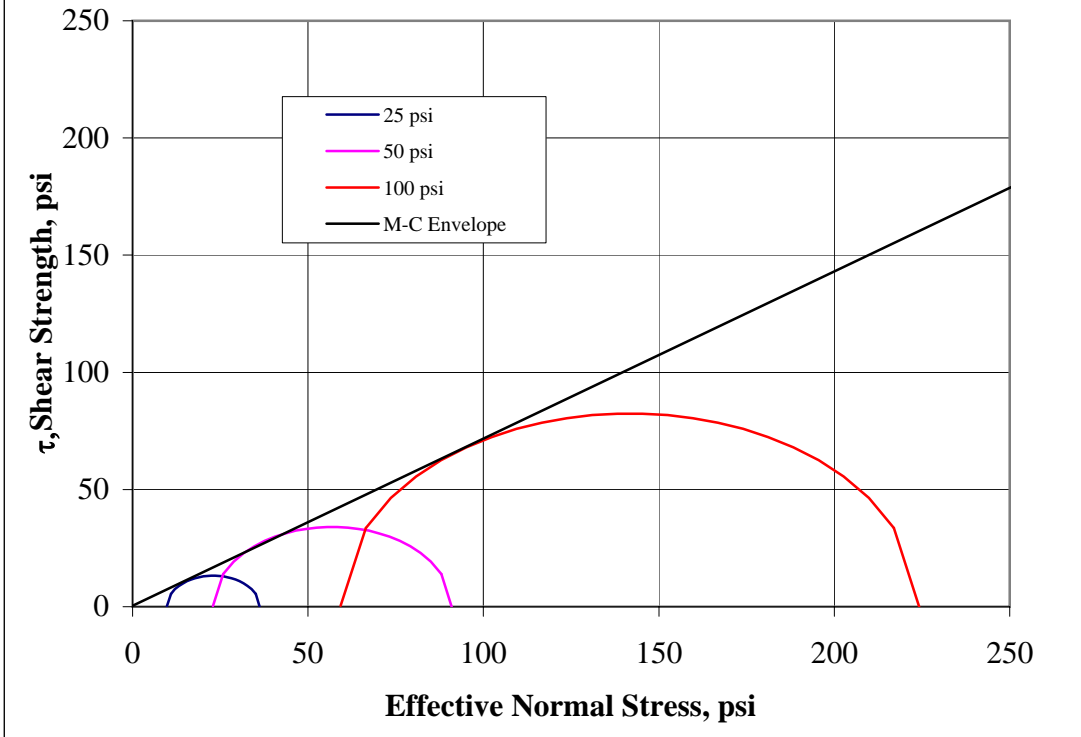


Stress Path Parameters

$\psi' = 30.1$ degrees
 $a' = 0.3$ psi

Golder Associates, Inc. Denver, Colorado		Title: CU TRIAXIAL SHEAR DATA STRESS PATH PLOT		
Job Short Title: G&K/TYRONE CLOSURE HEARING/AZ				
Sample No. TYTP 01-04	Reviewed: GE	Date: 11/30/01	Job Number: 013-1595	Figure: 3

Mohr Circle Diagram Effective Stress Parameters



Mohr-Coulomb Parameters

$\phi' = 35.5$ degrees
 $c' = 0.4$ psi

Golder Associates, Inc. Denver, Colorado		Title: CU TRIAXIAL SHEAR DATA MOHR CIRCLE DIAGRAM		
Job Short Title: G&K/TYRONE CLOSURE HEARING/AZ				
Sample No. TYTP 01-04	Reviewed: GE	Date: 11/30/01	Job Number: 013-1595	Figure: 4

Sample # = TYTP 01-7
Point # = 1

Initial
Length = 20.30 cm
Diameter = 10.13 cm
Wet Weight = 3387.40 g
Area = 80.6 sq.cm
Sample Area = 12.49 sq. in.

Volume = 1636.0 cc
Moisture Content = 6.3%
Specific Gravity = NA
Dry Weight of Solids = 3186.64 g
Wet Density = 2.07 g/cc
Dry Density = 1.95 g/cc
Wet Density = 129.2 pcf
Dry Density = 121.5 pcf

Cell Pressure = 100 psi
Back Pressure = 75 psi
Confining Pressure = 25 psi

Sample # = TYTP 01-7
Point # = 2

Initial
Length = 20.30 cm
Diameter = 10.13 cm
Wet Weight = 3387.40 g
Area = 80.6 sq.cm
Sample Area = 12.49 sq. in.

Volume = 1636.0 cc
Moisture Content = 6.3%
Specific Gravity = NA
Dry Weight of Solids = 3186.64 g
Wet Density = 2.07 g/cc
Dry Density = 1.95 g/cc
Wet Density = 129.2 pcf
Dry Density = 121.5 pcf

Cell Pressure = 100 psi
Back Pressure = 50 psi
Confining Pressure = 50 psi

Sample # = TYTP 01-7
Point # = 3

Initial
Length = 20.30 cm
Diameter = 10.13 cm
Wet Weight = 3387.40 g
Area = 80.6 sq.cm
Sample Area = 12.49 sq. in.

Volume = 1636.0 cc
Moisture Content = 6.3%
Specific Gravity = NA
Dry Weight of Solids = 3186.64 g
Wet Density = 2.07 g/cc
Dry Density = 1.95 g/cc
Wet Density = 129.2 pcf
Dry Density = 121.5 pcf

Cell Pressure = 120 psi
Back Pressure = 20 psi
Confining Pressure = 100 psi

- Notes: 1. Staged test at 25, 50 and 100 psi confining pressure
2. Sample remolded to 122.6 lb/ft³ and 6% moisture content

Golder Associates, Inc.
Denver, Colorado

Job Short Title:

G&K/TYRONE CLOSURE HEARING/AZ

Title:

TRIAXIAL SHEAR TEST REPORT
SAMPLE DATA AND CALCULATIONS

Sample No.

TYTP 01-7

Reviewed:

GE

Date:

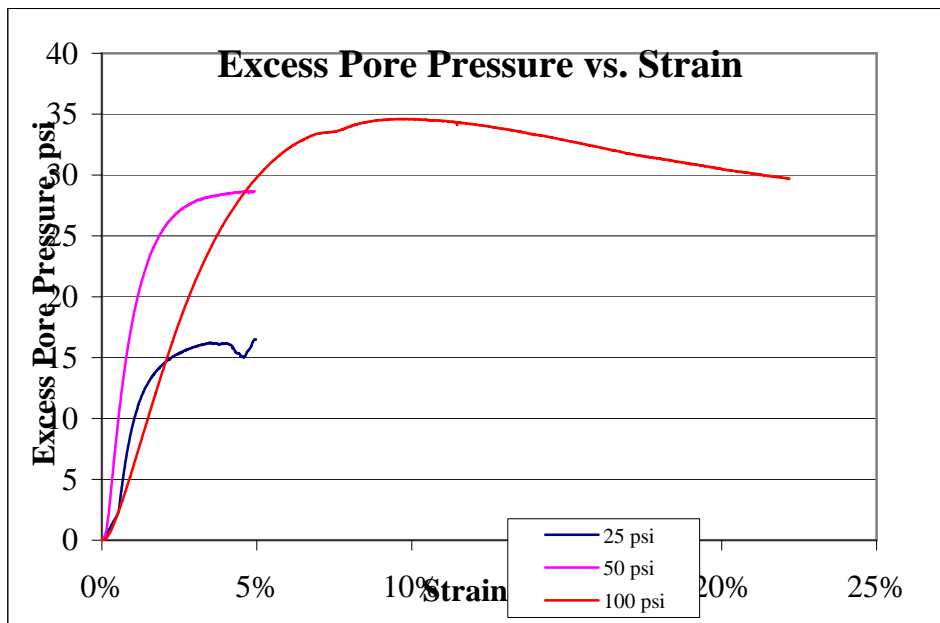
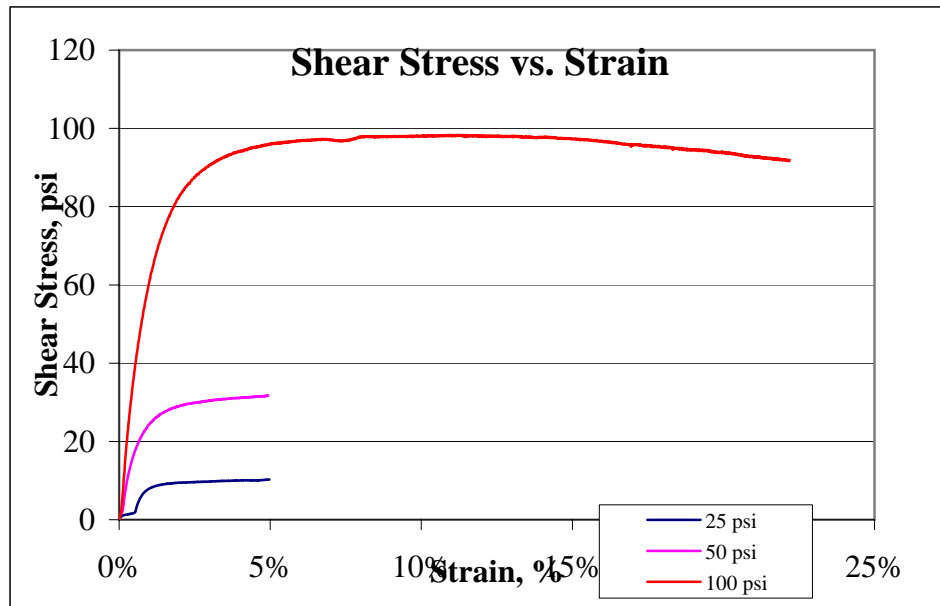
11/25/2001

Job Number:

013-1595

Figure:

1



Golder Associates, Inc.
Denver, Colorado

Title:

CU TRIAXIAL SHEAR DATA
STRESS AND Δ PORE PRESSURE PLOTS

Job Short Title:

G&K/TYRONE CLOSURE HEARING/AZ

Sample No.

TYTP 01-7

Reviewed:

GE

Date:

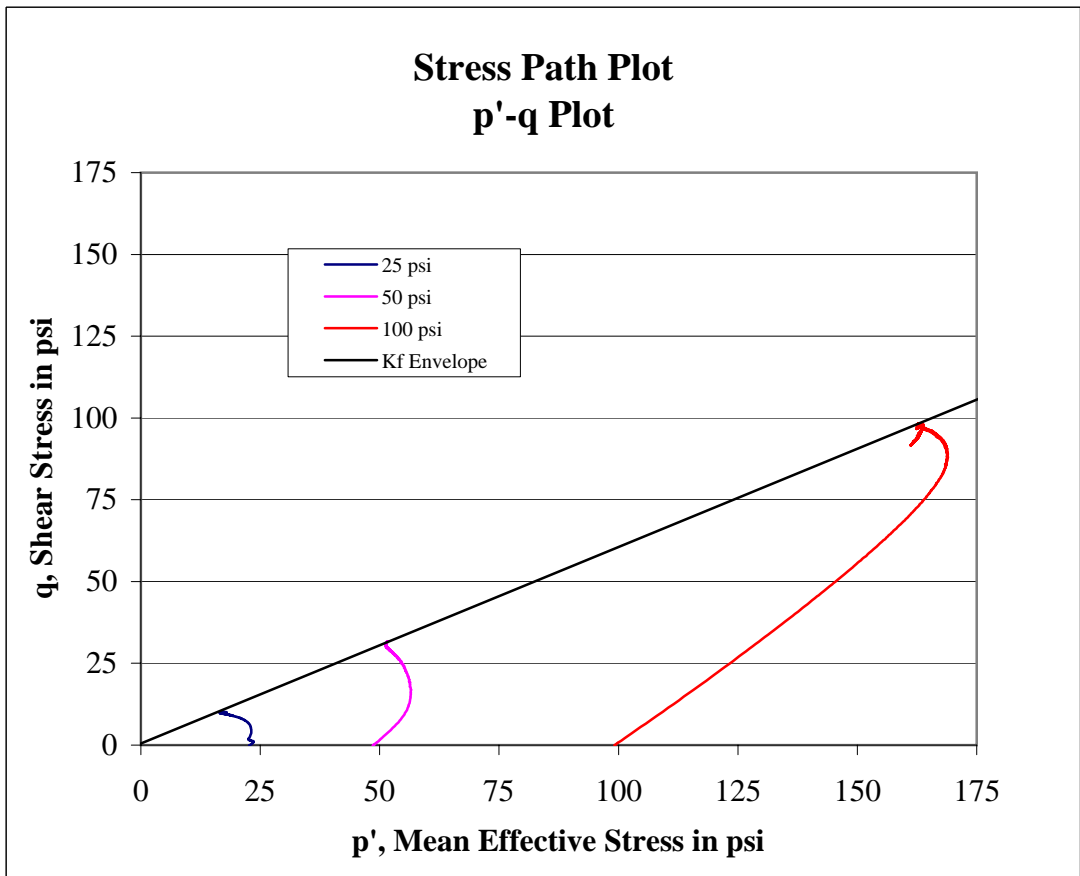
11/25/01

Job Number:

013-1595

Figure:

2

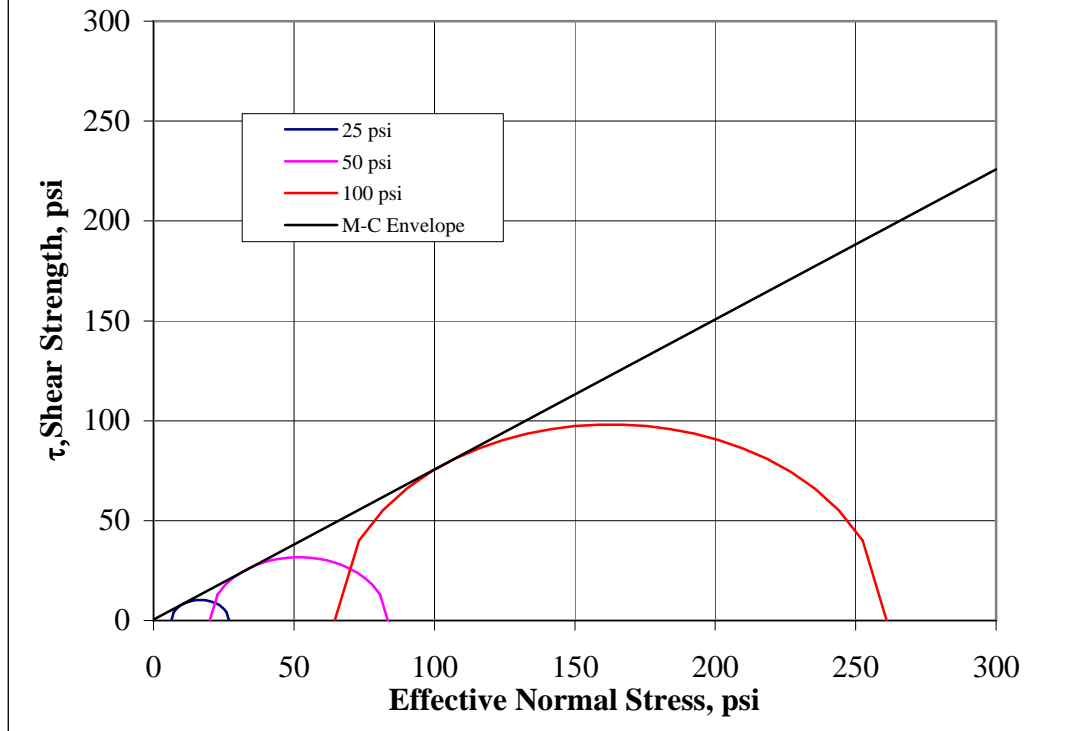


Stress Path Parameters

$\psi' = 31.0$ degrees
 $a' = 0.4$ psi

Golder Associates, Inc. Denver, Colorado		Title: CU TRIAXIAL SHEAR DATA STRESS PATH PLOT		
Job Short Title: G&K/TYRONE CLOSURE HEARING/AZ				
Sample No. TYTP 01-7	Reviewed: GE	Date: 11/25/01	Job Number: 013-1595	Figure: 3

Mohr Circle Diagram Effective Stress Parameters



Mohr-Coulomb Parameters

$\phi' = 36.9$ degrees
 $c' = 0.6$ psi

Golder Associates, Inc. Denver, Colorado		Title: CU TRIAXIAL SHEAR DATA MOHR CIRCLE DIAGRAM		
Job Short Title: G&K/TYRONE CLOSURE HEARING/AZ				
Sample No. TYTP 01-7	Reviewed: GE	Date: 11/25/01	Job Number: 013-1595	Figure: 4

Sample # = TYTP 01-9
 Point # = 1

Initial

Length = 20.35 cm
 Diameter = 10.12 cm
 Wet Weight = 3386.20 g
 Area = 80.4 sq.cm
 Sample Area = 12.47 sq. in.

Volume = 1636.8 cc
 Moisture Content = 6.1%
 Specific Gravity = NA
 Dry Weight of Solids = 3191.52 g
 Wet Density = 2.07 g/cc
 Dry Density = 1.95 g/cc
 Wet Density = 129.1 pcf
 Dry Density = 121.7 pcf

Cell Pressure = 100 psi
 Back Pressure = 75 psi
 Confining Pressure = 25 psi

Notes: 1. Sample remolded to 122.6 lb/ft³ and 6% mc
 2. Soil is gravelly clay material

Sample # = TYTP 01-9
 Point # = 2

Initial

Length = 20.35 cm
 Diameter = 10.12 cm
 Wet Weight = 3386.20 g
 Area = 80.4 sq.cm
 Sample Area = 12.47 sq. in.

Volume = 1636.8 cc
 Moisture Content = 6.1%
 Specific Gravity = NA
 Dry Weight of Solids = 3191.52 g
 Wet Density = 2.07 g/cc
 Dry Density = 1.95 g/cc
 Wet Density = 129.1 pcf
 Dry Density = 121.7 pcf

Cell Pressure = 100 psi
 Back Pressure = 50 psi
 Confining Pressure = 50 psi

Sample # = TYTP 01-9
 Point # = 3

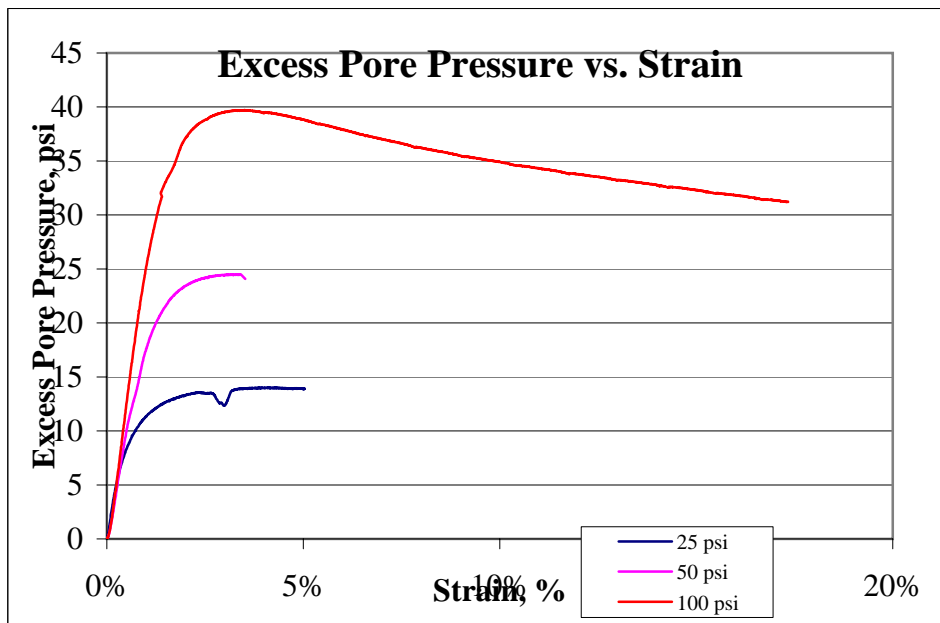
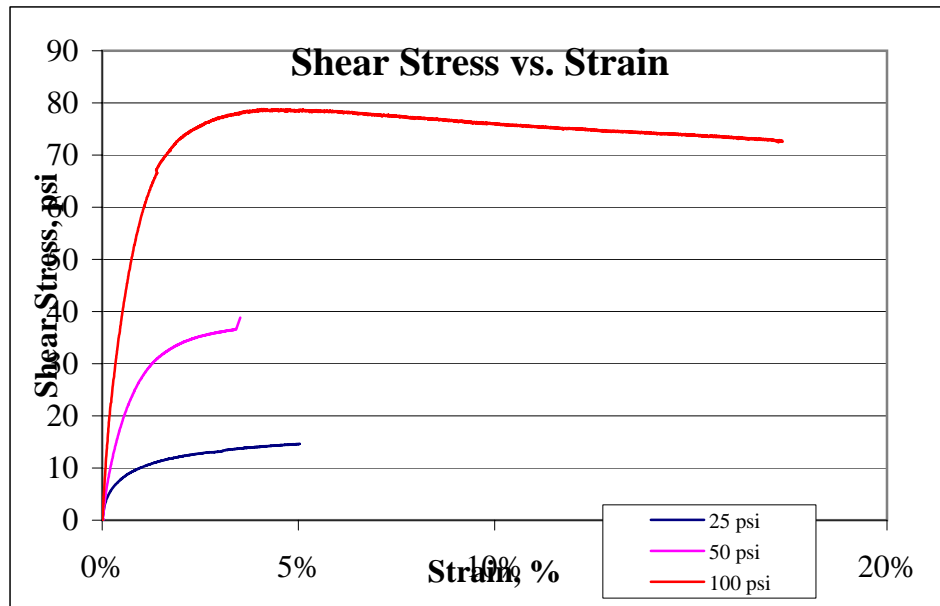
Initial

Length = 20.35 cm
 Diameter = 10.12 cm
 Wet Weight = 3386.20 g
 Area = 80.4 sq.cm
 Sample Area = 12.47 sq. in.

Volume = 1636.8 cc
 Moisture Content = 6.1%
 Specific Gravity = NA
 Dry Weight of Solids = 3191.52 g
 Wet Density = 2.07 g/cc
 Dry Density = 1.95 g/cc
 Wet Density = 129.1 pcf
 Dry Density = 121.7 pcf

Cell Pressure = 120 psi
 Back Pressure = 20 psi
 Confining Pressure = 100 psi

Golder Associates, Inc. Denver, Colorado		Title: TRIAXIAL SHEAR TEST REPORT SAMPLE DATA AND CALCULATIONS		
Job Short Title: G&K/TYRONE CLOSURE HEARING/AZ				
Sample No. TYTP 01-9	Reviewed: GE	Date: 12/5/2001	Job Number: 013-1595	Figure: 1



Golder Associates, Inc.
Denver, Colorado

Title:

CU TRIAXIAL SHEAR DATA
STRESS AND Δ PORE PRESSURE PLOTS

Job Short Title:

G&K/TYRONE CLOSURE HEARING/AZ

Sample No.

TYTP 01-9

Reviewed:

GE

Date:

12/05/01

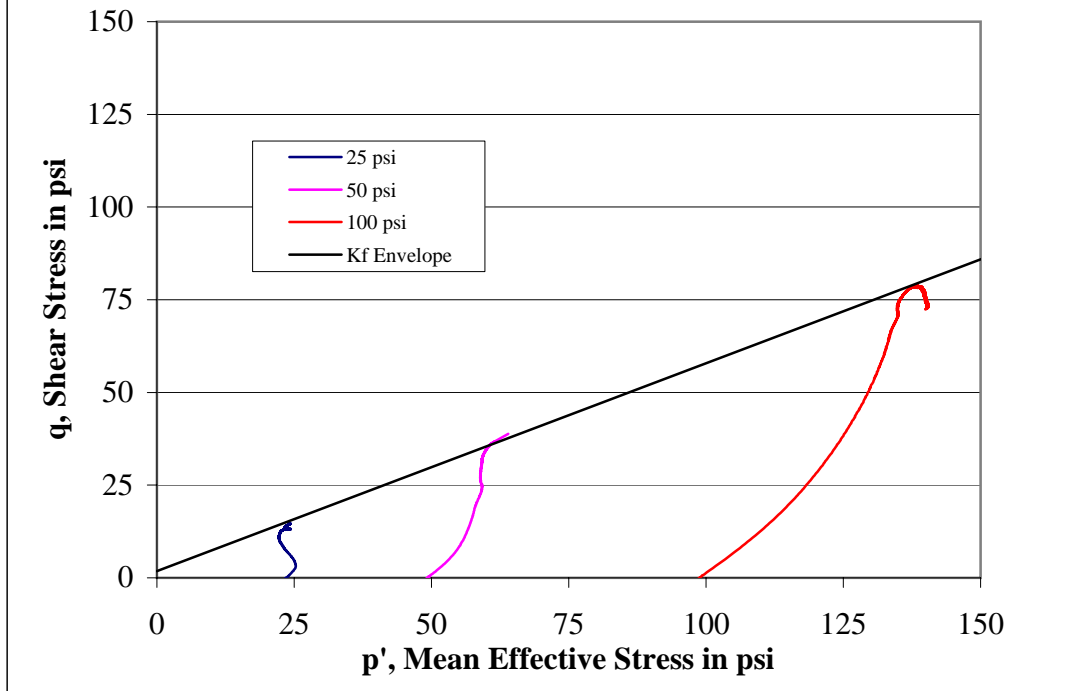
Job Number:

013-1595

Figure:

2

Stress Path Plot p'-q Plot

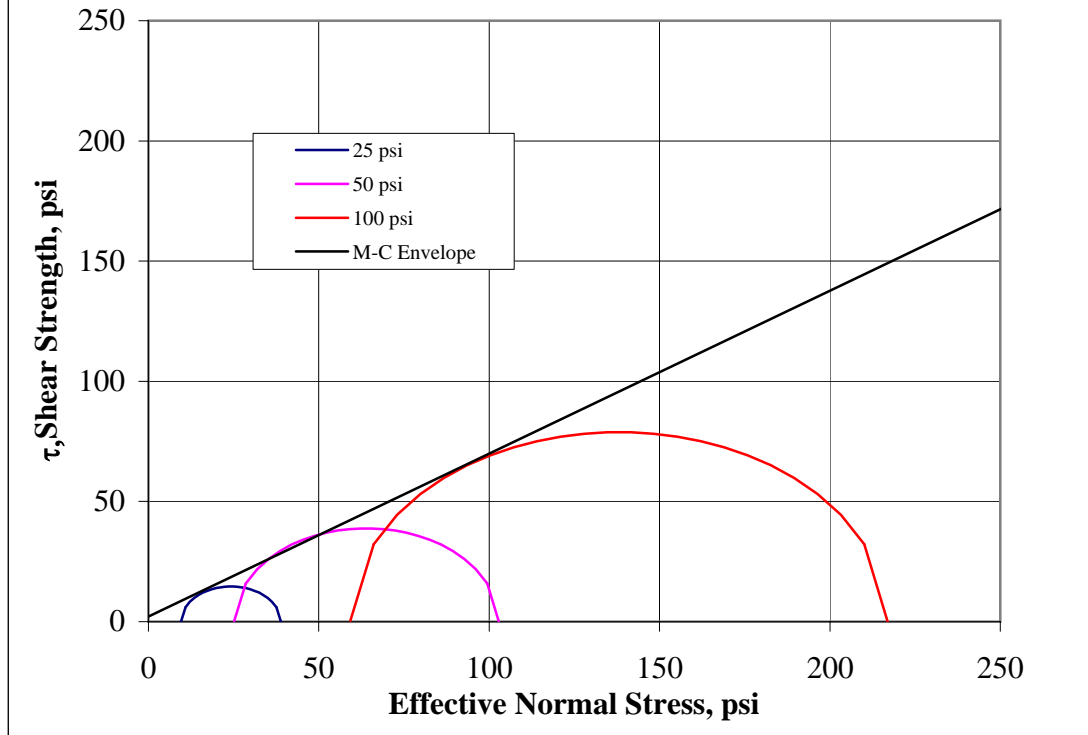


Stress Path Parameters

$\psi' = 29.3$ degrees
 $a' = 1.8$ psi

Golder Associates, Inc. Denver, Colorado		Title: CU TRIAXIAL SHEAR DATA STRESS PATH PLOT		
Job Short Title: G&K/TYRONE CLOSURE HEARING/AZ				
Sample No. TYTP 01-9	Reviewed: GE	Date: 12/05/01	Job Number: 013-1595	Figure: 3

Mohr Circle Diagram Effective Stress Parameters



Mohr-Coulomb Parameters

$\phi' = 34.1$ degrees
 $c' = 2.2$ psi

Golder Associates, Inc. Denver, Colorado		Title: CU TRIAXIAL SHEAR DATA MOHR CIRCLE DIAGRAM		
Job Short Title: G&K/TYRONE CLOSURE HEARING/AZ				
Sample No. TYTP 01-9	Reviewed: GE	Date: 12/05/01	Job Number: 013-1595	Figure: 4

APPENDIX D-3

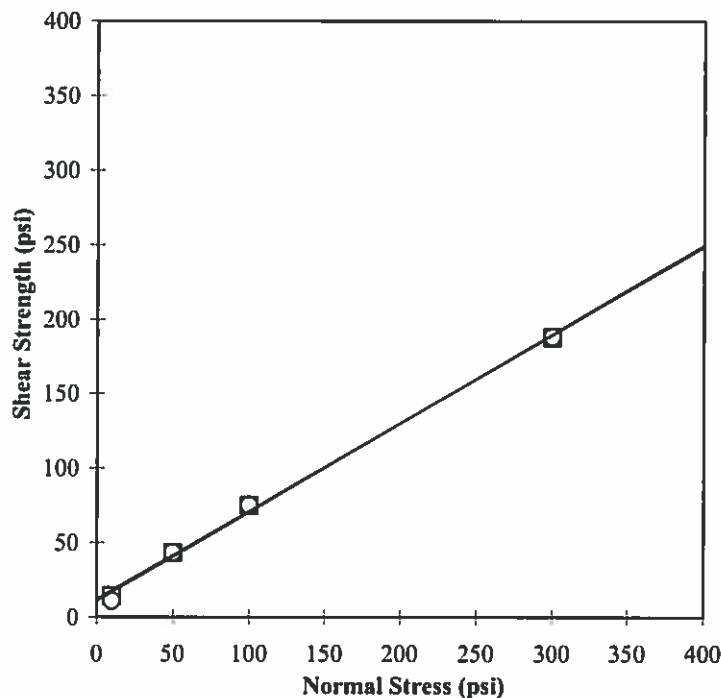
1C STOCKPILE DIRECT SHEAR TESTS



SGI TESTING SERVICES, LLC

INTERNAL DIRECT SHEAR TESTING (ASTM D 3080) GOLDER ASSOCIATES, INC. GOLDER PROJECT NO. 053-2503-001

Test Series No. 1:
internal strength of sample GA04-TY-1-GT material under soaked and consolidation conditions



Test Conditions:

Shear Box Size (in x in): 6 x 6

Shear Area (sq. in.): 36

Upper Box: Tamped In-Place
at as-received moisture

Lower Box: Tamped In-place
at as-received moisture

Soaking Stress (psi): N/A

Soaking Time (hour): N/A

Consolidation Stress (psi): 10

Consolidation Time (hour): 1

Shear Rate (in/min): 0.04

Normal Stress (psi)	Peak Load (lb)	Large Displ Load (lb)	Peak Strength (psi)	Large Displ Strength (psi)	Shear Strength Parameters			
					Peak		Large Displacement	
					Friction Angle (degree)	Adhesion (psf)	Friction Angle (degree)	Adhesion (psf)
10	499	385	13.86	10.69	31	1720	31	1495
50	1555	1555	43.19	43.19				
100	2689	2689	74.69	74.69				
300	6764	6764	187.89	187.89				

Note: Shear failure occurred within the soil at the mid-plane of the shear box after each test..

Project No.: SG15021
S5021-1

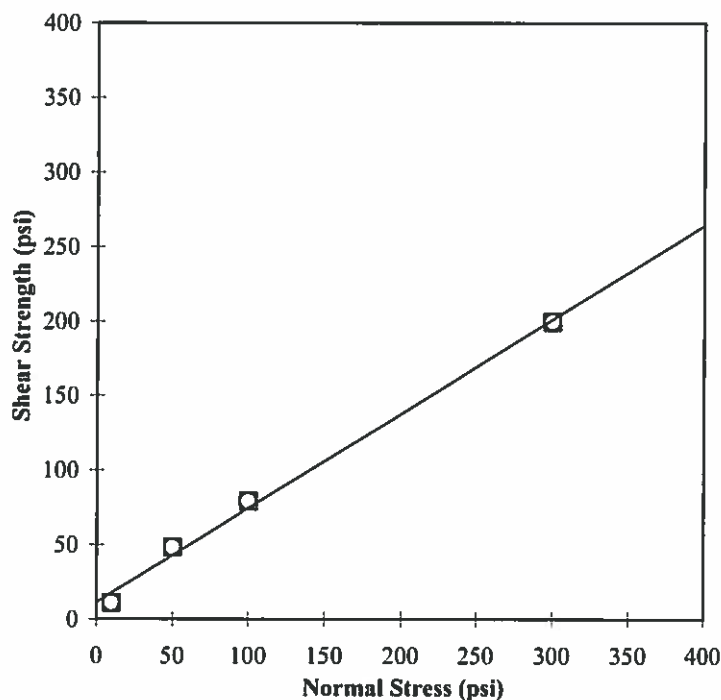


SGI TESTING SERVICES, LLC

INTERNAL DIRECT SHEAR TESTING (ASTM D 3080) GOLDER ASSOCIATES, INC. GOLDER PROJECT NO. 053-2503-001

Test Series No. 2:

internal strength of sample GA04-TY-5-GT material under soaked and consolidation conditions



Test Conditions:

Shear Box Size (in x in): 6 x 6

Shear Area (sq. in.): 36

Upper Box: Tamped In-Place
at as-received moisture

Lower Box: Tamped In-place
at as-received moisture

Soaking Stress (psi): N/A

Soaking Time (hour): N/A

Consolidation Stress (psi): 10

Consolidation Time (hour): 1

Shear Rate (in/min): 0.04

Normal Stress (psi)	Peak Load (lb)	Large Displ Load (lb)	Peak Strength (psi)	Large Displ Strength (psi)	Shear Strength Parameters			
					Peak		Large Displacement	
					Friction Angle (degree)	Adhesion (psf)	Friction Angle (degree)	Adhesion (psf)
10	381	381	10.58	10.58	32	1630	32	1630
50	1721	1721	47.81	47.81				
100	2839	2839	78.86	78.86				
300	7172	7172	199.22	199.22				

Note: Shear failure occurred within the soil at the mid-plane of the shear box after each test..

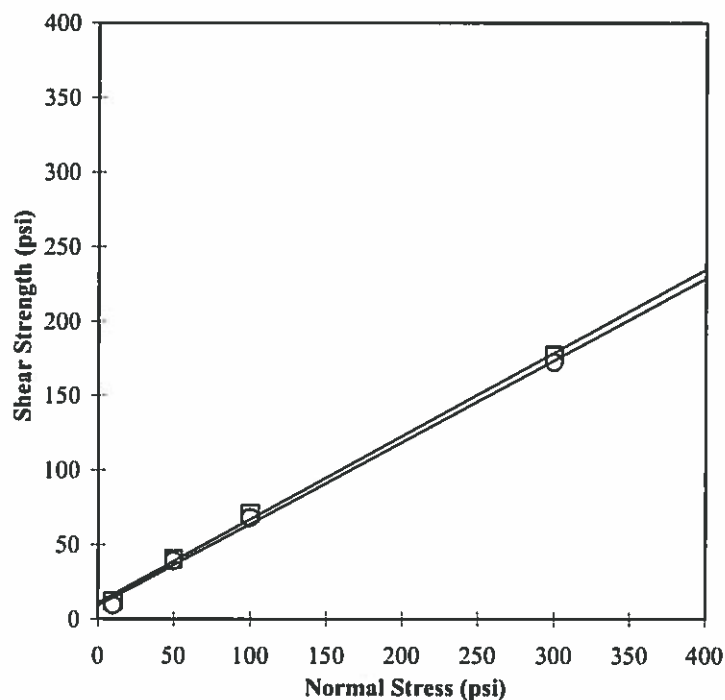
Project No.: SG15021
S5021-2



SGI TESTING SERVICES, LLC

INTERNAL DIRECT SHEAR TESTING (ASTM D 3080) GOLDER ASSOCIATES, INC. GOLDER PROJECT NO. 053-2503-001

Test Series No. 3:
internal strength of sample GA04-TY-8-GT material under soaked and consolidation conditions



Test Conditions:

Shear Box Size (in x in): 6 x 6

Shear Area (sq. in.): 36

Upper Box: Tamped In-Place
at as-received moisture

Lower Box: Tamped In-place
at as-received moisture

Soaking Stress (psi): N/A

Soaking Time (hour): N/A

Consolidation Stress (psi): 10

Consolidation Time (hour): 1

Shear Rate (in/min): 0.04

Normal Stress (psi)	Peak Load (lb)	Large Displ Load (lb)	Peak Strength (psi)	Large Displ Strength (psi)	Shear Strength Parameters			
					Peak		Large Displacement	
					Friction Angle (degree)	Adhesion (psf)	Friction Angle (degree)	Adhesion (psf)
10	436	329	12.11	9.14	29	1540	29	1260
50	1443	1399	40.08	38.86				
100	2551	2439	70.86	67.75				
300	6369	6190	176.92	171.94				

Note: Shear failure occurred within the soil at the mid-plane of the shear box after each test..

Project No.: SG15021
S5021-3

APPENDIX II

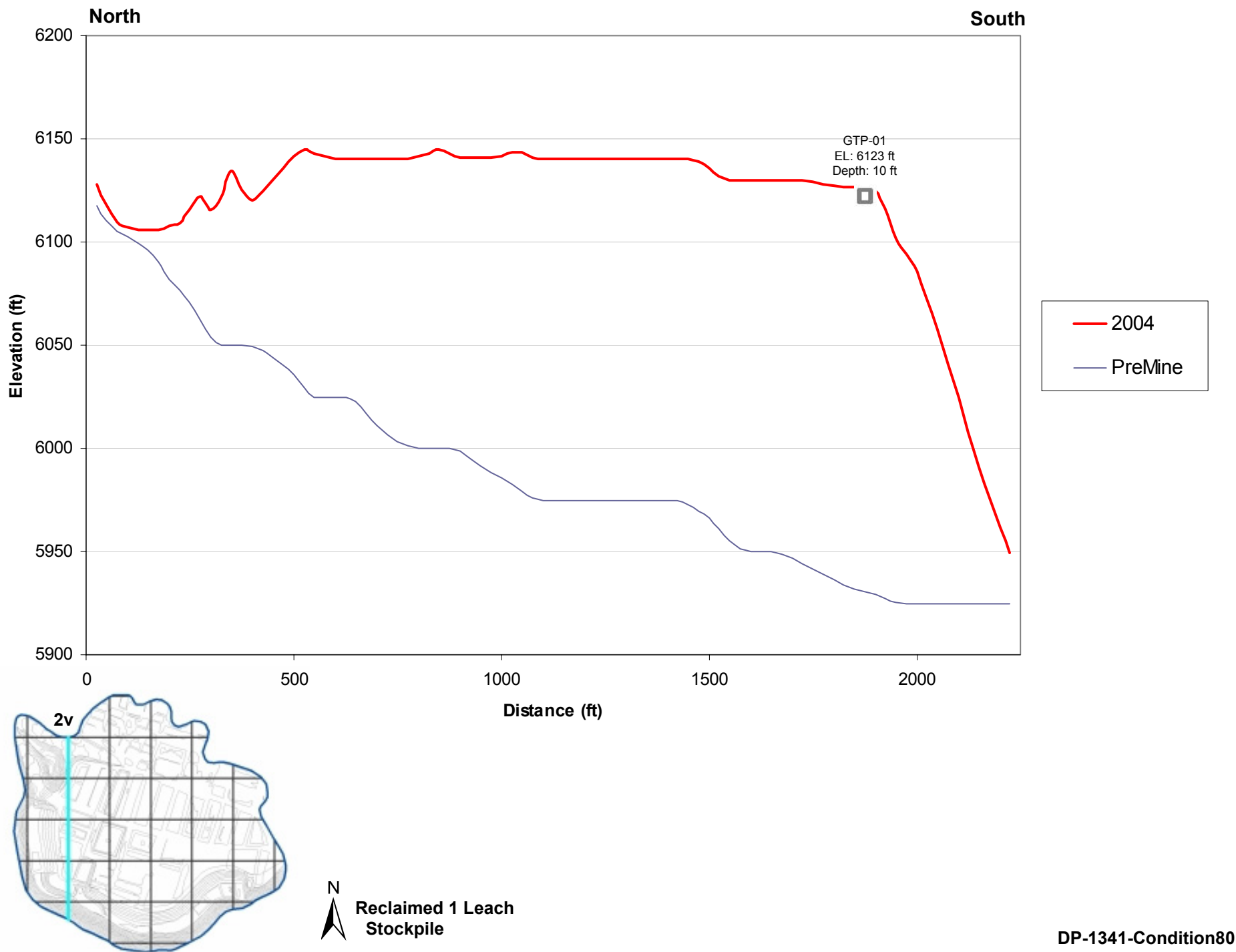
2005positional Model

APPENDIX II

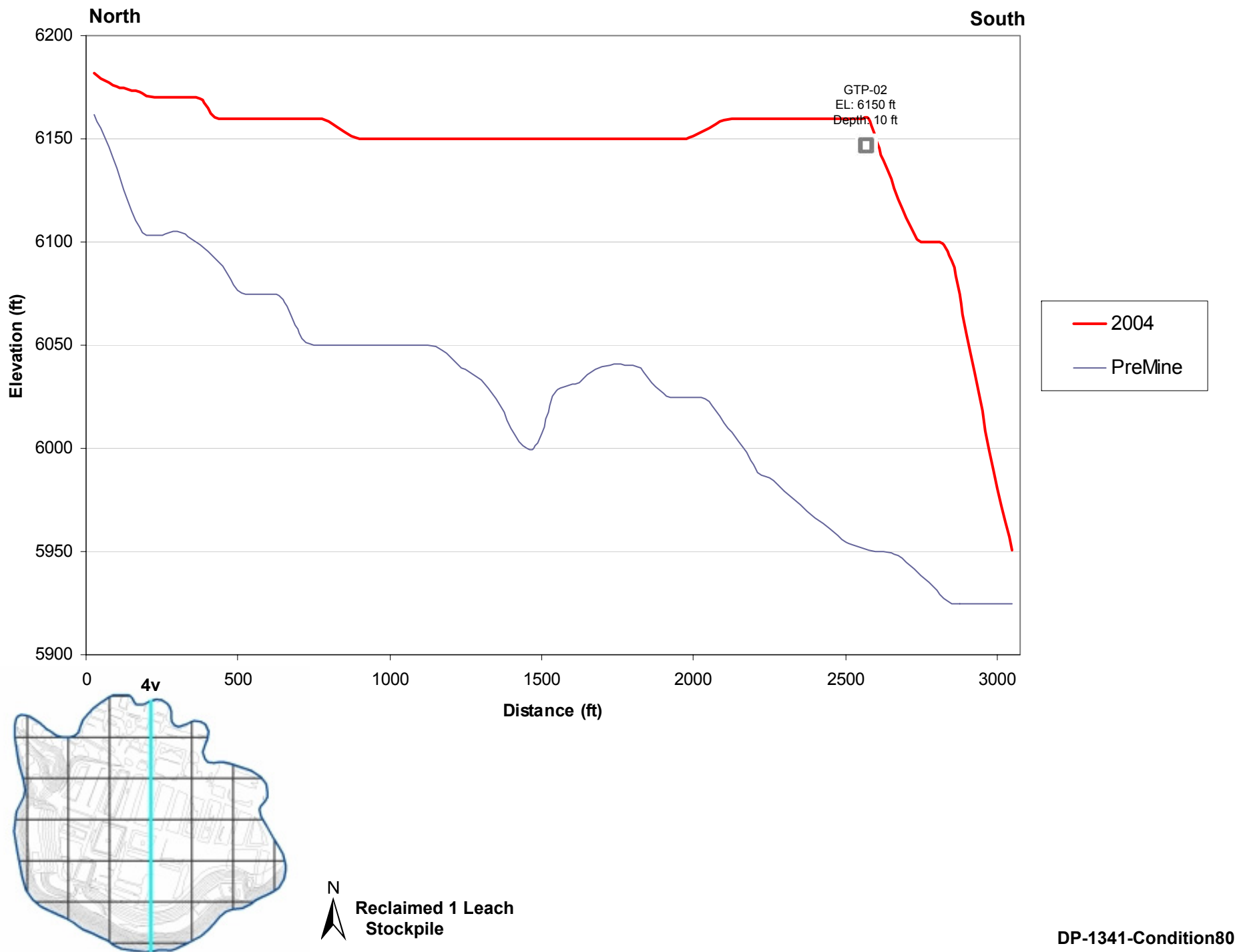
COMPOSITIONAL MODELS

COMPOSITIONAL MODELS – Reclaimed 1 Leach Stockpile

Tyrone Mine - Reclaimed 1 Leach Stockpile - Cross Section 2v



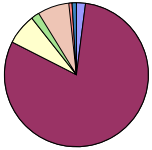
Tyrone Mine - Reclaimed 1 Leach Stockpile - Cross Section 4v



COMPOSITIONAL MODELS – 1A Leach Stockpile

1A Leach Stockpile

1973



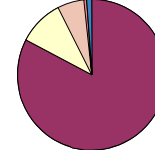
1975

No material
deposited

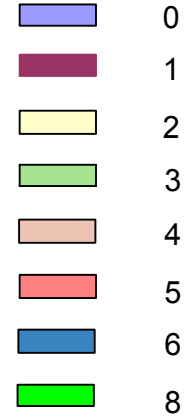
1976

No material
deposited

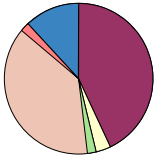
1978



Mineral
Assemblage
Code



1982



1986

No material
deposited

1987

No material
deposited

1988

No material
deposited

1989

No material
deposited

1990

No material
deposited

1992

No material
deposited

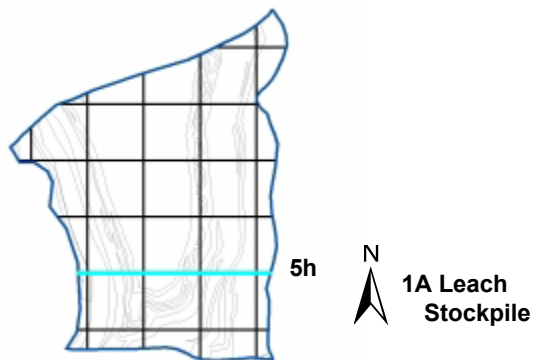
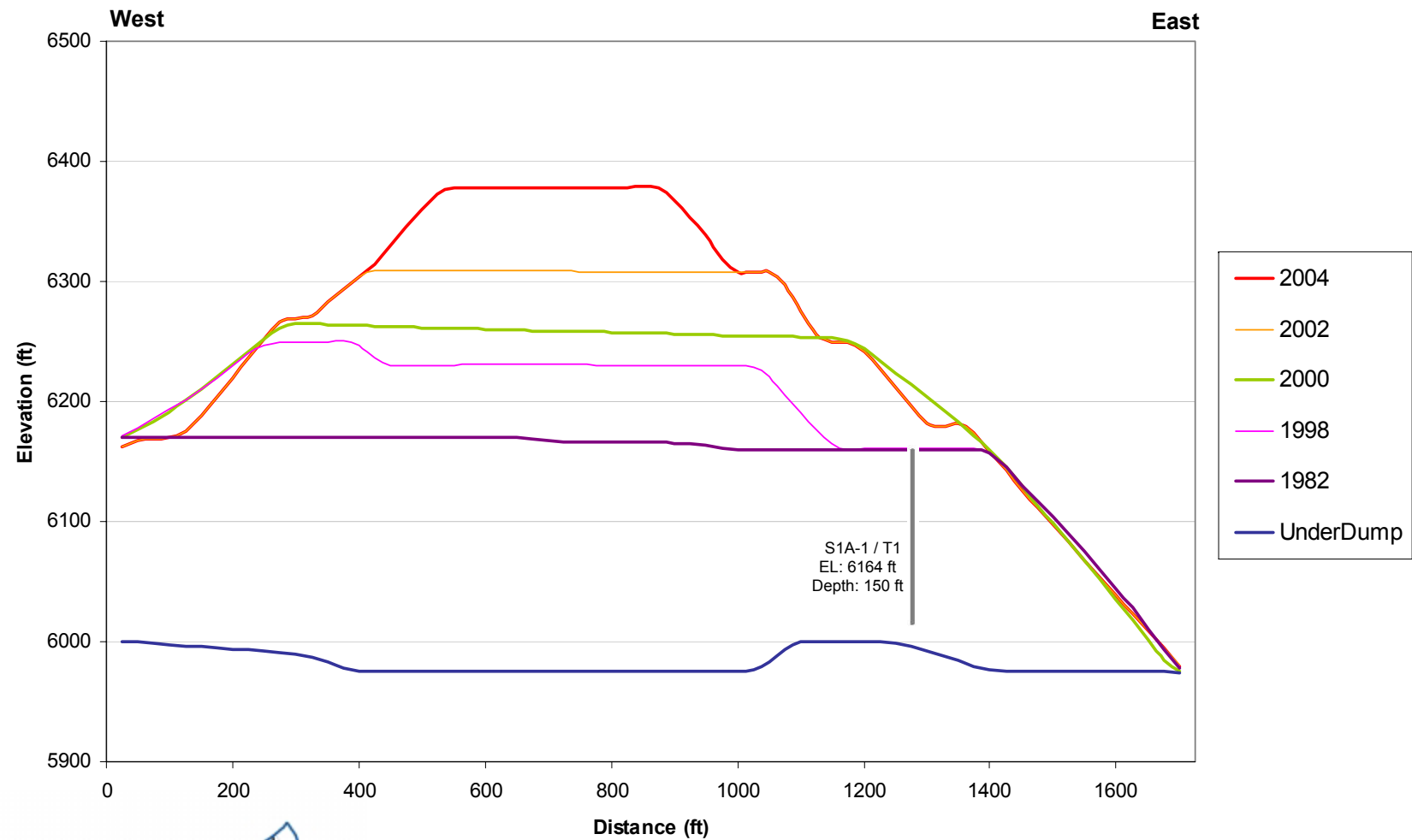
1995

No material
deposited

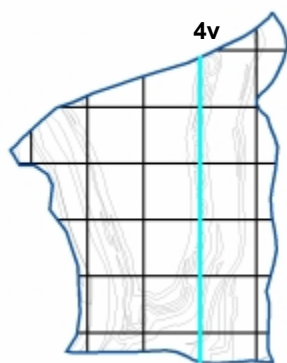
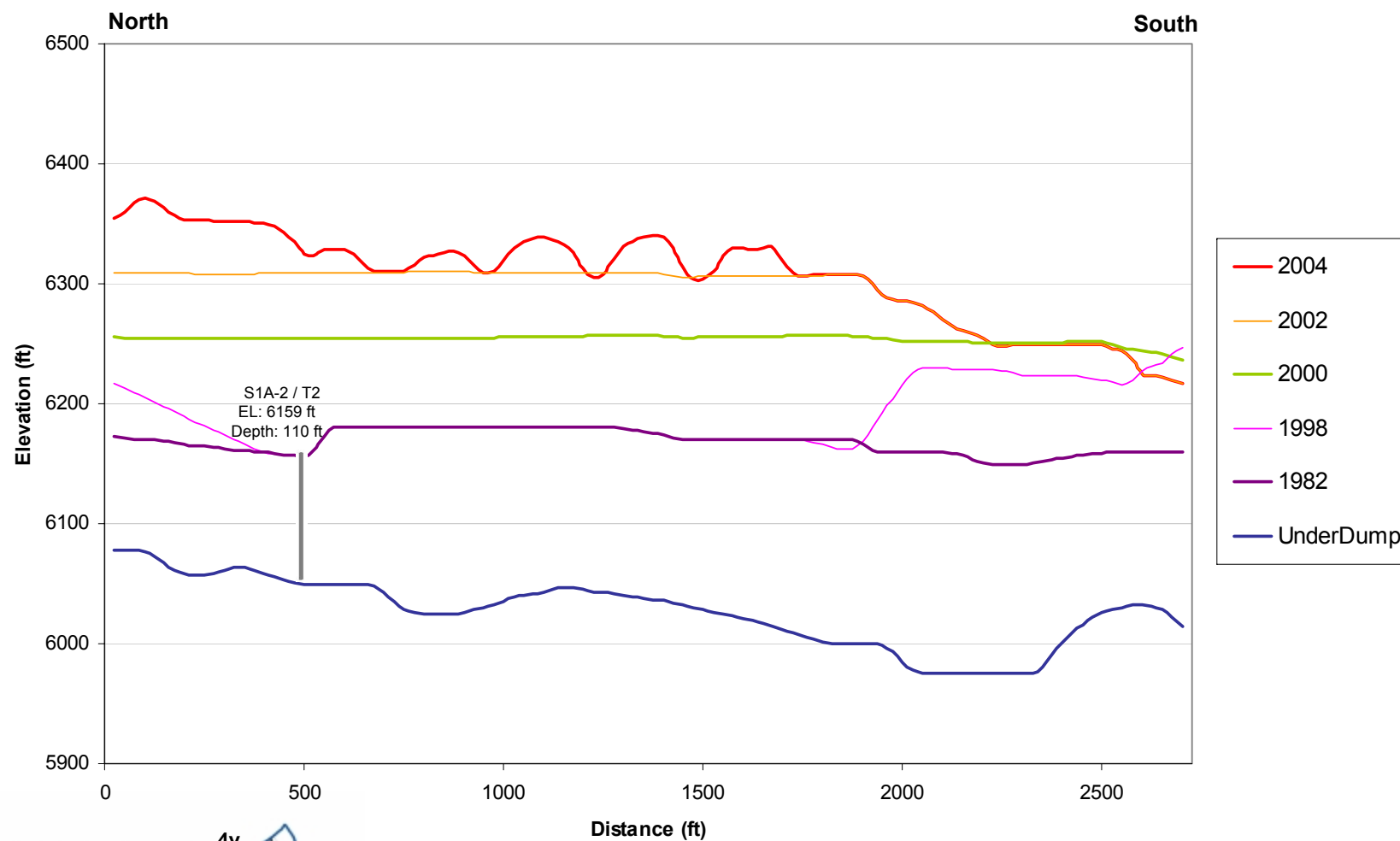
1996

No material
deposited

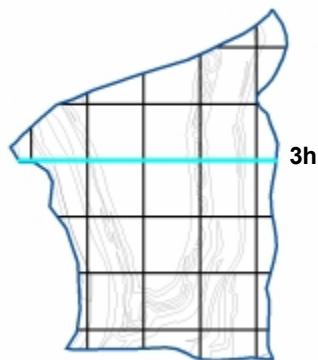
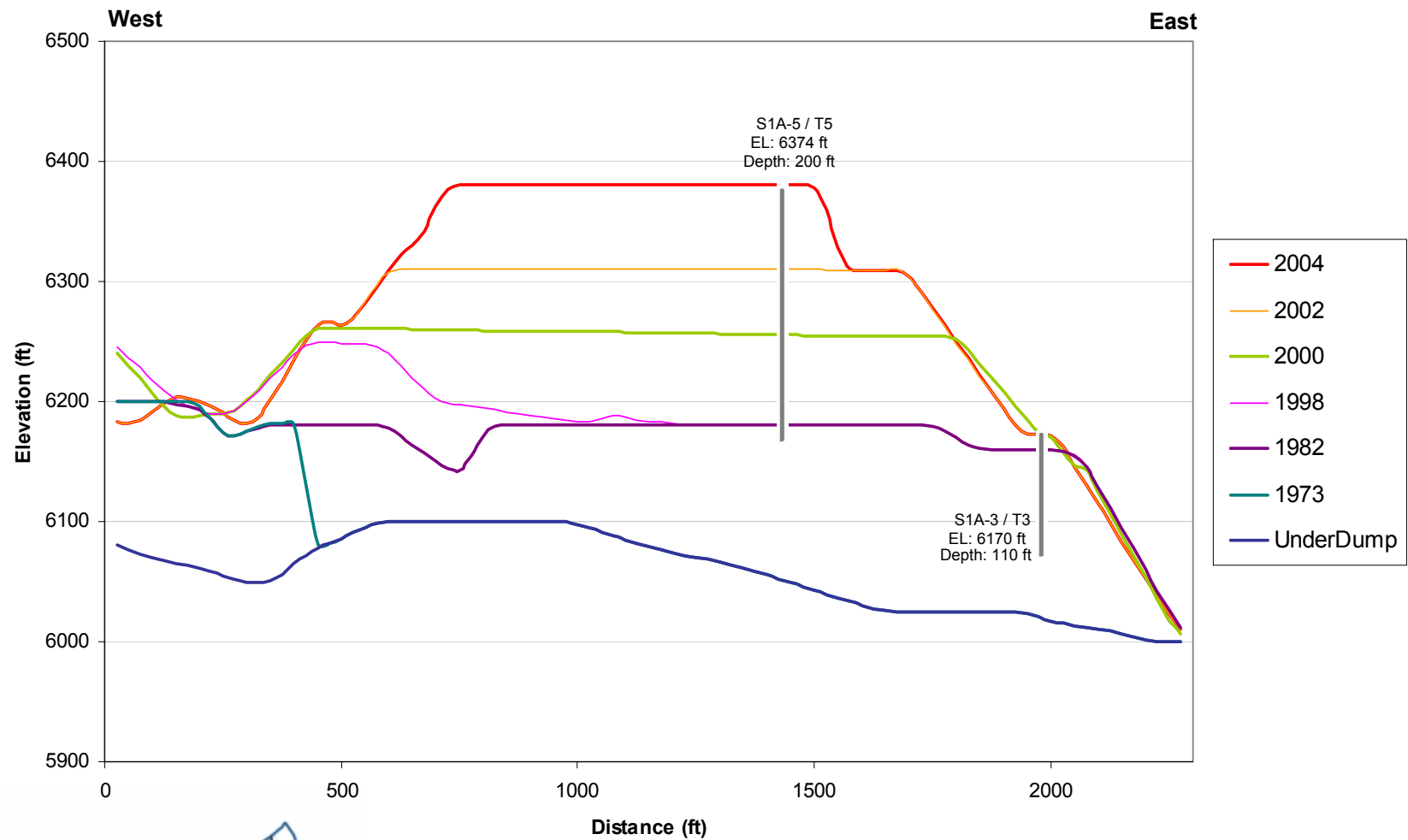
Tyrone Mine - 1A Leach Stockpile - Cross Section 5h



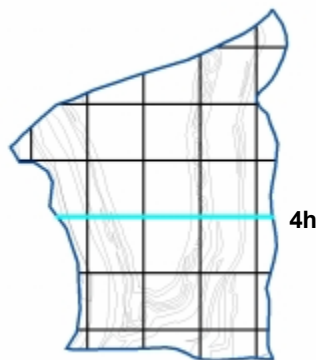
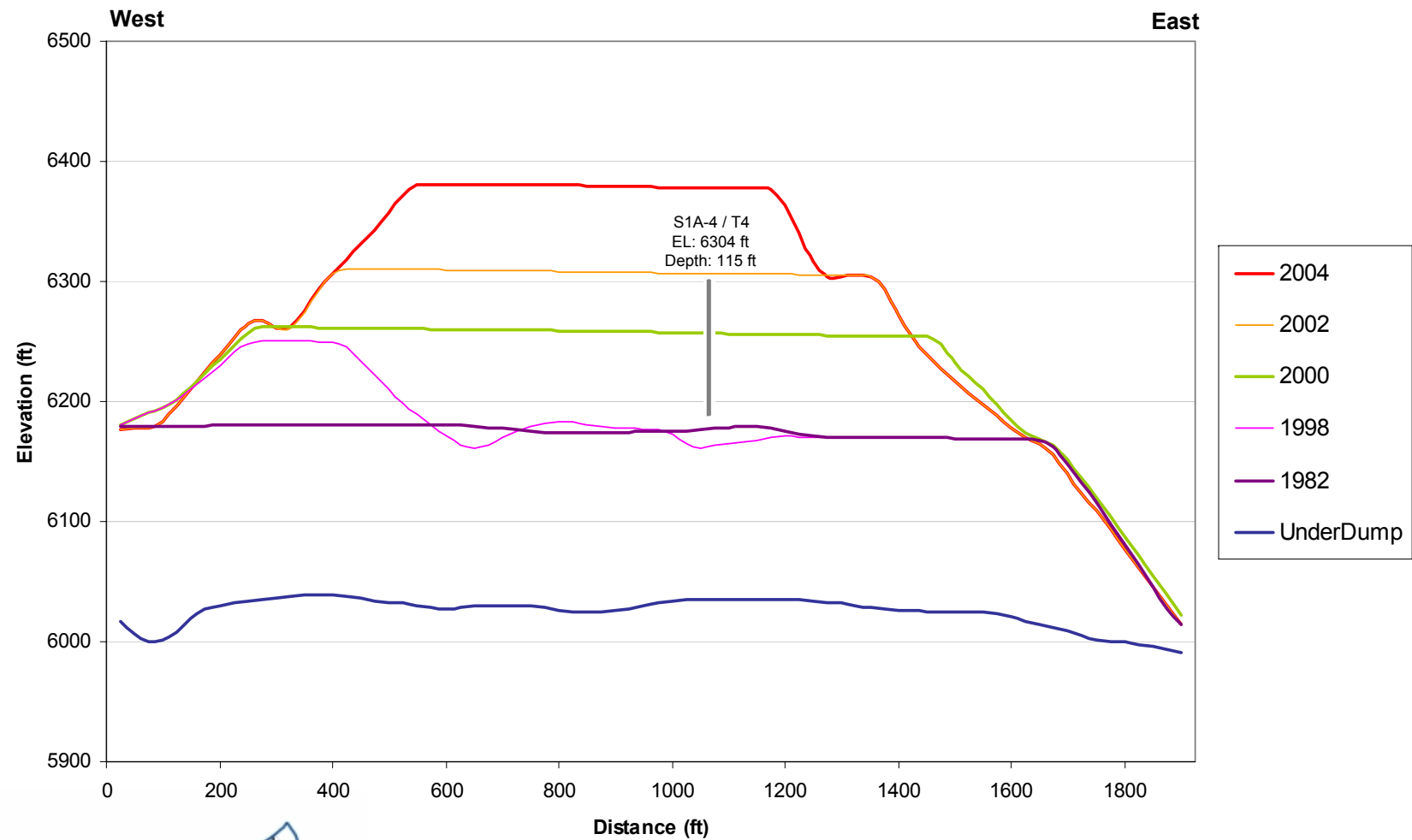
Tyrone Mine - 1A Leach Stockpile - Cross Section 4v



Tyrone Mine - 1A Leach Stockpile - Cross Section 3h



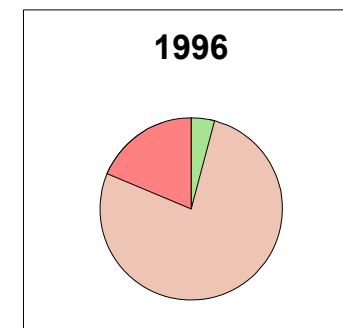
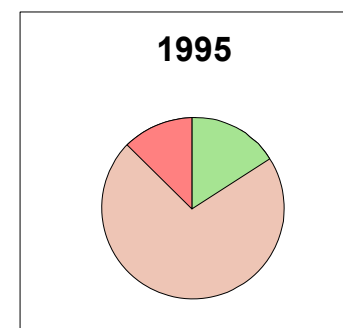
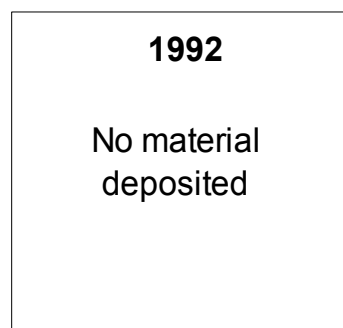
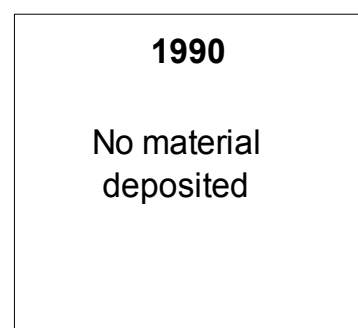
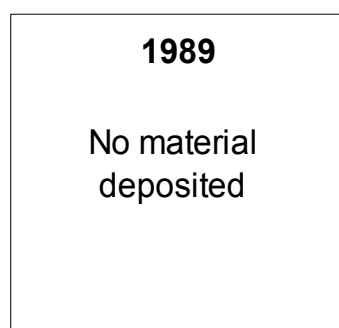
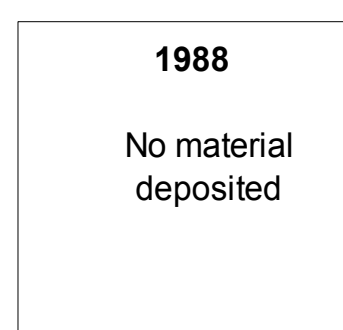
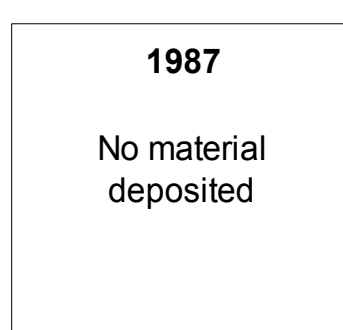
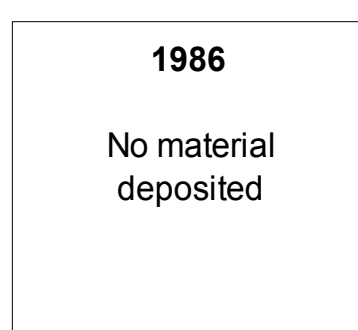
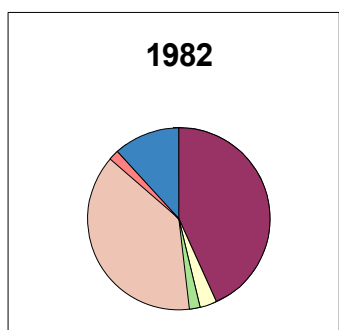
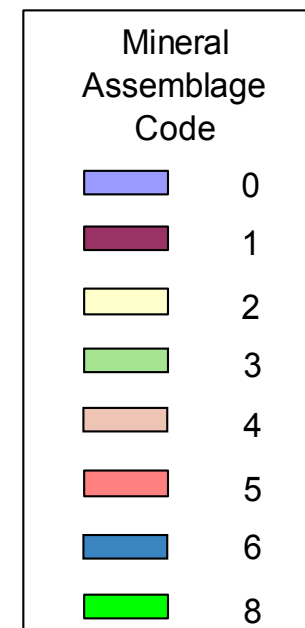
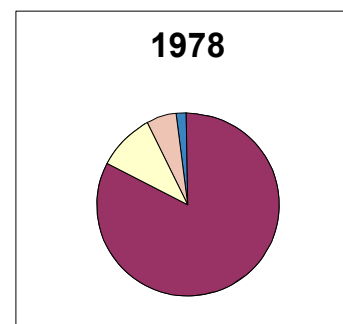
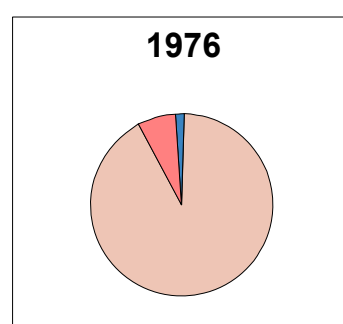
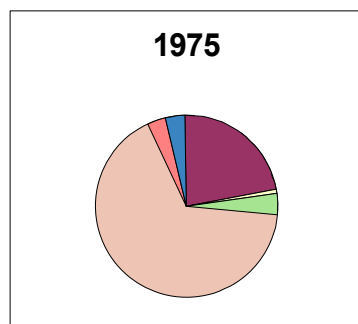
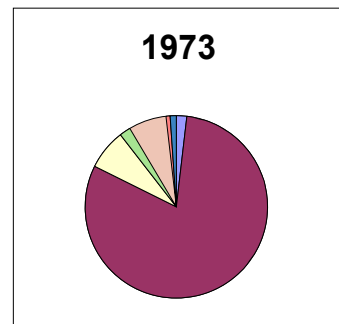
Tyrone Mine - 1A Leach Stockpile - Cross Section 4h



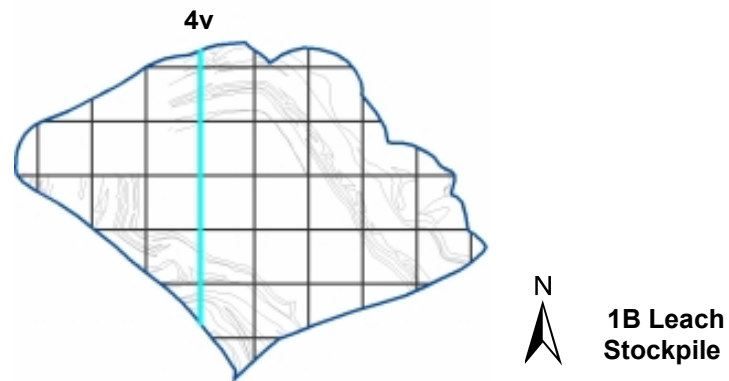
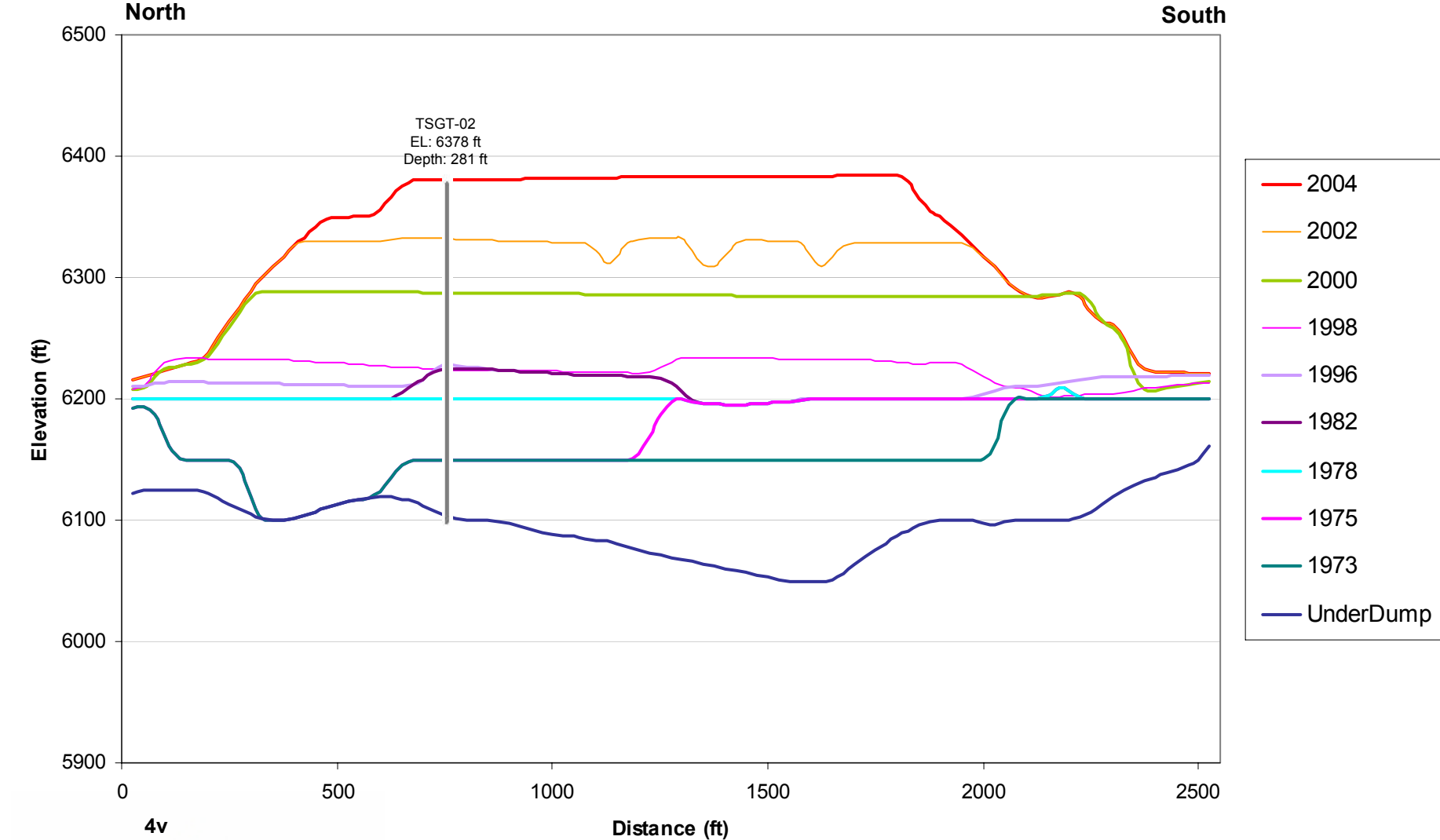
1A Leach
Stockpile

COMPOSITIONAL MODELS – 1B Leach Stockpile

1B Leach Stockpile



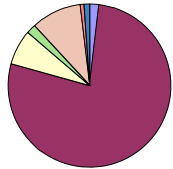
Tyrone Mine - 1B Leach Stockpile - Cross Section 4v



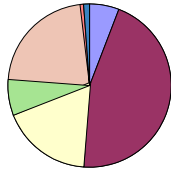
COMPOSITIONAL MODELS – Reclaimed 1C Waste Stockpile

Reclaimed 1C Waste Stockpile

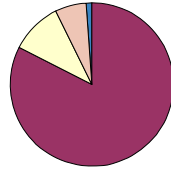
1973



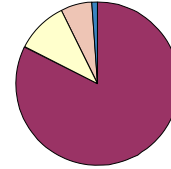
1975



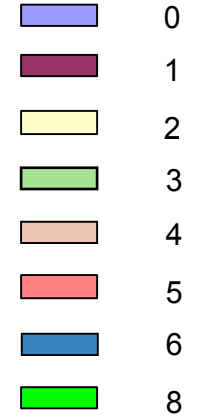
1976



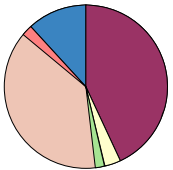
1978



Mineral
Assemblage
Code



1982



1986

No material
deposited

1987

No material
deposited

1988

No material
deposited

1989

No material
deposited

1990

No material
deposited

1992

No material
deposited

1995

No material
deposited

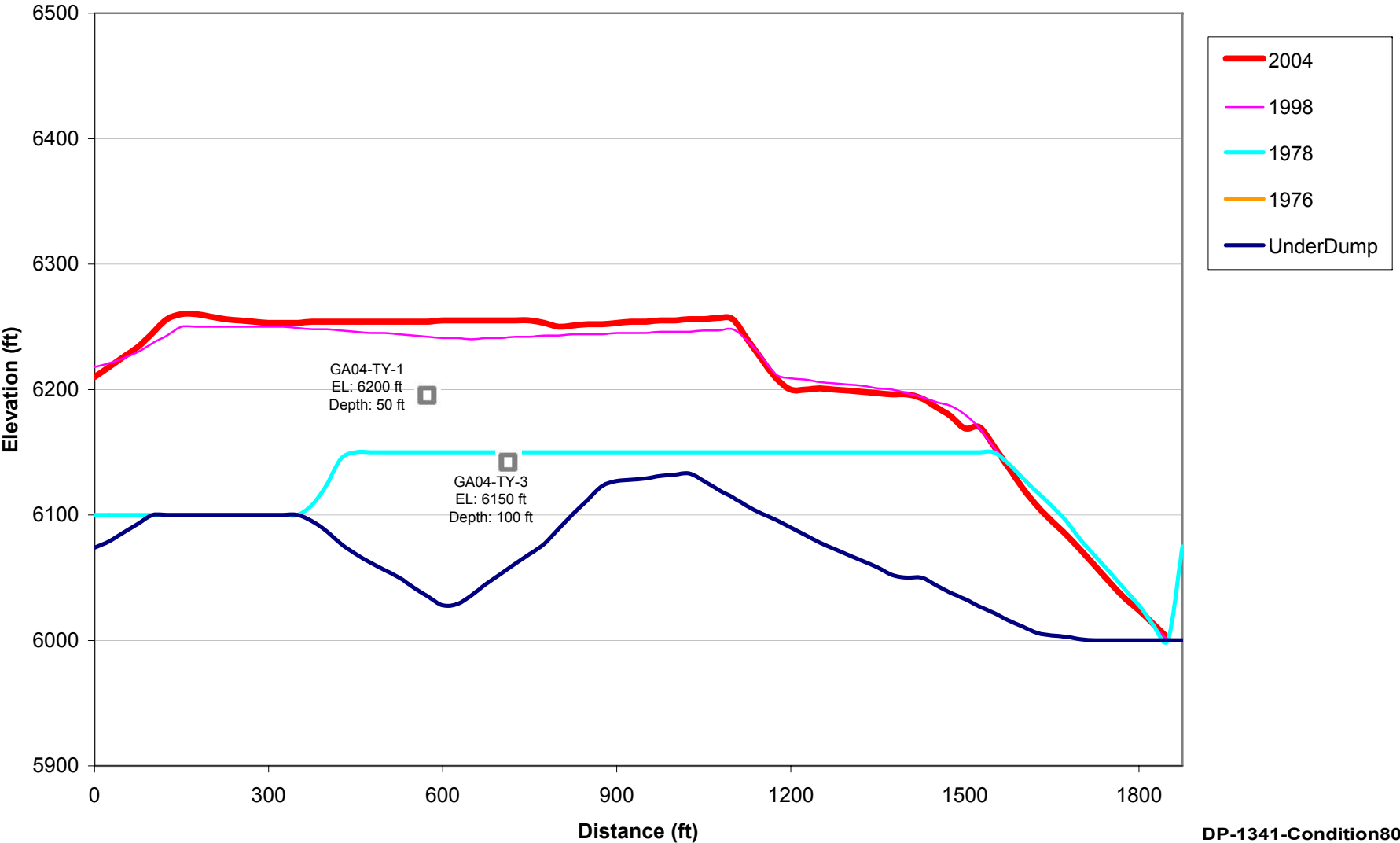
1996

Not
available

Tyrone Cross Section Reclaimed 1C Waste Cross Section 16v

North

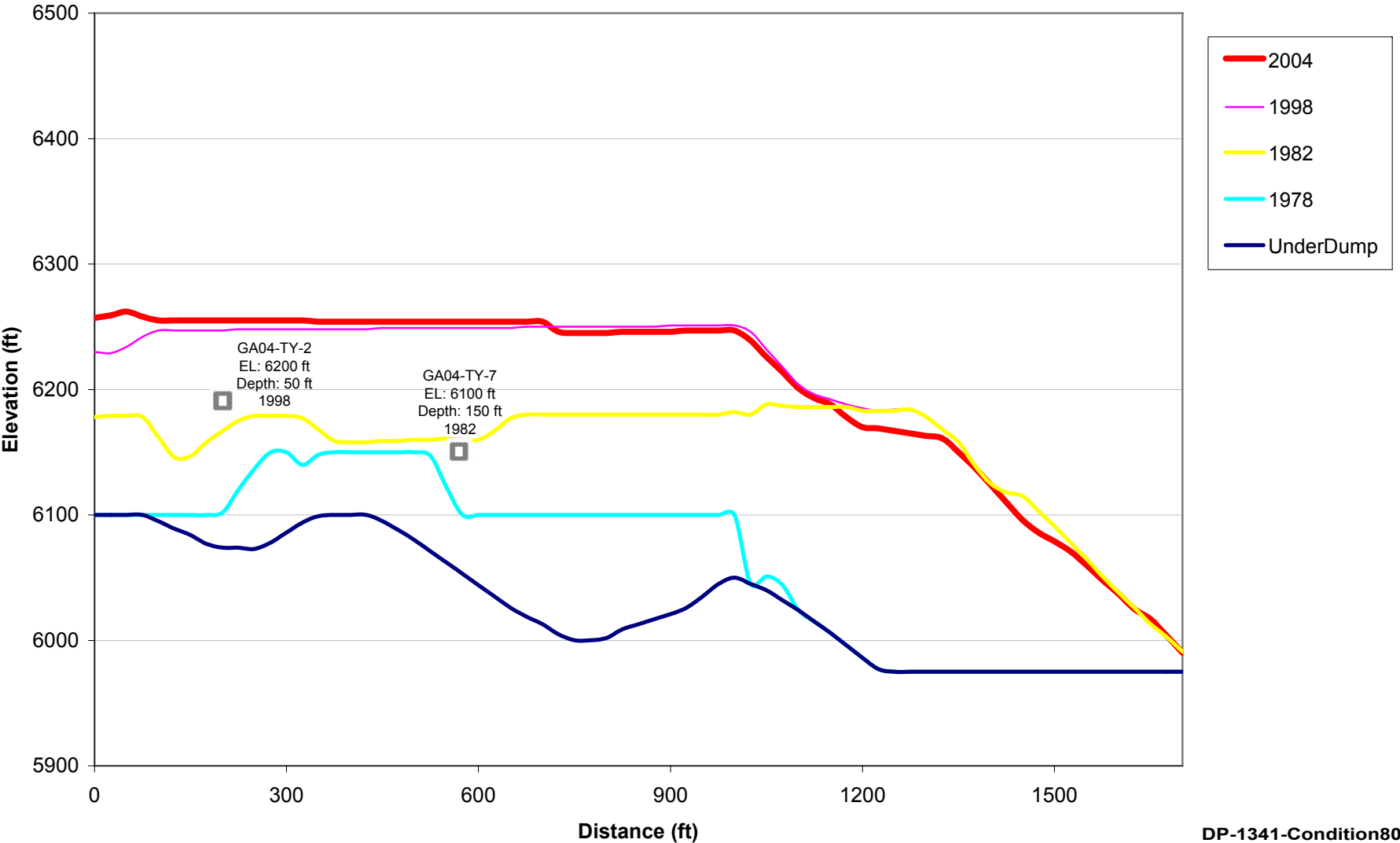
South



Tyrone Cross Section Reclaimed 1C Waste Cross Section 17v

North

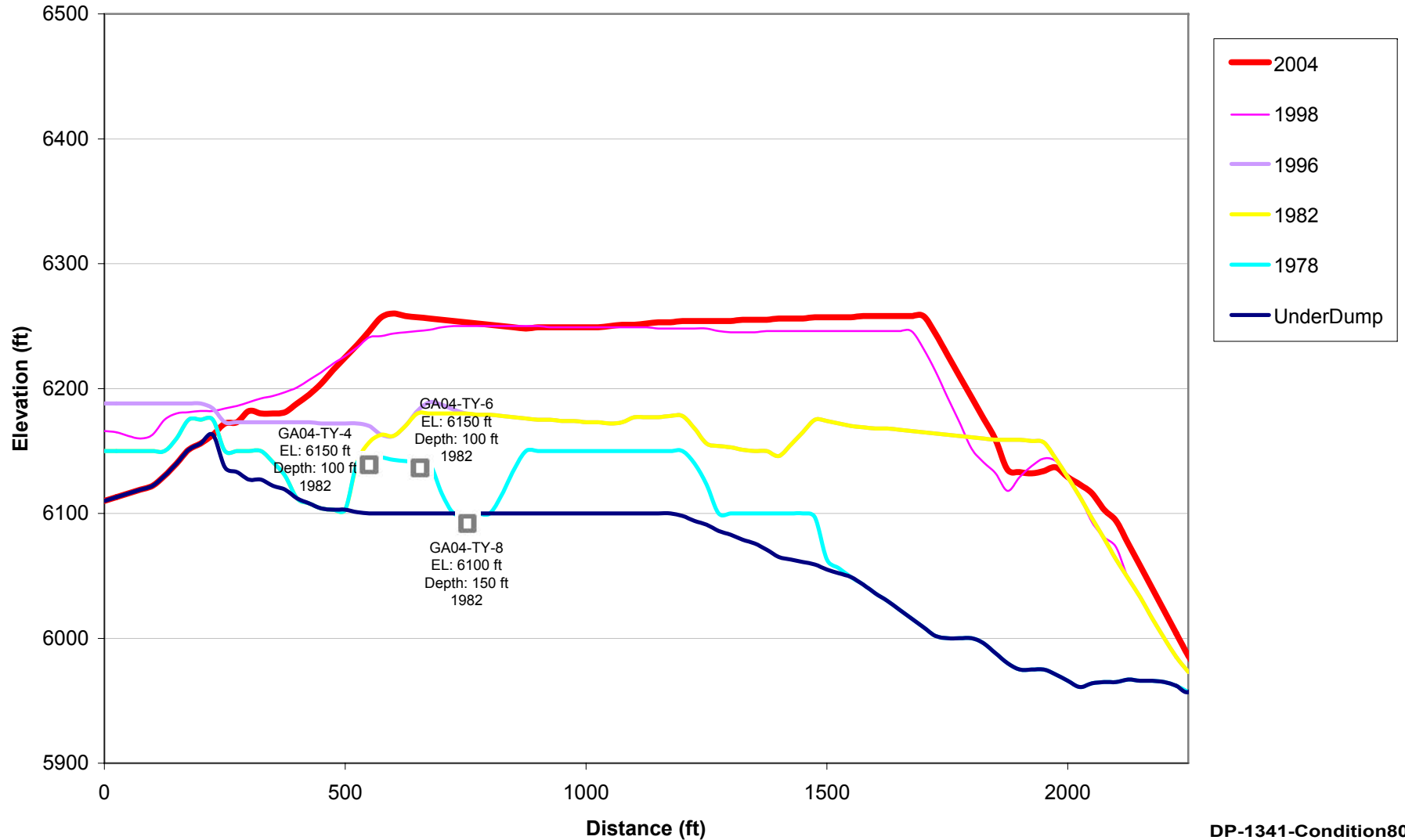
South



Tyrone Cross Section Reclaimed 1C Waste Cross Section 1h

West

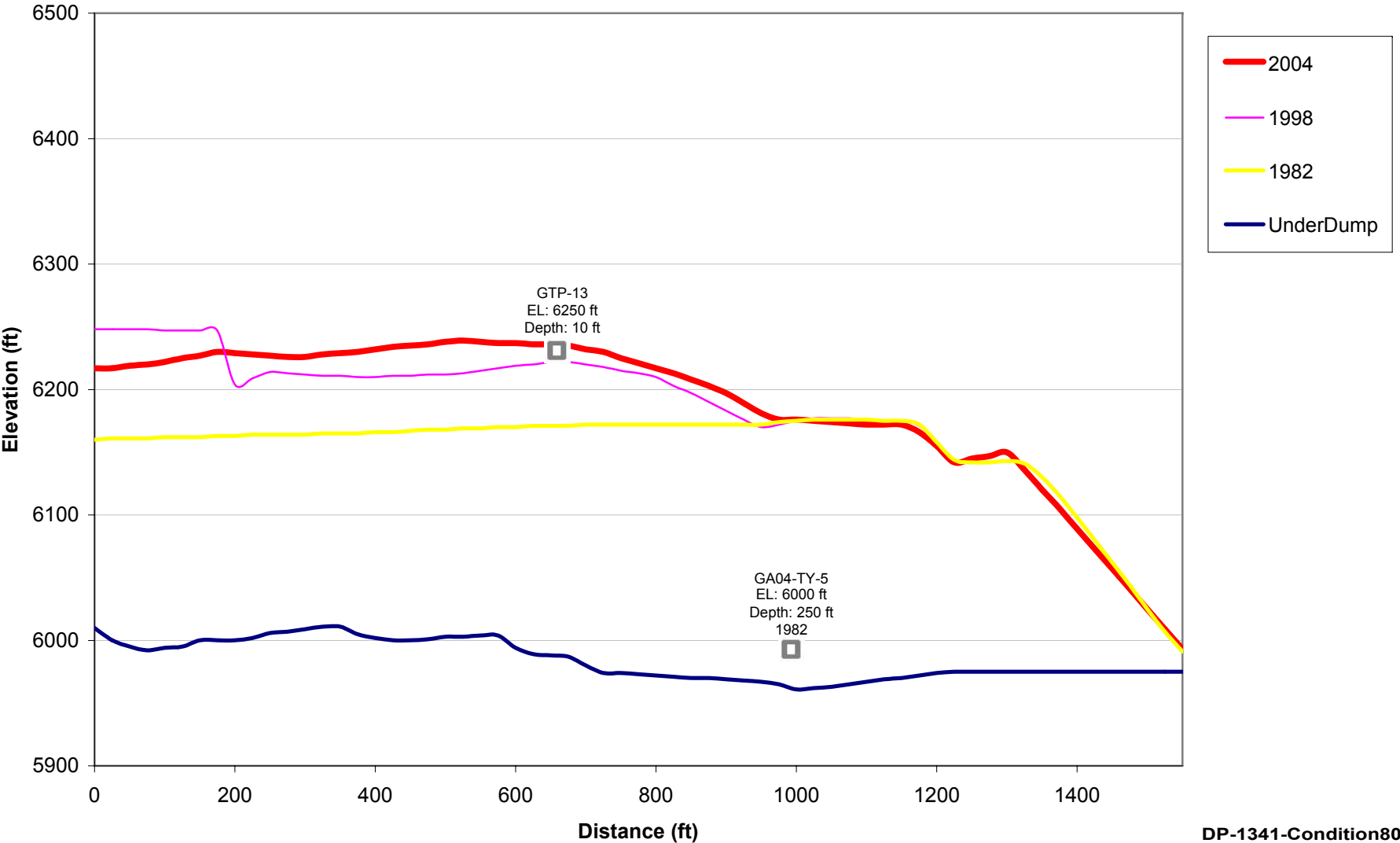
East



Tyrone Cross Section Reclaimed 1C Waste Cross Section 18v

North

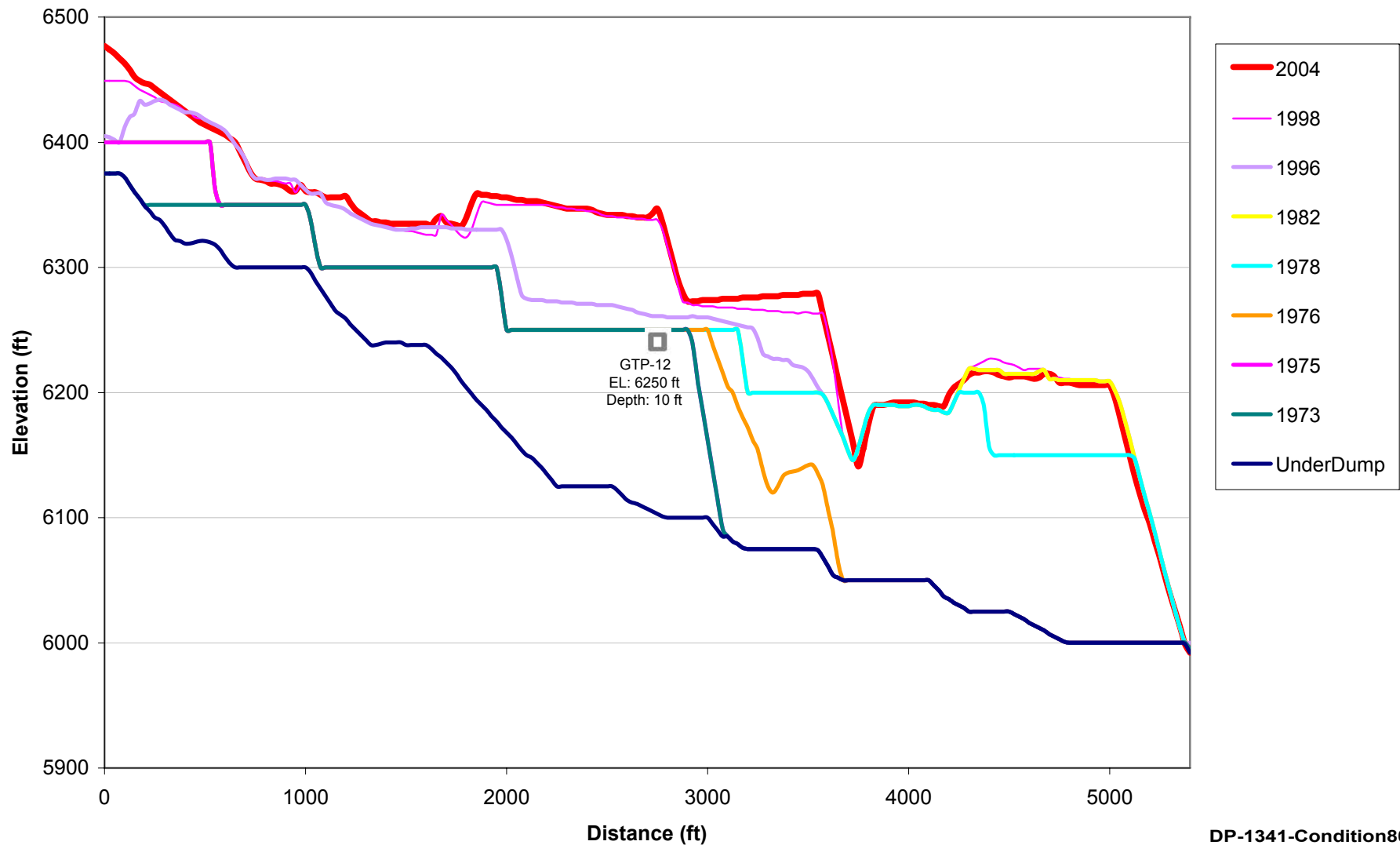
South



Tyrone Cross Section Reclaimed 1C Waste Cross Section 4h

West

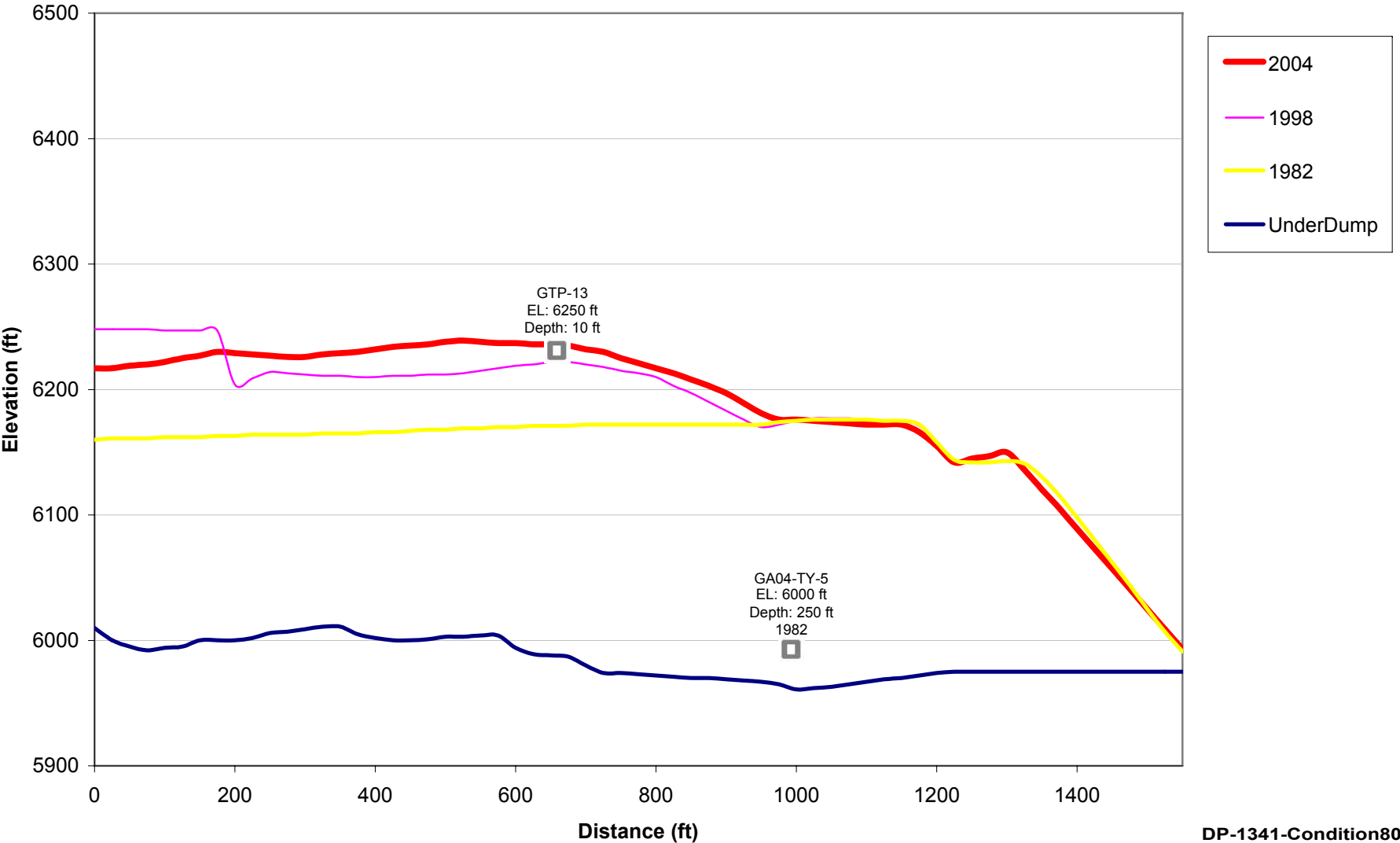
East



Tyrone Cross Section Reclaimed 1C Waste Cross Section 18v

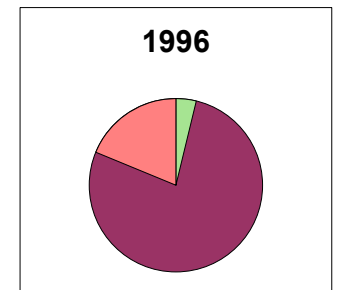
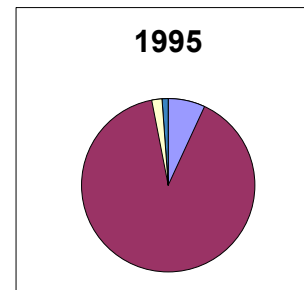
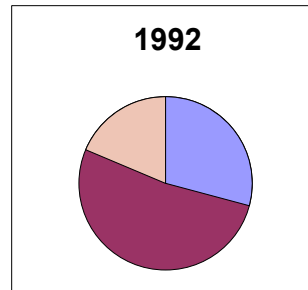
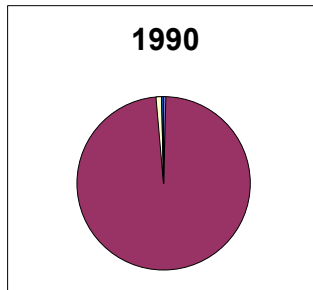
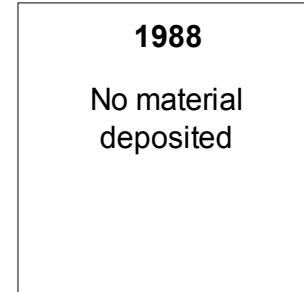
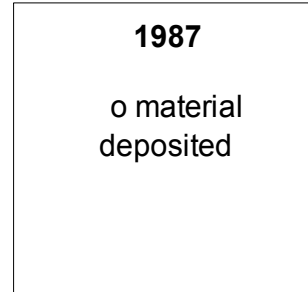
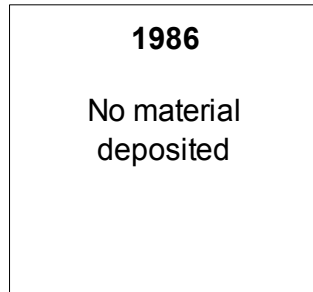
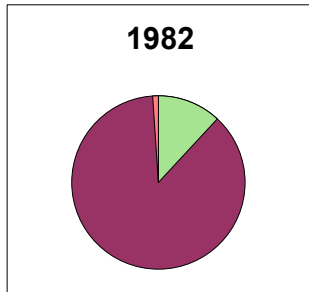
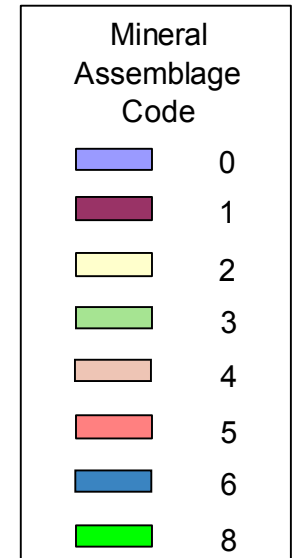
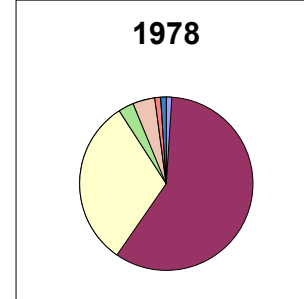
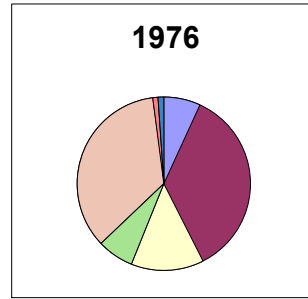
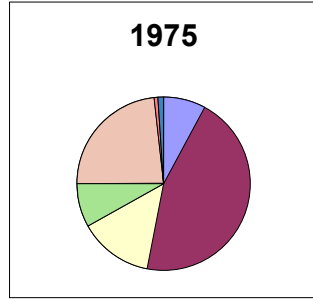
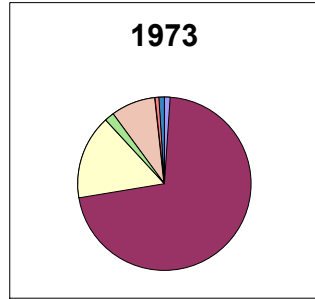
North

South

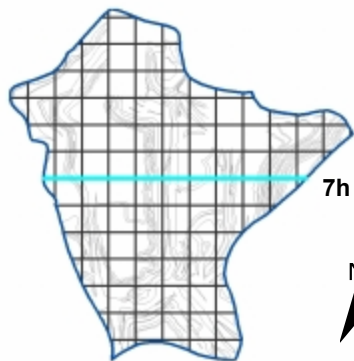
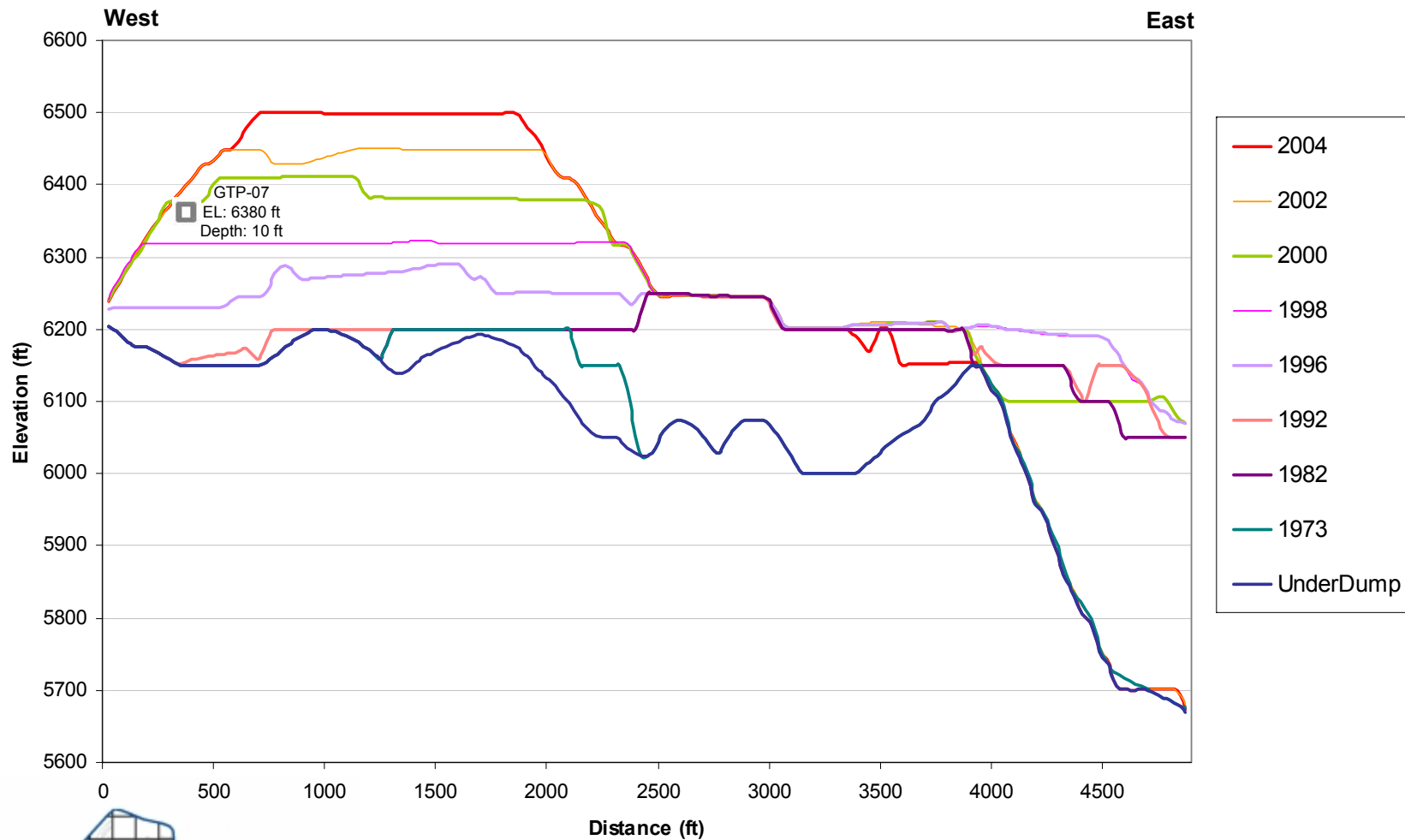


COMPOSITIONAL MODELS – 2A and 2B Leach Stockpiles

2A and 2B Leach Stockpiles

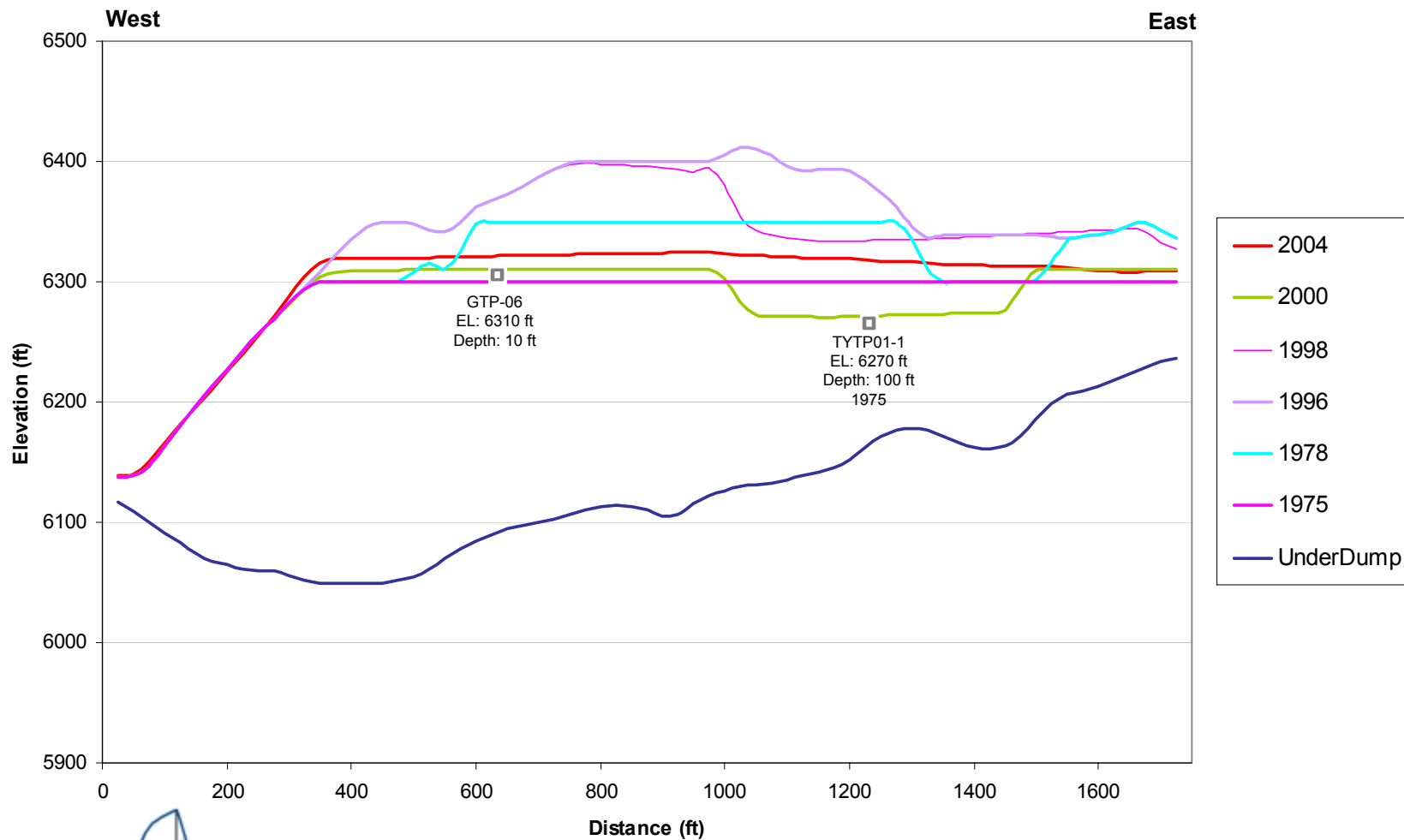


Tyrone Mine - 2A Leach Stockpile - Cross Section 7h



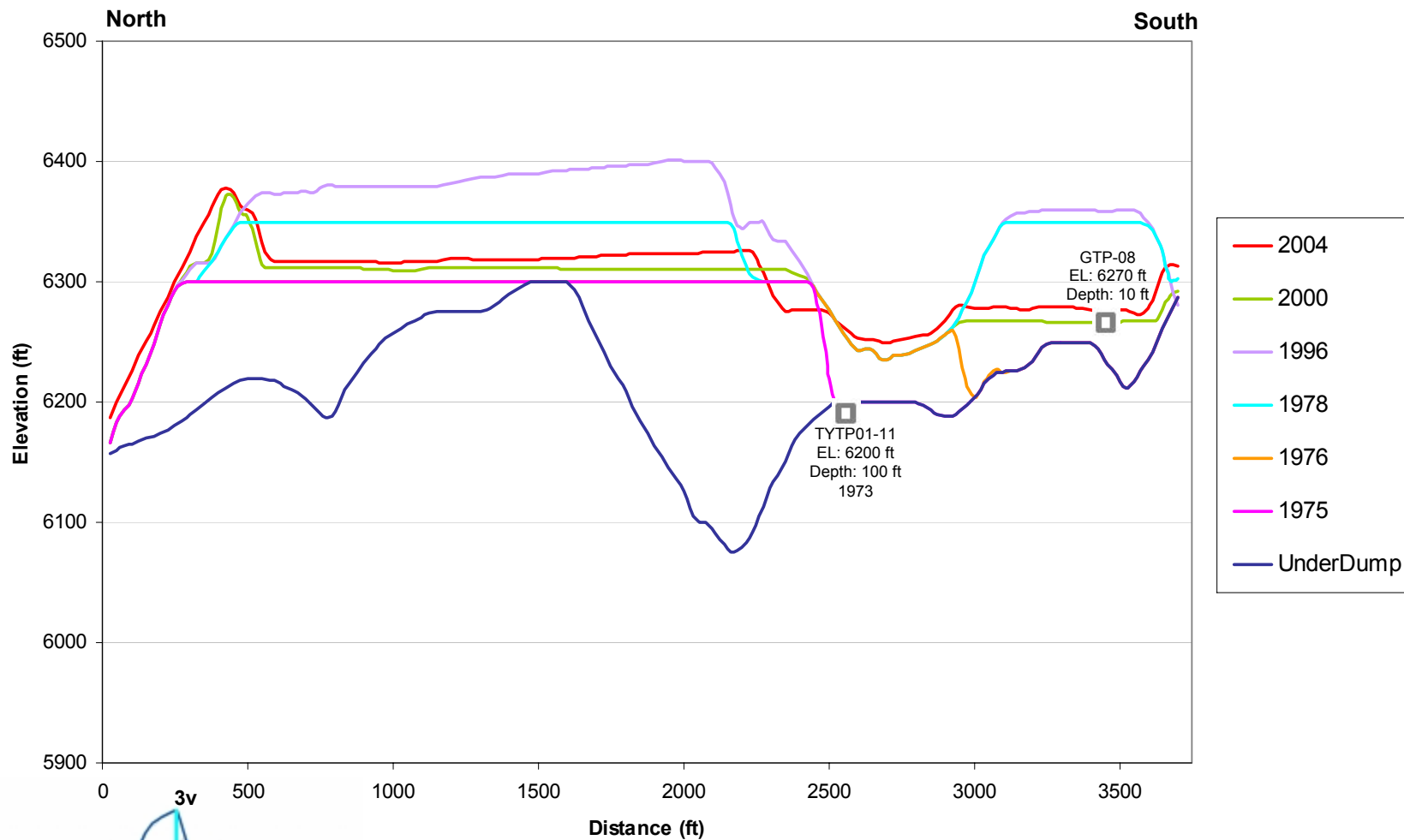
2A Leach
Stockpile

Tyrone Mine - 2B Leach Stockpile - Cross Section 4h



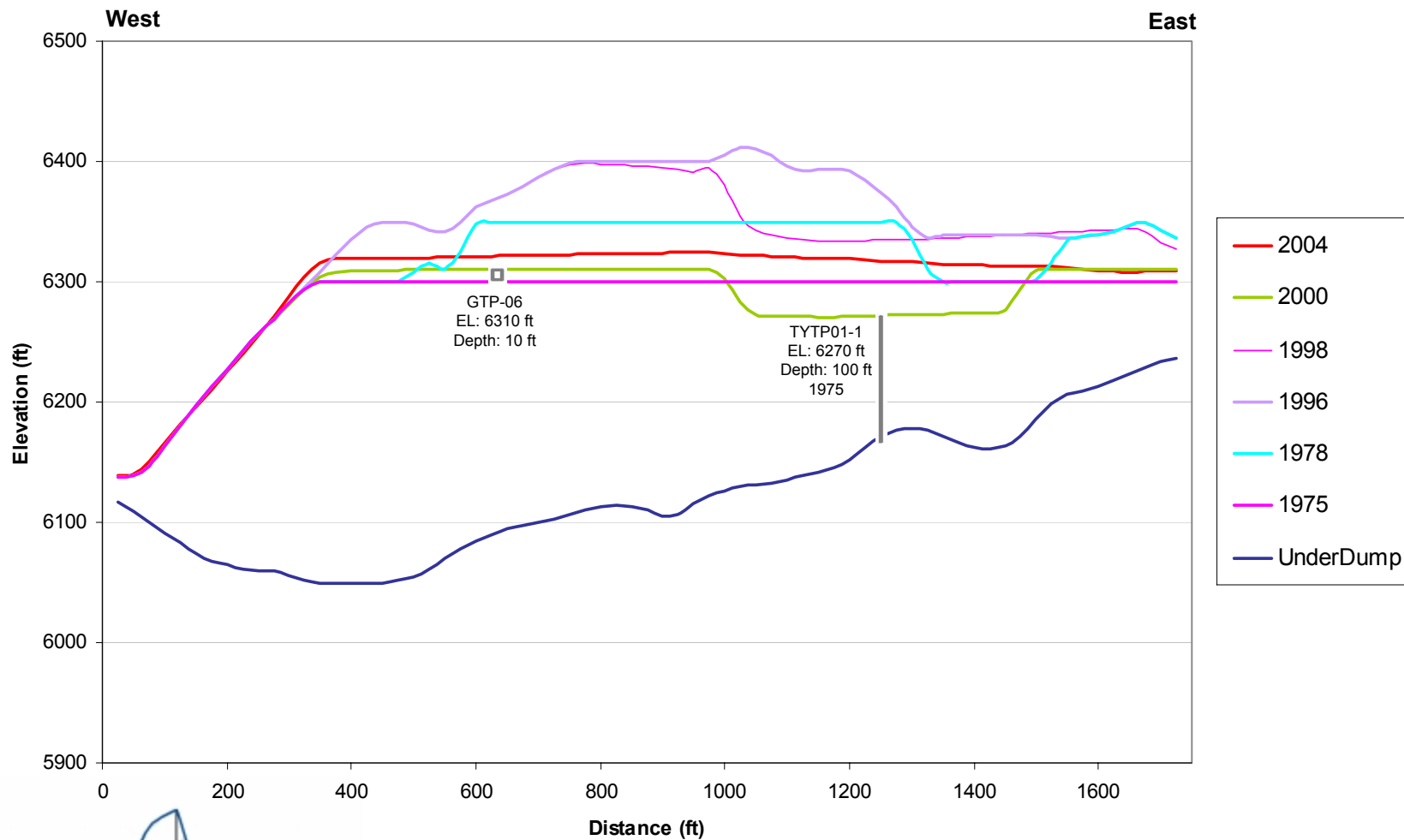
**2B Leach
Stockpile**

Tyrone Mine - 2B Leach Stockpile - Cross Section 3v



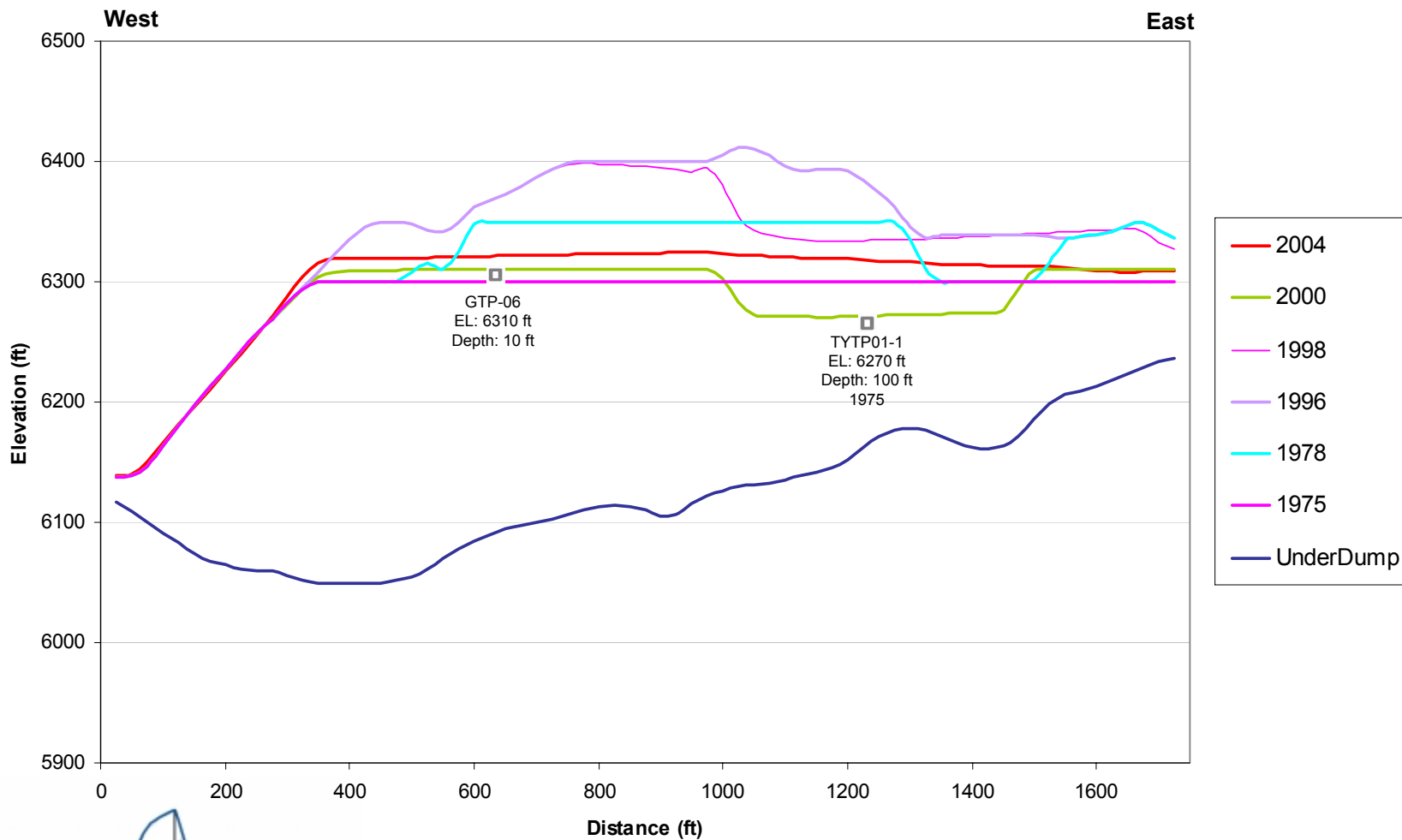
**2B Leach
Stockpile**

Tyrone Mine - 2B Leach Stockpile - Cross Section 4h



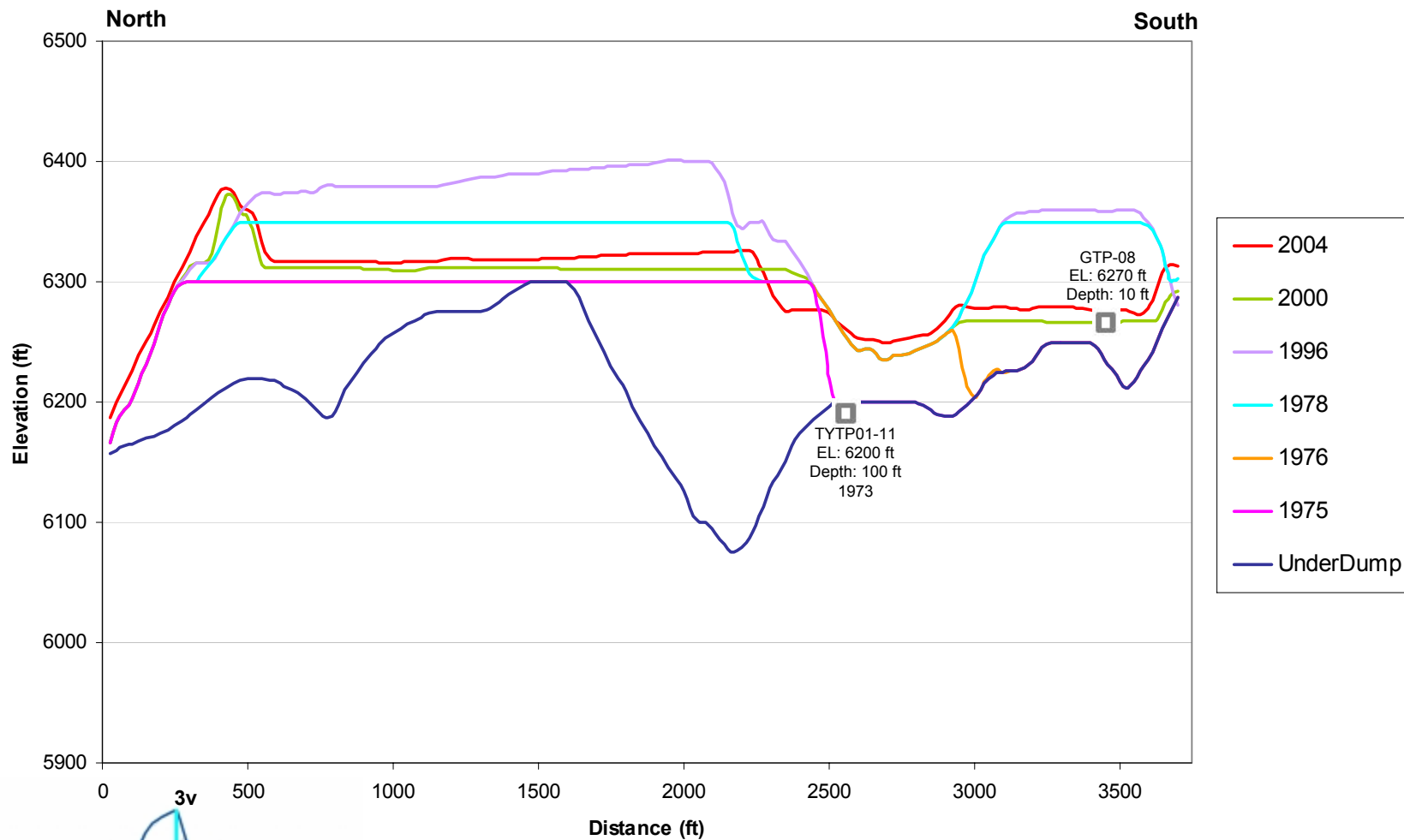
**2B Leach
Stockpile**

Tyrone Mine - 2B Leach Stockpile - Cross Section 4h



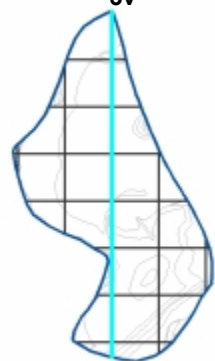
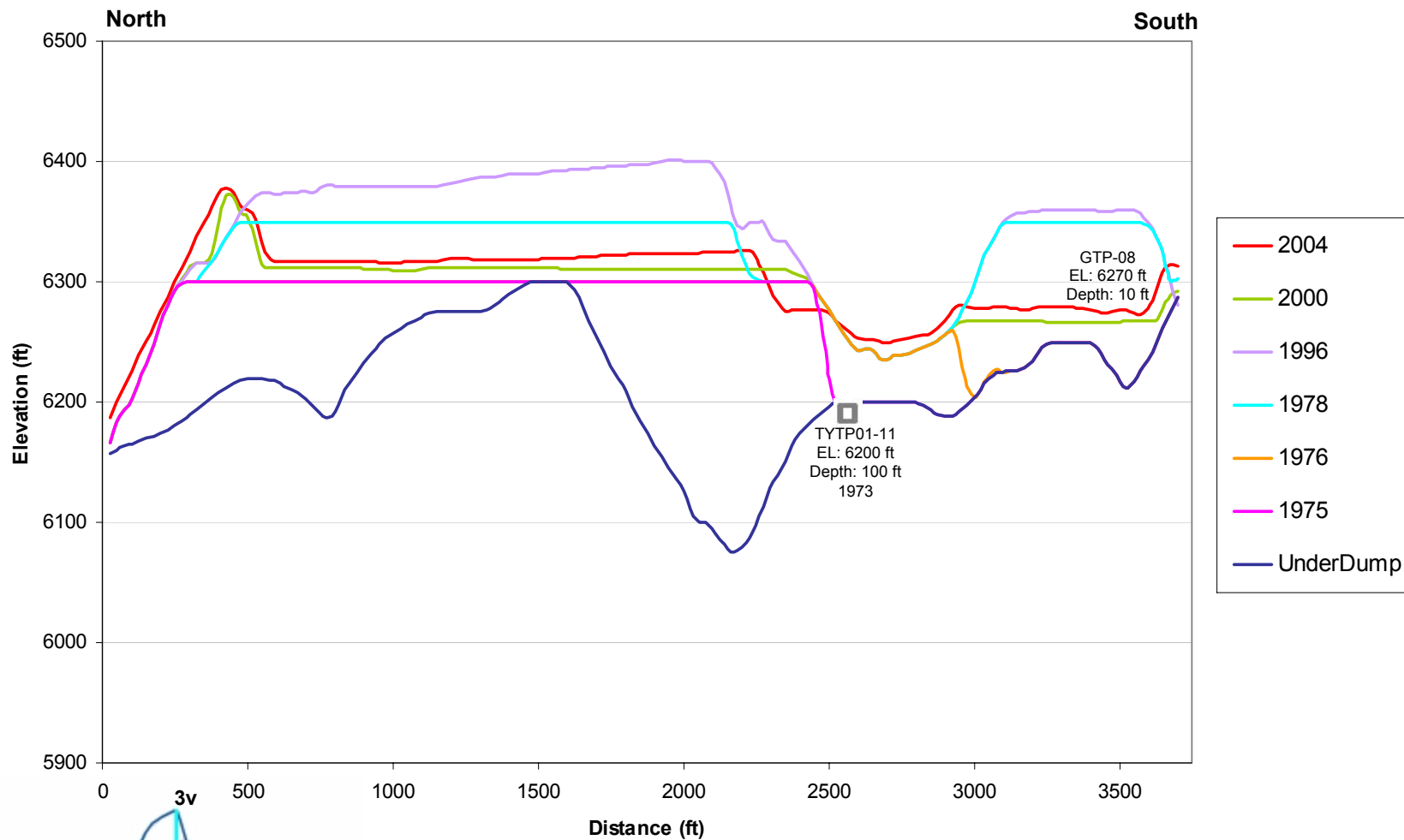
2B Leach
Stockpile

Tyrone Mine - 2B Leach Stockpile - Cross Section 3v



**2B Leach
Stockpile**

Tyrone Mine - 2B Leach Stockpile - Cross Section 3v



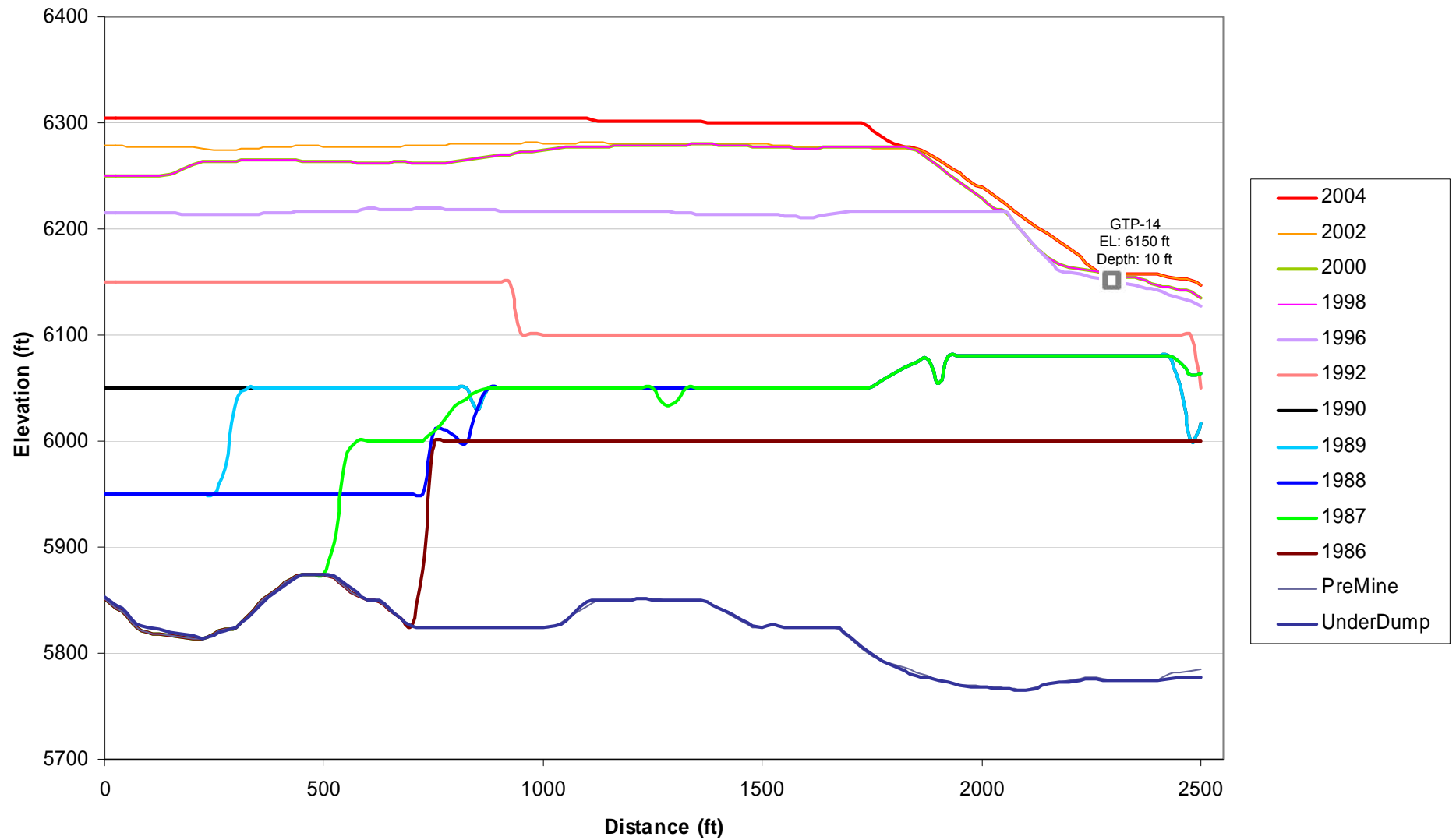
**2B Leach
Stockpile**

COMPOSITIONAL MODELS – 3A Leach Stockpile

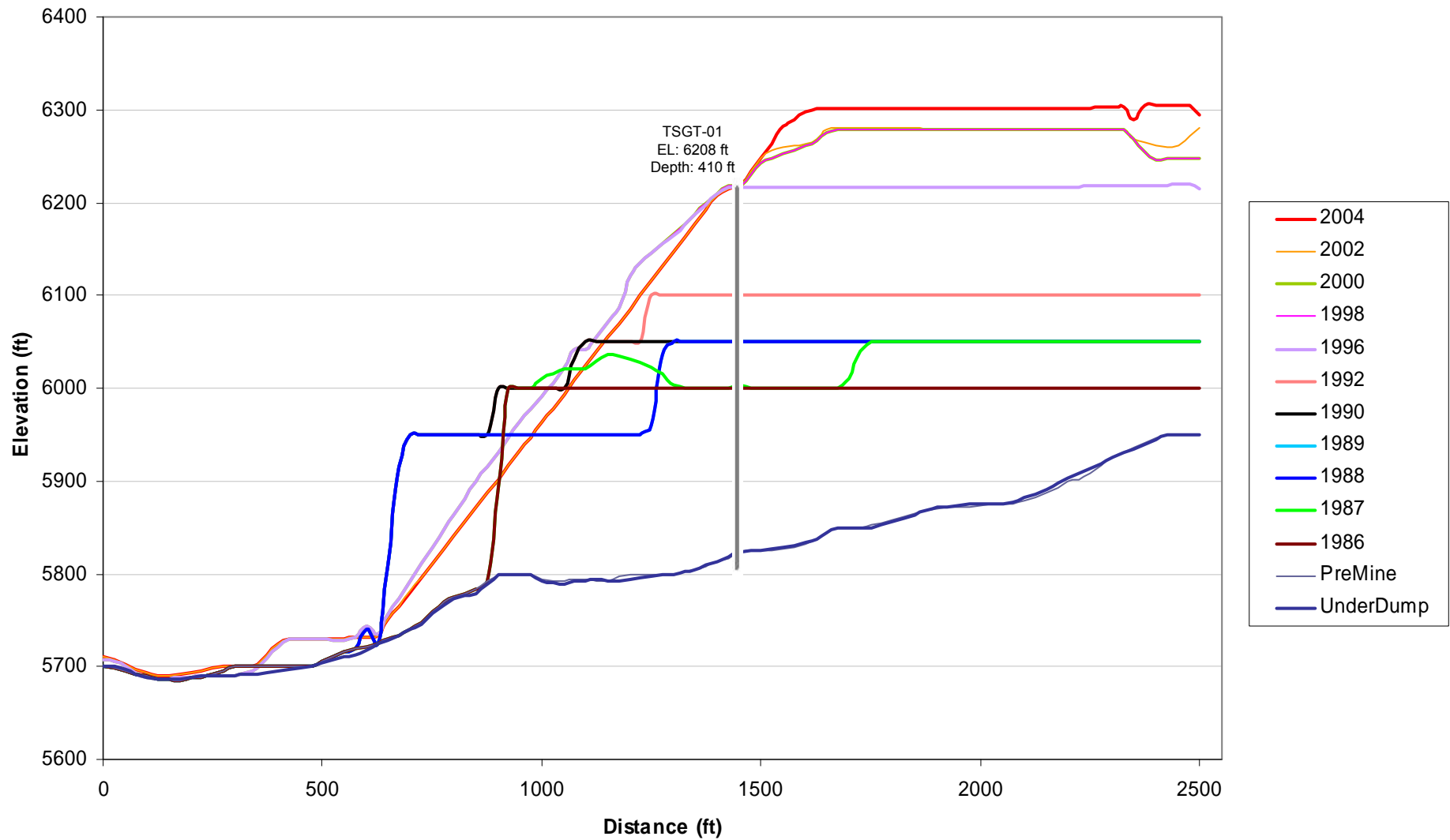
Tyrone Cross Section 3A Leach

West

East



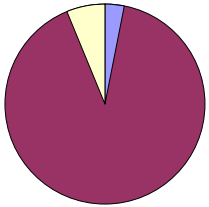
South



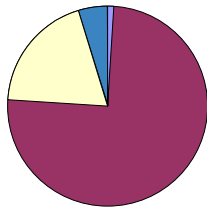
COMPOSITIONAL MODELS – 5A Waste Stockpile

5A Waste Stockpile

1973



1975



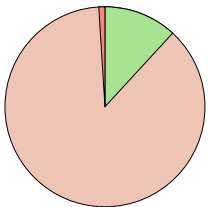
1976

No material deposited

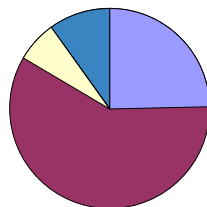
1978

No material deposited

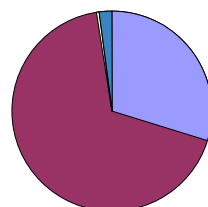
1982



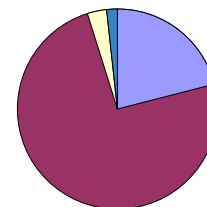
1986



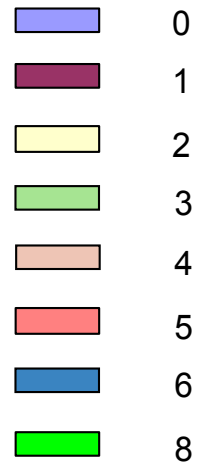
1987



1988



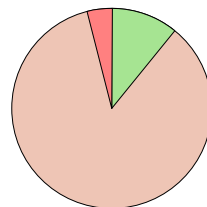
Mineral
Assemblage
Code



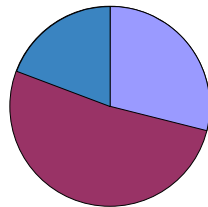
1989

No material deposited

1990



1992



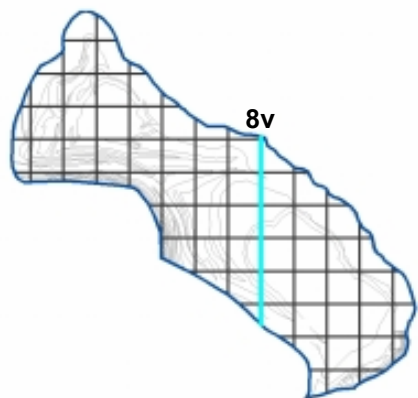
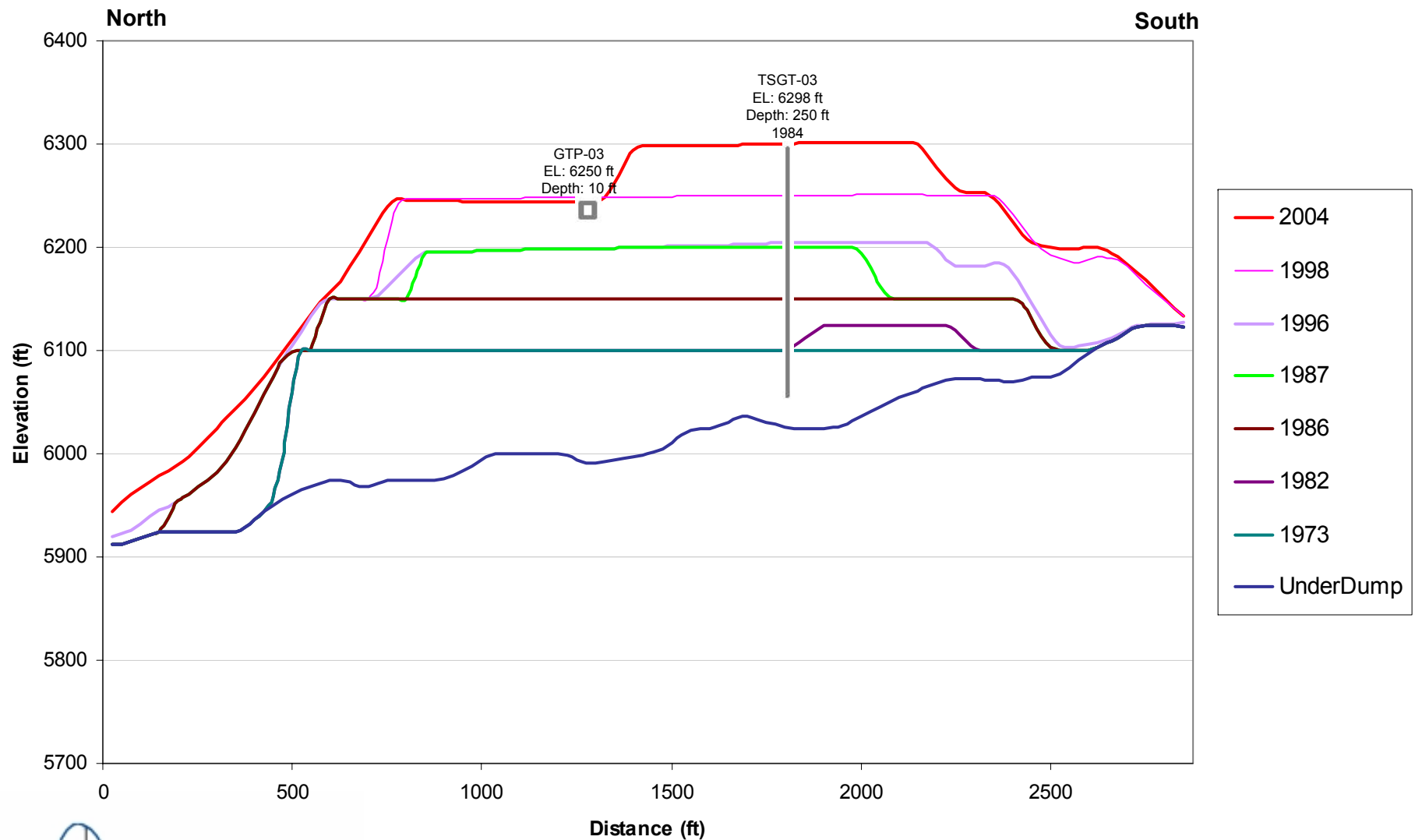
1995

No material deposited

1996

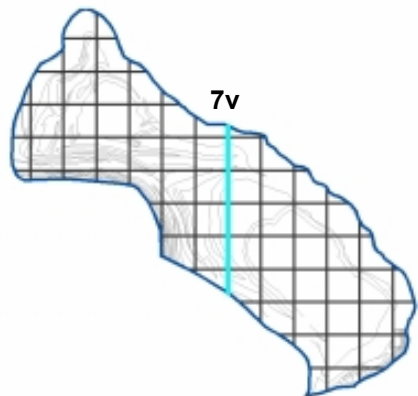
No material deposited

Tyrone Mine - 5A Waste Stockpile - Cross Section 8v



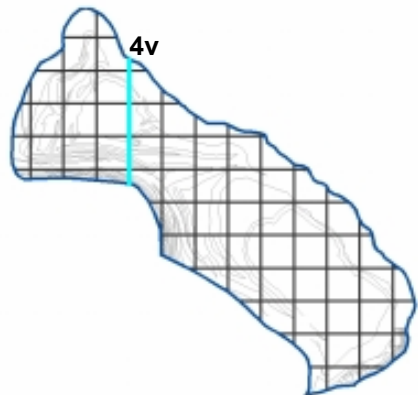
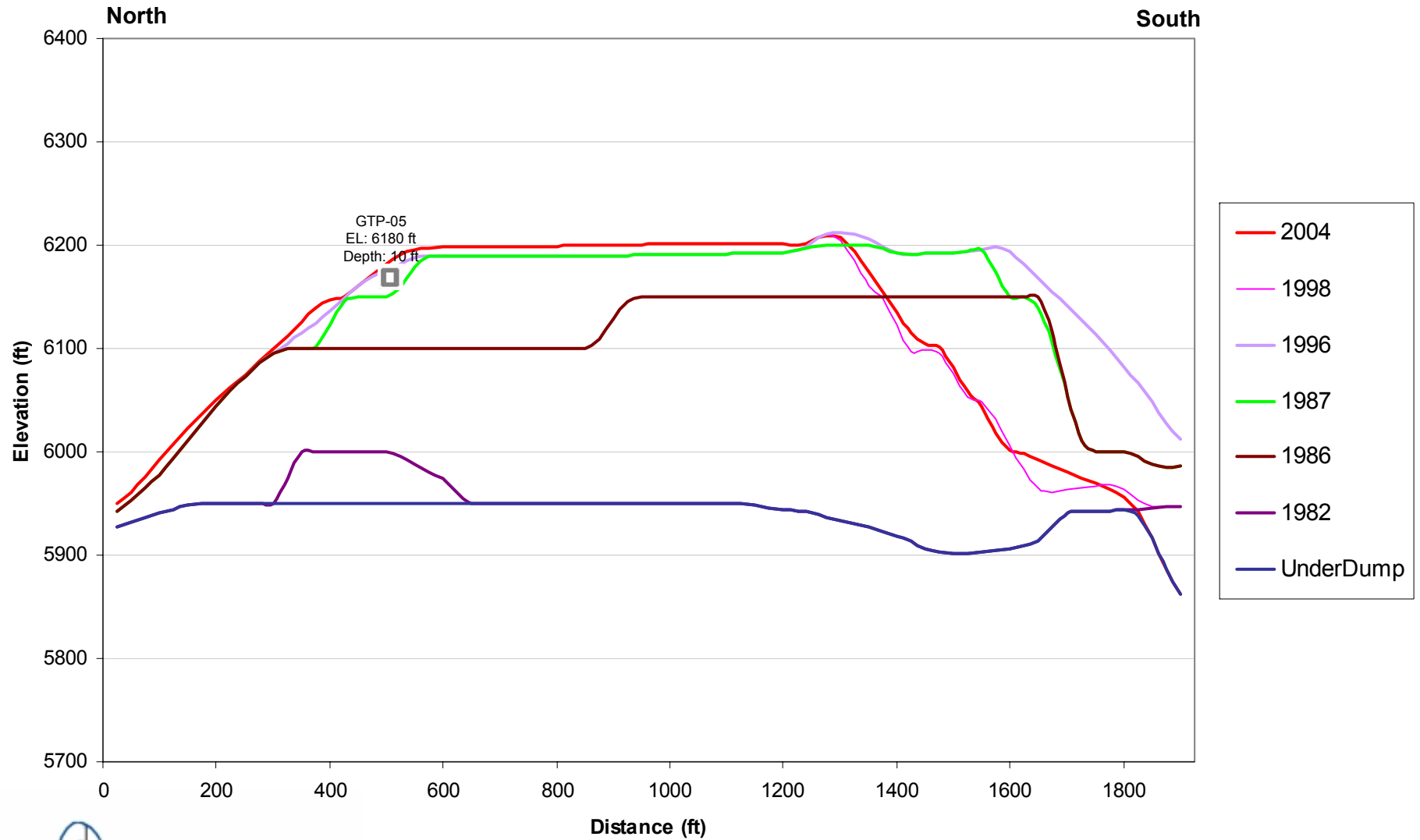
5A Waste
Stockpile

Tyrone Mine - 5A Waste Stockpile - Cross Section 7v



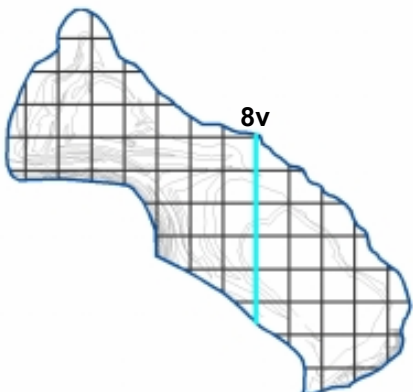
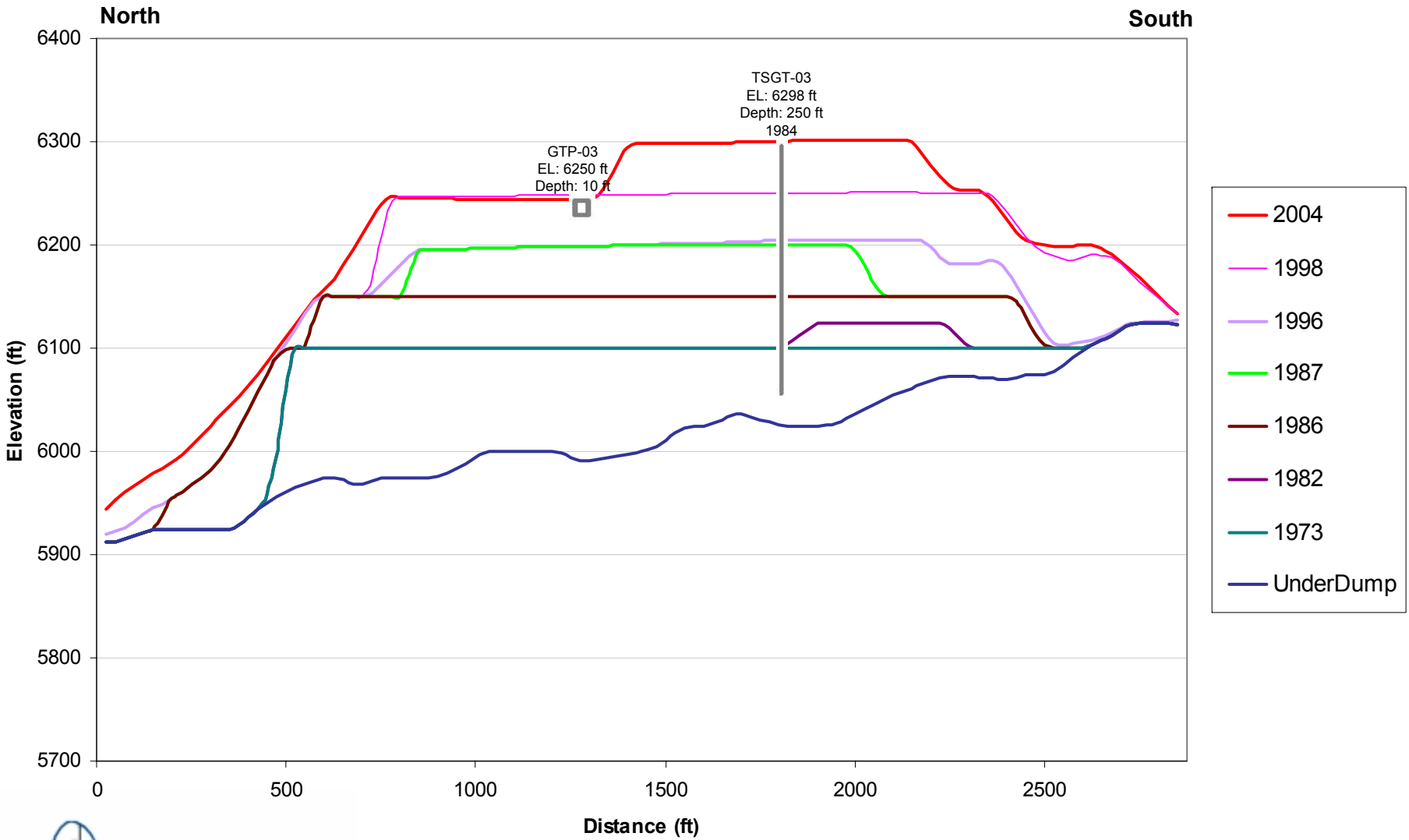
5A Waste
Stockpile

Tyrone Mine - 5A Waste Stockpile - Cross Section 4v



5A Waste
Stockpile

Tyrone Mine - 5A Waste Stockpile - Cross Section 8v

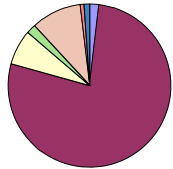


5A Waste
Stockpile

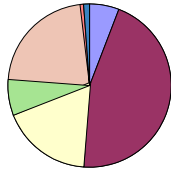
COMPOSITIONAL MODELS – Reclaimed 7A Waste Stockpiles

Reclaimed 7A Waste Stockpile

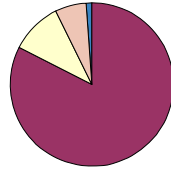
1973



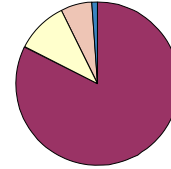
1975



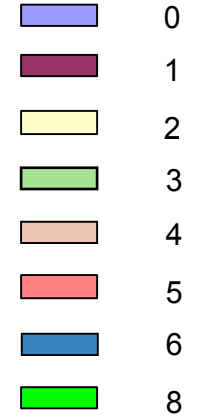
1976



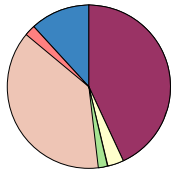
1978



Mineral
Assemblage
Code



1982



1986

No material
deposited

1987

No material
deposited

1988

No material
deposited

1989

No material
deposited

1990

No material
deposited

1992

No material
deposited

1995

No material
deposited

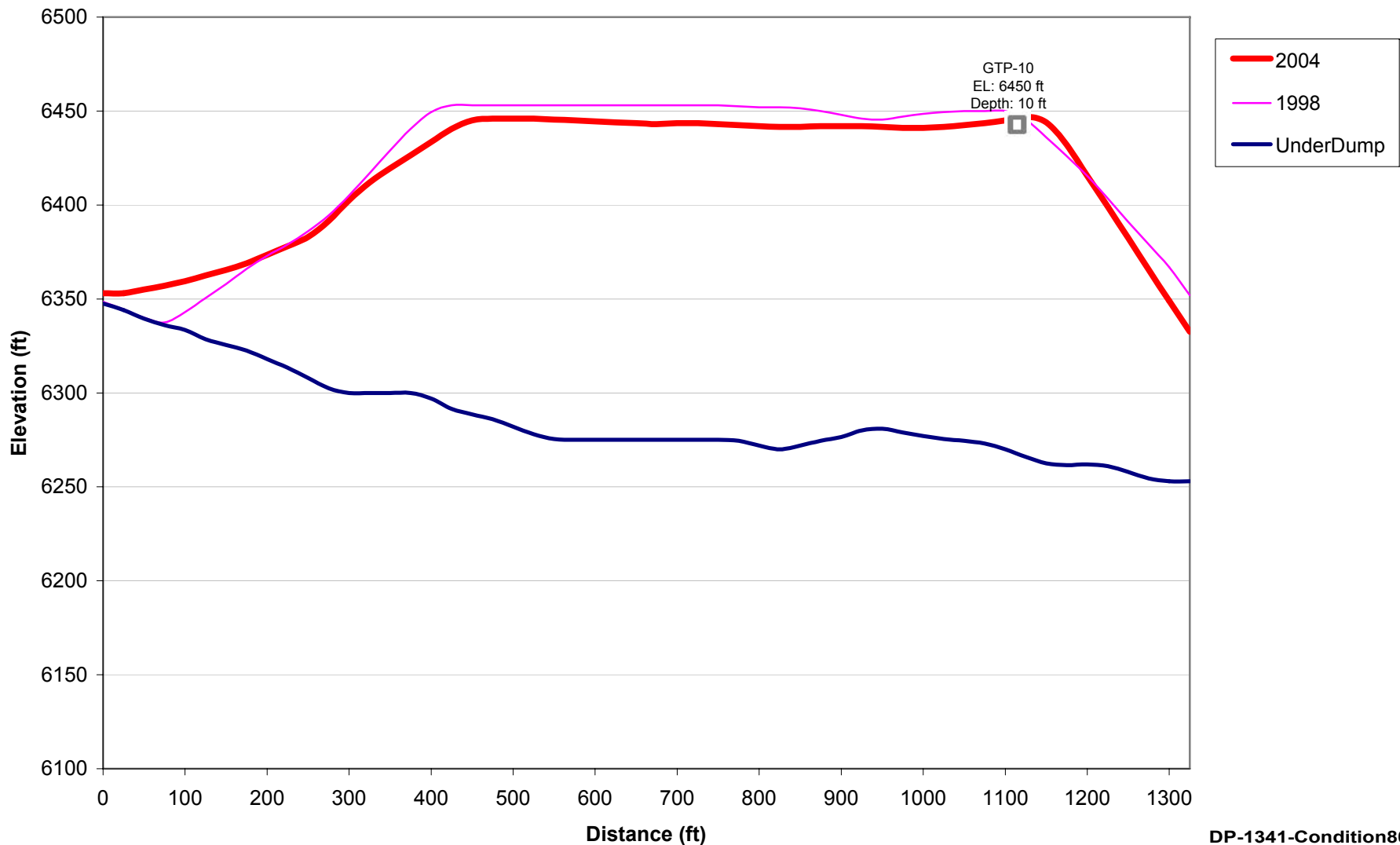
1996

Not
available

Tyrone Cross Section Reclaimed 7A Waste _8h

West

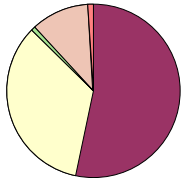
East



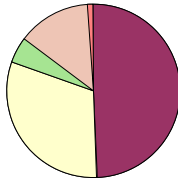
COMPOSITIONAL MODELS –Pit Leach Stockpiles

Copper Mountain Pit Leach Stockpile

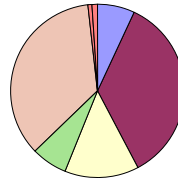
1973



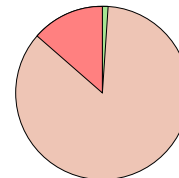
1975



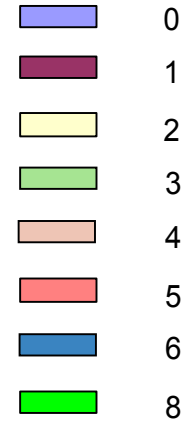
1976



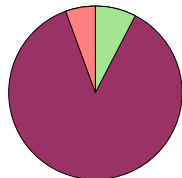
1978



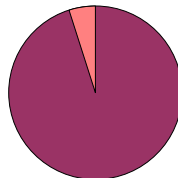
Mineral
Assemblage
Code



1982



1986



1987

No material
deposited

1988

No material
deposited

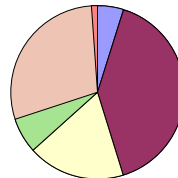
1989

No material
deposited

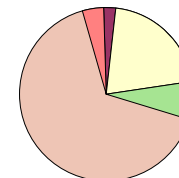
1990

No material
deposited

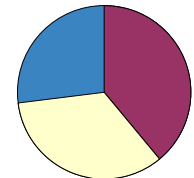
1992



1995

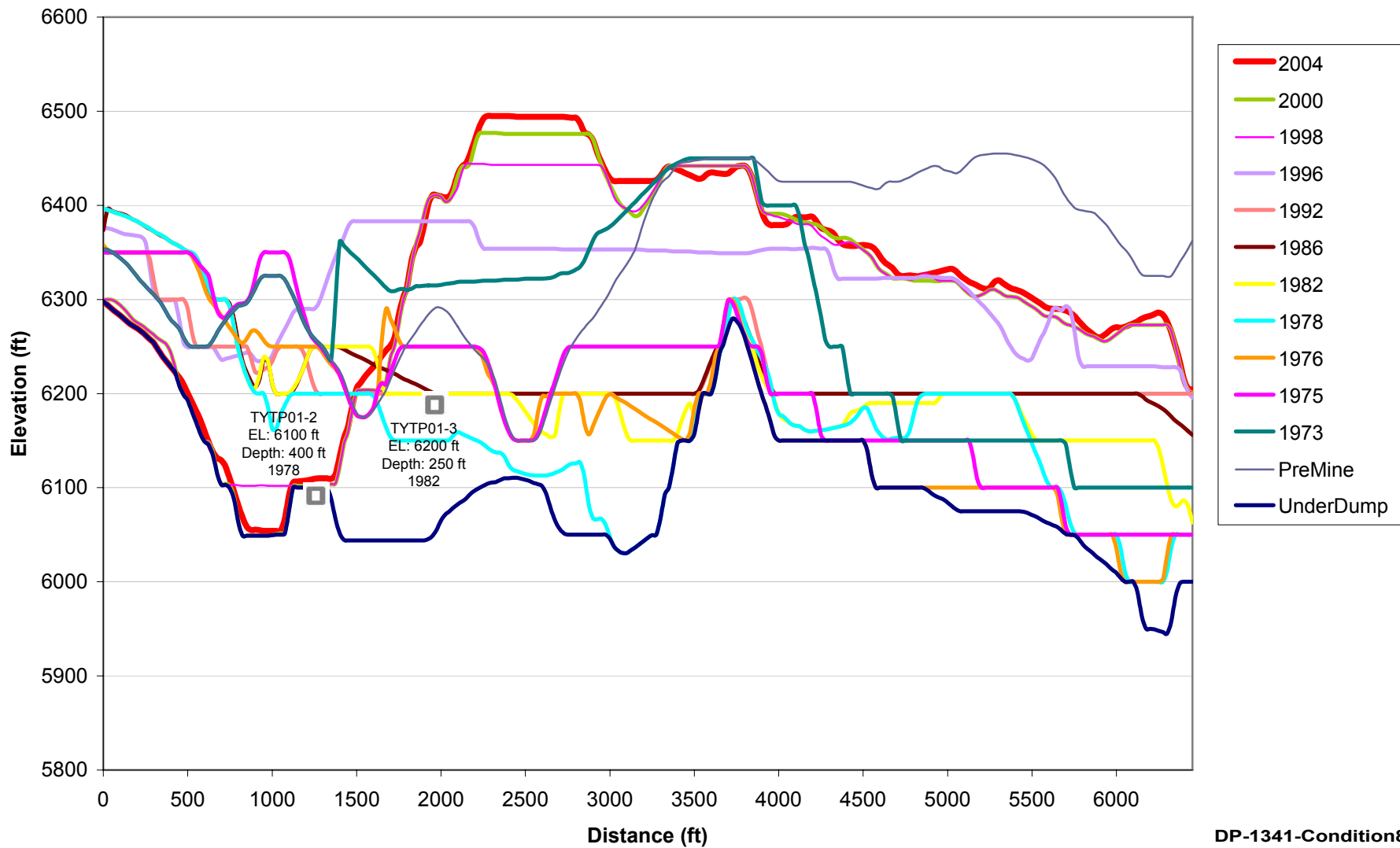


1996

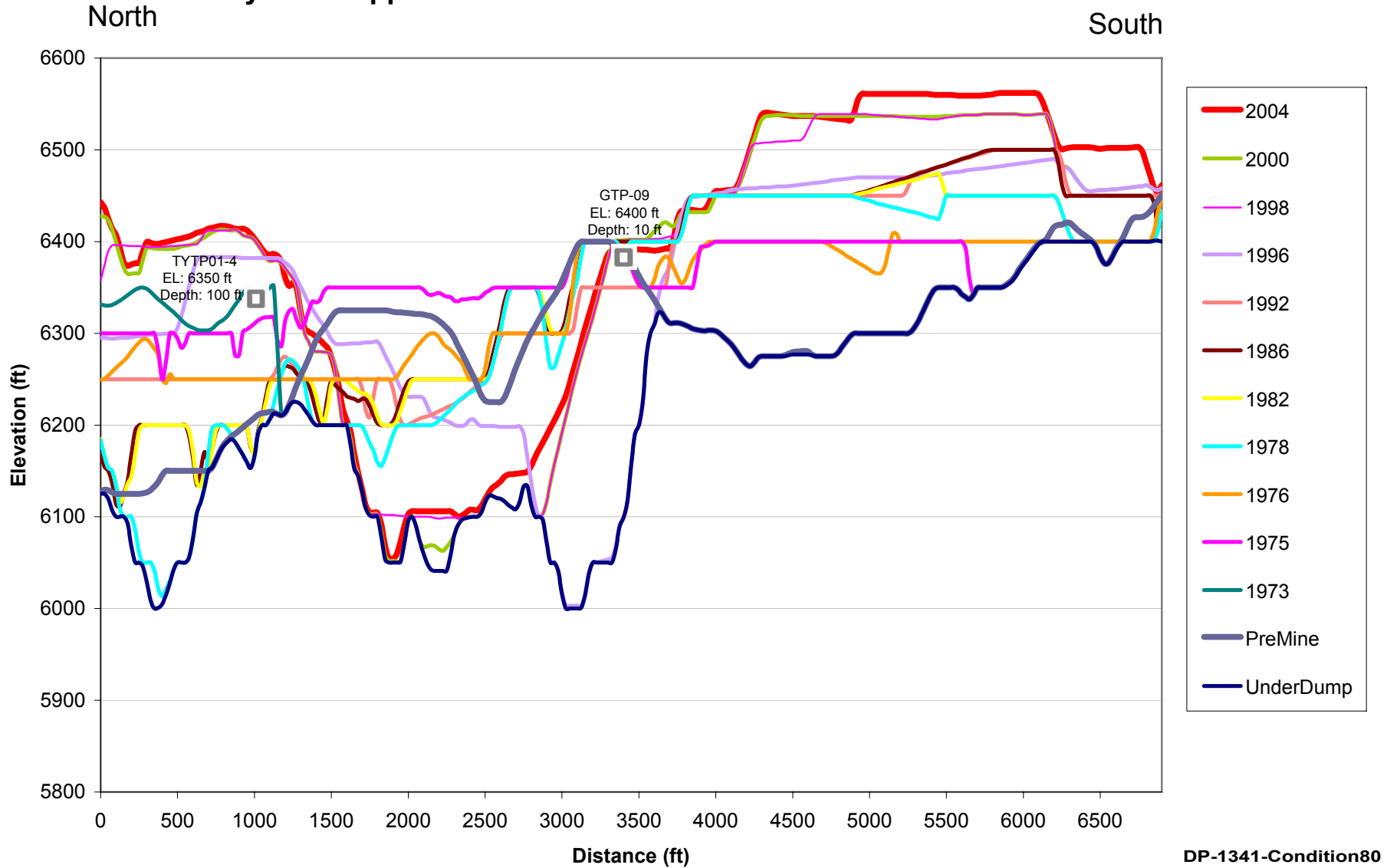


West

East



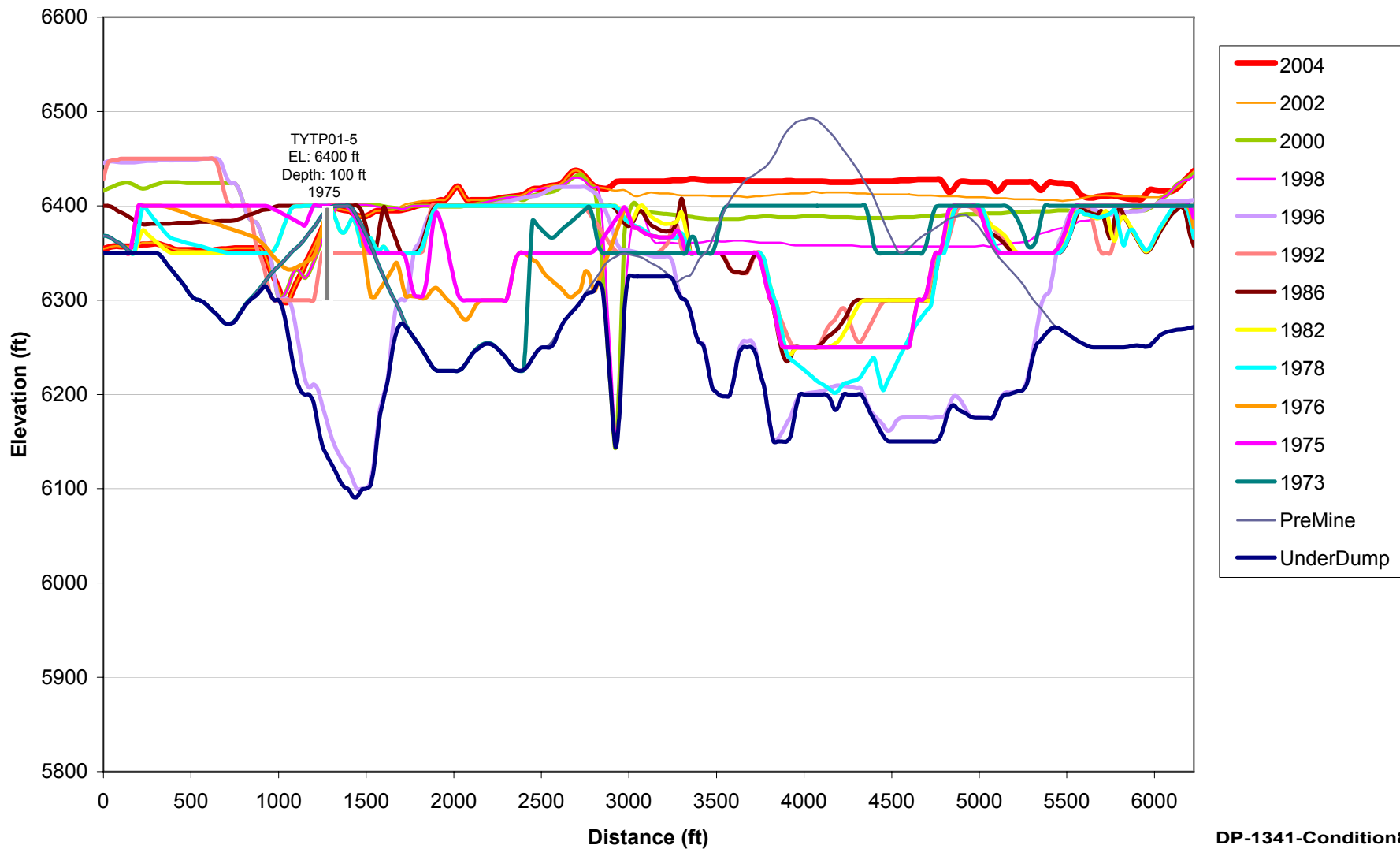
Tyrone Copper Mountain Pit Leach - Cross Section 6v



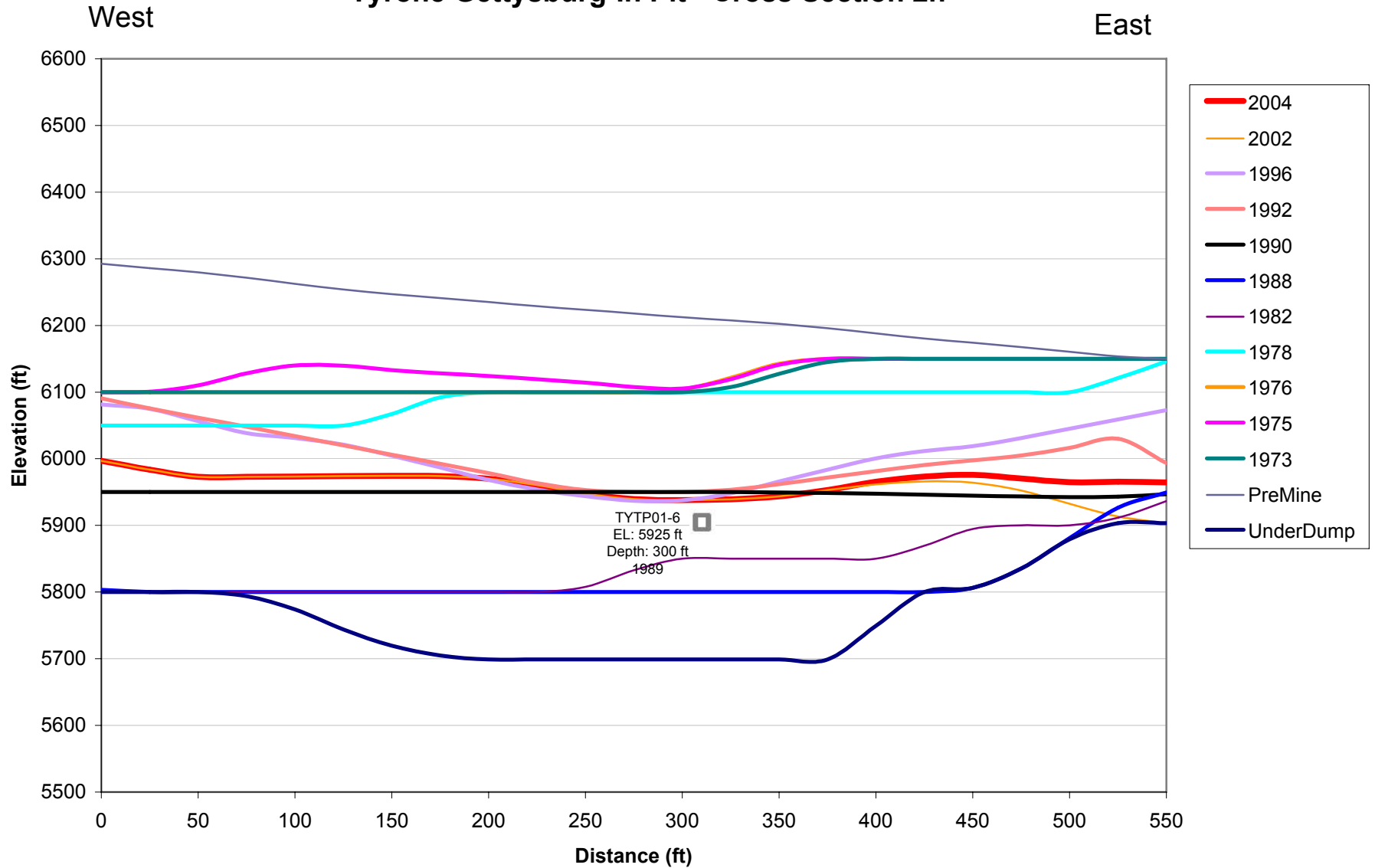
Tyrone Copper Mountain Pit Leach - Cross Section 7h

West

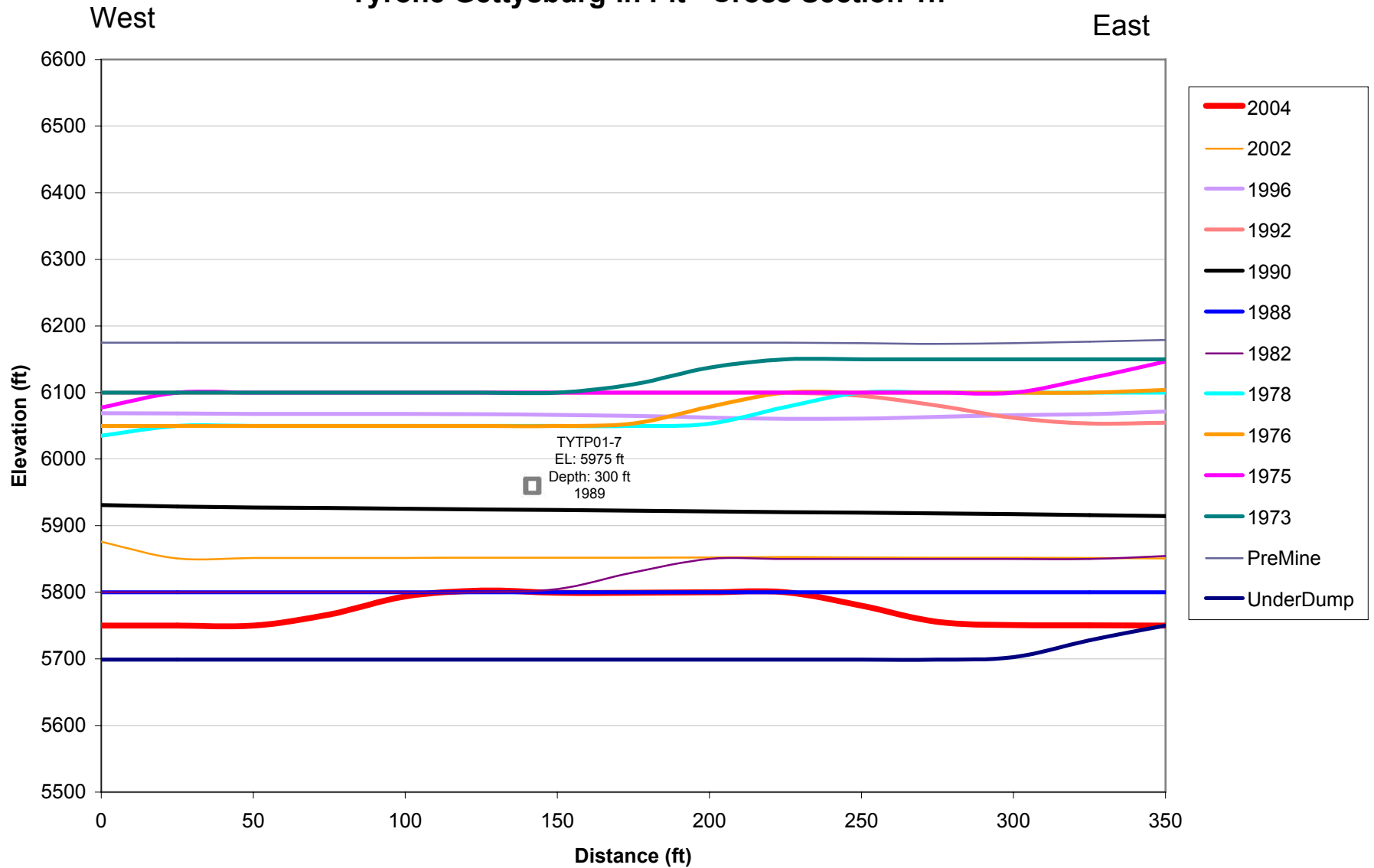
East



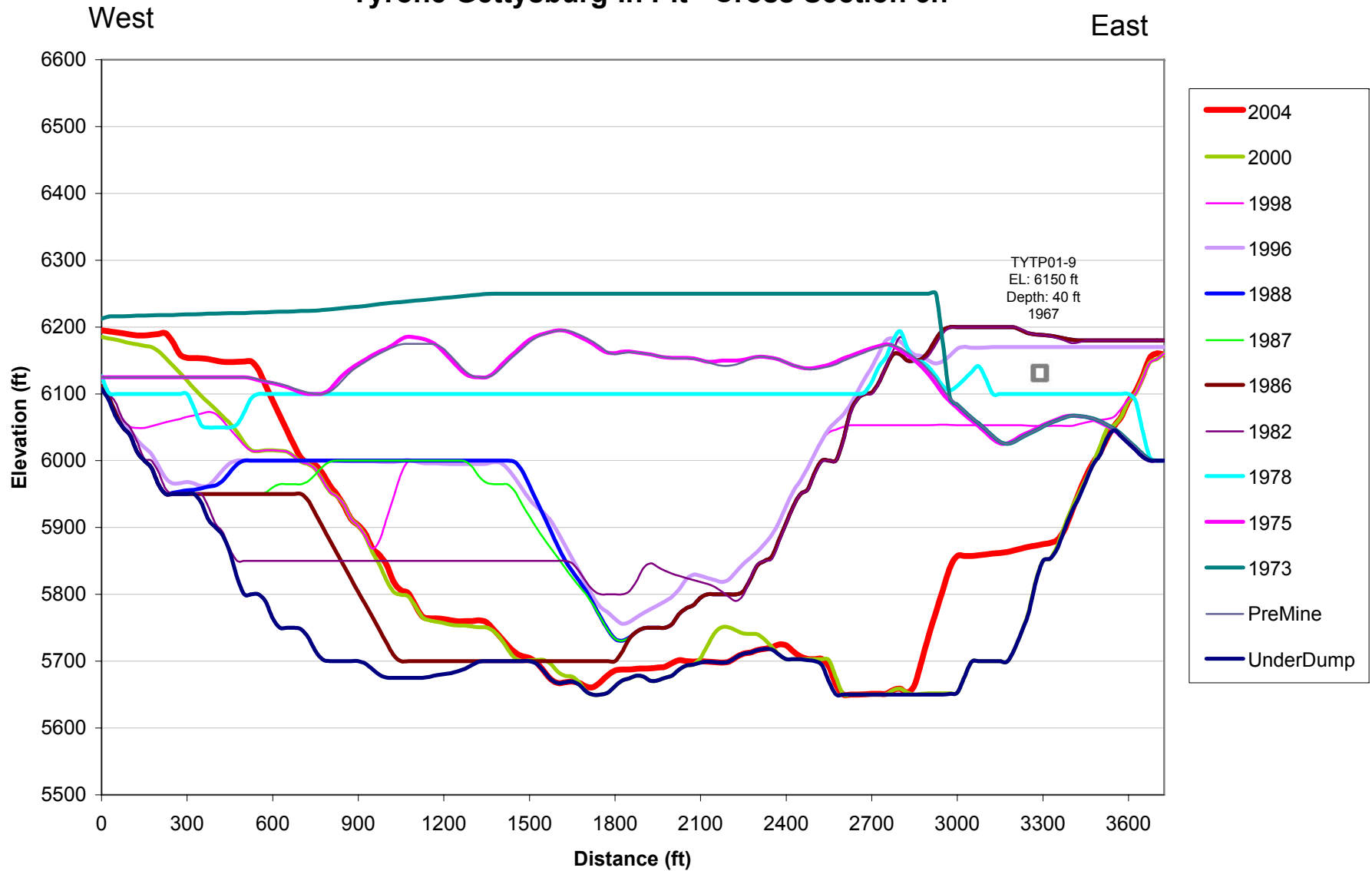
Tyrone Gettysburg-In Pit - Cross Section 2h



Tyrone Gettysburg-In Pit - Cross Section 1h



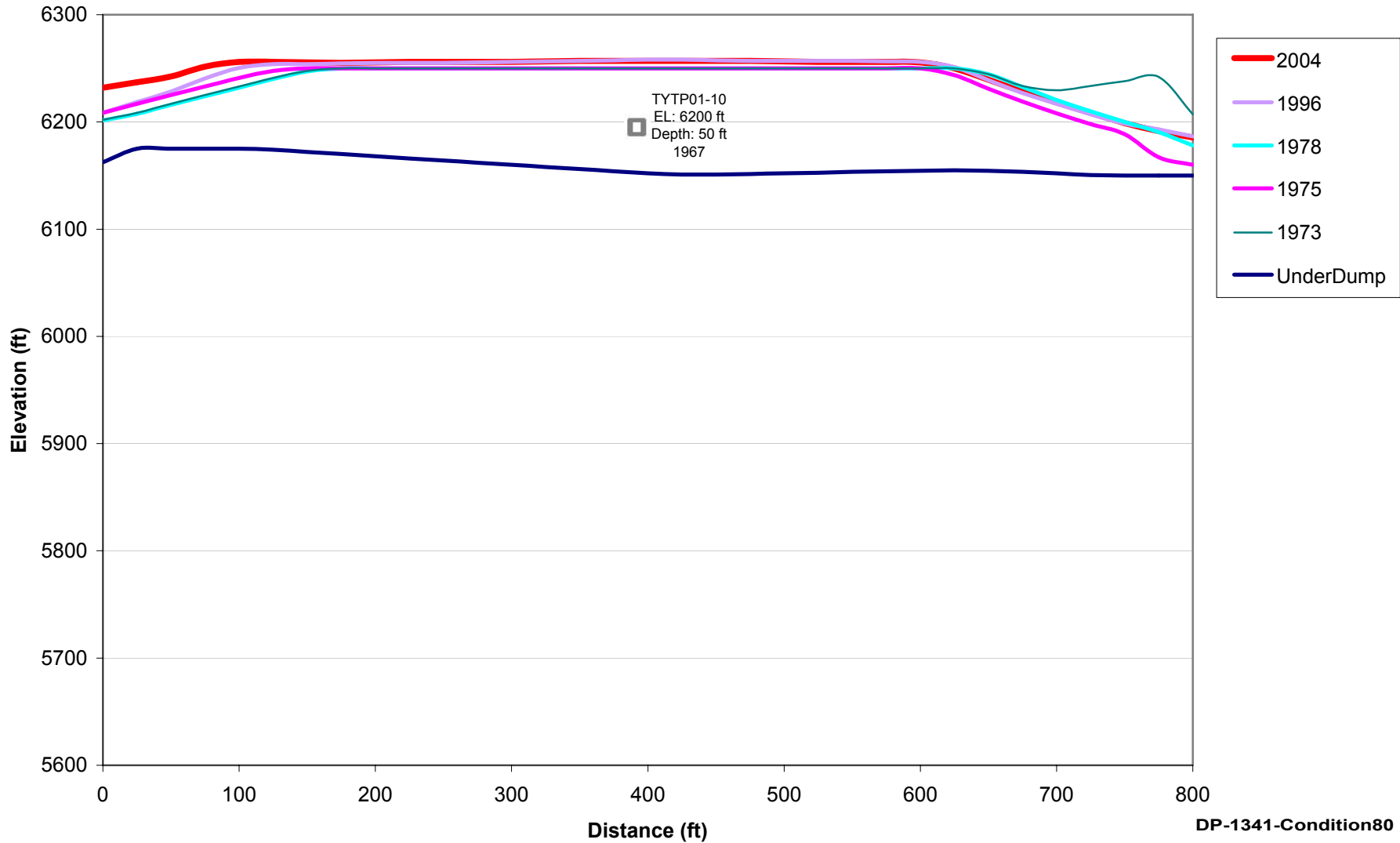
Tyrone Gettysburg-In Pit - Cross Section 5h



Tyrone 6A Leach (Savanna In Pit) - Cross Section 2h

West

East



APPENDIX III

Previously Unreported Testing

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

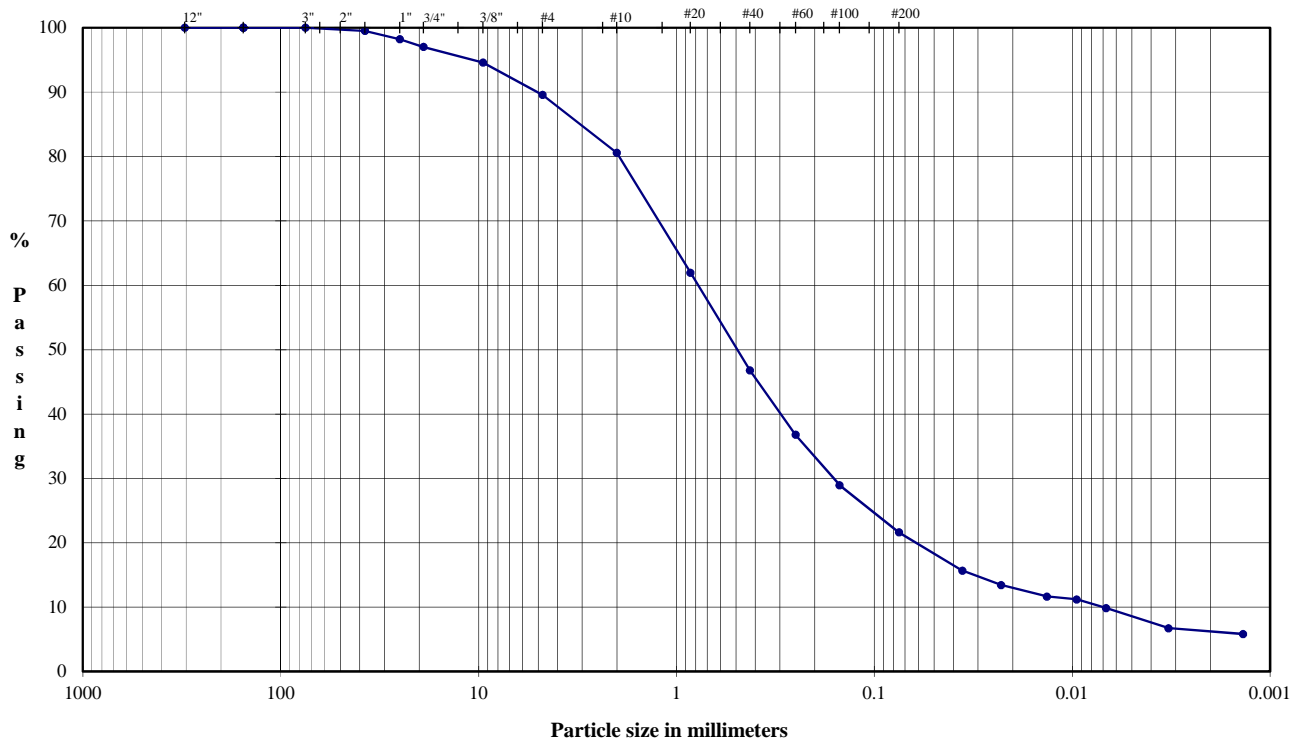
ASTM D421, D422, D4318

PROJECT NAME: **G&K/Tyrone Stockpile Geotech/AZ**

SAMPLE ID: **TSGT-04**

Depth (ft): **265-269**

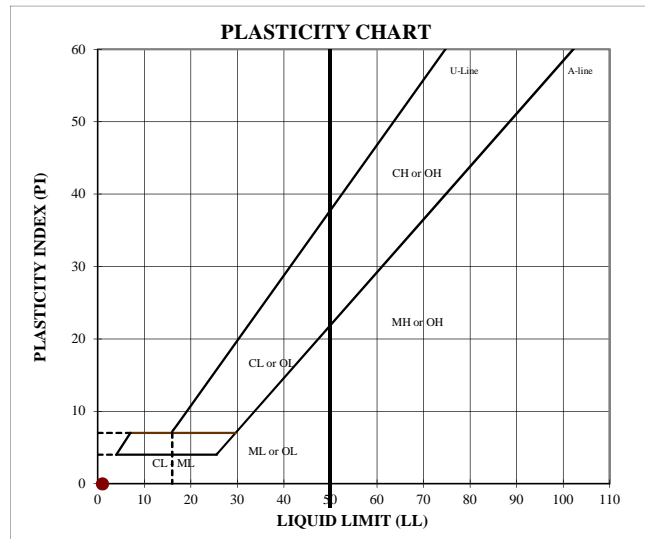
TYPE: **Pail**



	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
COBBLES	GRAVEL		SAND			FINES

U.S. Standard Sieves Sizes and Numbers	Particle Size (mm)		Particle Size Classification	
	(mm)	% Passing	Classification	Percentage
	12.0"	304.8	100.0	
	6.0"	154.2	100.0	
	6.0"	154.2	100.0	
	6.0"	154.2	100.0	
	3.0"	75.0	100.0	
	1.5"	37.5	99.5	
	1.0"	25.0	98.2	
	0.75"	19.0	97.0	
	0.375"	9.5	94.6	
	#4	4.8	89.6	
	#10	2.00	80.6	
	#20	0.85	61.9	
	#40	0.43	46.8	
	#60	0.25	36.8	
	#100	0.15	28.9	
	#200	0.075	21.6	

Hydrometer Analysis	(mm)	% Finer		
	0.036	15.7	Fines	21.60
	0.023	13.4		
	0.013	11.6		
	0.0095	11.2		
	0.0068	9.8	Silt or Clay	21.60
	0.0033	6.7		
	0.0014	5.8		



ATTERBERG LIMITS

M _c	LL	PL	PI	SpG (assumed)
--	NP	NP	NP	2.70

DESCRIPTION: Light brown silty sand

USCS: SM

TECH: DS
DATE: 6/28/2005
REVIEW: MB

Sample # = TSTG-04
Point # = 1

Initial
Length = 14.67 cm
Diameter = 7.27 cm
Wet Weight = 1235.20 g
Area = 41.5 cm²
Sample Area = 6.43 in²

Volume = 608.9 cm³
Moisture Content = 11.0%
Specific Gravity = -
Dry Weight of Solids = 1112.79 g
Wet Unit Weight = 2.03 g/cm³
Dry Unit Weight = 1.83 g/cm³
Wet Unit Weight = 126.6 pcf
Dry Unit Weight = 114.0 pcf

Cell Pressure = 80 psi
Back Pressure = 50 psi
Confining Pressure = 30 psi

Sample # = TSTG-04
Point # = 2

Initial
Length = 14.67 cm
Diameter = 7.27 cm
Wet Weight = 1235.20 g
Area = 41.5 cm²
Sample Area = 6.43 in²

Volume = 608.9 cm³
Moisture Content = 11.0%
Specific Gravity = -
Dry Weight of Solids = 1112.79 g
Wet Unit Weight = 2.03 g/cm³
Dry Unit Weight = 1.83 g/cm³
Wet Unit Weight = 126.6 pcf
Dry Unit Weight = 114.0 pcf

Cell Pressure = 150 psi
Back Pressure = 50 psi
Confining Pressure = 100 psi

Sample # = TSTG-04
Point # = 3

Initial
Length = 14.67 cm
Diameter = 7.27 cm
Wet Weight = 1235.20 g
Area = 41.5 cm²
Sample Area = 6.43 in²

Volume = 608.9 cm³
Moisture Content = 11.0%
Specific Gravity = -
Dry Weight of Solids = 1112.79 g
Wet Unit Weight = 2.03 g/cm³
Dry Unit Weight = 1.83 g/cm³
Wet Unit Weight = 126.6 pcf
Dry Unit Weight = 114.0 pcf

Cell Pressure = 250 psi
Back Pressure = 50 psi
Confining Pressure = 200 psi

Notes: Material visually described as sand, reddish-brown, with clay and fine gravel.
Specimen remolded with a light to moderate tamp at visually estimated optimum moisture content.
Failure defined as maximum principal stress ratio.
The strain rate was 0.1mm/min, and t₅₀ was 0.1 minutes.
Test was a staged triaxial test.

Golder Associates, Inc.
Denver, Colorado

Title:

TRIAXIAL SHEAR TEST REPORT
SAMPLE DATA AND CALCULATIONS

Job Short Title:

PD Tyrone/Stockpile Geotech

Sample Number:

TSTG-04 @ 265-268

Reviewed:

JEO

Date:

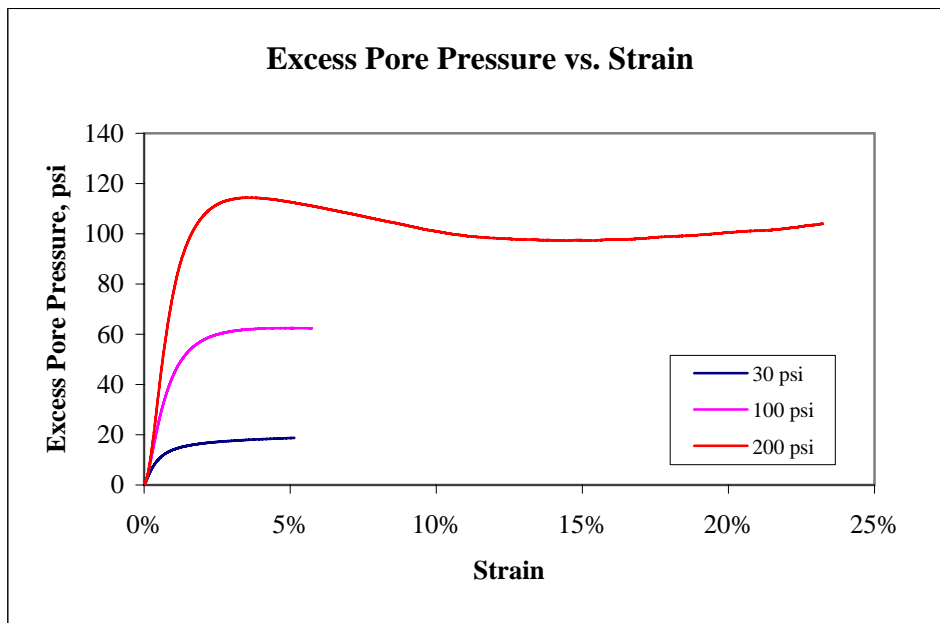
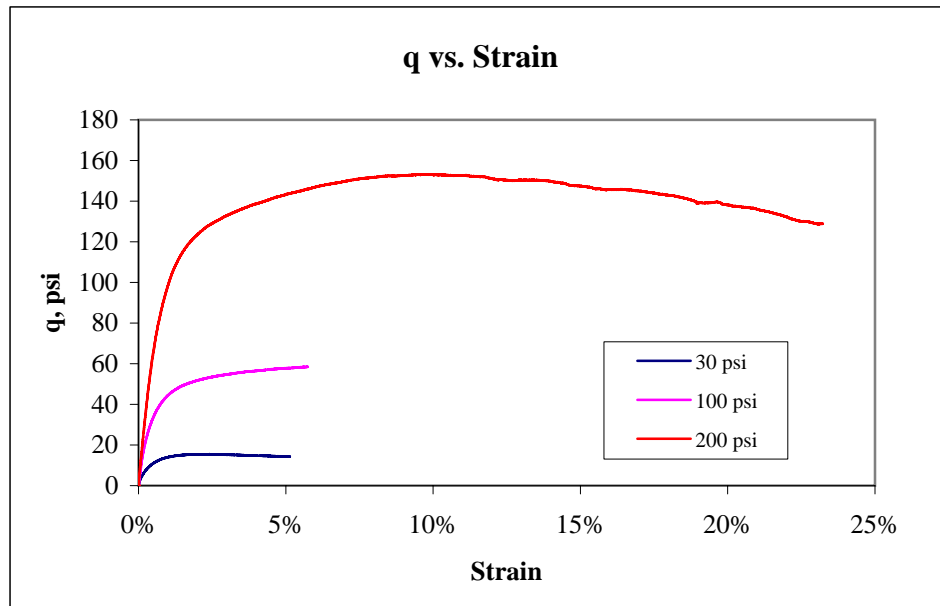
6/7/2006

Job Number:

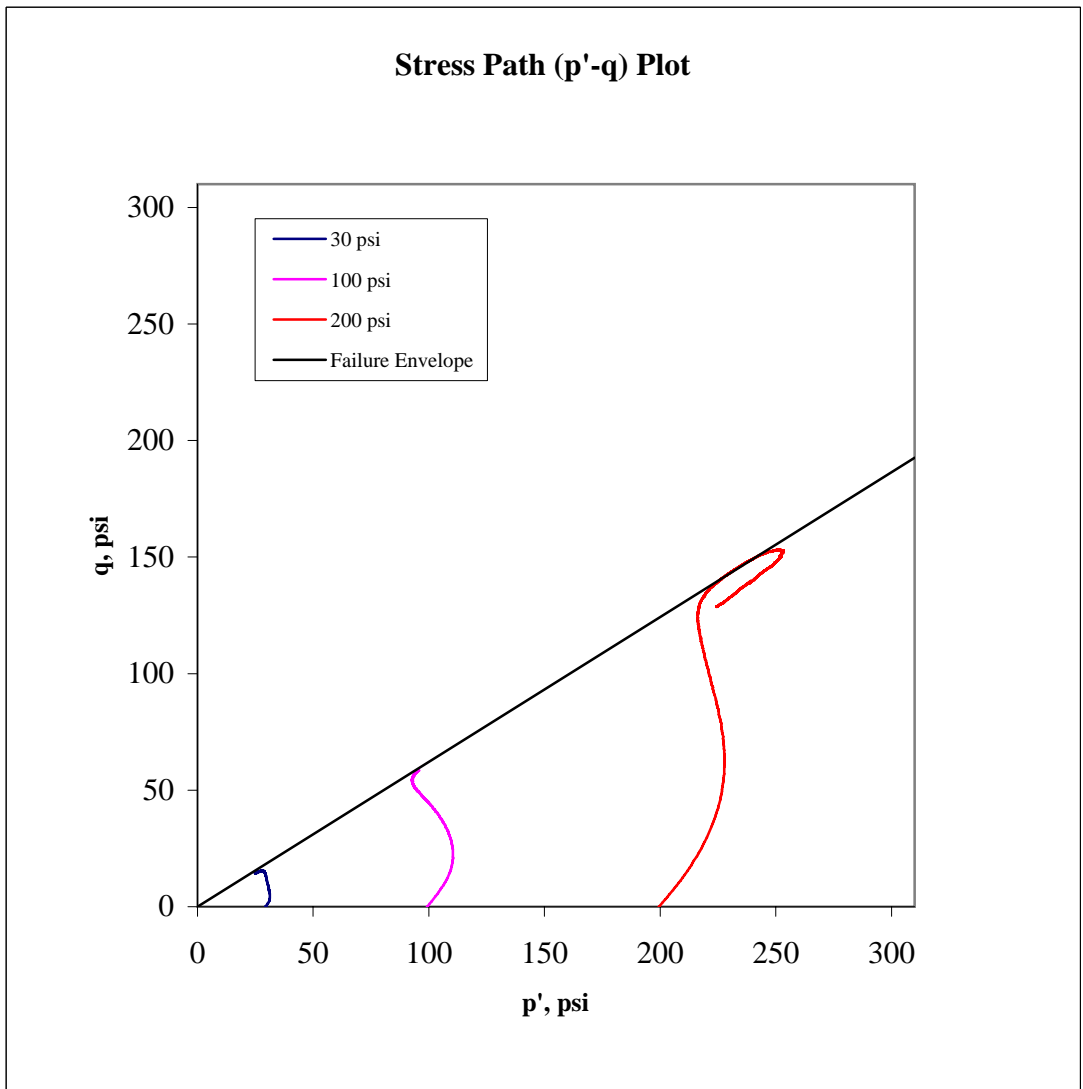
053-2550

Figure:

1



Golder Associates, Inc. Denver, Colorado		Title: C-U TRIAXIAL SHEAR DATA			
Job Short Title: PD Tyrone/Stockpile Geotech		q AND EXCESS PORE PRESSURE PLOTS			
Sample Number: TSTG-04 @ 265-268	Reviewed: JEO	Date: 06/07/06	Job Number: 053-2550	Figure: 2	

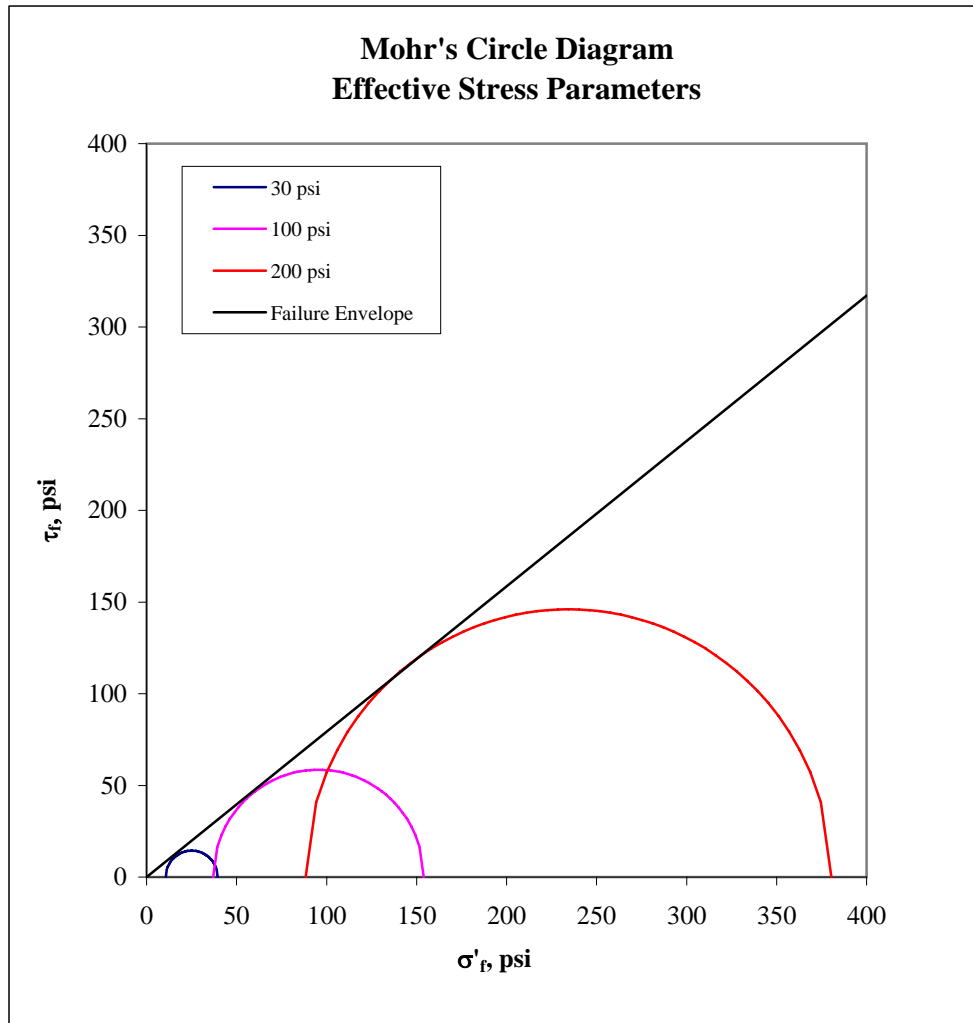


Stress Path Parameters

$\psi' = 31.8$ degrees

$a' = 0.0$ psi

Golder Associates, Inc. Denver, Colorado		Title: C-U TRIAXIAL SHEAR DATA STRESS PATH PLOT		
Job Short Title: PD Tyrone/Stockpile Geotech				
Sample Number: TSTG-04 @ 265-268	Reviewed: JEO	Date: 6/7/2006	Job Number: 053-2550	Figure: 3



Effective Stress Shear Strength Parameters

$\phi' = 38.4$ degrees

$c' = 0.0$ psi

Golder Associates, Inc. Denver, Colorado		Title: C-U TRIAXIAL SHEAR DATA MOHR'S CIRCLE DIAGRAM			
Job Short Title: PD Tyrone/Stockpile Geotech					
Sample Number: TSTG-04 @ 265-268		Reviewed: JEO	Date: 6/7/2006	Job Number: 053-2550	Figure: 4

Consolidated-Undrained Triaxial Lab Data

From: GOLDER ASSOCIATES, INC.

Project: PD Tyrone/Stockpile Geotech

Project Number: 053-2550

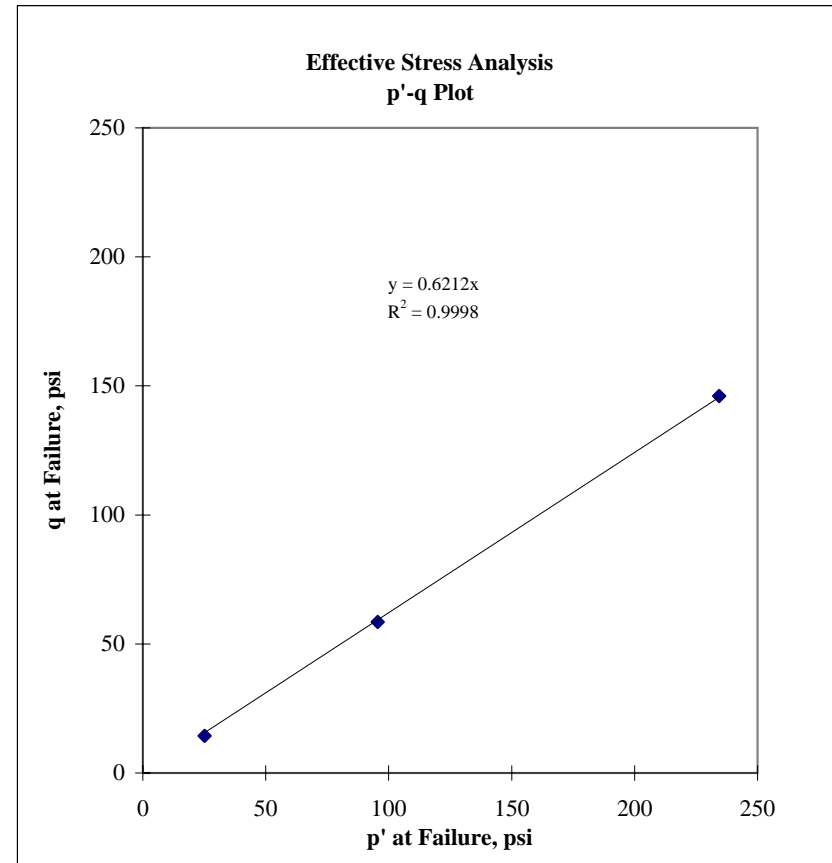
Sample Number TSTG-04 @ 265-268

Effective Stress Analysis

Point Number	p' (psi)	q (psi)
1	25.1	14.4
2	95.5	58.5
3	234.4	146.0

$$\tan(\psi') = 0.6212$$
$$a' = 0.0 \text{ psi}$$

$$\phi' = 38.4 \text{ degrees}$$
$$c' = 0.0 \text{ psi}$$



Consolidated-Undrained Triaxial Lab Data

From: GOLDER ASSOCIATES, INC.

Project: PD Tyrone/Stockpile Geotech

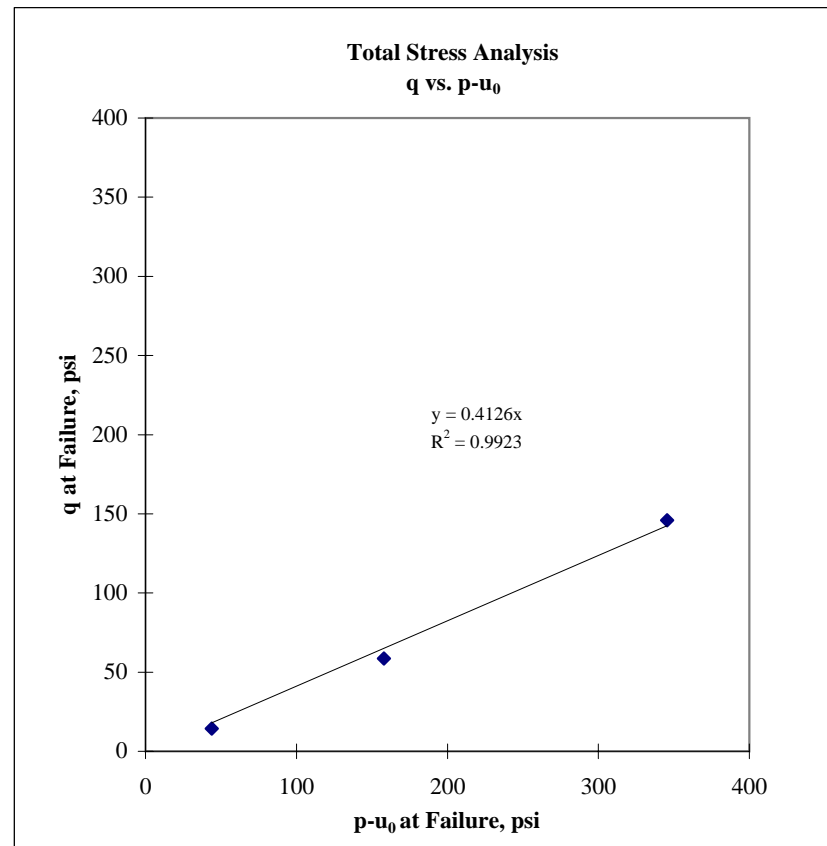
Project Number: 053-2550

Sample Number TSTG-04 @ 265-268

Total Stress Analysis

Point Number	p-u ₀ (psi)	q (psi)
1	43.8	14.4
2	157.8	58.5
3	345.5	146.0

$\tan(\psi) = 0.4126$
a = 0.0 psi
 $\phi = 24.4$ degrees
c = 0.0 psi



Consolidated-Undrained Triaxial Lab Data

From: GOLDER ASSOCIATES, INC.

Project: PD Tyrone/Stockpile Geotech

Project Number: 053-2550

Mohr-Coulomb Failure Criteria:

$$\tau_{ff} = c' + \sigma'_{ff} \tan(\phi')$$

$$\tau_{ff} = c + \sigma_{ff} \tan(\phi)$$

Where:

c' , c = effective and total stress cohesion intercepts

ϕ' , ϕ = effective and total stress friction angles

τ_{ff} = shear strength on the failure surface at failure

σ'_{ff} , σ_{ff} = effective and total normal stresses on the failure surface at failure

Stress Path Space:

$$q = \frac{\sigma_1 - \sigma_3}{2} \quad p' = \frac{\sigma'_1 + \sigma'_3}{2} \quad p = \frac{\sigma_1 + \sigma_3}{2}$$

Where:

q = maximum shear stress

p' , p = mean effective and total stresses

σ'_1 , σ_1 = effective and total axial stresses

σ'_3 , σ_3 = effective and total confining stresses

Stress Path Failure Criteria:

$$q = a' + p' \tan(\psi')$$

$$q = a + (p - u_0) \tan(\psi)$$

Where:

a' , a = intercepts of the q -axis in effective stress and total stress spaces

ψ' , ψ = angles of the failure envelopes in effective stress and total stress spaces

q = maximum shear stress at failure

p' = mean effective stress at failure

$p - u_0$ = mean total stress at failure minus the initial pore pressure

The relationships between ψ and ϕ and a and c are as follows:

$$\tan(\psi) = \sin(\phi)$$

$$a = c \cos(\phi)$$

The relationships between ψ' and ϕ' and a' and c' are as follows:

$$\tan(\psi') = \sin(\phi')$$

$$a' = c' \cos(\phi')$$



PD TYRONE/ STOCKPILE GEOTEC

053-2550

TSTG=4 @265-268

STAGED TRIAXIAL SHEAR TEST

Golder Associates, Inc.
Denver, Colorado

January-19

18106417.3B

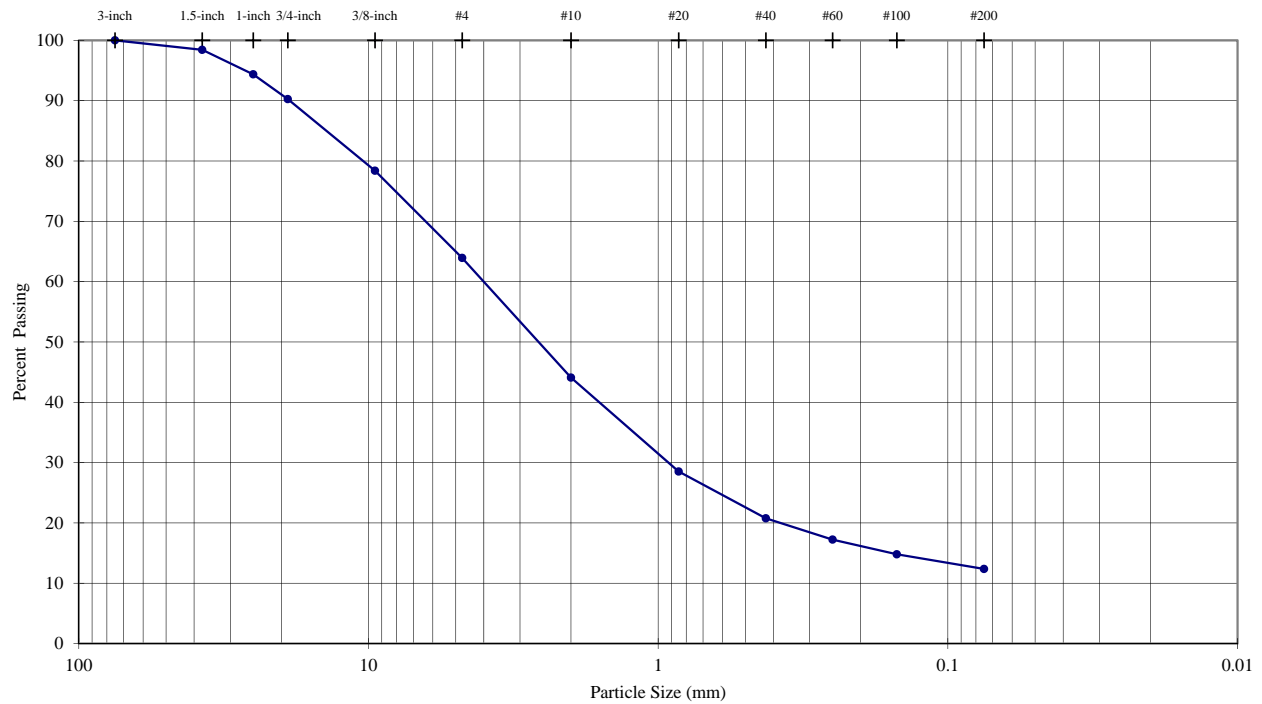
PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

ASTM D421, D422, D4318

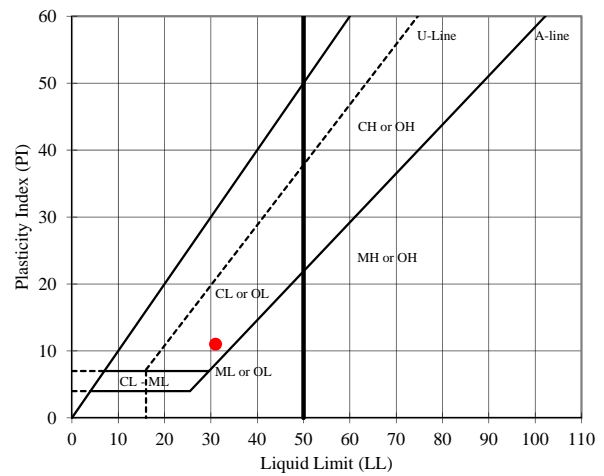
PROJECT NAME: **FMI/TyroneCCP Support 2018-2019**

SAMPLE ID: **TY18-01**

DEPTH (ft): --

TYPE: **Pail**


Sieve Analysis (Initial Separation on No. 4 Sieve)	Sieve	Particle Size (mm)	% Passing	Description	Percentage
	3-inch	75.0	100.0	Coarse Gravel	9.74
	1.5-inch	37.5	98.4		
	1-inch	25.0	94.4		
	3/4-inch	19.0	90.3	Fine Gravel	26.33
	3/8-inch	9.5	78.4		
	#4	4.75	63.9	Coarse Sand	19.84
	#10	2.0	44.1		
	#20	0.850	28.5	Medium Sand	23.33
	#40	0.425	20.8		
	#60	0.250	17.2	Fine Sand	8.40
	#100	0.150	14.8		
	#200	0.075	12.4	Silt or Clay Fines	12.36



USCS Description (ASTM D 2487):

Clayey sand with gravel, reddish brown, moist

As-Received Moisture Content (%)

--

USCS Group Symbol

SC

Notes: 0 g of particles up to 75.0 mm maximum size were removed from particle size analysis sample prior to testing

Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed

Sample prepared for Atterberg Limits testing by the dry method

Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving

Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH JP

DATE 4-Jan-2019

REVIEW MB

January-19

18106417.3B

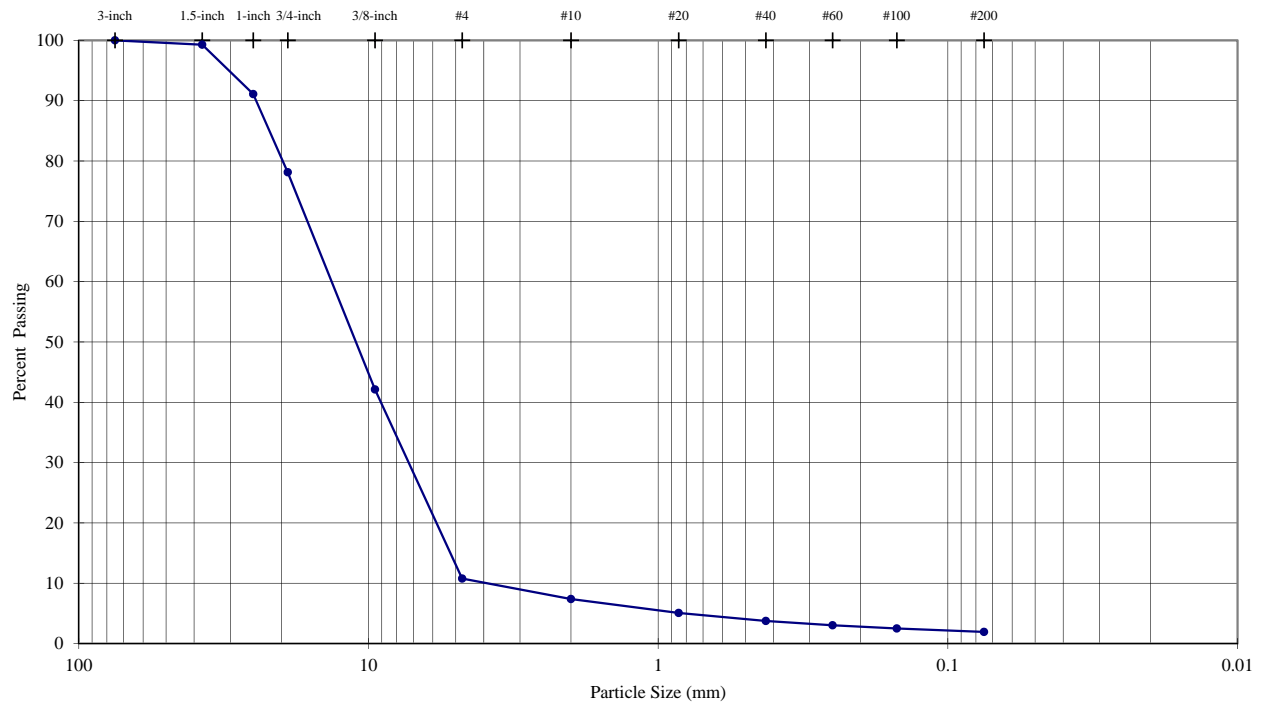
PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

ASTM D421, D422, D4318

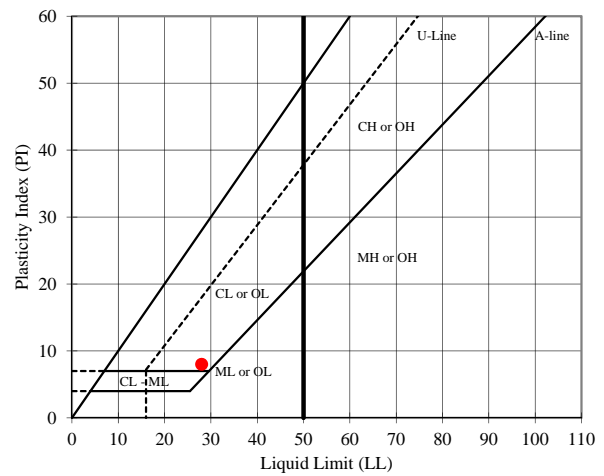
PROJECT NAME: **FMI/TyroneCCP Support 2018-2019**

SAMPLE ID: **TY18-02**

DEPTH (ft): --

TYPE: **Pail**


Sieve Analysis (Initial Separation on No. 4 Sieve)	Sieve	Particle Size (mm)	% Passing	Description	Percentage
	3-inch	75.0	100.0	Coarse Gravel	21.87
	1.5-inch	37.5	99.3		
	1-inch	25.0	91.1		
	3/4-inch	19.0	78.1	Fine Gravel	67.35
	3/8-inch	9.5	42.1		
	#4	4.75	10.8	Coarse Sand	3.39
	#10	2.0	7.4		
	#20	0.850	5.1	Medium Sand	3.63
	#40	0.425	3.8		
	#60	0.250	3.0	Fine Sand	1.82
	#100	0.150	2.5		
	#200	0.075	1.9	Silt or Clay Fines	1.93



USCS Description (ASTM D 2487):

Poorly graded gravel, dark yellowish brown, moist

As-Received Moisture Content (%)

--

USCS Group Symbol

GP

Notes: 0 g of particles up to 75.0 mm maximum size were removed from particle size analysis sample prior to testing

Particle size analysis sample was not mechanically dispersed; hydrometer test was not performed

Sample prepared for Atterberg Limits testing by the dry method

Material retained on No. 40 sieve removed from Atterberg Limits sample by sieving

Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using mechanical device

TECH **EH**

DATE **7-Jan-2019**

REVIEW **MB**

Boring or Test Pit: Comp
Sample: TY18-01/02
Depth: 0-8 ft
Point No.: 1

Boring or Test Pit: Comp
Sample: TY18-01/02
Depth: 0-8 ft
Point No.: 2

Boring or Test Pit: Comp
Sample: TY18-01/02
Depth: 0-8 ft
Point No.: 3

Initial	Initial	Initial
Bottom Thickness = 3.00 in	Bottom Thickness = 2.00 in	Bottom Thickness = 2.00 in
Top Thickness = 3.00 in	Top Thickness = 3.00 in	Top Thickness = 2.00 in
Bottom Length = 12.00 in	Bottom Length = 9.00 in	Bottom Length = 6.00 in
Top Length = 12.00 in	Top Length = 9.00 in	Top Length = 6.00 in
Width = 12.00 in	Width = 9.00 in	Width = 6.00 in
Wet Mass = 65.21 lb	Wet Mass = 30.72 lb	Wet Mass = 10.89 lb
Volume = 864 in ³	Volume = 405 in ³	Volume = 144 in ³
Specific Gravity = 2.70 (Assumed)	Specific Gravity = 2.70 (Assumed)	Specific Gravity = 2.70 (Assumed)
Dry Mass of Solids = 62.47 lb	Dry Mass of Solids = 29.31 lb	Dry Mass of Solids = 10.40 lb
Moisture Content = 4.4%	Moisture Content = 4.8%	Moisture Content = 4.7%
Wet Unit Weight = 130.4 pcf	Wet Unit Weight = 131.1 pcf	Wet Unit Weight = 130.7 pcf
Dry Unit Weight = 124.9 pcf	Dry Unit Weight = 125.1 pcf	Dry Unit Weight = 124.8 pcf
Void Ratio = 0.35	Void Ratio = 0.35	Void Ratio = 0.35
Percent Saturation = 34%	Percent Saturation = 38%	Percent Saturation = 36%

Pre-Shear	Pre-Shear	Pre-Shear
Top Thickness = 2.96 in	Top Thickness = 2.96 in	Top Thickness = 1.88 in
Volume = 858 in ³	Volume = 402 in ³	Volume = 140 in ³
Moisture Content = 9.3%	Moisture Content = 8.5%	Moisture Content = 7.8%
Wet Unit Weight = 137.5 pcf	Wet Unit Weight = 136.7 pcf	Wet Unit Weight = 138.9 pcf
Dry Unit Weight = 125.8 pcf	Dry Unit Weight = 126.0 pcf	Dry Unit Weight = 128.8 pcf
Void Ratio = 0.34	Void Ratio = 0.34	Void Ratio = 0.31
Percent Saturation = 75%	Percent Saturation = 68%	Percent Saturation = 69%

Shear Rate = 0.0195 in/min
Normal Stress = 5,760 psf

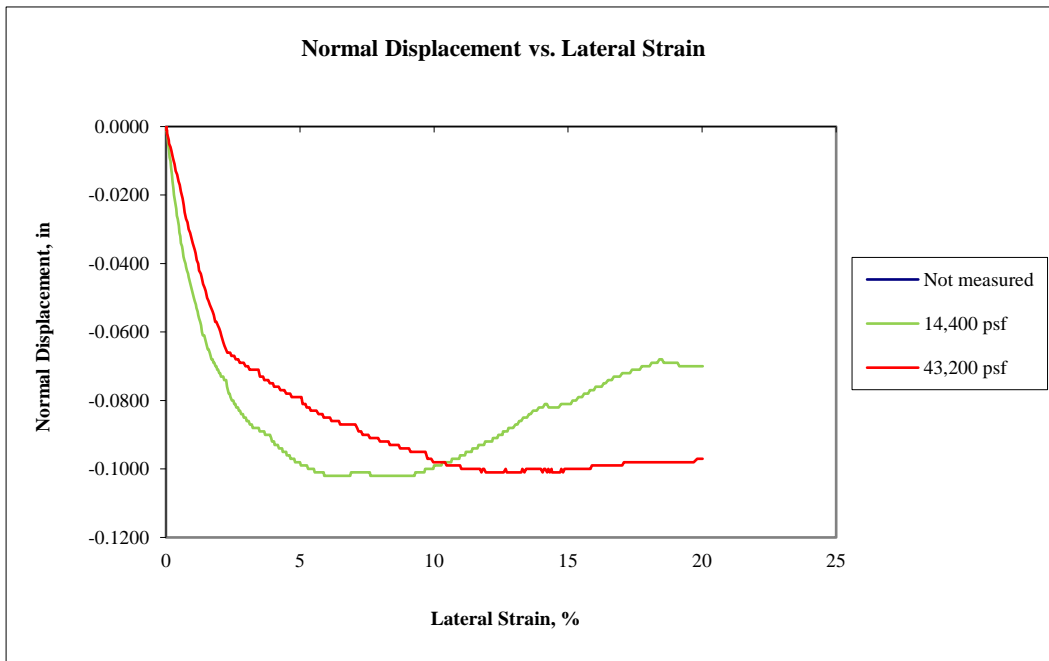
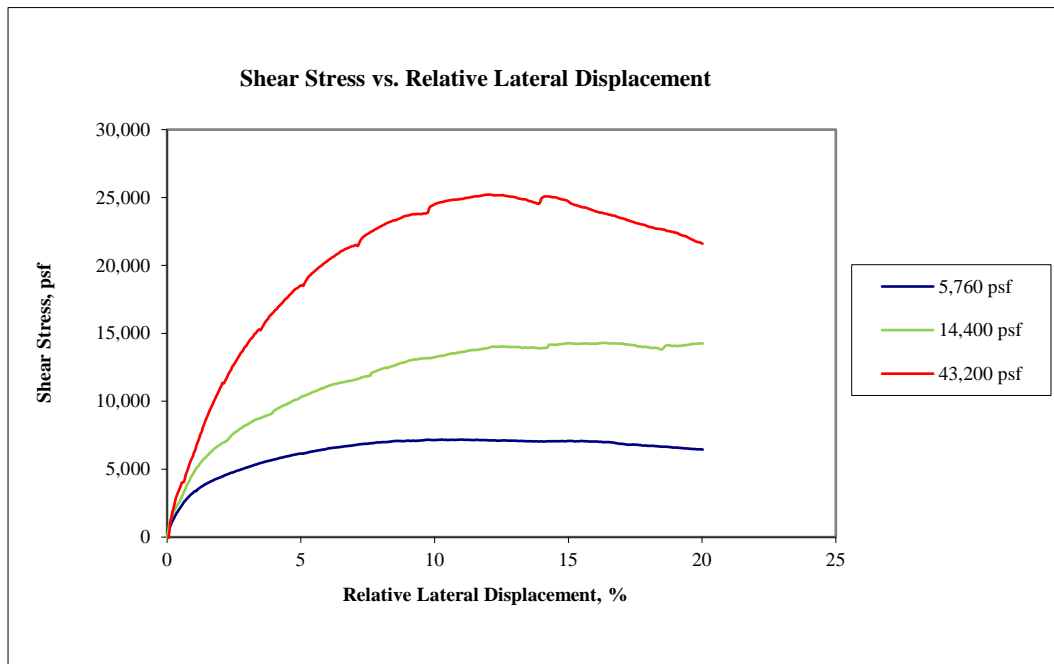
Shear Rate = 0.0192 in/min
Normal Stress = 14,400 psf

Shear Rate = 0.0192 in/min
Normal Stress = 43,200 psf

Notes:

Visual description: Sandy gravel, brown, moist
Atterberg limits: LL = -- PL = -- PI = -- (-- indicates test was not performed)
Percent finer: 3/4 in. = -- No. 4 = -- No. 200 = -- (-- indicates test was not performed)
Specimen type: ☐ Intact ☒ Reconstituted Remold targets: 125.0 pcf (dry) at Delivered moisture
Inundation: Specimens were inundated after normal load was applied
Apparatus: DGS large scale shear device
In accordance with ASTM D3080, no area correction is made; nominal shear stresses and normal stresses are reported.
Gap between top and bottom shear boxes was approximately 0.25 inches during shear.

Project Name: FMI/Tyrone CCP Support 2018-2019	ASTM D3080 - MODIFIED CONSOLIDATED DRAINED DIRECT SHEAR TEST REPORT SAMPLE AND TEST DATA				
Project Number: 18106417.3B					
Sample ID: Composite TY18-01 / TY18-02	Technician: EH	Checked: PRH	Reviewed: MK	Date: 30-Jan-2018	Figure: 1



Project Name:
FMI/Tyrone CCP Support 2018-2019

Project Number:
18106417.3B

Sample ID:
Composite TY18-01 / TY18-02

ASTM D3080 - MODIFIED
CONSOLIDATED DRAINED DIRECT SHEAR TEST REPORT
SHEAR STRESS AND NORMAL DISPLACEMENT PLOTS

Technician:
EH

Checked:
PRH

Reviewed:
MK

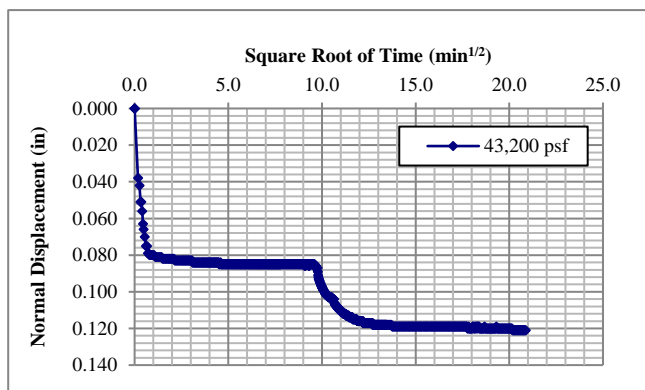
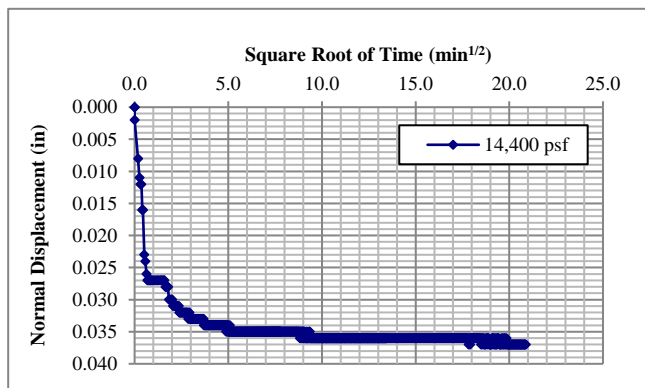
Date:
30-Jan-2018

Figure:
2



The graph shows the relationship between Normal Displacement (in) and the Square Root of Time ($\text{min}^{1/2}$) for a load of 5,760 psf. The displacement starts at 0.000 inches at time 0 and rapidly decreases, reaching a steady state of approximately 0.060 inches after about 2.5 minutes. The data points are plotted as blue diamonds connected by a line.

Square Root of Time ($\text{min}^{1/2}$)	Normal Displacement (in)
0.0	0.000
0.2	0.028
0.4	0.040
0.6	0.045
0.8	0.048
1.0	0.050
1.2	0.052
1.4	0.054
1.6	0.055
1.8	0.056
2.0	0.057
2.2	0.058
2.4	0.059
2.6	0.060
2.8	0.060
3.0	0.060
3.2	0.060
3.4	0.060
3.6	0.060
3.8	0.060
4.0	0.060
4.2	0.060
4.4	0.060
4.6	0.060
4.8	0.060
5.0	0.060
5.2	0.060
5.4	0.060
5.6	0.060
5.8	0.060
6.0	0.060
6.2	0.060
6.4	0.060
6.6	0.060
6.8	0.060
7.0	0.060
7.2	0.060
7.4	0.060
7.6	0.060
7.8	0.060
8.0	0.060
8.2	0.060
8.4	0.060
8.6	0.060
8.8	0.060
9.0	0.060
9.2	0.060
9.4	0.060
9.6	0.060
9.8	0.060
10.0	0.060
10.2	0.060
10.4	0.060
10.6	0.060
10.8	0.060
11.0	0.060
11.2	0.060
11.4	0.060
11.6	0.060
11.8	0.060
12.0	0.060
12.2	0.060
12.4	0.060
12.6	0.060
12.8	0.060
13.0	0.060
13.2	0.060
13.4	0.060
13.6	0.060
13.8	0.060
14.0	0.060
14.2	0.060
14.4	0.060
14.6	0.060
14.8	0.060
15.0	0.060
15.2	0.060
15.4	0.060
15.6	0.060
15.8	0.060
16.0	0.060
16.2	0.060
16.4	0.060
16.6	0.060
16.8	0.060
17.0	0.060
17.2	0.060
17.4	0.060
17.6	0.060
17.8	0.060
18.0	0.060
18.2	0.060
18.4	0.060
18.6	0.060
18.8	0.060
19.0	0.060
19.2	0.060
19.4	0.060
19.6	0.060
19.8	0.060
20.0	0.060

[illegible]

Technician:	Checked:	Reviewed:	Date:	Figure:
EH	PRH	MK	30-Jan-2018	3



GOLDER

Point No.: 1
 Normal Stress = 5,760 psf
 Shear Rate = 0.0195 in/min

Shear Stress psf	Relative Displacement	
	Lateral %	Normal in
2,155	0.5	-
3,293	1.0	-
3,946	1.5	-
4,411	2.0	-
4,793	2.5	-
5,134	3.0	-
5,447	3.5	-
5,705	4.0	-
5,945	4.5	-
6,146	5.0	-
6,323	5.5	-
6,510	6.0	-
6,632	6.5	-
6,764	7.0	-
6,881	7.5	-
6,983	8.0	-
7,091	9.0	-
7,142	10.0	-
7,175	11.0	-
7,134	11.9	-
7,099	13.0	-
7,052	14.0	-
7,070	15.0	-
7,043	16.0	-
6,863	17.0	-
6,729	18.0	-
6,587	19.0	-
6,458	20.0	-

Point No.: 2
 Normal Stress = 14,400 psf
 Shear Rate = 0.0192 in/min

Shear Stress psf	Relative Displacement	
	Lateral %	Normal in
2,780	0.5	-0.031
4,688	1.0	-0.048
5,973	1.5	-0.063
6,821	2.0	-0.072
7,619	2.5	-0.080
8,277	3.0	-0.085
8,766	3.5	-0.089
9,283	4.0	-0.092
9,779	4.5	-0.095
10,293	5.0	-0.098
10,691	5.5	-0.100
11,088	6.0	-0.102
11,367	6.5	-0.102
11,564	7.0	-0.101
11,838	7.5	-0.101
12,357	8.0	-0.102
12,951	9.0	-0.102
13,242	10.0	-0.099
13,626	11.0	-0.096
13,921	12.0	-0.092
13,994	13.0	-0.088
13,921	14.0	-0.082
14,263	14.9	-0.081
14,243	16.0	-0.077
14,232	16.9	-0.073
13,984	18.0	-0.070
14,090	19.0	-0.069
14,268	20.0	-0.070

Point No.: 3
 Normal Stress = 43,200 psf
 Shear Rate = 0.0192 in/min

Shear Stress psf	Relative Displacement	
	Lateral %	Normal in
3,569	0.5	-0.016
6,193	1.0	-0.034
8,793	1.5	-0.048
10,961	2.0	-0.059
12,705	2.5	-0.067
14,109	3.0	-0.070
15,233	3.5	-0.073
16,569	4.0	-0.075
17,613	4.5	-0.078
18,517	5.0	-0.079
19,485	5.5	-0.083
20,341	6.0	-0.085
21,045	6.5	-0.087
21,481	7.0	-0.087
22,317	7.5	-0.090
22,865	8.0	-0.092
23,657	9.0	-0.094
24,465	10.0	-0.098
24,893	11.0	-0.099
25,213	12.0	-0.101
25,033	13.0	-0.101
24,905	14.0	-0.100
24,745	15.0	-0.100
24,001	16.0	-0.099
23,497	17.0	-0.099
22,881	18.0	-0.098
22,409	19.0	-0.098
21,701	19.9	-0.097

Project Name: FMI/Tyrone CCP Support 2018-2019		ASTM D3080 - MODIFIED CONSOLIDATED DRAINED DIRECT SHEAR TEST REPORT SHEAR DATA				
Project Number: 18106417.3B						
Sample ID: Composite TY18-01 / TY18-02		Technician: EH	Checked: PRH	Reviewed: MK	Date: 30-Jan-2018	Figure: 4



Project Name: FMI/Tyrone CCP Support 2018-2019	ASTM D3080 - MODIFIED CONSOLIDATED DRAINED DIRECT SHEAR TEST REPORT SPECIMEN PHOTOGRAPH - 5,760 psf				
Project Number: 18106417.3B					
Sample ID: Composite TY18-01 / TY18-02	Technician: EH	Checked: PRH	Reviewed: MK	Date: 30-Jan-2018	Figure: 5



Project Name: FMI/Tyrone CCP Support 2018-2019	ASTM D3080 - MODIFIED CONSOLIDATED DRAINED DIRECT SHEAR TEST REPORT SPECIMEN PHOTOGRAPH - 14,400 psf				
Project Number: 18106417.3B					
Sample ID: Composite TY18-01 / TY18-02	Technician: EH	Checked: PRH	Reviewed: MK	Date: 30-Jan-2018	Figure: 6



Project Name: FMI/Tyrone CCP Support 2018-2019	ASTM D3080 - MODIFIED CONSOLIDATED DRAINED DIRECT SHEAR TEST REPORT SPECIMEN PHOTOGRAPH - 43,200 psf				
Project Number: 18106417.3B					
Sample ID: Composite TY18-01 / TY18-02	Technician: EH	Checked: PRH	Reviewed: MK	Date: 30-Jan-2018	Figure: 7

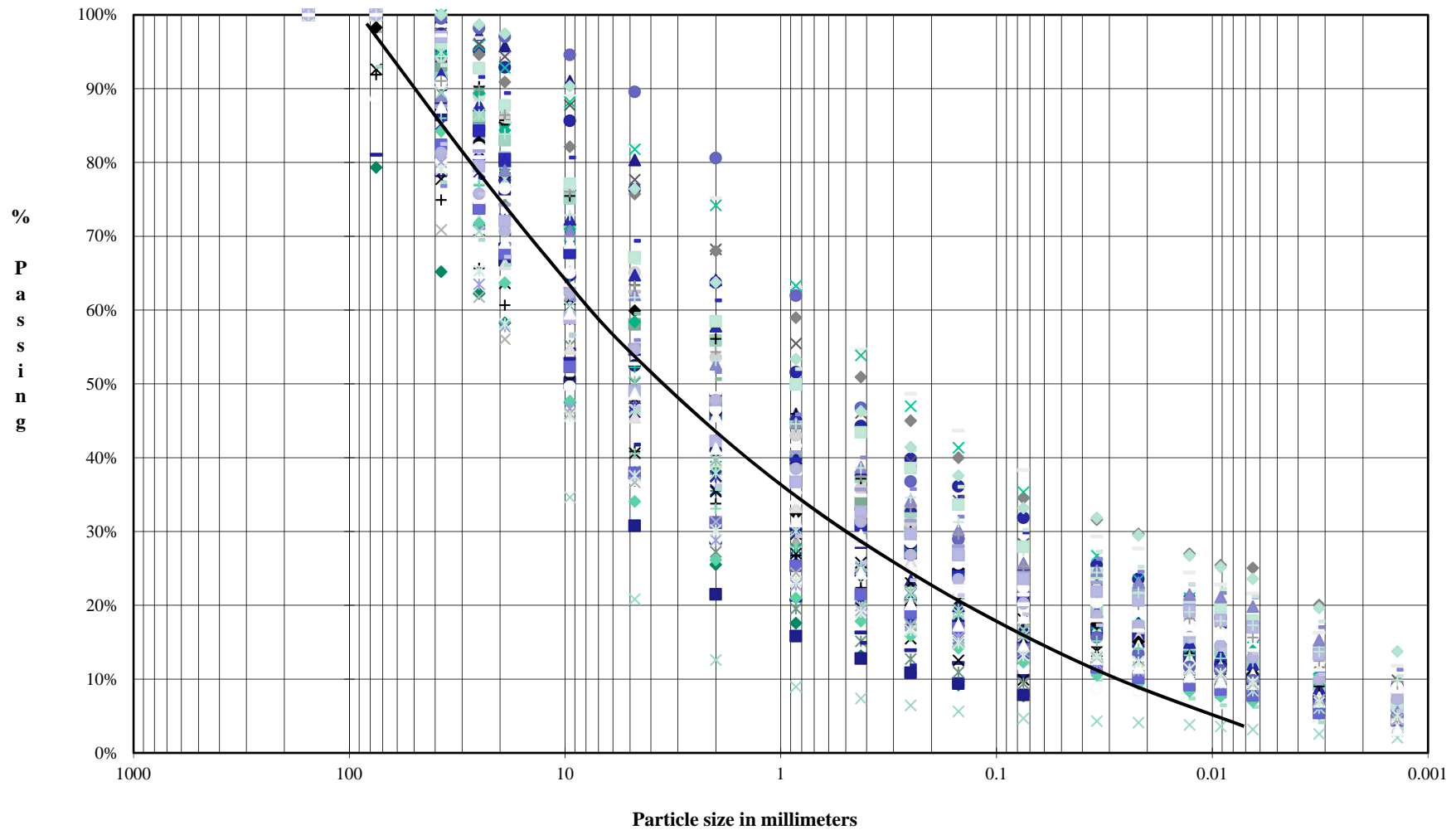
APPENDIX IV

Laboratory Testing Summary

TABLE IV-1
Laboratory Testing Summary

									Coarse Gravel	Coarse Gravel	Fine Gravel	Coarse S	Medium Sand	Fine Sand		Fines																Triaxial	Direct Shear			
			Mine Coords.					6.000"	3.000"	1.500"	1.000"	0.750"	0.375"	#4	#10	#20	#40	#60	#100	#200												φ'	c'	φ'	c'	
Date Sampled	Number	Depth	Easting	Northing	Stockpile	Year Placed	%>3 inch	154.2	75	37.5	25	19	9.5	4.75	2	0.85	0.425	0.25	0.15	0.075	0.034	0.022	0.013	0.009	0.006	0.003	0.001	Activity	LL	PL	PI	USCS	(°)	psi	(°)	psi
1/22/2000	GTP-01/01	10	21349	11668	No. 1		15	100.0%	100.0%	89.2%	85.3%	81.2%	66.4%	48.6%	38.6%	30.3%	25.0%	21.9%	19.4%	16.5%								47	22	25	GC					
1/22/2000	GTP-03/02	10	15937	14890	1D		20	100.0%	100.0%	94.6%	89.1%	84.5%	73.0%	60.0%	48.4%	34.5%	27.1%	23.2%	20.3%	17.4%								33	20	13	SC	34.6	5.8			
1/22/2000	GTP-06/03	10	4038	11230	2, 2A North		20	100.0%	81.1%	78.3%	73.9%	68.9%	53.4%	37.3%	28.5%	20.8%	16.3%	13.9%	12.2%	10.4%								42	20	22	GP-GC	32.8	8.3			
1/22/2000	GTP-09/05	10	6075	6910	2, 2A South		45	100.0%	79.3%	65.2%	62.3%	58.3%	47.5%	36.9%	25.5%	17.6%	13.2%	10.9%	9.2%	7.7%								39	20	19	GP-GC					
1/22/2000	GTP-10/04	10	11416	3546	S. Rim		25	100.0%	100.0%	83.5%	76.0%	70.3%	54.0%	38.2%	30.2%	23.3%	18.9%	16.4%	14.4%	12.1%								47	20	26	GC					
1/22/2000	GTP-13/06	10	18450	6622	1 C	2000	20	100.0%	100.0%	87.0%	81.4%	74.4%	60.8%	48.6%	36.5%	26.0%	19.6%	16.0%	13.6%	11.2%								38	19	19	GP-GC					
1/22/2000	GTP-14/07	10	10230	19063	3A		35	100.0%	100.0%	86.4%	73.8%	66.8%	50.1%	30.8%	21.5%	15.8%	12.8%	10.9%	9.4%	7.9%								28	15	13	GP-GC					
10/19/2001	TYTP1-1	100	4757	10882	2, 2A South	1975	40	100.0%	98.4%	91.3%	83.2%	77.4%	62.1%	48.1%	38.5%	28.6%	21.9%	18.1%	15.1%	12.5%								38	17	21	GC					
10/19/2001	TYTP1-2	400	6335	8367	Copper Mtn.	1978	30	100.0%	100.0%	91.8%	84.2%	77.9%	61.5%	48.0%	37.5%	27.0%	19.7%	15.5%	12.5%	9.9%								30	14	16	GW-GC	36.2	0.6			
10/19/2001	TYTP1-3	250	6643	8275	Copper Mtn	1982	50	100.0%	100.0%	93.0%	83.1%	74.9%	55.2%	38.1%	27.2%	19.5%	15.1%	12.7%	10.9%	9.4%								36	18	18	GP-GC					
10/19/2001	TYTP1-4	100	5945	9155	Copper Mtn	1986	15	100.0%	100.0%	88.1%	80.5%	75.9%	64.7%	52.5%	41.0%	30.4%	24.2%	20.8%	17.8%	14.8%								40	17	23	GC	35.5	0.4			
10/19/2001	TYTP1-5	100	5766	6482	4C North	1975	10	100.0%	100.0%	97.0%	87.3%	79.0%	64.5%	49.1%	35.5%	25.8%	21.0%	18.4%	16.5%	14.5%								39	16	23	GC					
10/19/2001	TYTP1-6	300	13643	9578	Savanna	1989	50	100.0%	100.0%	90.0%	77.9%	70.3%	54.7%	41.8%	30.6%	20.6%	14.9%	11.9%	9.8%	7.9%								29	16	13	GW-GC					
10/19/2001	TYTP1-7	300	13282	9913	Savanna	1989	40	100.0%	100.0%	92.7%	85.4%	81.3%	71.4%	62.5%	46.9%	33.0%	24.8%	20.5%	17.4%	14.7%								28	15	13	SC	36.9	0.6			
10/19/2001	TYTP1-8	250	13523	9429	Savanna	1990	30	100.0%	98.3%	94.2%	89.2%	85.4%	76.2%	59.9%	47.4%	32.4%	24.0%	20.0%	17.3%	15.0%								24	16	8	SC					
10/19/2001	TYTP1-9	40	17378	8051	Gettysberg	1967	30	100.0%	100.0%	93.1%	85.2%	80.1%	67.7%	58.1%	47.8%	39.6%	33.9%	30.0%	26.7%	23.6%								30	18	12	GC	34.1	2.2			
10/19/2001	TYTP1-10	50	14429	12245	Savanna	1967	5	100.0%	100.0%	99.7%	97.3%	95.8%	91.0%	80.3%	64.1%	40.0%	28.5%	23.4%	19.9%	17.0%								30	18	12	SC					
10/19/2001	TYTP1-11	100	4473	10612	2, 2A	1973	10	100.0%	100.0%	94.6%	89.7%	84.5%	68.7%	50.1%	38.8%	27.7%	21.1%	17.4%	14.7%	12.5%								37	17	20	GC					
10/19/2001	TYTP1-12	100	10770	15320	No 3 Leach	1975	40	100.0%	100.0%	94.3%	83.6%	76.9%	60.5%	46.2%	35.6%	26.4%	20.9%	17.7%	15.5%	13.5%								30	16	14	GC					
9/8/2004	GA04-TY-1	50	17363.7	6537.2302	1C Waste	1998	10	100.0%	100.0%	84.7%	75.0%	70.2%	56.0%	40.7%	36.3%	31.5%	27.1%	23.9%	21.0%	18.1%	16.0%	14.5%	13.0%	11.9%	10.4%	7.8%	5.4%	2.1	34	20	14	GC			31.0	10.4
9/8/2004	GA04-TY-2	50	18049.785	7148.82	1C Waste	1998	10	100.0%	100.0%	93.0%	85.0%	78.8%	65.9%	51.6%	54.8%	38.5%	32.8%	28.9%	25.4%	21.9%	18.2%	16.9%	15.8%	14.5%	12.9%	9.7%	6.2%	1.8	35	21	14	GC				
9/8/2004	GA04-TY-3	100	17366.983	6367.1624	1C Waste	1982	50	100.0%	100.0%	86.6%	76.8%	69.0%	52.9%	39.7%	35.7%	30.5%	25.9%	22.6%	17.7%	16.8%	14.5%	13.2%	12.6%	11.3%	10.4%	7.6%	5.2%	3.0	40	21	19	GC				
9/8/2004	GA04-TY-4	100	18112.156	6939.5058	1C Waste	1982	40	100.0%	100.0%	85.9%	61.7%	46.2%	24.9%	14.9%	12.8%	10.7%	9.2%	8.2%	7.3%	6.1%	5.3%	4.8%	4.5%	4.2%	3.9%	3.2%	2.6%	50	17	33	GP-GC					
9/8/2004	GA04-TY-5	250	18417.447	6252.6937	1C Waste	1982	15	100.0%	97.0%	78.1%	66.3%	59.1%	44.9%	33.1%	29.9%	25.0%	21.2%	18.5%	16.3%	13.9%	12.3%	11.0%	10.3%	9.1%	8.2%	5.8%	4.1%	2.8	33	19	14	GC			32.0	11.8
9/8/2004	GA04-TY-6	100	18220.485	7116.1147	1C Waste	1982	20	100.0%	97.6%	83.8%	73.9%	65.8%	50.4%	42.2%	39.2%	33.6%	28.8%	25.8%	22.9%	19.9%	17.2%	16.1%	14.6%	13.1%	11.8%	9.0%	6.6%	2.0	35	19	16	GC				
9/8/2004	GA04-TY-7	150	18158.114	6690.9453	1C Waste	1982	20	100.0%	100.0%	86.2%	73.3%	63.8%	45.4%	35.9%	30.0%	25.0%	21.3%	18.8%	16.7%	14.3%	12.5%	11.7%	10.9%	10.4%	9.2%	7.2%	4.5%	2.9	36	19	17	GC				
9/8/2004	GA04-TY-8	150	18404.316	7044.1629	1C Waste	1982	30	100.0%	100.0%	85.5%	75.0%	68.3%	53.7%	42.2%	37.4%	31.0%	26.2%	22.9%	20.2%	17.3%	14.8%	13.4%	12.7%	11.6%	10.2%	8.4%	6.1%	1.5	30	19	11	GC			29.0	8.8
10/13/2004	TSGT-1	19	9537	19405	3A Leach			100.0%	100.0%	94.3%	89.1%	86.1%	76.5%	65.1%	53.6%	43.0%	35.8%	31.1%	26.9%	22.6%	17.2%	16.0%	14.4%	13.5%	11.9%	9.4%	5.0%	1.5	25	14	11	SC				
10/13/2004	TSGT-1	47	9537	19405	3A Leach			100.0%	91.9%	74.9%	65.7%	60.7%	50.2%	40.8%	33.8%	26.8%	22.4%	19.6%	17.4%	15.0%	12.1%	11.0%	10.2%	9.6%	8.8%	6.7%	4.7%	2.3	29	16	13	GC				
10/13/2004	TSGT-1	75	9537	19405	3A Leach			100.0%	100.0%	87.6%	83.4%	81.1%	71.5%	59.5%	50.7%	38.1%	29.9%	24.8%	21.0%	17.2%	12.5%	11.0%	10.7%	9.5%	9.2%	6.5%	3.3%	1.4	26	19	7	SC-SM				
10/13/2004	TSGT-1	88	9537	19405	3A Leach			100.0%	100.0%	94.6%	89.4%	84.4%	71.0%	58.4%	46.9%	37.7%	32.3%	28.9%	26.0%	22.7%	18.5%	17.6%	15.9%	14.7%	13.8%	10.7%	7.2%	1.1	28	18	10	GC				
10/13/2004	TSGT-1	102	9537	19405	3A Leach			100.0%	100.0%	91.1%	84.3%	80.4%	67.8%	54.6%	45.8%	36.8%	30.9%	27.1%	24.2%	21.0%	16.7%	15.6%	13.1%	11.7%	10.0%	6.7%	4.4%	2.3	33	20	13	GC				
10/13/2004	TSGT-1	140	9537	19405	3A Leach			100.0%	100.0%	82.8%	72.2%	66.3%	54.8%	45.5%	39.6%	33.3%	28.9%	25.9%	23.5%	20.6%	16.8%	15.6%	14.7%	13.6%	12.4%	10.1%	4.8%	1.5	29	18	11	GC				
10/13/2004	TSGT-1	158.5	9537	19405	3A Leach			100.0%	92.6%	77.7%	70.8%	63.6%	50.3%	40.6%	35.4%	29.6%	25.8%	23.0%	20.7%	18.1%	13.8%	12.5%	11.3%	10.9%	10.7%	7.1%	5.9%	2.3	34	19	15	GC				
10/13/2004	TSGT-1	184	9537	19405	3A Leach			100.0%	100.0%	82.4%	74.7%	71.8%	62.7%	50.3%	39.6%	30.5%	25.3%	21.9%	19.5%	16.7%	12.9%	11.6%	10.2%	8.9%	7.9%	5.0%	3.0%	2.5	30	20	10	GC				
10/13/2004	TSGT-1	211	9537	19405	3A Leach			100.0%	100.0%	97.5%	95.1%	92.9%	85.6%	76.4%	63.7%	51.6%	44.3%	39.8%	36.1%	31.9%	25.5%	23.6%	20.9%	18.2%	17.5%	13.7%	9.1%	1.0	27	16	11	SC				
10/13/2004	TSGT-1	228	9537	19405	3A Leach			100.0%	100.0%	84.2%	76.9%	71.6%	62.1%	52.3%	45.5%	37.5%	31.7%	27.7%	24.3%	20.5%	16.9%	15.1%	13.8%	11.9%	10.9%	7.8%	5.7%	1.2	23	15	8	GC				
10/13/2004	TSGT-1	250.5	9537	19405	3A Leach			100.0%	100.0%	97.3%	91.6%	89.4%	80.7%	69.4%	61.3%	50.6%	43.6%	38.9%	34.7%	29.9%	24.8%	22.3%	19.2%	17.1%	15.4%	11.2%	7.3%	1.2	27	16	11	SC				
10/13/2004	TSGT-1	298	9537	19405	3A Leach			100.0%	100.0%	100.0%	96.9%	94.6%	89.0%	82.3%	75.2%	63.1%	54.7%	48.7%	43.7%	38.3%	29.3%	27.7%	24.4%	22.8%	21.6%	16.3%	11.8%	1.0	28	14	14	SC				
10/13/2004	TSGT-1	310	9537	19405	3A Leach			100.0%	100.0%	90.1%	82.7%	78.0%	65.1%	54.7%	46.1%	37.6%	31.8%	28.0%	24.9%	21.3%	17.5%	15.1%	13.1%	12.0%	10.7%	7.7%	5.2%	2.0								

Leach



9 Monroe Parkway Suite 270
Portland, Oregon 97035
Ph: +1 (503) 607 1820

TITLE:

Grain Size Distribution Leach Material

CLIENT AND PROJECT:

**FREEPORT MCMORAN
TYRONE INC.**

**TYRONE STOCKPILE
STABILITY
2019 CCP UPDATE**

ANALYSIS: Stockpile Stability

PROJECT NO.: 18106417

PHASE:

REV:

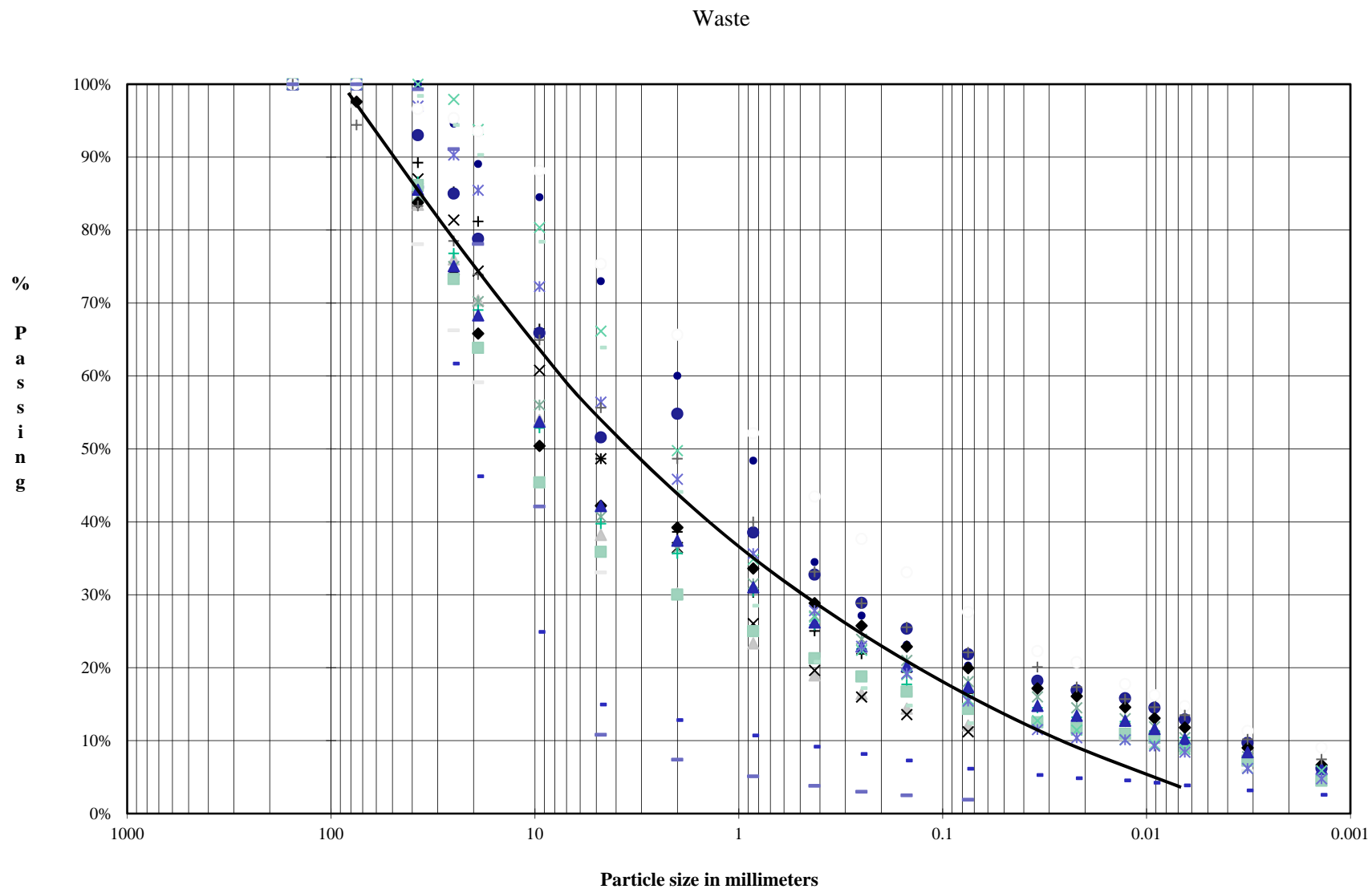
PREPARED BY: TJW

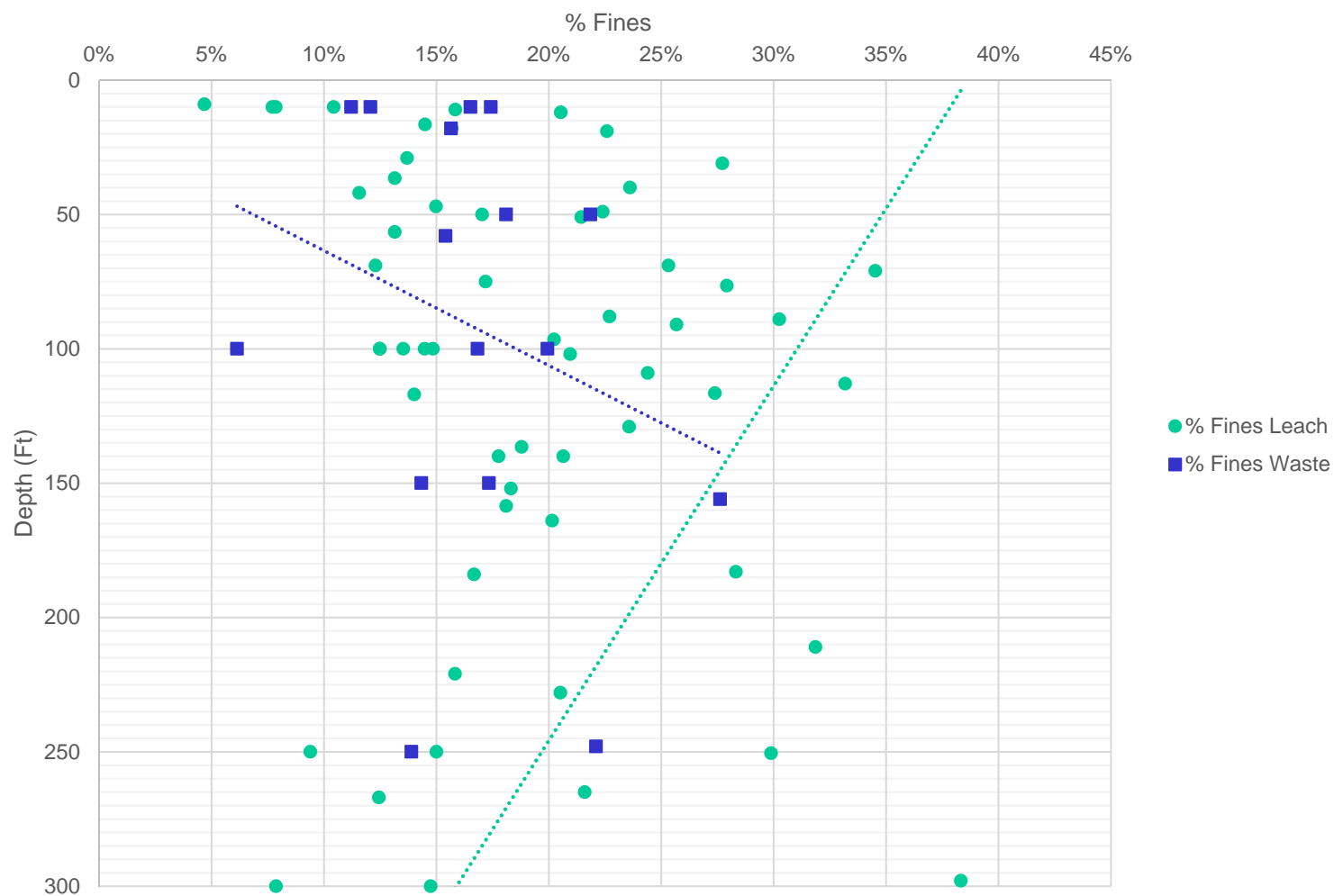
REVIEWED BY: TJW

APPROVED BY: DAK

DATE: 04/05/2019

FIGURE- IV-1





9 Monroe Parkway Suite 270
Portland, Oregon 97035
Ph: +1 (503) 607 1820

TITLE:

Percent Fines versus Depth

CLIENT AND PROJECT:

**FREEPORT MCMORAN
TYRONE INC.**

**TYRONE STOCKPILE
STABILITY
2019 CCP UPDATE**

ANALYSIS: Stockpile Stability

PROJECT NO.: 18106417

PHASE:

REV:

PREPARED BY: TJW

REVIEWED BY: TJW

APPROVED BY: DAK

DATE: 04/05/2019

FIGURE- IV-3



9 Monroe Parkway Suite 270
Portland, Oregon 97035
Ph: +1 (503) 607 1820

TITLE:

Percent Sand and Fines versus Depth

CLIENT AND PROJECT:

**FREEPORT MCMORAN
TYRONE INC.**

**TYRONE STOCKPILE
STABILITY
2019 CCP UPDATE**

ANALYSIS: Stockpile Stability

PROJECT NO.: 18106417

PHASE:

REV:

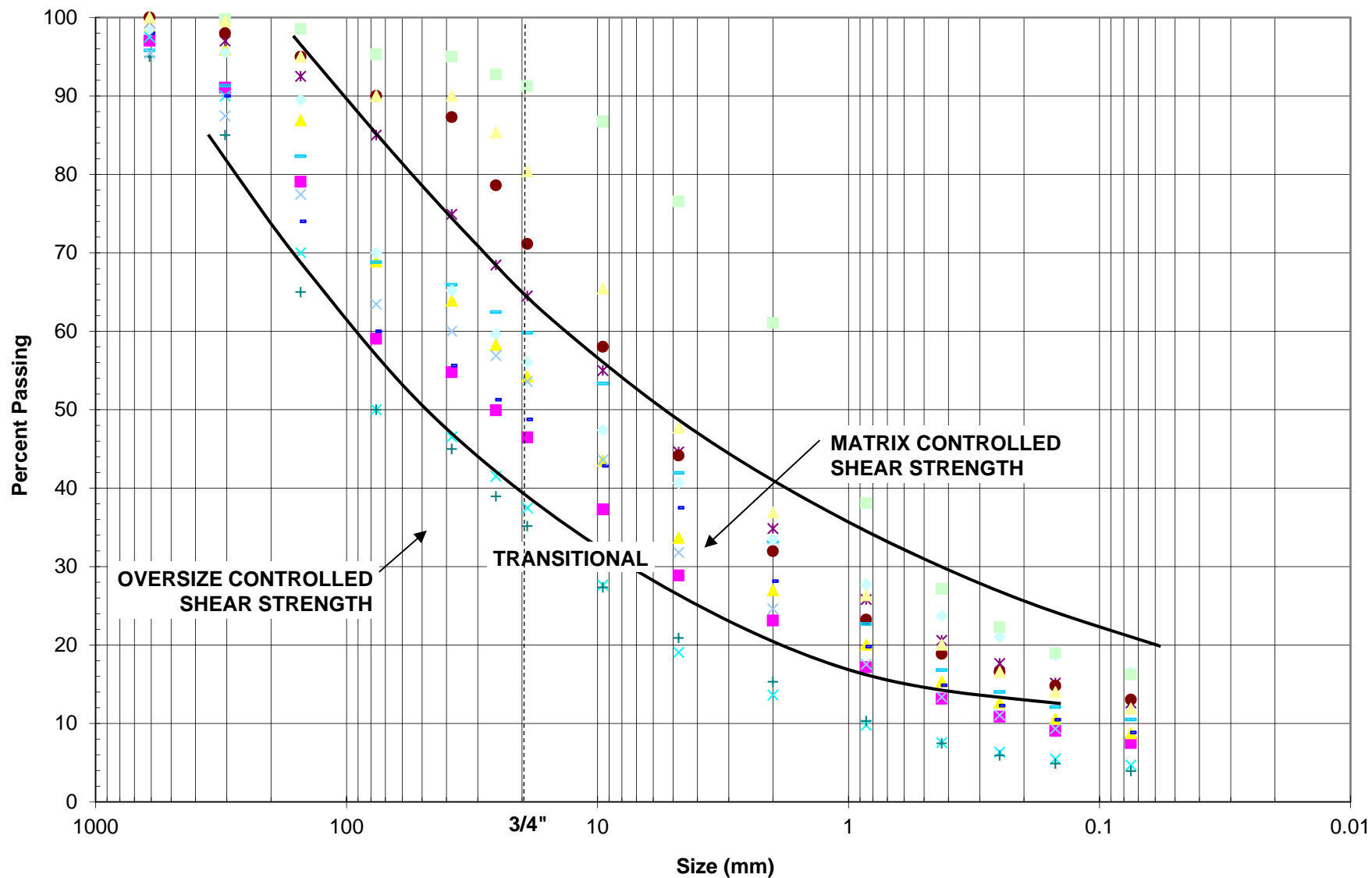
PREPARED BY: TJW

REVIEWED BY: TJW

APPROVED BY: DAK

DATE: 04/05/2019

FIGURE- IV-4



9 Monroe Parkway Suite 270
Portland, Oregon 97035
Ph: +1 (503) 607 1820

TITLE:

Extended Grain Size Distribution Leach Material

CLIENT AND PROJECT:

**FREEPORT MCMORAN
TYRONE INC.**

**TYRONE STOCKPILE
STABILITY
2019 CCP UPDATE**

ANALYSIS: Stockpile Stability

PROJECT NO.: 18106417

PHASE:

REV:

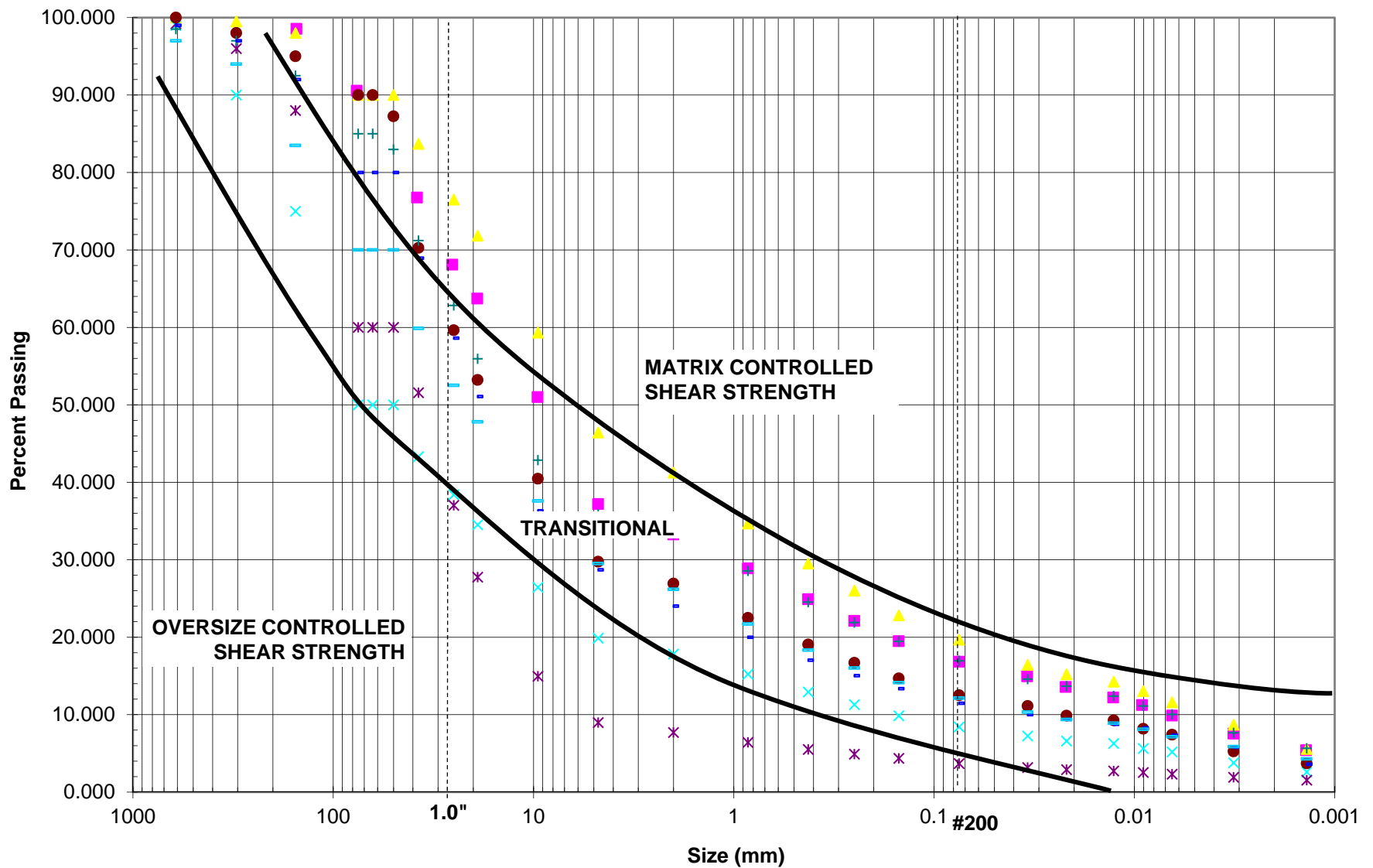
PREPARED BY: TJW

REVIEWED BY: TJW

APPROVED BY: DAK

DATE: 04/05/2019

FIGURE- IV-5



9 Monroe Parkway Suite 270
Portland, Oregon 97035
Ph: +1 (503) 607 1820

TITLE:

Extended Grain Size Distribution Waste Material

CLIENT AND PROJECT:

**FREEPORT MCMORAN
TYRONE INC.**

**TYRONE STOCKPILE
STABILITY
2019 CCP UPDATE**

ANALYSIS: Stockpile Stability

PROJECT NO.: 18106417

PHASE:

REV:

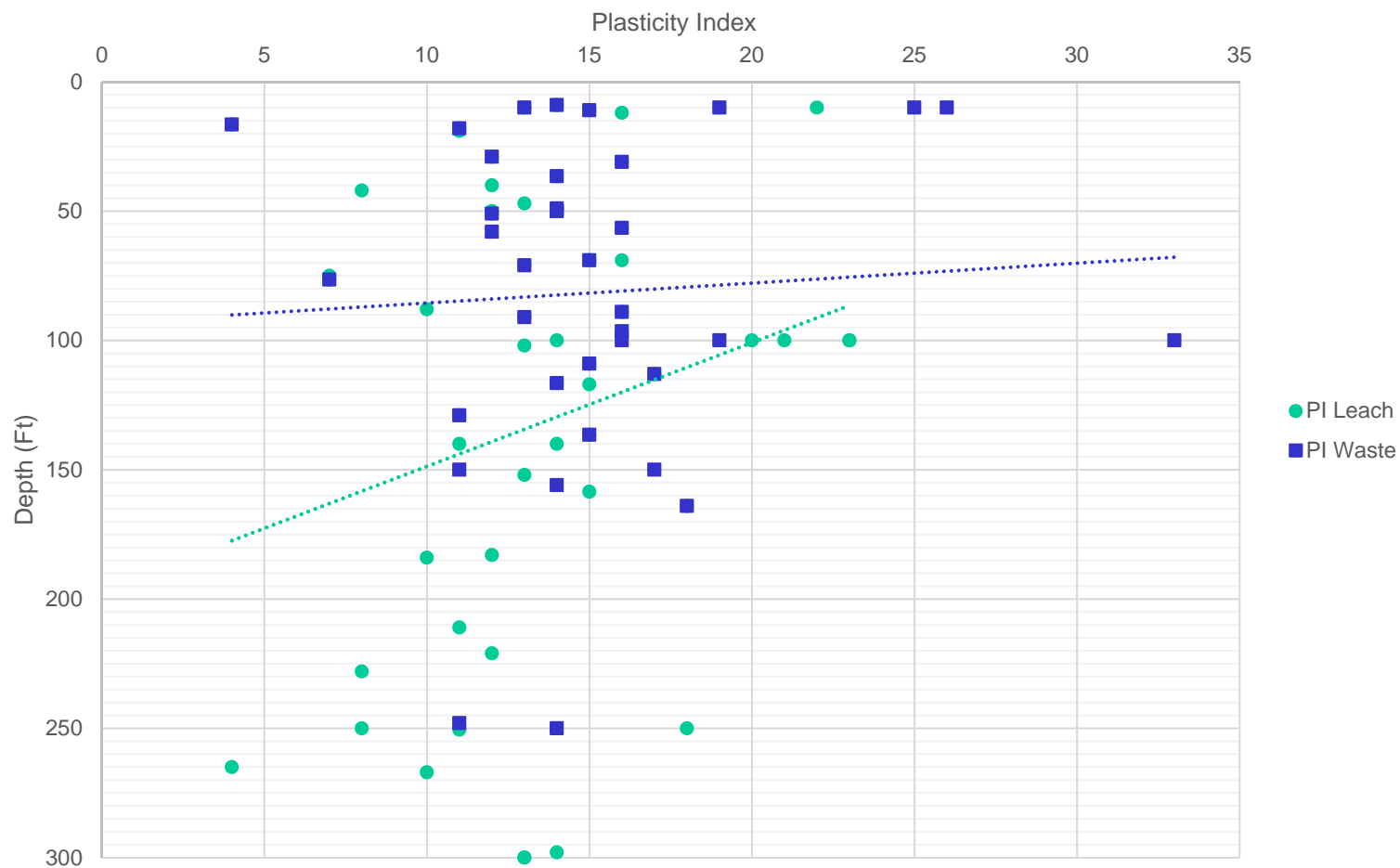
PREPARED BY: TJW

REVIEWED BY: TJW

APPROVED BY: DAK

DATE: 04/05/2019

FIGURE- IV-6



9 Monroe Parkway Suite 270
Portland, Oregon 97035
Ph: +1 (503) 607 1820

TITLE:

Plasticity Index versus Depth

CLIENT AND PROJECT:

**FREEPORT MCMORAN
TYRONE INC.**

**TYRONE STOCKPILE
STABILITY
2019 CCP UPDATE**

ANALYSIS: Stockpile Stability

PROJECT NO.: 18106417

PHASE:

REV:

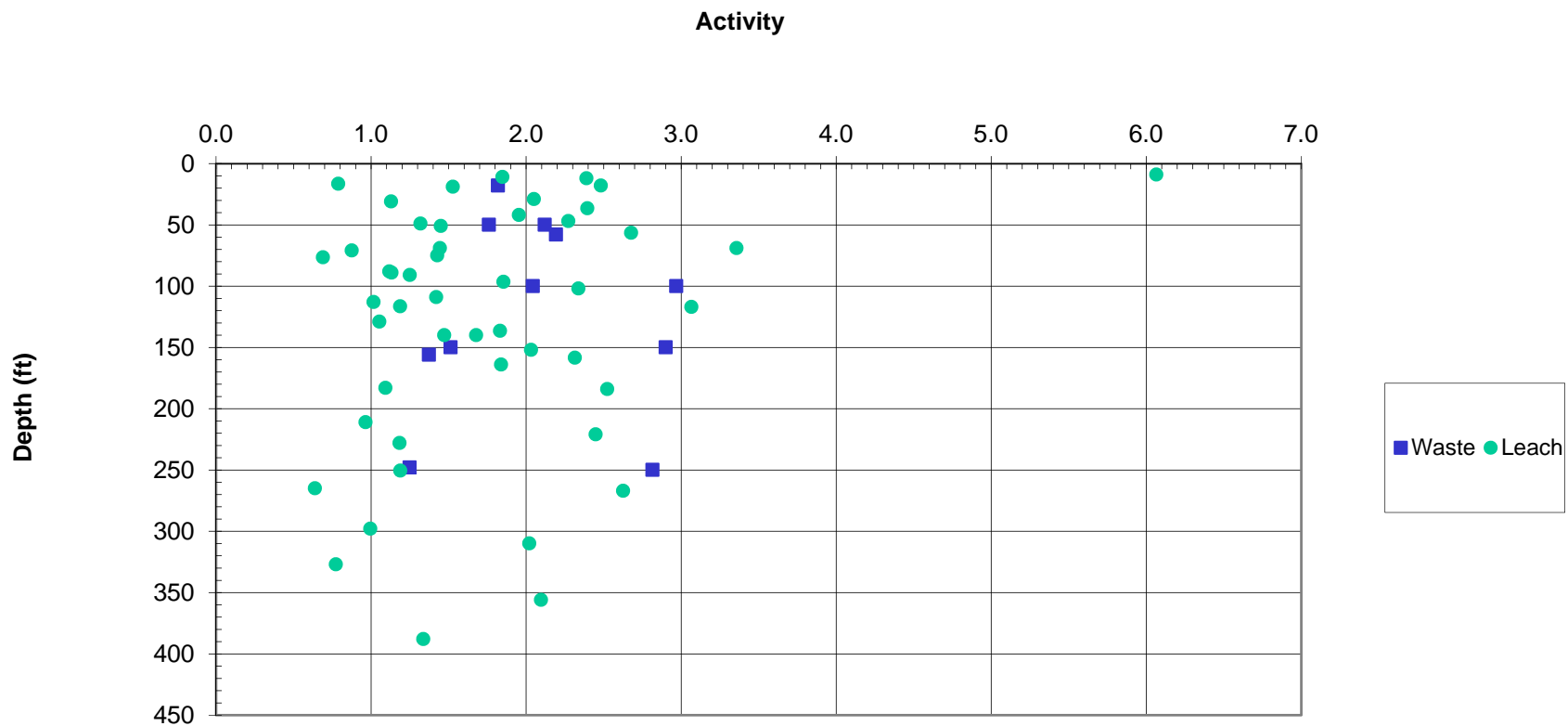
PREPARED BY: TJW

REVIEWED BY: TJW

APPROVED BY: DAK

DATE: 04/05/2019

FIGURE- IV-7



9 Monroe Parkway Suite 270
Portland, Oregon 97035
Ph: +1 (503) 607 1820

TITLE:

Clay Activity versus Depth

CLIENT AND PROJECT:

**FREEPORT MCMORAN
TYRONE INC.**

**TYRONE STOCKPILE
STABILITY
2019 CCP UPDATE**

ANALYSIS: Stockpile Stability

PROJECT NO.: 18106417

PHASE:

REV:

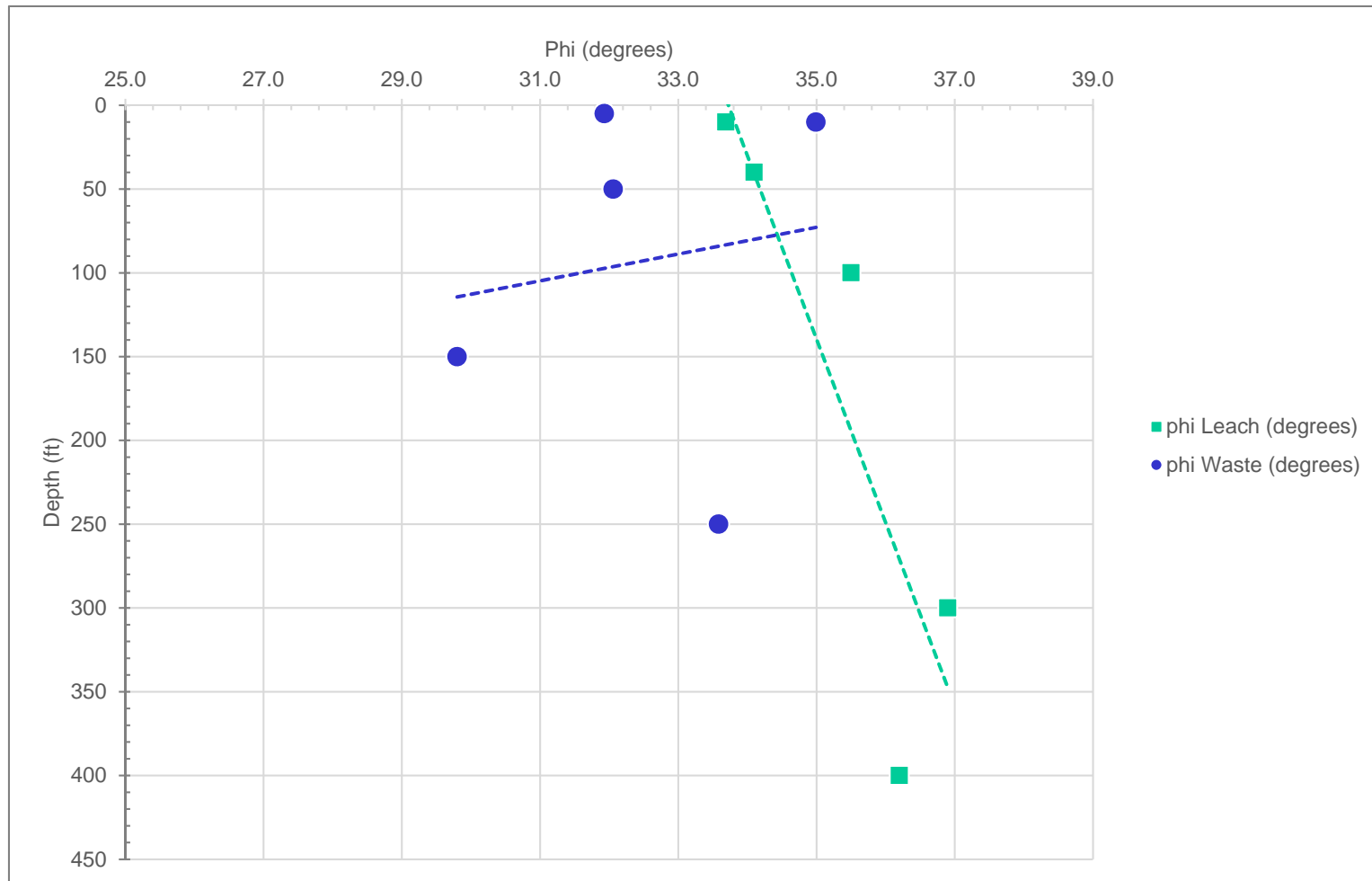
PREPARED BY: TJW

REVIEWED BY: TJW

APPROVED BY: DAK

DATE: 04/05/2019

FIGURE- IV-8



9 Monroe Parkway Suite 270
Portland, Oregon 97035
Ph: +1 (503) 607 1820

TITLE:

Laboratory Measured Friction Angle versus Depth

CLIENT AND PROJECT:

**FREEPORT MCMORAN
TYRONE INC.**

**TYRONE STOCKPILE
STABILITY
2019 CCP UPDATE**

ANALYSIS: Stockpile Stability

PROJECT NO.: 18106417

PHASE:

REV:

PREPARED BY: TJW

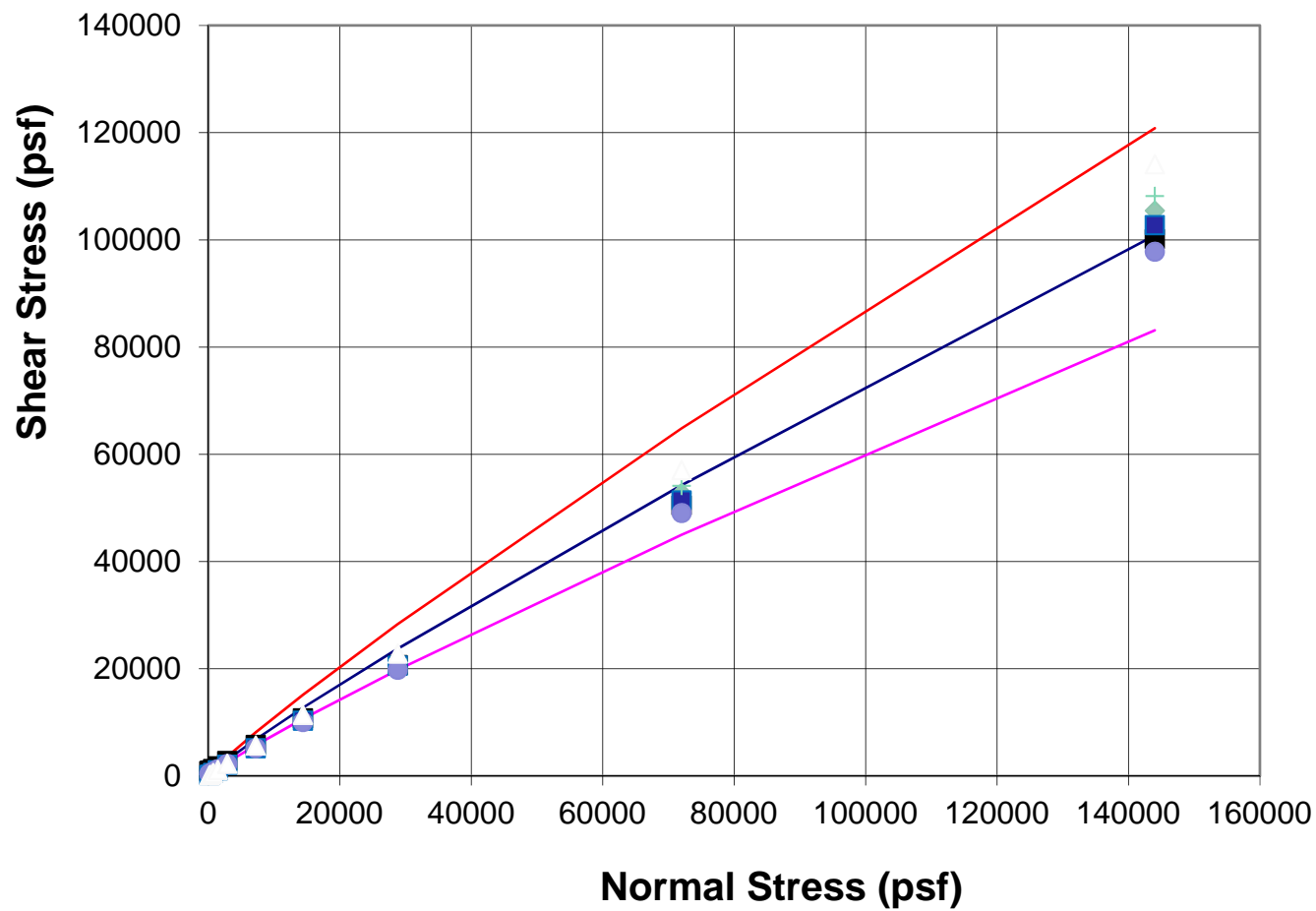
REVIEWED BY: TJW

APPROVED BY: DAK

DATE: 04/05/2019

FIGURE- IV-9

Leach Ore Compared to Leps



GOLDER

9 Monroe Parkway Suite 270
Portland, Oregon 97035
Ph: +1 (503) 607 1820

TITLE:

Leached Ore Compared to Leps (1970)

CLIENT AND PROJECT:

**FREEPORT MCMORAN
TYRONE INC.**

**TYRONE STOCKPILE
STABILITY
2019 CCP UPDATE**

ANALYSIS: Stockpile Stability

PROJECT NO.: 18106417

PHASE:

REV:

PREPARED BY: TJW

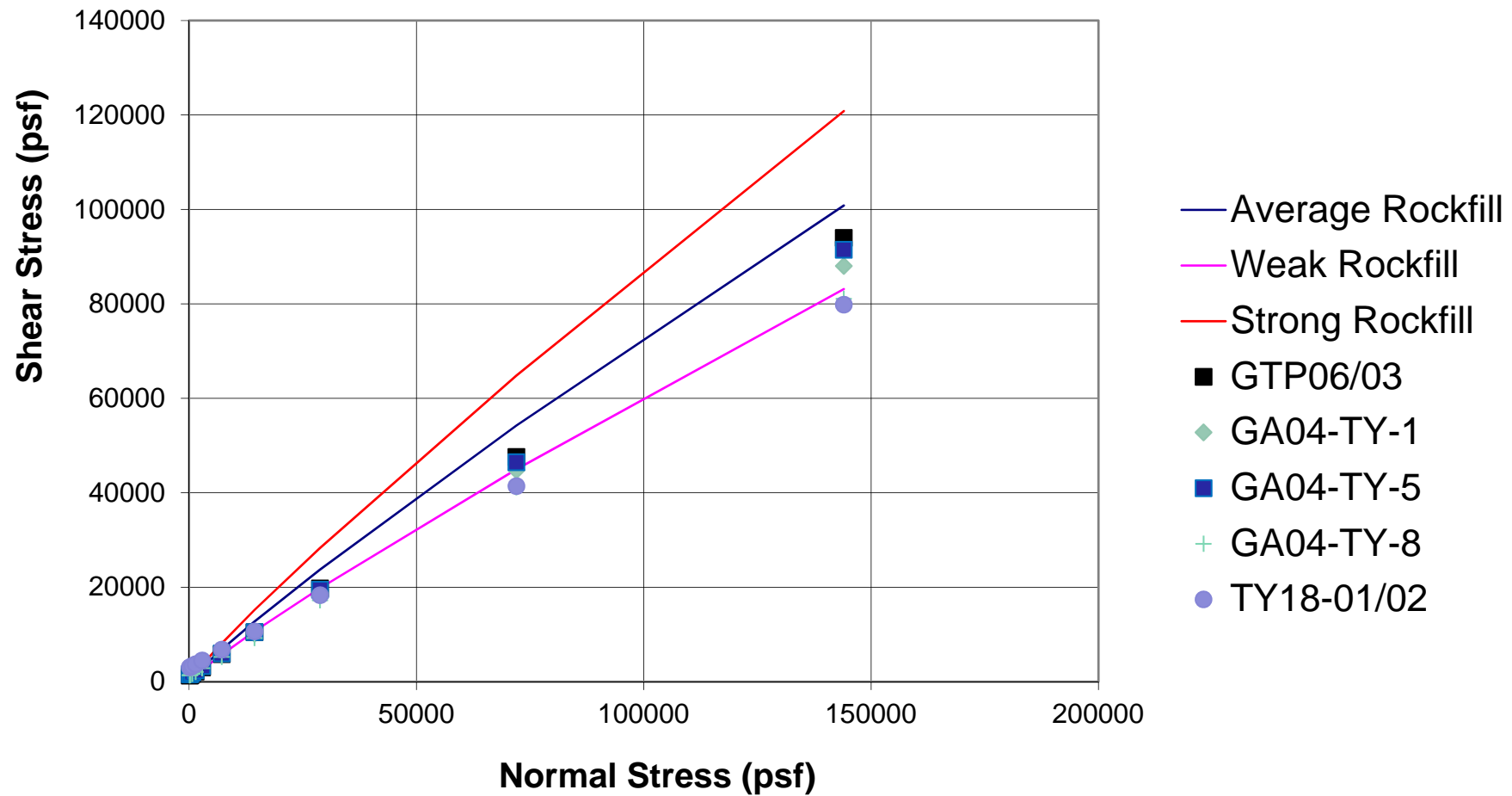
REVIEWED BY: TJW

APPROVED BY: DAK

DATE: 04/05/2019

FIGURE- IV-10

Waste Rock Compared to Leps



GOLDER

9 Monroe Parkway Suite 270
Portland, Oregon 97035
Ph: +1 (503) 607 1820

TITLE:

Waste Rock Compared to Leps (1970)

CLIENT AND PROJECT:

**FREEPORT MCMORAN
TYRONE INC.**

**TYRONE STOCKPILE
STABILITY
2019 CCP UPDATE**

ANALYSIS: Stockpile Stability

PROJECT NO.: 18106417

PHASE:

REV:

PREPARED BY: TJW

REVIEWED BY: TJW

APPROVED BY: DAK

DATE: 04/05/2019

FIGURE- IV-11

APPENDIX V

Liquefaction and Foundation Investigation Borehole Logs

TABLE V-1

All Qal SPT Data, 1A and 1B Leach Stockpile

Corrected N Value (As per Youd et al, 1996 and 1998 NCEER Workshops)

Qal moist unit weight		120 PCF												
Qal sat unit weight		130 PCF												
Borehole dia correction	1.05 (Cb)	Youd et al												
Rod length corr	0.8 (Cr)	Youd et al												
Energy Ratio	1 (Ce)	Youd et al												
Sampling Method Corr	1 (Cs)	Youd et al												
Correction Product	0.84 (Less Ovb corr, Column J)													
Max surface Accel	0.14 g	USGS Unified Hazard Tool		Site Class (760 m/sec)										
	No. 1A-1B	30% Amplification												
Percent Fines	10 %													
Max Quake Magnitude	6.7	URS Seismicity Study (2017)												
Mag Scale Factor	1.234426	Idriss and Boulanger (2008)												
Regrade Burial Depth	0 Feet													
Surcharge Pressure	0 PSF at surface													
				Cn (ovb)	N1(60)									
				Correction	BPF	rd	CSR	CRR	FS					
				(Youd eqn 9)	(Youd eqn 8)	(Youd eqn 2a)	(Youd eqn 1)	(LIQFAC Lookup)	(Youd eqn 23)					
Hole	Depth to Water (bgs)	Sample Depth	Material	Blows/ft (uncorr)	Depth below gws	u (pcf)	Total Stress	Effective Stress						
GA-05-01	11	5	Fill	12	0	0	600	600	1.70	17.14	0.99	0.090	0.2975	NL
GA-05-01	11	10	Fill	26	0	0	1200	1200	1.29	28.20	0.98	0.089	2.4689	NL
GA-05-01	11	15	Fill	6	4	249.6	1840	1590.4	1.12	5.65	0.97	0.102	0.1234	1.5
GA-05-01	11	20	Fill	13	9	561.6	2490	1928.4	1.02	11.12	0.95	0.112	0.2061	2.3
GA-05-01	11	25	Qal	29	14	873.6	3140	2266.4	0.94	22.88	0.94	0.119	0.4209	4.4
GA-05-02	7	5	Fill	20	0	0	600	600	1.70	28.56	0.99	0.090	2.4689	NL
GA-05-02	7	10	Fill	2	3	187.2	1230	1042.8	1.38	2.33	0.98	0.105	0.0864	1.0 Void (PLS Line?)
GA-05-03	0	20	Qal	20	20	1248	2600	1352	1.22	20.43	0.95	0.167	0.3605	2.7
GA-05-03	0	25	Qal	23	25	1560	3250	1690	1.09	21.02	0.94	0.165	0.3901	2.9
GA-05-03	0	23	Qal	87	23	1435.2	2990	1554.8	1.13	82.89	0.95	0.166	2.4689	NL
GA-05-04	16	5	Fill	28	0	0	600	600	1.70	39.98	0.99	0.090	2.4689	NL
GA-05-04	16	10	Fill	14	0	0	1200	1200	1.29	15.18	0.98	0.089	0.2679	NL
GA-05-04	16	15	Fill	9	0	0	1800	1800	1.05	7.97	0.97	0.088	0.1506	NL
GA-05-04	16	20	Fill	18	4	249.6	2440	2190.4	0.96	14.45	0.95	0.097	0.2494	3.2
GA-05-04	16	25	Fill	29	9	561.6	3090	2528.4	0.89	21.67	0.94	0.105	0.3901	4.6
GA-05-04	16	30	Qtg	50	14	873.6	3740	2866.4	0.84	35.08	0.93	0.110	2.4689	NL
GA-05-05	14	17.5	Qal	6	3.5	218.4	2135	1916.6	1.02	5.15	0.96	0.097	0.1234	1.6 PLS Pond Sedime
GA-05-05	14	22.5	Qtg	52	8.5	530.4	2785	2254.6	0.94	41.14	0.95	0.107	2.4689	NL
GA-05-06	20	5	Qal	32	0	0	600	600	1.70	45.70	0.99	0.090	2.4689	NL
GA-05-06	20	10	Qal	43	0	0	1200	1200	1.29	46.63	0.98	0.089	2.4689	NL
GA-05-06	20	15	Qtg	50	0	0	1800	1800	1.05	44.27	0.97	0.088	2.4689	NL
GA-05-07	21	5	Fill	31	0	0	600	600	1.70	44.27	0.99	0.090	2.4689	NL
GA-05-07	21	10	Qal	6	0	0	1200	1200	1.29	6.51	0.98	0.089	0.1383	NL
GA-05-07	21	15	Qal	7	0	0	1800	1800	1.05	6.20	0.97	0.088	0.1383	NL
GA-05-07	21	20	Qtg	50	0	0	2400	2400	0.91	38.34	0.95	0.087	2.4689	NL
GA-05-08	29	5	Fill	16	0	0	600	600	1.70	22.85	0.99	0.090	0.4209	NL
GA-05-08	29	10	Fill	12	0	0	1200	1200	1.29	13.01	0.98	0.089	0.2358	NL
GA-05-08	29	15	Fill	18	0	0	1800	1800	1.05	15.94	0.97	0.088	0.2679	NL
GA-05-08	29	20	Fill	28	0	0	2400	2400	0.91	21.47	0.95	0.087	0.3901	NL
GA-05-08	29	25	Fill	25	0	0	3000	3000	0.82	17.15	0.94	0.086	0.2975	NL
GA-05-08	29	30	Fill	46	1	62.4	3610	3547.6	0.75	29.01	0.93	0.086	2.4689	35.4
GA-05-08	29	35	Qtg	50	6	374.4	4260	3885.6	0.72	30.13	0.92	0.092	2.4689	NL
GA-05-09	31	20	Fill	50	0	0	2400	2400	0.91	38.34	0.95	0.087	2.4689	NL
GA-05-09	31	25	Fill	40	0	0	3000	3000	0.82	27.43	0.94	0.086	1.2344	NL
GA-05-09	31	30	Qtg	50	0	0	3600	3600	0.75	31.30	0.93	0.085	2.4689	NL

TABLE V-2
1C Liquefaction Analysis

All Qal SPT Data, Reclaimed 1C Waste Transects 1-3
Corrected N Value (As per Youd et al, 1996 and 1998 NCEER Workshops)

Qal moist unit weight		120 PCF	
Qal sat unit weight		130 PCF	
Borehole dia correction	1.05 (Cb)	Youd et al	
Rod length corr	0.8 (Cr)	Youd et al	
Energy Ratio	1 (Ce)	Youd et al	
Sampling Method Corr	1 (Cs)	Youd et al	
Correction Product	0.84 (Less Ovb corr, Column J)		
Max surface Accel	0.14 g	USGS Unified Hazard Tool	Site Class (760 m/sec)
No. 1C		30% Amplification	
Percent Fines	10 %		
Max Quake Magnitude	6.7	URS Seismicity Study (2017)	
Mag Scale Factor	1.234426	Idriss and Boulanger (2008)	
Regrade Burial Depth	0 Feet		
Surcharge Pressure	0 PSF at surface		
</			

TABLE V-2
1C Liquefaction Analysis

All Qal SPT Data, Reclaimed 1C Waste Transects 1-3
Corrected N Value (As per Youd et al, 1996 and 1998 NCEER Workshops)

Qal moist unit weight		120 PCF	
Qal sat unit weight		130 PCF	
Borehole dia correction	1.05 (Cb)	Youd et al	
Rod length corr	0.8 (Cr)	Youd et al	
Energy Ratio	1 (Ce)	Youd et al	
Sampling Method Corr	1 (Cs)	Youd et al	
Correction Product	0.84 (Less Ovb corr, Column J)		
Max surface Accel	0.14 g	USGS Unified Hazard Tool	Site Class (760 m/sec)
	No. 1C		30% Amplification
Percent Fines	10 %		
Max Quake Magnitude	6.7	URS Seismicity Study (2017)	
Mag Scale Factor	1.234426	Idriss and Boulanger (2008)	
Regrade Burial Depth	0 Feet		
Surcharge Pressure	0 PSF at surface		

TABLE V-2
1C Liquefaction Analysis

All Qal SPT Data, Reclaimed 1C Waste Transects 1-3
Corrected N Value (As per Youd et al, 1996 and 1998 NCEER Workshops)

[illegible]



BOREHOLE LOG

BIC-1

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine

DRILLING METHOD: Hollow Stem Auger

BORING NO.

Transect 1

SAMPLING METHOD: Split Spoon

SHEET

PAGE OF 1

DRILLING

START FINISH

NORTHING

EASTING:

3613191

0749026

WATER LEVEL

TIME

DATE

CASING DEPTH

SURFACE CONDITIONS:

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR (moist)	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
1'					SL (18%) 2.54 5/3, NE 40% grl. subrounded, (5% of grl. is fine), dry		40	35	25	2.5Y 5/3	soft	SS, SP	
5'	16	100	(S-S.S)		SL (18%), NE, dry, grl are subrounded.	0	40	35	25	2.5Y 5/3	FR	SS, SP	
10'	50	100	(10-10.5)		one 1" grl of granite. upper 3" SL (15% clay) 10YR 5/4, dry, grl subrounded	0	40	35	25	10YR 5/4	FR	SS, SP	
15'					lower 3" CoL S (5% clay) 10YR 4/4 (m), dry, NE grl subrounded.	0	40	50	10	10YR 4/4	H	SO, PO	
20'	15'	50	0	15'	on rock.								
25'	50	100	QTz		most blocks hit rock on top. SL (15%) 10YR 5/4 (m) dry, NE grl are subrounded, 1" broken rock of Qtz diorite.	?	45	33	22	10YR 5/4	FR	SS, SP	
30'	50	100	QTz		SL (18% clay), NE grl subrounded, dry	0	45	33	22	10YR 5/3	FR	SS, SP	
35'					* Photo of Alluvial Gila contact in split spoon. Largest grl in spoon. 1 1/2"								
40'					- driller said harder drilling at 13'								
					photo 38 of sample 10-10.5"								

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

- drill pad cut into original surface about 1' + is constructed of Gila.

no samples

start drilling 2:15

TD 25.5'

photo 39 ? of sample

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

LOGGED BY: Dave Buscher

DATE: 1/25/05

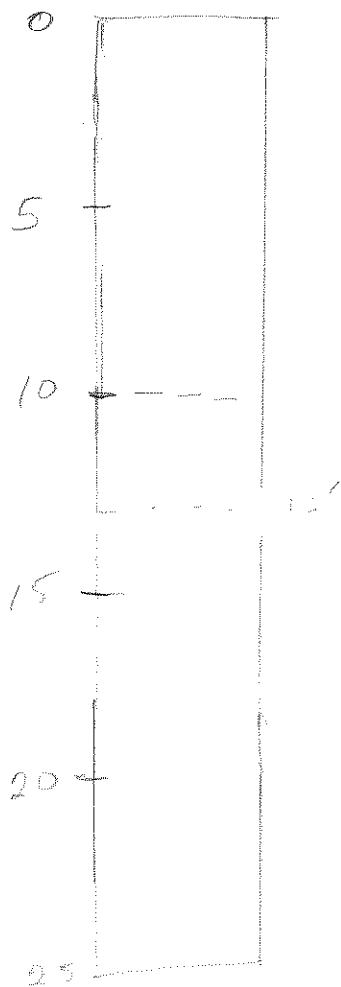
JOB NO. 043-2319-0002

FILE NAME: 043-2319-0002-BH Logs

(29)

Summary over

BCI-1



0 - 10' relatively
soft Gila

10' - 25' hard
Gila



BOREHOLE LOG

BIC 2

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine		DRILLING METHOD: Hollow Stem Auger	BORING NO. <u>BIC-2</u>
Transsect 1		SAMPLING METHOD: Split Spoon	SHEET PAGE 1 OF 1
NORTHING 3613183		DRILLING START FINISH	
EASTING: 0749020		4:00 6:00	
DRILL RIG: ANGLE: 90		DATE DATE	
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.		SURFACE CONDITIONS:	

DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ /CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	18	100	Qal		(5-6.5') ColS (5% clay), 50% grl. subrounded, 1/2" largest gravel, moist	0	50	40	10	1042 4/3	FR	SO PD	
10	14		Qal		10-11' 104R 3/4 CL (25% clay) 5% subrounded subangular fine + med gravel, moist	0	5	30	65		FR	SP	
15					11-11.5' 104R 4/3 ColS (7% clay) 50% grl. subangular, angular, 3/4" largest grl, moist	0	50	40	10		FR	SO PD	
20					15' - Gila								
25	50	100			15-15.5' 104R 3/4 SCL (28% clay) NE (one 2" grl)	0	20	40	40		VH	SP	
30					15.5-15.7' 104R CLS, NE (7% clay), grl subangular, angular, 1/2" largest grl		50	40	10		FR	SO PD	
35					17' Gila Hard Dry								
40	50	100			20-20.5' 104R 3/4 SCL (25% clay), 20% fine + med. grl - subrounded, Dry, NE	0	20	50	30		VH	SP	

- Notes:
- Percent > 3 inch.
 - Sum of gravel, sand, and fines = 100%
 - For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.
 - For noncohesive soil: weak, moderate, strong.
 - Pocket penetrometer, torevane, in situ density, etc.

Completed drilling 5:45
Completed tripping 6:00

Sample: BIC-2 20-20.5'

pad is ~ 2' above original ground level

TD 20.5'

28

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

Summary over

LOGGED BY: Dave Buscher

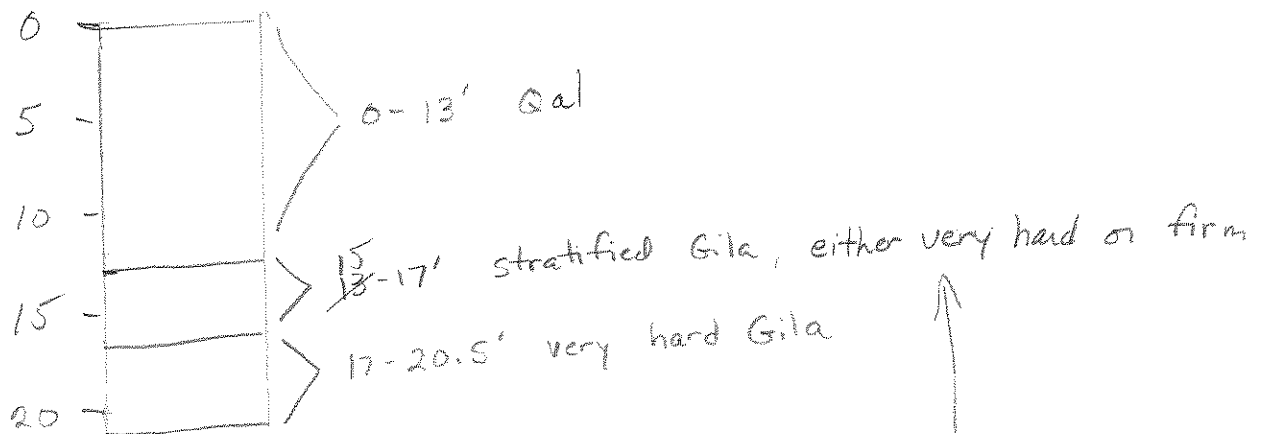
DATE: 1/25/05

JOB NO 043-2319-0002

FILE NAME: 043-2319-0002-BH Logs

later, probably all hard
pounding on spoon breaks up
cementation
Gila
15-17
Stratified
w/ very
hard +
Firm
layers.
> 17'
all
v. hard.

BC1-2



later!
probably all hard,
cement breaks down
during pounding on
spoon



BORING NO.

Transect 1

BCI-3 is in between BCI-1
↓ BCI-2

EASTING:

SHEET

PAGE OF

DRILLING

START	FINISH
-------	--------

[illegible]

7:30	7:45
------	------

DATE	DATE
------	------

--	--

BEARING: -

ANGLE: 90

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

FILE NAME: 043-2319-0002-BH Logs

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.^a For noncohesive soil; weak, moderate, strong.^s Pocket penetrometer, torevane, in situ density, etc.

birds on site 7:30

drilling at 7.45

15.5

rain .. d

drill pad $\cong 1'$ below original ground surface +
Constructed of GMA material

0
Qa1
5-

10- QTg Gila 7-15'

15



BOREHOLE LOG

BIC-4

raining

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine Transsect 1 center of drainage		DRILLING METHOD: Hollow Stem Auger	BORING NO. BIC-4
NORTHING		SAMPLING METHOD: Split Spoon	SHEET PAGE 1 OF 2
EASTING:		WATER LEVEL	DRILLING START FINISH
		TIME	9:00 12:00
		DATE	DATE
DRILL RIG: ANGLE: 90 BEARING: -		CASING DEPTH	
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.		SURFACE CONDITIONS:	

drillers took lunch, went to town at ~12:00

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ^{3/} CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	5	80	Qal		10YR 2/2 CoSL (13% clay), moist, original surface, grl are subrounded to angular, NE 5-6.5'	0	40	40	20	10YR 2/2	FR	SS sp	
10	20	80	Qal		10-11.5 10YR 5/4 CoLS, (5% clay), moist, grl are subrounded to angular, some granodiorite 2.5", some red altered grl. NE	5	50	35	10		FR	SO PO	
15	20	100	Qal		15-16.5 10YR 3/4 CoLS, (6% clay), NE, moist, grl is subrounded to angular, some diorite + altered grl.	0	50	40	10		FR	SO PO	
20	50	0	rock		20' on cobble								
25	40	10	rock		21' piece of diorite								
30	24	100			25-26.5 10YR 4/4 CoLS, grl subrounded to subangular, NE, moist, 3/4" largest grl.	0	60	35	5		FR	SO PO	
35	50	100	Stg	?	30-30.5 10YR 5/4 SL, 14% clay, moist	0	50	30	20		FR	SS SP	
40					Water at 32'								
45	50	0			35' - rock								

LOGGED BY: Dave Buscher

DATE: 11/24/05

JOB NO. 043-2319-0002

FILE NAME: 043-2319-0002-BH Logs

Notes:

- ¹ Percent > 3 inch.
- ² Sum of gravel, sand, and fines = 100%
- ³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.
- ⁴ For noncohesive soil: weak, moderate, strong.
- ⁵ Pocket penetrometer, torevane, in situ density, etc.

drill pad is about 4' above original ground surface.

Samples: BIC-4 5-6.5'
10-11.5'
15-16.5'
25-26.5'
30-30.5'
40-40.4'

drilling slightly harder at 30'



BOREHOLE LOG

BIC-5

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine <i>Transect 1</i>		DRILLING METHOD: Hollow Stem Auger	BORING NO. <i>BIC-5</i>
		SAMPLING METHOD: Split Spoon	SHEET PAGE OF DRILLING
NORTHING	EASTING:	WATER LEVEL	START FINISH
		TIME	2:30 4:00
		DATE	DATE
		CASING DEPTH	
DRILL RIG: ANGLE: 90 BEARING: -		SURFACE CONDITIONS:	
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.			

DEPTH IN METERS (ELEVATION)	BLOW/ 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	45	20	Qal		5-6' hit rock CoLS								
	30		Qal		Gila material but still in road base.								
10	50	0	rock		6-9' 10YR 2/2 SCL (original surface between 6'-9')						FR	SP	
15	50	0	rock		12'								
	50	80	rock		15-15.3' 10YR 6/2 SCL, 22% clay, moist.	10	15	40	35		FR	SP	
20			Qal		grl are subrounded to subangular + fine to medium. NE. Much of the blow due to rock.								
25	50	10	OTg		20-20.5' rock - granodiorite + small amt. of 10YR 9/3 (12%), moist	0	30	40	30		?	SS SP	
30													
35	50	100	OTg		22-22.5' 10YR 5/3 CoLS, grl are subrounded to subangular. Some 1" granodiorite. NE slightly moist	0	50	40	10		lo	sopo	
40											↑	pulverized	

Notes:

¹ Percent > 3inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.Samples: BIC-5 15-15.3'
22-22.5'

drill pad is elevated ~ 5'
above original surface - pad constructed
of Gila material

TD = 22'

raining

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

LOGGED BY: Dave Buscher

FILE NAME: 043-2319-0002-BH Logs

JOB NO. 043-2319-0002

DATE: 1/26/05



BOREHOLE LOG *BIC-4* *Cont.*

raining

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine <i>Transect 1</i> <i>center of drainage</i>		DRILLING METHOD: Hollow Stem Auger	BOREHOLE NO: <i>BIC-4</i>
		SAMPLING METHOD: Split Spoon	SHEET PAGE <i>2</i> OF <i>2</i>
NORTHING: EASTING:		WATER LEVEL	DRILLING START FINISH
		TIME	DATE DATE
		DATE	
DRILL RIG: ANGLE: 90 BEARING: -		CASING DEPTH	
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.		SURFACE CONDITIONS:	

DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5					<i>32' mud, 10YR 5/2</i>								
10					<i>Co SL, 30% grl. NE</i>								
15					<i>pH of mud = 4.5</i>								
20	<i>50</i>	<i>0</i>			<i>pH of water = 4.0</i>								
25					<i>according to driller</i>								
30	<i>50</i>	<i>0</i>			<i>peached water ends</i>								
35					<i>at about 38' ± a</i>								
40					<i>couple feet</i>								
	<i>50</i>	<i>100</i>	<i>OTg</i>		<i>35' - rock</i>								
					<i>37' - rock</i>								
					<i>40-40.4' 10YR 5/4</i>	<i>0</i>	<i>60</i>	<i>25</i>	<i>15</i>		<i>FR</i>	<i>SS SP</i>	
					<i>Co SL (14% clay) NE,</i>								
					<i>moist, grl subrounded</i>								

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

LOGGED BY: Dave Buscher

DATE: *1/26/05*

JOB NO. 043-2319-0002

FILE NAME: 043-2319-0002-BH Logs

Notes:

- ¹ Percent > 3 inch.
- ² Sum of gravel, sand, and fines = 100%
- ³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.
- ⁴ For noncohesive soil: weak, moderate, strong.
- ⁵ Pocket penetrometer, torenvane, in situ density, etc.

TD = 40'



BOREHOLE LOG BIC-6

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine

DRILLING METHOD: Hollow Stem Auger

BORING NO.

Transsect 1

SAMPLING METHOD: Split Spoon

SHEET
PAGE OF
DRILLING

NORTHING

EASTING:

WATER LEVEL
TIME
DATE
CASING DEPTH
DATESTART
FINISH
9:30 12:00DATE
DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	3	70	Qal		Hit original surface @ 4'								
10	50	0			5-6.5' 10YR2/2 SLL (15% clay), moist NE, grt subrounded rock	0	40	55			FR	SS sp	
15	24	100	Qal		15-16 10YR4/2 SL, NE, grt subrounded to subangular.	5	35	55			FD	SS sp	
20	25	80	Qal		20-21' 10YR5/4 CoLS (7% clay) NE, moist. grt generally subrounded moist and at times red stained grt	~	60	80			FR	SS sp	
25	0	rock			25'- rock								
30	50	90	QTz		26-26.5 10YR5/3 CoSL 13% clay, grt subrounded to angular, moist	0	60	60			FR	SS sp	
35			QTz		26.5-27 10YR5/3 SCL 22% clay, grt subrounded to subangular, moist	0	40	40			MH	SP	
40	50	80	QTz		30-30.5 10YR5/4 CoSL dry, NE, grt are subrounded	0	50	60			10	SS sp	
45	50	100	QTz		35-25.5 10YR5/3 SCL, slightly (22% clay), NE moist grt are subrounded	0	70	60			H	SP	

Notes:

¹ Percent > 3inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.Drill pad is elevated about 4'
above original surface.

drilled an additional 5' to check for water

Drillers on
site at 9:10
due to rain
Drilling at
9:30

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

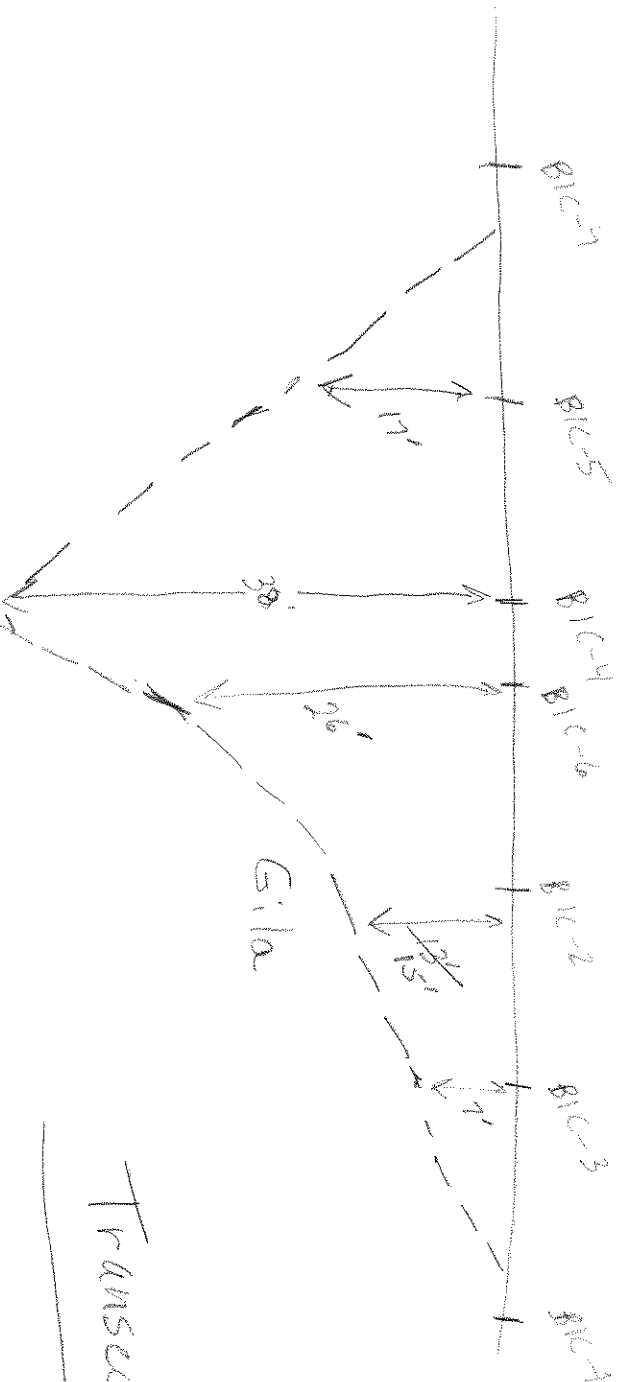
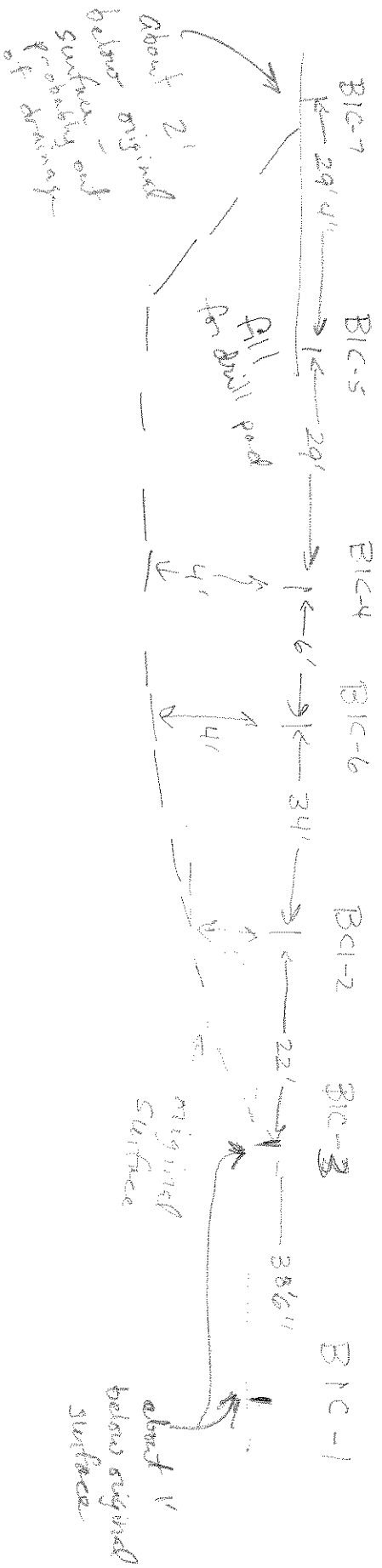
LOGGED BY: Dave Buscher

DATE: 1/27/05

JOB NO. 043-2319-0002

FILE NAME: 043-2319-0002-BH Logs

→ E



Transect 1

all nature is art + science



BOREHOLE LOG

(2nd from W. end of transect) BIC-9

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine		DRILLING METHOD: Hollow Stem Auger	BORING NO. BIC-9
Transect 2		SAMPLING METHOD: Split Spoon	SHEET PAGE 1 OF 1
NORTHING		DRILLING START FINISH	
EASTING:		DATE 1/27/05	
DRILL RIG: ANGLE: 90 BEARING: -		DATE 1/27/05	
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.		SURFACE CONDITIONS:	

DEPTH IN METERS (ELEVATION)	BLOW 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (pp. l, m, h)	OTHER TESTS ⁵
5	10				0-1 10YR 2/2 SCL								
10	49			①	5-6.5 9" recovery 3" SCL 10YR 3/2, sl. moist grad. to med.	30	40	30			soft	sp	NE
15	25				6" Co SL 10YR 4/4 grl is subangular to subrounded, fine to medium dry	50	30	20			soft	sp, po	NE
20	25			②	10-11.5 14" res, 9" 7.5YR 5/4 Co SL, grl. subang to sub rounded, dry	35	55	10			soft-firm	so, sp	NE
25	25				3" SCL 10YR 3/2, grl. fine to med, s. ang to s. round.	25	45	30			firm	so, sp, to pl	NE
30	10			③	15-16.5 10" recovery 6" 10YR 5/3 SCL dry	25	45	30			hard	ss, sp.	
35	45/4"				* likely contact at 15.5' to 16' 4" case, grl. sub rounded, fine to med, 10YR 5/4, dry sl. moist	55	35	10			prob. (soft)	sp, po	NE
40	59/6"			④	20-20.5 10YR 6/4 6" recovery, Co SL grl is fine, sub rounded, sl. moist	45	45	10			firm moist	sp, po	NE
					TD 20.5'								

Notes:

- Percent > 3 inch.
- Sum of gravel, sand, and fines = 100%
- For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.
- For noncohesive soil: weak, moderate, strong.
- Pocket penetrometer, torevane, in situ density, etc.

Samples:

5-6.5'
10-11.5'
15-16.5'
20-20.5'

Borehole surface at original grade.

sp = slightly plastic

LOGGED BY: Dave Buscher / Mike Gaber DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

DATE: 1/27/05

JOB NO. 043-2319-0002

FILE NAME: 043-2319-0002-BH Logs



BOREHOLE LOG

BIC-10

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine		DRILLING METHOD: Hollow Stem Auger	BORING NO. BIC-10
Transsect 2		SAMPLING METHOD: Split Spoon	SHEET PAGE OF DRILLING
NORTHING		WATER LEVEL	START FINISH
EASTING:		TIME	DATE DATE
DRILL RIG: ANGLE: 90 BEARING: -		SURFACE CONDITIONS:	
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.			

DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (pp, l, m, h)	OTHER TESTS ⁵
5	12	100	Qal		5-6.5' 10YR 4/4 CoSL, NE most grl are fine & subangular dry. hit original surface 10YR 2 1/2 SCL at about 2'		65	20	15		SP	SP	
10													
15	8	60	Qal		10-11' hit rock at 10.5' 7.5YR 4/4 CoSL 5% clay, moist, grl are subangular to subangular. NE	5	60	20	15		SP	SP	
20													
25	17	100	Qal		15-16.5' SCL (35% clay) 7.5YR 4/3, NE, grl are fine, moist	0	15	50	35		HA	SP	
30													
35	50	100			20-20.5 one broken rock in sample, may caused fine. 50 blow counts. 7.5YR 4/4 SCL. 25% clay, NE moist	5	10	45	40		HA	SP	
40													
45	145	100	OTg		25-25.5 SYR 4/4 CoSL 13% clay, NE, moist most grl are fine	0	60	25	15		MH		
50													
55	100	100	OTg		27-28.5 27-28.5 5YR 4/4 SCL 28% clay, NE, grl 10%	0	10	-	-		FR-MH	SP	

Notes:

1 Percent > 3 inch.

2 Sum of gravel, sand, and fines = 100%

3 For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.

4 For noncohesive soil: weak, moderate, strong.

5 Pocket penetrometer, torevane, in situ density, etc.

28-28.5 5YR 4/4 CoLS
20% subrounded grl,
slightly moist. NE
FR, SP, PD

water running across the drill pad
coming from bottom of stockpile, pH=3
boring hole 2' above original surface
boring is across from stake 15

drillers on site
at 7:20completed
by 9:25

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

LOGGED BY: Dave Buscher

DATE: 1/28/05

JOB NO. 043-2319-0002

FILE NAME: 043-2319-0002-BH Logs

23' - drill indicated north drilling at 23'

23'



BOREHOLE LOG

BIC-11

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine

DRILLING METHOD: Hollow Stem Auger

BORING NO.

Mine

Transect 2

BIC-11

SAMPLING METHOD: Split Spoon

SHEET

PAGE OF

DRILLING

START FINISH

NORTHING

EASTING:

WATER LEVEL

TIME

DATE

CASING DEPTH

9:30

10:30

DATE

DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN METERS (ELEVATION)	BLOW 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY/ ³ CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	10 46 50	100	Qal QTg		5-5.5 10YR 4/4 CoSL 15% clay, wet, grl are subrounded to subangular	40	40	20		FR	SSP		
10					5.5-6.5 7.5YR 4/4 SCL 34% clay, dry NE	15	50	35		HR- VH	SP		
15	50	100			10-10.5 7.5YR 3/3 SCL 34% clay, dry, NE grl is fine	15	50	35					
20	50	100			15-15.6 (7"-50 blows) 7.5YR 3/3 SCL (30% clay) NE, dry, 15% fine grl subround to subangular	15	50	35		HR- MD	SP		
25													
30													
35													
40													

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

- boring hole about 1' above original surface

- across from stake 20.

Samples: BIC-11 5-6.5'

10-10.5'

15-15.6'

- photo 41 of Qal/QTg contact

note wet 5-5.5' + dry 5.5-6.5'

photo 40 drilling T2

not saturated

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

LOGGED BY: Dave Buscher

DATE: 1/28/05

JOB NO. 043-2319-0002

FILE NAME: 043-2319-0002-BH Logs

relatively easy drilling to 10'
high blow count possible from
tight clay, going 5' more.

drilling a bit
harder below 10'



BOREHOLE LOG

BIC-12

* did not
drill next
to stake 16,
one to the
North

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine <i>Transect 2</i>		DRILLING METHOD: Hollow Stem Auger	BORING NO.
		SAMPLING METHOD: Split Spoon	SHEET PAGE OF DRILLING
NORTHING EASTING:		WATER LEVEL TIME DATE CASING DEPTH	START FINISH
			11:00 11:30
DRILL RIG: ANGLE: 90 BEARING: - SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.		SURFACE CONDITIONS:	

DEPTH IN METERS (ELEVATION)	BLOW 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (mp, l, m, h)	OTHER TESTS ⁵
5	25	100	Qal		5-6.5 7.5YR 4/4 CoLS 3% clay, 3% grl most is fine subrounded to subangular, NE. bottom 1" may be Gila	0	30	65	5		FR	50 Po	
10	25												
15	50	100	QTz		10-10.3 (50 blows 4") 7.5YR 4/4 CoSL 10% clay, NE	0	50	35	15		HA VH	50 Po	
20													
25													
30													
35													
40													

Notes:

- Percent > 3inch.
- Sum of gravel, sand, and fines = 100%
- For cohesive soil: soft, firm, hard, v. hard.
- For noncohesive soil: weak, moderate, strong.
- Pocket penetrometer, torevane, in situ density, etc.

boring is in between
BIC-8 + 9.

boring hole is about 1'
below original surface

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

LOGGED BY: Dave Buscher

DATE: 1/28/05

JOB NO. 043-2319-0002

FILE NAME: 043-2319-0002-BH Logs

(14)

BIC-8

← 14' →

BIC-12

← 12' →

BIC-9

← 25 1/2' →

(19)

(15)

BIC-10

← 24' →

(26)

BIC-11

← 21' →

didn't drill

BIC-11 fell down
stake in water. Surveyor had
27' from 10 but should
be 24'



BOREHOLE LOG

BCI-8

(stake marked #44 Western most
BH on transect #2 - rig pulled to location)

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine

DRILLING METHOD: Hollow Stem Auger

BORING NO.

BCI-8

Transsect 2

SAMPLING METHOD: Split Spoon

SHEET

PAGE 1 OF 1

NORTHING

EASTING:

DRILLING

START FINISH

1500 1530

DATE DATE

1/27/05 1/27/05

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN METERS (ELEVATION)	BLOW 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ CEMENTATION ⁴	PLASTICITY (pp. l, m, h)	OTHER TESTS ⁵
5	50/4"	QTY	①	5-5.5'	10YR 5/6 LS, gr is subangular, fine to medium, dry	30	60	10			soft- PHL unified	SO DO PH	NE
10	50/3"	QTY	②	10-10.5'	10YR 5/4, 6LS 10% clay gr is subrounded to subangular, fine to med. dry	45	40	15			PR- HARD	SO DO NE	
15													
20													
25													
30													
35													
40													

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.Drill pad is ~~between~~ 2-3'
below original gradeSamples: BCI-8 5-5.4'
10-10.3'drill rig
stuck at
1:00
Unstuck at
2:45pm
(still waiting on
equip. truck road
being fixed).
3:05 - fully
operational

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

LOGGED BY: Dave Buscher / M. Gabor

DATE: 01/27/05

JOB NO. 043-2319-0002

FILE NAME: 043-2319-0002-BH Logs



BOREHOLE LOG

(2nd from W. end of transect) BCI - 9

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone

DRILLING METHOD: Hollow Stem Auger

BORING NO.

Mine

Transect 2

BCI-9

SAMPLING METHOD: Split Spoon

SHEET

PAGE 1 OF 1

DRILLING

START FINISH

NORTHING

EASTING:

WATER LEVEL

TIME

DATE

CASING DEPTH

5.45

DATE

1/27/05

DATE

1/27

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (mp, l, m, h)	OTHER TESTS ⁵
0					0-1 104R 2/2 SCL								
5	10				① 5-6.5								
10	44				9" recovery								
15	50				3" SCL 104R 3/2, sl. moist	30	40	30			soft	sp	NE
20	25				6" CoSL 104R 4/4	50	30	20			soft	sp	NE
25	25				grl is subangular to								
30	25				subrounded, fine to medium								
35	10				dry								
40	45/44				② 10-11.5 14" rec, 9" 7.5" R 5/4	35	50	10			soft-firm	sp, sp.	NE
45					2" SCL, grl. subang to sub rounded,								
50					dry								
55					3" SCL 104R 3/2, grl. fine	25	45	30			firm	sp, sp.	NE
60					to med, s. angl to s. round.								
65					③ 15-16.5								
70					10" recovery Gila 16'								
75					6" 104R 5/3 SCL	25	45	30			hard	ss, sp.	
80					dry								
85					* likely contact at 15.5' to 16'								
90					4" CoSL, grl. are subrounded,	55	35	10			prob. (soft)	sp, sp.	NE
95					fine to med, 104R 5/4, dry								
100	59/6"				sl. moist								
105					④ 20-20.5 104R 6/4	45	45	10			firm	sp, sp.	NE
110					6" recovery, CoSL								
115					grl is fine, subrounded,								
120					sl. moist								
125					TD 20.5'								

Notes:

¹ Percent > 3inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

Borehole surface at original grade.

sp = slightly plastic

Samples:

5-6.5'

10-11.5'

15-16.5'

20-20.5'

LOGGED BY: Dave Buscher/Mike Gabore DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

DATE: 1/27/05

JOB NO. 043-2319-0002

FILE NAME: 043-2319-0002-BH Logs



BOREHOLE LOG

BCI-10

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone

Mine

Transect 2

DRILLING METHOD: Hollow Stem Auger

BORING NO.

BCI-10

SAMPLING METHOD: Split Spoon

SHEET

PAGE

OF

DRILLING

NORTHING

EASTING:

WATER LEVEL

TIME

DATE

CASING DEPTH

SURFACE CONDITIONS:

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

drillers on site
at 7:20

Completed

9:25

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

LOGGED BY: Dave Buscher

DATE: 1/28/05

JOB NO. 043-2319-0002

FILE NAME: 043-2319-0002-BH Logs

DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (mp, l, m, h)	OTHER TESTS ⁵
5	8	100	Qal		5-6.5' 10YR 4/4 CoSL, NE most grl are fine & subrounded, dry. hit original surface 10YR 2/2 SCL at about 2'		65				FR	SOPO	
10	8	60	Qal		10-11' hit rock at 10.5' 7.5YR 4/4 CoSL 15% clay, moist, grl are subround to subangular, NE	5	60	20	15		FR	SP	
15	8	100	Qal		15-16.5' SCL (35% clay) 7.5YR 4/3, NE, grl are fine, moist	0	15				HA	SP	
20	17	50	Qal		20-20.5' one broken rock in sample; may cause the 50 blow counts. 7.5YR 4/4 SCL, 25% clay, NE, moist	0	10				HA	SP	
25	17	100	Qal		25-25.5' SYR 4/4 CoSL 13% clay, NE, moist most grl are fine	0	60				MH		
30	16	100	Qal		27-28.5' 27-28' 5YR 4/4 SCL 28% clay, NE, grl 10'	0	10				FR-MH	SP	

Notes:

¹ Percent > 3/8 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torenvane, in situ density, etc.

28-28.5 SYR 4/4 CoSL
20% subrounded pl,
slightly moist. NE
FR, SO, PO

water running across the drill pad
coming from bottom of stockpile, pH=3

boring hole 1-2' above original surface.
boring is across from stake 15

23' - driller indicated hard drilling at 23'



BOREHOLE LOG

BIC-11

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine <i>Transect 2</i>	DRILLING METHOD: Hollow Stem Auger	BORING NO. <i>BIC-11</i>
	SAMPLING METHOD: Split Spoon	SHEET PAGE OF
	DRILLING	START FINISH
	WATER LEVEL	9:30 10:30
NORTHING	EASTING:	DATE DATE
DRILL RIG:	SURFACE CONDITIONS:	
ANGLE: 90		
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.		

DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ CEMENTATION ⁴	PLASTICITY (np, 1, m, h)	OTHER TESTS ⁵
5	10	46	100	Qal	5-5.5 10YR 4/4 GsL	40					FR	SS SP	
10	50			QTg	15% clay, wet, grl are subrounded to subangular	15					HR-VH	SP	
15	50	100			5.5-6.5 7.5YR 4/4 SCL	15							
20	50	100			34% clay, dry NE								
25					10-10.5 7.5YR 3/3 SCL	15							
30					34% clay, dry, NE grl is fine								
35					15-15.6 (7"-50 blows) 7.5YR 3/3 SCL (30% clay) NE, dry, 15% fine grl subround to subangular	15					HR-MD	SP	
40													

- Notes:
- Percent > 3inch.
 - Sum of gravel, sand, and fines = 100%
 - For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.
 - For noncohesive soil: weak, moderate, strong.
 - Pocket penetrometer, torevane, in situ density, etc.

- boring hole about 1' above original surface.

- across from stake 20.

Samples: BIC-11 5-6.5'
10-10.5'
15-15.6'

- photo of Qal/QTg contact
note wet 5-5.5' + dry 5.5-6.5'

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

LOGGED BY: Dave Buscher

DATE: 1/28/05

JOB NO. 043-2319-0002

FILE NAME: 043-2319-0002-BH Logs

relatively easy drilling to 10'
high blow count possibly from
tight clay, going 5' more

drilling a bit
harder below 10'



BOREHOLE LOG

BIC-12

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine

DRILLING METHOD: Hollow Stem Auger

BORING NO.

Transect 2

SAMPLING METHOD: Split Spoon

SHEET

PAGE

OF

DRILLING

NORTHING

EASTING:

WATER LEVEL

TIME

DATE

CASING DEPTH

SURFACE CONDITIONS:

START

FINISH

DATE

DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	25	100	Qal		5-6.5 7.5YR 4/4 CoLS	0	30	65	5		FR	50 PO	
6	25				3% clay, 30% grl								
7	50				most is fine subrounded								
8					to subangular, NE.								
9					bottom 1" may be Gila								
10													
11													
12													
13													
14													
15	50	100	QTA		10-10.3 (50 blows 4")	0	50				HA	50 PO	
16					7.5YR 4/4 CoSL						VH		
17					10% clay, NE								
18													
19													
20													
21													
22													
23													
24													
25													
26													
27													
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41													
42													
43													
44													
45													
46													
47													
48													
49													
50													

Notes:

¹ Percent > 3inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.boring is in between
BIC-8 + 9.boring hole is about 1'
below original surface* did not
drill next
to stake 16,
one to the
North11:30
ready to
move on
standby
dozer fixing
road.

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

LOGGED BY: Dave Buscher

DATE: 11/28/05

JOB NO. 043-2319-0002

FILE NAME: 043-2319-0002-BH Logs



BOREHOLE LOG

BIC-13

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone

DRILLING METHOD: Hollow Stem Auger

BORING NO.

Mine

Transtect 3

at NE end.

SAMPLING METHOD: Split Spoon

SHEET

PAGE OF

DRILLING

START FINISH

NORTHING

EASTING:

WATER LEVEL

TIME

DATE

CASING DEPTH

SURFACE CONDITIONS:

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

DEPTH IN METERS (ELEVATION)	BLOW 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	50	100			Upper 2' road fill - but 2-5' 10YR 4/4 SCL, NE 24% clay, 30% subrounded grl								close to original surface level
10	50	100			5-5.4 10YR 5/3 CoSL (16% clay), 50% grl that breaks apart - highly weathered. Either a weathered diorite or close to bedrock. - No	50	30	20			H	SS SP	
15	20	100			10-11' 10-10.5' 7.5YR 5/6 SCL (27% clay), NE 10.5-11' pulverized granodiorite, dry	5	15	50	30		FR	SP	
20	40	100			15-16' 10R 4/6 SCL, 28% clay, grl are red & angular. NE, dry	60	25	15			MH	SP	
25	20	70			20-21' 10R 4/6 SCL, 32% clay, NE, dry grl are red, subrounded to subangular, most are fine. Rock at 20-7-21'	5	35	40	20		MH	SP	
30	50	100			25-25.5 10R 4/6 SCL 28% clay, grl are red subangula to subround, hit several larger rocks, dry	10	40	30	20		H	SP	
35	50	100			30-30.5 - hard weathered diorite. 7.5YR 6/2 rock						VH		

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

TD 30"

Samples: BK-13 10-10.5'
15-16'
20-21'
25-25.5'
30-30.5'Weathered diorite bedrock is 33' to the NE
along drill pad.

Photo 42 - drilling

Photo 43 of 10-11' Spoon sample

level, but not
absolutely sure.

at stake 9

Start by
over at
12:15

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

LOGGED BY: Dave Buscher

DATE:

JOB NO. 043-2319-0002

FILE NAME: 043-2319-0002-BH Logs



BOREHOLE LOG

BIC-14

at stake 8

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine

DRILLING METHOD: Hollow Stem Auger

BORING NO.

Mine

Transect 3

SAMPLING METHOD: Split Spoon

SHEET

PAGE OF

DRILLING

START FINISH

NORTHING

EASTING:

WATER LEVEL

TIME

DATE

CASING DEPTH

2120

3145

DATE

DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN METERS (ELEVATION)	BLOW 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	14	40			5-6.5' 7.5YR 4/3 SCL 28% clay, grl are angular - subangular disint. moist	5	30	35	25	FR	SP		
10	30	40			10-11.5 5YR 4/4 SCL 32% clay, grl are red & subrounded to angular moist	5	40	33	22	H			
15	20	100			15-16.5 5YR 4/4 SC 38% clay, moist grl are fine, NE	0	5	50	45	H-VH	SP		
20	20	100			20-21.5 5YR 4/4 SC 50% clay, slightly moist, gravels are fine NE	0	5	45	50	H-VH	VSVP		
25	50	80			25' 5YR 4/6 SCL 34% clay, saturated (only got 2', either on large rock or hard bed rock) refusal at 25.2', thin layer of water on top. → pH of mud = 6.5	0	5	50	45	—	SP		

Notes:

¹ Percent > 3inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

- bore hole at about original surface level.

- photo 44 15-16.5' sample

Samples: BIC14

5-6.5'

10-11.5

15-16.5

20-21.5

25'

- Spoon had about 0.5" mud on the outside

- saturated zone 0.5-1' thick.

- close to original ground level

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

LOGGED BY: Dave Buscher

DATE: 1/23/05

JOB NO. 043-2319-0002

FILE NAME: 043-2319-0002-BH Logs



BOREHOLE LOG BIC-15

on site 7:00,
arrived to site
would not
drilling 2:00

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine		DRILLING METHOD: Hollow Stem Auger	BORING NO.
Transsect 3		SAMPLING METHOD: Split Spoon	SHEET PAGE OF DRILLING
NORTHING	EASTING:	WATER LEVEL	START FINISH
		TIME	8:00 10:00
		DATE	DATE
		CASING DEPTH	8:30 10:30
DRILL RIG: ANGLE: 90 BEARING: -		SURFACE CONDITIONS:	
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.			

10:00 discontinued
hole unable to
sample due to
mud in hole,
1/29/05
2/2/05

DEPTH IN METERS (ELEVATION)	BLOW/ 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	30	100			5-6' 10YR5/4 CoSL, 15% clay, grl are fine NE 5-6.5' moist 4.5-6' saturated. pH of mud 4.6		30	40	30		Fr	SS	
10	25	100			10-11.6' 10YR4/4 SCL 28% clay, NE, grl are fine, 10-11' saturated pH of water = 4.0, moist → 11-11.6' weathered diorite.		15	50	35		HR	SP	
15	30				15- no sample, problem with water in the hole. 20' unable to sample. mud in hole.								
20					15' tried collecting sample twice, last one 60 blows 4" & sample spoon was bouncing, but mud sucked into spoon. Sample consisted of mud + diorite chips.								
25	7				20-21' 7.5YR5/4 SCL, moist NE, with broken diorite fragments.		5	30	0				
30					25-25.3 hand diorite dry, duped								
35													
40													
45													
50	50	100											

2/2/05
8:00 filled
water tank
8:30 drilling

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)
LOGGED BY: Dave Buscher
DATE: 1/29/05 - 2/2/05

JOB NO 043-2319-0002
FILE NAME: 043-2319-0002-BH Logs

Notes:

- ¹ Percent > 3 inch.
- ² Sum of gravel, sand, and fines = 100%
- ³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.
- ⁴ For noncohesive soil: weak, moderate, strong.
- ⁵ Pocket penetrometer, torevane, in situ density, etc.

Samples: BIC-15 5-6'

TD = 25'

10-11.6'
20-21'
25-25.3

photo 45 of 5-6'
photo 46 showing diorite/alluvium contact
4-5' below original surface



BOREHOLE LOG BIC-16

at stake 1

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine

DRILLING METHOD: Hollow Stem Auger

BORING NO.

BIC-16

Mine

Transect 3
SW end

SAMPLING METHOD: Split Spoon

SHEET

PAGE OF

DRILLING

START FINISH

NORTHING

EASTING:

WATER LEVEL

TIME

DATE

CASING DEPTH

DATE

DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN METERS (ELEVATION)	BLOW / 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (pp. l, m, h)	OTHER TESTS ⁵
5	10	100	soil		upper foot 7.5YR 2.5/2 SC (34% clay) - probably on original surface								
10	16	100			5-6.5 5YR 4/3 SC (36% clay), ES, 15% fine gravel, moist	0	10	45	45		H	SP	
15	30	100	soil		10-11': 10-10.5' 5YR 3/2 SC (34% clay), ES, moist, 16% fine gravel	0	10	50	40		H	SP	
20	50		bedrock		10.5-11' 7.5YR 4/3 CoLS (3% clay); NE weathered qtz. diorite (or qtz. monzonite), dry								
25					photo 47 of 10-11'								
30					refusal at 12'								
35													
40													

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

Samples: BIC-16

5-6.5'

- close to original
ground level

10-10.5'

10.5-11'

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

LOGGED BY: Dave Buscher

DATE: 1/29/05

JOB NO 043-2319-0002

FILE NAME: 043-2319-0002-BH Logs



BOREHOLE LOG BIC-17

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine		DRILLING METHOD: Hollow Stem Auger	BORING NO.	
Transect 3 2nd from SW end		SAMPLING METHOD: Split Spoon	SHEET PAGE 1 OF 1	
NORTHING		EASTING:		DRILLING
		WATER LEVEL	TIME	START
		DATE	DATE	FINISH
DRILL RIG:		SURFACE CONDITIONS:		
ANGLE: 90				
BEARING: -				
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.				

DEPTH IN METERS (ELEVATION)	BLOW 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (mp, l, m, h)	OTHER TESTS ⁵
5	20	100			about 1', 10YR 2/1 SCL, maybe original surface.								
10	20	100			5-6.5' 10YR 3/3 SC 37% clay, NE, moist	10	75	45			H	SP	
15	20	100	Qal	?	10-11.5' 10-11' 7.5YR 3/3 SC, NE, slightly moist. (37% clay) grt are fine	15	45	40			H	SP	
20	22	100			11-11.5' 10YR 4/4 COLS, dry, most grt are fine subangular	35	60	5			FR	SO PO	
25	24	100	Qal	?	15-16.5' stratified 10YR 4/4 SCL (34% clay) 1" 10YR 4/4 COLS and at bottom 5YR 4/4 SC 38% clay, all NE slightly moist	10	45	45			MH FR	SP SO PO	
30	30	100	Soil		20-21.5' 5YR 4/6 SCL 32% clay, NE, dry, grt are subangular	20	75	5			H	SP	
35	20	100	Soil		25-26.5' 5YR 4/6 SCL 34% clay, NE, dry	10	60	30					
40	25	100			30-31' weathered diorite	10	55	35			H		
45	30	100	R										

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

drilled additional 5' to make sure 30' wasn't just a rock

50 blows 4" 35-35.4 - hard diorite (VH)

- close to original ground level

- photo 48 of 10-11.5' sample

Samples: BIC-17 5-6.5' BIC-17 30-31'
10-11' 35-35.4'
11-11.5'
15-16.5'
20-21.5'
25-26.5'

TD = 35'

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

LOGGED BY: Dave Buscher

DATE: 1/29/05

JOB NO. 043-2319-0002

FILE NAME: 043-2319-0002-BH Logs

Soil derived from diorite
30' weathered diorite

Qal

16'

BCI-13 74' →
BCI-14

BCI-17 85' →
BCI-16



BOREHOLE LOG

BIC-18

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine

DRILLING METHOD: Hollow Stem Auger

BORING NO.

Mine

Transect 3

SAMPLING METHOD: Split Spoon

SHEET
PAGE OF

DRILLING

NORTHING

EASTING:

WATER LEVEL

TIME

DATE

CASING DEPTH

START

FINISH

3:00 5:20

DATE

DATE

DRILL RIG:

ANGLE: 80

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN METERS (ELEVATION)	BLOW 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, I, m, h)	OTHER TESTS ⁵
5	12	100			5-6.5' 10YR3/3 SCL (26% clay), moist, NE grt are subangular, contains some mica.	0	30	40	30		MH	S P	
10	15												
15	15												
20	20				10-11.5' 10YR 4/4 CoSL 12% clay, moist, NE grt are subangular + most are fine. Root at bottom	0	35	40	25		FR	SS/SP	
25	36	100	Qal										
30	50												
35	20				15-16.5' 10YR4/4 CoSL 10% clay, NE, grt are subangular subangul.	0	35	40	25		FR	SS/SP	
40	30	80	Qal										
45	28												
50	20				20-21.5' 7.5YR 4/3, SC 45% clay, NE, moist	0	20	40	40		HA	SSP	
55	30	100											
60	50				25-26.5' 7.5YR 4/4 SCL, 27% clay, NE moist grt are fine	0	10	55	35		MH	S P	
65	20												
70	35	100			30-31.5' 5YR 4/4 SCL 35% clay, NE, moist, grt are fine (weathered from diorite).	0	5	60	35		MH	S P	
75	50												
80	20				35-35.5' weathered diorite, with 2" of mud on top, 5YR 4/4 SCL						H	-	
85	30	100											
90	50				37-37.5' - 7" red hard weathered diorite						VH	-	
95	20												
100	30	100											

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soft, v. soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

Weathered diorite is dry Samples: BIC-18

- about 3' below original ground level

mud in spoon from hydraulic head.

PH of mud = 5.0, water-filled
the hole, etc.

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

LOGGED BY: Dave Buscher

DATE: 1/29/05

JOB NO. 043-2319-0002

FILE NAME: 043-2319-0002-BH Logs

5:30-

trip 6:00

decon. 6:00-

left site 6:15

Next to
Stake 35-6.5'
10-11.5'
15-16.5'
20-21.5'
25-26.5'
30-31.5'
35-35.5'
37-37.5'

see back



BOREHOLE LOG BIC-19

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine		DRILLING METHOD: Hollow Stem Auger	BORING NO.
Transect 3		SAMPLING METHOD: Split Spoon	SHEET PAGE OF DRILLING
		WATER LEVEL TIME	START FINISH
NORTHING	EASTING:	DATE	DATE
DRILL RIG: ANGLE: 90 BEARING: -		SURFACE CONDITIONS:	
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.			

cold ground is frozen
7:50 moved rig to site
10:30 moving to transect 4.
10:00-10:30 trapping & clean some augers

DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY/ ³ CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	27	80			about 2' 10YR 2/1 SCL								
10	27				Saturated soil ~ 3' from recent rains PH of mud = 6.0								
15	14	100			5-6.5' 7.5YR 3/3 SCL	0	5	55	40		MH	SP	
20	14				22% clay, mica flakes, sl moist, NE, SL fine grl								
25	19				10-11.5' 7.5YR 3/3 SC	0	5	50	45		MH	vs VP	
30	25	60			45% clay, moist, NE								
35	30				5% fine grl,								
40	35				15-16.5 5YR 4/4 SCL	5	5	55	40		MH	SP	
45	23	100			32% clay, NE, moist								
50	23				SL fine grl								
55	16	100			20-21.5 5YR 4/6 SCL	0	5	55	40		MH	SP	
60	23				34% clay, NE, sl moist, S grl								
65					25-25.3 red weathered diorite (fines are 2.5R 4/4 SCL 22% clay) dry						VH	SP	
70					27-27.3 - red weathered diorite, dry NE						VH		
75					TD 27'								

- Notes:
- ¹ Percent > 3in.
 - ² Sum of gravel, sand, and fines = 100%
 - ³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.
 - ⁴ For noncohesive soil: weak, moderate, strong.
 - ⁵ Pocket penetrometer, torevane, in situ density, etc.

Samples: BIC-19 5-6.5'
10-11.5'
15-16.5'
20-21.5'
25-25.3'
27-27.3'

- close to original ground level

* large sed pond wall, ~ 20' to west, has alluvium about 20' S of BIC-19. Thought was this boring would be in alluvium.

weathered diorite drills fairly easy

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

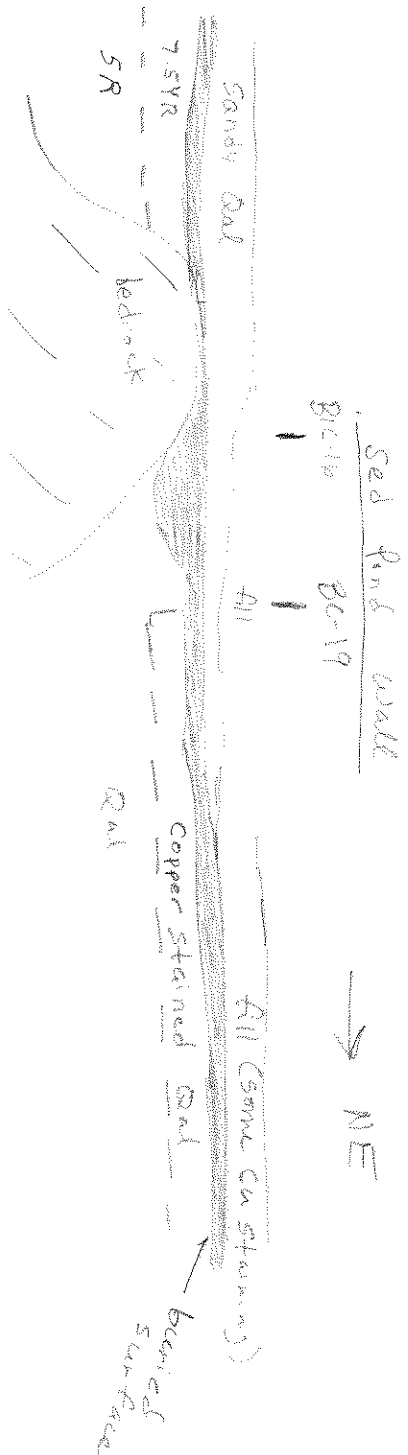
LOGGED BY: Dave Buscher

DATE: 1/30/05

JOB NO. 043-2319-0002

FILE NAME: 043-2319-0002-BH Logs

(3) → SW
 (2) →
 (1) →
 BP-18 BP-17 BP-19 BP-16
 ← 44' → ← 49 1/2' → ← 35 1/2' →



at stake 5



BOREHOLE LOG BIC-24

8:00 filled
water tank,
moved rig.

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine		DRILLING METHOD: Hollow Stem Auger	BORING NO. BIC-24
Transect 3		SAMPLING METHOD: Split Spoon	SHEET PAGE OF DRILLING
NORTHING	EASTING:	WATER LEVEL	START FINISH
		TIME	8:30 9:30
		DATE	DATE
		CASING DEPTH	
DRILL RIG: ANGLE: 90 BEARING: -		SURFACE CONDITIONS:	
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.			

DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	10	60	Qal		S-6.5 10YR 4/4 CLS, 5% clay, NE, SL moist, grl au. subround/subangular	0	25	70	5		soft	so po	
10	13	21	10'										
10	5	80	10'		10-11' 5YR 4/4 SCL, NE moist, (34% clay) contains much qtz. + mica.	0	5	60	35		MH	S	P
15	15	20	15'		15-15.4' 5YR 4/4 COSL, NE dry, 14% clay, - either rock or weathered diorite.						M	SS	SP
20	50	100	20'		20-20.4' same as 15-15.4' - weathered diorite, dry						M		
25	20	50	100										
30					about 6" of slab of 7.5YR 4/4 CL, MH moist, 35% clay, 40% sand, NE - probably somewhere between 16-20'								
35													
40					TD 20'								

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

LOGGED BY: Dave Buscher

DATE: 2/1/05

JOB NO. 043-2319-0002

FILE NAME: 043-2319-0002-BH Logs

Notes:

- Percent > 3inch.
- Sum of gravel, sand, and fines = 100%
- For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.
- For noncohesive soil: weak, moderate, strong.
- Pocket penetrometer, torevane, in situ density, etc.

- about 3' below original surface
photo 52 water seeping at bottom of waste pile.
photo 53 small diorite outcrop across from BIC-24
Samples: BIC-24 5-6.5'
10-11'
15-15.4'
20-20.4'

driller indicated upper 6" slab



BOREHOLE LOG BIC-25

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine	DRILLING METHOD: Hollow Stem Auger	BORING NO.
	SAMPLING METHOD: Split Spoon	SHEET PAGE OF DRILLING
	WATER LEVEL	START FINISH
	TIME	DATE DATE
	DATE	DATE
NORTHING	EASTING:	
DRILL RIG:	BEARING: -	
ANGLE: 90		
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.	SURFACE CONDITIONS:	

DEPTH IN METERS (ELEVATION)	BLOW 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ³	% FINES ³	COLOR	CONSISTENCY ⁴ / CEMENTATION ⁵	PLASTICITY (mp, l, m, h)	OTHER TESTS ⁵
5	50	100			5-5.5' 54R4/4 COSL 10% clay, dry, broken granodiorite, NE						H	SO PO	
10					above 5' 10YR3/3 SCL (34% clay) moist, MH, SP, NE								
15	50	100		1-2'	10-10.3' same as 5-5.5' weathered, dry granodiorite much K-spar; which gives the 54R color.						VH		
20													
25													
30													
35													
40													

Notes:

- Percent > 3 inch.
- Sum of gravel, sand, and fines = 100%
- For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.
- For noncohesive soil: weak, moderate, strong.
- Pocket penetrometer, torvane, in situ density, etc.

bore hole about 3.4' below original ground level

TD 10'

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

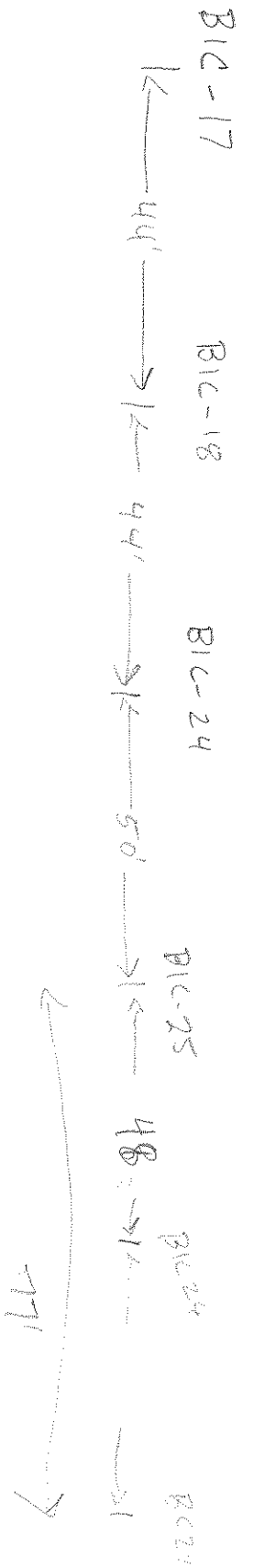
LOGGED BY: Dave Butcher

DATE: 2/1/05

JOB NO. 043-2319-0002

FILE NAME: 043-2319-0002-BH1 Logs

(stack 5)



at stake 6



BOREHOLE LOG BIC-26

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine		DRILLING METHOD: Hollow Stem Auger	BORING NO.
Transect 3		SAMPLING METHOD: Split Spoon	SHEET PAGE OF DRILLING
NORTHING		WATER LEVEL	START FINISH
EASTING:		TIME	10:30 1:00
DRILL RIG: ANGLE: 90		DATE	DATE
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.		CASING DEPTH	
BEARING: -		SURFACE CONDITIONS:	

completed
cleaning
augers 1:20

about 1" of sluf
side of mud
sample is
in side
slightly
from auger

DEPTH IN METERS (ELEVATION)	BLOW 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, MCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ CEMENTATION ⁴	PLASTICITY (mp, l, m, h)	OTHER TESTS ⁵
5					on top of drill pad cut - alluvium - 10YR 4/3 CoSL (15% clay) 1" thick over a few feet of 10YR 2 1/2 SCL (22% clay) S-6.5' 10YR 4 1/4 CoLS/sand	0	20				FR	SO PO	
10	10 30 30	100	Qal		Wet, NE, grl subrounded to subangular, some red grl (saturated)	0	30					SO PO	
15	25 25 26		Qal		10-11' 10YR 4 1/4 CoLS Wet, NE (saturated)	0	30					SO PO	
20	30 30	100	Qal		15-16' 10YR 4 1/4 CoSL Wet, NE, 1/2" largest grl, grl subrounded/ subangular.	0	40					SS SP	
25				20'	pH of water = 4.0								
30	30	100			20-20.5 (upper 1' all sluf). 5YR 4 1/3 SC, (clay?). NE, sh moist	0	S				VH	S P	
35	30	100			25-25.5' 2.5YR 4 1/6 SCL (34% clay), NE slightly moist.	0	15				H	S P	
40	50	100		30'	30-30.4' 7.5YR 5 1/4 CoSL, weathered diorite, dry mainly broken rock						VH	SS SP	
	50	100			35-35.4' weathered diorite - like 30-30.4'						VH		

DRILLING CONTRACTOR: Layne-Weather (Chandler, AZ)

LOGGED BY: Dave Bucher

DATE: 2/1/05

JOB NO 043-2319-0002

FILE NAME: 043-2319-0002-BH Logs

- Notes:
- ¹ Percent > 3/16 in.
 - ² Sum of gravel, sand, and fines = 100%
 - ³ For cohesive soil: soft, soft, firm, hard, v. hard.
 - ⁴ For noncohesive soil: weak, moderate, strong.
 - ⁵ Pocket penetrometer, torvaens, in situ density, etc.

water S ≈ 20'

boring hole about 5' below original ground level
pH of mud between 5-10' = 4 or less,
(scale only goes to 4)

Samples BIC-26 5-6.5'
10-11'
15-16'
20-20.5'
20-30.4'
35-35.4'

TD 35'

- Sample spoon is
plating copper



BOREHOLE LOG B16-27

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine

DRILLING METHOD: Hollow Stem Auger

BORING NO.

Transect 3

SAMPLING METHOD: Split Spoon

SHEET

PAGE OF

DRILLING

START FINISH

NORTHING

EASTING:

WATER LEVEL

TIME

DATE

CASING DEPTH

SURFACE CONDITIONS:

DRILL RIG:

ANGLE: 90

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

BEARING: -

DEPTH IN METERS (ELEVATION)	BLOW 8 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, MCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
10	10	100			5-6.5 10YR 3/4 CL (35% clay) moist NE	0	5				FR	S	
20	20				bottom 2" 7.5YR 4/3 SL (18% clay) moist	0	10						
10	20	100			10-11.5' 10YR 4/4 SCL, 30% clay, moist, NE	0	10	60			FR	S	
20	30				bottom 1" 7.5YR 5/4 CoSL 12% clay.								
21	100		Ral		15-16.5' 10YR 5/4 CoSand	0	40				soft	SO	
23	20				Wet, NE, grl are subrounded/subangular							PO	
20					pH of mud = 4.5 pH of water = 4.0								
25	100				20-20.5' (upper 1' slab)	6	30				MH	SO	
40					7.5YR 4/4 CoSL & 7.5YR 4/4 SCL (30% clay)		5				MH	S	
30	100				moist, NE							P	
30	100				7.5YR 5/4 CoSL (15% clay)	0	10				MH	SS	
35					moist, NE							SP	
25-25.5'													
30-31	100				2.5YR 4/6 CL (35% clay) SL moist, NE	0	5				MH	S	
	50				over weathered diorite at 30.5'						H		
					36-35.5 2.5YR weathered diorite						VH		
					dry								

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torvane, in situ density, etc.

water 15-20'

in drill
pad cutbore hole about 5' below original
ground level

0-1.5' 10YR 4/3 CoSL (12% clay) 30% grl
 1.5-2.5' 10YR 2/1 SCL (28% clay) 5% grl
 2.5-5' 10YR 2/2 CL (35% clay) 10% grl

4:00-5:00
 tripped,
 washed augers,
 moved rig to
 next site.

DRILLING CONTRACTOR: Layne-Wedem (Chandler, AZ)

LOGGED BY: Dave Buscher

DATE: 2/1/05

JOB NO. 043-2319-0002

FILE NAME: 043-2319-0002-BH1 Logs



BOREHOLE LOG BIC-28

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine <i>Transsect 3</i>		DRILLING METHOD: Hollow Stem Auger	BORING NO.
NORTHING		SAMPLING METHOD: Split Spoon	SHEET PAGE OF DRILLING
EASTING:		WATER LEVEL	START FINISH
		TIME	DATE DATE
		DATE	
		CASING DEPTH	
DRILL RIG: ANGLE: 90 BEARING: -		SURFACE CONDITIONS:	
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.			

DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ /CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5					0-6.5' - no sample Qal. water @ 5'								
10	10 15 34	100			10-11.5 10-11 7.5YR 4/4 CoSL, 13% clay, wet NE: 11-11.5 10YR 4/4 SCL, 25% clay, NE moist. water pH = 4	0	15				FR	SS PO	
15	13 13 15	100			15-16.5' lenses of 10YR 4/4 CoSL, wet NE + 7.5YR 4/4 clay (40% clay) NE slightly moist	0	20				soft	SO PO	
20					20-21' 7.5YR 4/6 SCL, 25% clay, NE moist.	0	0				H	VS VP	
25	35 35	100			25-26.5' 7.5 YR 5/4 CoSL, wet with layers of 7.5YR 3/3 SCL, 32% clay, NE moist	0	5				MH	S P	
30	21 20 23				30-30.4' rock at bottom (diorite) + some 7.5YR 4/4 SCL 35% clay, moist	0	40				soft	SO PO	
35						0	5				MH	S P	
40	50 60	80 100			35-35.4' deep red weathered diorite, dry- very hard	50	5				MH	S P	
											VH	—	

- Notes:
- Percent > 3 inch.
 - Sum of gravel, sand, and fines = 100%
 - For cohesive soil: soft, firm, hard, v. hard.
 - For noncohesive soil: weak, moderate, strong.
 - Pocket penetrometer, torevane, in situ density, etc.

TD = 35'

Samples: BIC-28 10-11.5'
15-16.5'
20-21'
25-26.5'
30-30.4', 35-35.4'

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

LOGGED BY: Dave Buscher

FILE NAME: 043-2319-0002-BH Logs

JOB NO. 043-2319-0002

DATE: 2/2/05

- Boring is about 5' below original surface



BOREHOLE LOG BIC-29 ≈ 10' N of BIC-18

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine		DRILLING METHOD: Hollow Stem Auger	BORING NO. <u>BIC-29</u>
Transsect 3		SAMPLING METHOD: Split Spoon	SHEET PAGE OF DRILLING
NORTHING	EASTING:	WATER LEVEL	START FINISH
		TIME	DATE DATE
		DATE	
		CASING DEPTH	
DRILL RIG: ANGLE: 90		SURFACE CONDITIONS:	
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.			

DEPTH IN METERS (ELEVATION)	BLOW 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ CEMENTATION ⁴	PLASTICITY (mp, l, m, h)	OTHER TESTS ⁵
13	45	60	Qal		at 10' pulled augers up + was slightly moist at bottom of auger - Qal								
15	45				15-16.5 upper 6" 7.5YR 5/4 SCL Saturated 25% clay in bottom 6" 10YR 3/3 Clay, 45% clay, sl. moist	0	5					S	P
20	10	100	?		20-21.5 7.5YR 4/4 + 5YR 4/4 SCL 40% clay, moist	0	0				H	VS	VP
25	15										MH	S	P
26	20												
28	10	100			25-26.5 5YR 4/4 SCL 38% clay, moist, bottom 2" pulverized diorite	0	10				MH	S	P
30	20												
35	43												
38	25	100			30-31.5 2.5YR 4/6 SCL 28% clay, moist, with diorite fragments within the soil	0	15				FR	S	P
40	25												
42	30												
45	50	6"			35-36 2.5YR 4/6 Clay (40% clay), moist	0	5				H	VS	VP
48	50	5"			35-35.5' 35.6-36' hard, weathered diorite, dry						VH	-	

Notes:

- Percent > 3inch.
- Sum of gravel, sand, and fines = 100%
- For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.
- For noncohesive soil: weak, moderate, strong.
- Pocket penetrometer, torvane, in situ density, etc.

boring hole is ≈ 2' below original surface

Samples: BIC-29
15-16.5'
20.-21.5'
25-26.5'
30-31.5'
35-36'

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

LOGGED BY: Dave Buscher

FILE NAME: 043-2319-0002-BH Logs

JOB NO 043-2319-0002

DATE: 2/2/05

small amt. of water did not bring any mud to surface

hit water at ≈ 12' pushing rock

between 31.5 + 35' thin layer of water, saturated mud - 2.5Y 4/6 SCL

brought up some red mud to surface

- tripping out of hole, last auger wet + caked in red mud

- pH of red mud = 6.0



BOREHOLE LOG

BIC-30

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine

DRILLING METHOD: Hollow Stem Auger

BORING NO.

Transect 3
~35' N of BIC-15

SAMPLING METHOD: Split Spoon

SHEET
PAGE OF
DRILLING

NORTHING

EASTING:

WATER LEVEL	TIME	DATE	CASING DEPTH	START	FINISH
				4:00	5:30

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	10 15 30	100			5-6.5' 10YR3/3 SCL 35% clay, NE moist grl subrounded/subangular	0	10				FR	SP	
10	10 30 40	100	Qal	15	10-11.5' 10YR4/4 COSL, moist, NE, grl are subrounded.	0	60				FR	SP	
15	10 27 30	60	red soil	20	15-16.5' 2.5YR4/6 SC (38% clay) moist, NE, few diorite frags.	0	5						
20	15 45 50	100			20-21' deep red weathered diorite 2.5YR4/8 SCL, 22% clay, sl. moist/dry						VH		
25			diorite		25-25.5' grayish- white weathered diorite, dry						VH		
30	60												
35													
40													
					TD 25'								

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

top of boring ≈ 1 1/2' below original surface

total 120' in day

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

LOGGED BY: Dave Buscher

DATE: 2/2/05

JOB NO. 043-2318-0002

FILE NAME: 043-2318-0002-BH Logs



BOREHOLE LOG

BIC-31

SITE NAME AND LOCATION: 1C Stockpile Reclaim Area, Tyrone Mine <i>Transect</i>		DRILLING METHOD: Hollow Stem Auger	BORING NO.		
NORTHING: EASTING:		SAMPLING METHOD: Split Spoon	SHEET PAGE OF DRILLING		
		WATER LEVEL	START	FINISH	
		TIME	8:00	9:00	
		DATE	DATE	DATE	
DRILL RIG: ANGLE: 90 BEARING: - SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.		SURFACE CONDITIONS:			

DEPTH IN METERS (ELEVATION)	BLOW 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	15				5-6.5' 10YR 4/3 CoSL, NE, slightly moist. gr/one subrounded/ subangular	0	40				H	SO PO	
10	25				10-10.4' 10YR 4/4 CoLS weathered diorite. mainly broken rock. dry, NE						VH	SO PO	
15	30	50	100		15-15.5' broken diorite (like 10-10.4'), dry						1/2H	SO PO	
20					TD 15'								
25													
30													
35													
40													

Notes:

¹ Percent > 3inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

boring close to original surface level

photo 54 samples: BIC-31 5-6.5'

photo 55 of cut showing 10-10.4'

possible North drainage. 15-15.5'

7:30 drillers
moved rig
& filled
water
tank

10: drillers
deconing
truck, auger
I left at
10:00

DRILLING CONTRACTOR: Layne-Western (Chandler, AZ)

LOGGED BY: Dave Buscher

DATE: 2/3/05

JOB NO. 043-2319-0002

FILE NAME: 043-2319-0002-BH Logs

TABLE V-1
1A-1B Liquefaction Assessment

All Qal SPT Data, 1A and 1B Leach Stockpile
Corrected N Value (As per Youd et al, 1996 and 1998 NCEER Workshops)

Qal moist unit weight 120 PCF
Qal sat unit weight 130 PCF

Borehole dia correction 1.05 (Cb) Youd et al
Rod length corr 0.8 (Cr) Youd et al
Energy Ratio 1 (Ce) Youd et al
Sampling Method Corr 1 (Cs) Youd et al
Correction Product 0.84 (Less Ovb corr, Column J)
Max surface Accel 0.14 g USGS Unified Hazard Tool

Site Class (760 m/sec)
30% Amplification

No. 1A-1B
Percent Fines 10 %
Max Quake Magnitude 6.7 URS Seismicity Study (2017)
Mag Scale Factor 1.234426 Idriss and Boulanger (2008)
Regrade Burial Depth 0 Feet
Surcharge Pressure 0 PSF at surface

Hole	Depth to Water (bgs)	Sample Depth	Material	Blows/ft (uncorr)	Depth below gws	u (pcf)	Total Stress	Effective Stress	Cn (ovb) Correction (Youd eqn 9)	N1(60) BPF (Youd eqn 8)	rd (Youd eqn 2a)	CSR (Youd eqn 1)	CRR (LIQFAC Lookup)	FS (Youd eqn 23)
GA-05-10	12	5	Fill	22	0	0	600	600	1.70	31.42	0.99	0.090	2.4689	NL
GA-05-10	12	10	Fill	7	0	0	1200	1200	1.29	7.59	0.98	0.089	0.1506	NL
GA-05-10	12	15	Qal	54	3	187.2	1830	1642.8	1.10	50.05	0.97	0.098	2.4689	NL
GA-05-10	12	20	Qal	61	8	499.2	2480	1980.8	1.00	51.49	0.95	0.109	2.4689	NL
GA-05-10	12	25	Qtg	50	13	811.2	3130	2318.8	0.93	39.01	0.94	0.116	2.4689	NL
GA-05-11	27.5	5	Fill	13	0	0	600	600	1.70	18.56	0.99	0.090	0.3172	NL
GA-05-11	27.5	10	Fill	9	0	0	1200	1200	1.29	9.76	0.98	0.089	0.1790	NL
GA-05-11	27.5	15	Fill	13	0	0	1800	1800	1.05	11.51	0.97	0.088	0.2061	NL
GA-05-11	27.5	20	Fill	17	0	0	2400	2400	0.91	13.04	0.95	0.087	0.2358	NL
GA-05-11	27.5	25	Fill	24	0	0	3000	3000	0.82	16.46	0.94	0.086	0.2777	NL
GA-05-11	27.5	30	Qtg	50	2.5	156	3625	3469	0.76	31.89	0.93	0.088	2.4689	NL
GA-05-11	27.5	35	Qtg	50	7.5	468	4275	3807	0.72	30.44	0.92	0.094	2.4689	NL
GA-05-12	36	5	Fill	13	0	0	600	600	1.70	18.56	0.99	0.090	0.3172	NL
GA-05-12	36	10	Fill	15	0	0	1200	1200	1.29	16.27	0.98	0.089	0.2777	NL
GA-05-12	36	15	Fill	7	0	0	1800	1800	1.05	6.20	0.97	0.088	0.1383	NL
GA-05-12	36	20	Fill	21	0	0	2400	2400	0.91	16.10	0.95	0.087	0.2777	NL
GA-05-12	36	25	Qal	57	0	0	3000	3000	0.82	39.09	0.94	0.086	2.4689	NL
GA-05-12	36	30	Qtg	50	0	0	3600	3600	0.75	31.30	0.93	0.085	2.4689	NL
GA-05-12	36	35	Qtg	50	0	0	4200	4200	0.69	28.98	0.92	0.084	2.4689	NL
1B-1	32.7	24		27	0	0	2880	2880	0.83	18.90	0.94	0.086	0.3172	NL
1B-1	32.7	29		50	0	0	3480	3480	0.76	31.84	0.93	0.085	2.4689	NL
1B-1	32.7	34		50	1.3	81.12	4093	4011.88	0.71	29.65	0.92	0.085	2.4689	35.7
1B-1	32.7	39		50	6.3	393.12	4743	4349.88	0.68	28.48	0.91	0.090	2.4689	33.8
1B-5	24.4	9	Qal	13	0	0	1080	1080	1.36	14.86	0.98	0.089	0.2494	NL
1B-5	24.4	14	Qal	28	0	0	1680	1680	1.09	25.66	0.97	0.088	0.7777	NL
1B-5	24.4	19	Qal	50	0	0	2280	2280	0.94	39.34	0.96	0.087	2.4689	NL
1B-5	24.4	24	Qal	50	0	0	2880	2880	0.83	35.00	0.94	0.086	2.4689	NL
1B-5	24.4	29	Qtg	50	4.6	287.04	3526	3238.96	0.79	33.00	0.93	0.092	2.4689	NL
1B-6	23.5	9		23	0	0	1080	1080	1.36	26.29	0.98	0.089	0.9875	NL
1B-6	23.5	14		22	0	0	1680	1680	1.09	20.16	0.97	0.088	0.3605	NL
1B-6	23.5	19		22	0	0	2280	2280	0.94	17.31	0.96	0.087	0.2975	NL
1B-6	23.5	24		50	0.5	31.2	2885	2853.8	0.84	35.16	0.94	0.087	2.4689	NL
1B-6	23.5	29	Qtg	50	5.5	343.2	3535	3191.8	0.79	33.25	0.93	0.094	2.4689	NL
1B-6	23.5	34	Qtg	50	10.5	655.2	4185	3529.8	0.75	31.61	0.92	0.099	2.4689	NL
1B-7	23.5	19		56	0	0	2280	2280	0.94	44.06	0.96	0.087	2.4689	NL
1B-7	23.5	24		50	0.5	31.2	2885	2853.8	0.84	35.16	0.94	0.087	2.4689	NL

TABLE V-1
1A-1B Liquefaction Assessment

**All Qal SPT Data, 1A and 1B Leach Stockpile
Corrected N Value (As per Youd et al, 1996 and 1998 NCEER Workshops)**

Qal moist unit weight		120 PCF														
Qal sat unit weight		130 PCF														
Borehole dia correction		1.05 (Cb)		Youd et al												
Rod length corr		0.8 (Cr)		Youd et al												
Energy Ratio		1 (Ce)		Youd et al												
Sampling Method Corr		1 (Cs)		Youd et al												
Correction Product		0.84 (Less Ovb corr, Column J)														
Max surface Accel		0.14 g		USGS Unified Hazard Tool		Site Class (760 m/sec)										
		No. 1A-1B				30% Amplification										
Percent Fines		10 %														
Max Quake Magnitude		6.7		URS Seismicity Study (2017)												
Mag Scale Factor		1.234426		Idriss and Boulanger (2008)												
Regrade Burial Depth		0 Feet														
Surcharge Pressure		0 PSF at surface														
									Cn (ovb)		N1(60)					
									Correction		BPF		rd		CSR	CRR
									(Youd eqn 9)		(Youd eqn 8)		(Youd eqn 2a)		(Youd eqn 1)	(LIQFAC Lookup)
																(Youd eqn 23)
Hole	Depth to	Sample	Material	Blows/ft	Depth below	u	Total	Effective								
	Water (bgs)	Depth		(uncorr)	gws	(pcf)	Stress	Stress								
1B-7	23.5	29		50	5.5	343.2	3535	3191.8	0.79		33.25	0.93		0.094	2.4689	N1



DRAFT

BOREHOLE LOG GA-05-01

SITE NAME AND LOCATION: name and location:

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET

1 of 2

DRILLING

START FINISH

NORTHING:

EASTING:

DATUM: amsl

ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DATE DATE

DRILL RIG:

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	4	0	fill	NO Blows tube Shelby tube failed (crack)	Waste Rock, gravel, sand Cobbles, light reddish brown sand, silty, fine to med, reddish brown, silty						2.5% 4/4	Soft M	NP PH 4.5 PH 7.0
10	2 9 17	0 100 100	fill	110- 115	Geo fabric Quartzite gravel, silty, dark reddish SPG Waste rock, gravel, sand, silty						Eye firm 4/2		PH 4.5
15	3 3 3										Soft		PH 4.0
20	1 5 5										Soft		PH 4.0
25			PAL ?		Flowing sand, coarse, sub angular, brown						7.5% 4/3		

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR

LOGGED BY:

DATE:

JOB NO.

FILE NAME:



DRAFT

BOREHOLE LOG GA-05-01

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET 2 of 2

DRILLING

START FINISH

NORTHING
DATUM: amslEASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DATE

DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS
25	12		QAL		Sand, A.A.					10 yr	hard		PH
	15		Qig		Drilling much trouble.					7/3			Y.O.
					Sand, gravelly, silty								
					yellowish brown								
30	30				TD: 30 ft					10 yr	hard		PH
	34									6/4			Y.O.

DRILLING CONTRACTOR QSI

LOGGED BY: Clay Sturze

DATE: 9-20-05

JOB NO.

FILE NAME:

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.



DRAFT

BOREHOLE LOG GA-05-02

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger		BORING NO.	
NORTHING DATUM: amsl		EASTING: ELEVATION:		SHEET 1 of 1	
				DRILLING	
				START	
				FINISH	
DRILL RIG: ANGLE: 90 SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.		WATER LEVEL		DATE	
		TIME		DATE	
		CASING DEPTH		DATE	
		SURFACE CONDITIONS:		DATE	

DEPTH IN METERS (ELEVATION)	BLOW/ 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	9 11 9	0 0 0	fill		Waste Rock, sand, silty, pink					5/8 7/4	firm	NP	PH: 6.5
10	5 1 1	0 0 20	fill		Gravel (Quartzite) Sandy Gravel Void 11 ft to 12 ft (PLS Line?)								
15	24 50+		Qal Qty		Shelby sand, silty, angular, clastic breccia, saturated Gravel, sandy, silty very pale brown					7.5/8 4/3	soft	M	PH: 4.0
20	50+				TD: 20					10/16 7/4	hard	NP	PH: 4.0

Notes:

¹ Percent > 3inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

GSI

DRILLING CONTRACTOR

LOGGED BY: Clay Kinnear

DATE: 5-22-05

JOB NO.

FILE NAME:



DRAFT

BOREHOLE LOG GA-05-04

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET

1 of 2

DRILLING

START FINISH

NORTHING
DATUM: amslEASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DATE

DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY/ CEMENTATION ³	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
5	10	100			Fill								
10	10	100			Sand, silt, gravelly, sl. moist					5 yr 4/6	Soft	L	PH 5.5
15	10	100			Gravel, silt, sand, sl. moist					5 yr 4/6	Firm	N	PH 5.5
20	10	100			Gravel, sandy, silty, clay red, v. moist					2.5 4/6 3/6			PH 4.0
25	10	100			Gravel, sandy, silty dark red, moist					2.5 4/6 3/6			PH 4.5

Saturated
→
stringsShelby
tube
11-17

DRILLING CONTRACTOR GSI

LOGGED BY: C. G. Kilmer

DATE: 9-2-05

JOB NO.

FILE NAME:

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.



DRAFT

BOREHOLE LOG *QA-05-04*

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger		BORING NO.			
				SHEET <i>2 of 2</i>			
		SAMPLING METHOD: SPT, Shelby					
				DRILLING			
NORTHING DATUM: amsl		EASTING: ELEVATION:		START		FINISH	
				WATER LEVEL			
				TIME			
				DATE			
DRILL RIG: ANGLE: 90 SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.		SURFACE CONDITIONS:		DATE		DATE	

DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ³	% FINES ²	COLOR	CONSISTENCY ⁴ / CEMENTATION ⁵	PLASTICITY (np, l, m, n)	OTHER TESTS ⁶
25	12 18 11	0 0 100	Fill gtg		Sand, gravelly, medium, sl. moist, reddish yellow Drilling muds (w/ce gravel, sandy, yellow sl. moist TD: 30 ft					7.5 ye 6/6		pH: 4.0	
30	42 50	0 100 100								100 7/6		pH 4.0	
35													

Notes:

- ¹ Percent > 3inch.
- ² Sum of gravel, sand, and fines = 100%
- ³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.
- ⁴ For noncohesive soil: weak, moderate, strong.
- ⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR *GSI*LOGGED BY: *Clay Kilmer*DATE: *9-21-05*JOB NO.
FILE NAME:

Golder Associates

*DENNISON sample: PG 870 Scranton PA Bulfinch
717-585-2061*



**Golder
Associates**

DRAFT

BOREHOLE LOG GA-05-05

SITE NAME AND LOCATION: name and location

DRILLING METHOD: ~~Hollow Stem Auger~~

BORING NO.

Tube

SHEET 1 of 1

SAMPLING METHOD: SPT, Shelby

DRILLING

START FINISH

NORTHING
DATUM: amsl

EASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DATE DATE

5-22-05

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5					No Sampling, previous observations at Test Pit 8 8-19-05								
10					5-14: Boulders (waste rock)								
15					Sand, silty, plastic								
20					Shelby Punk Bacon, Salinated								
25					Gila Sand, coarse, gravelly, silty, reddish yellow								
					TD Drilled - 22								

Notes:

¹ Percent > 3in.

² Sum of gravel, sand, and fines = 100%

³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.

⁴ For noncohesive soil: weak, moderate, strong.

⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR GSI

LOGGED BY: Clay Kilmer

DATE: 9-22-05

JOB NO.

FILE NAME:

992-3068 - Annalie Jones
538-7181 - Janas K.

DRAFT																																																																																																													
Golder Associates		BOREHOLE LOG GA-05-06																																																																																																											
SITE NAME AND LOCATION: name and location NORTHING DATUM: amsl EASTING: ELEVATION: DRILL RIG: ANGLE: 90 BEARING: - SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.				DRILLING METHOD: Hollow Stem Auger				BORING NO.																																																																																																					
				SAMPLING METHOD: SPT, Shelby				SHEET 1 of 1																																																																																																					
								DRILLING																																																																																																					
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				DATE				DATE DATE																																																																																																					
				CASING DEPTH																																																																																																									
				SURFACE CONDITIONS: Road bed in fill																																																																																																									
<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th style="width:10%;">DEPTH IN METERS (ELEVATION)</th> <th style="width:5%;">BLOW/6 IN.</th> <th style="width:5%;">RECOVERY</th> <th style="width:5%;">SYMBOL</th> <th style="width:5%;">SAMPLE NO.</th> <th style="width:30%;">SAMPLE NUMBER AND DESCRIPTION OF MATERIAL <small>(i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)</small></th> <th style="width:5%;">% OVERSIZE¹</th> <th style="width:5%;">% GRAVEL²</th> <th style="width:5%;">% SAND²</th> <th style="width:5%;">% FINES²</th> <th style="width:5%;">COLOR</th> <th style="width:5%;">CONSISTENCY³/ CEMENTATION⁴</th> <th style="width:5%;">PLASTICITY (np, l, m, h)</th> <th style="width:5%;">OTHER TESTS⁵</th> </tr> </thead> <tbody> <tr> <td></td> <td></td> <td></td> <td style="text-align: center;">fill</td> <td></td> <td>Fill sand, gravelly silty, reddish brown sl. moist</td> <td></td> <td></td> <td></td> <td></td> <td>5/8 fine</td> <td>L</td> <td>PH: 7.0</td> <td></td> </tr> <tr> <td>5</td> <td>18 13 19</td> <td>0 75 100</td> <td style="text-align: center;">Qal</td> <td></td> <td>Sand, silty, gravelly, yellowish red, sl. moist</td> <td></td> <td></td> <td></td> <td></td> <td>5/8 fine</td> <td>L</td> <td>PH: 7.0</td> <td></td> </tr> <tr> <td>10</td> <td>13 23 20</td> <td>0 20 100</td> <td></td> <td></td> <td>Sand, gravelly, coarse silty, dark yellowish brown, sl. moist</td> <td></td> <td></td> <td></td> <td></td> <td>10/16 v. hard</td> <td>L</td> <td>PH: 7.0</td> <td></td> </tr> <tr> <td>15</td> <td>23 20</td> <td>50 100</td> <td style="text-align: center;">Q_g</td> <td></td> <td>Drilling v. hard sand, coarse, gravelly, pale yellow, dry</td> <td></td> <td></td> <td></td> <td></td> <td>2.5 yr 8/8 v. hard</td> <td>L</td> <td>PH: 7.0</td> <td></td> </tr> <tr> <td>20</td> <td></td> <td></td> <td></td> <td></td> <td>TD: 15 ft</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>25</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> </tbody> </table>												DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL <small>(i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)</small>	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵				fill		Fill sand, gravelly silty, reddish brown sl. moist					5/8 fine	L	PH: 7.0		5	18 13 19	0 75 100	Qal		Sand, silty, gravelly, yellowish red, sl. moist					5/8 fine	L	PH: 7.0		10	13 23 20	0 20 100			Sand, gravelly, coarse silty, dark yellowish brown, sl. moist					10/16 v. hard	L	PH: 7.0		15	23 20	50 100	Q _g		Drilling v. hard sand, coarse, gravelly, pale yellow, dry					2.5 yr 8/8 v. hard	L	PH: 7.0		20					TD: 15 ft									25													
DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL <small>(i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)</small>	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵																																																																																																
			fill		Fill sand, gravelly silty, reddish brown sl. moist					5/8 fine	L	PH: 7.0																																																																																																	
5	18 13 19	0 75 100	Qal		Sand, silty, gravelly, yellowish red, sl. moist					5/8 fine	L	PH: 7.0																																																																																																	
10	13 23 20	0 20 100			Sand, gravelly, coarse silty, dark yellowish brown, sl. moist					10/16 v. hard	L	PH: 7.0																																																																																																	
15	23 20	50 100	Q _g		Drilling v. hard sand, coarse, gravelly, pale yellow, dry					2.5 yr 8/8 v. hard	L	PH: 7.0																																																																																																	
20					TD: 15 ft																																																																																																								
25																																																																																																													

Notes:

- ¹ Percent > 3 inch.
- ² Sum of gravel, sand, and fines = 100%
- ³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.
- ⁴ For noncohesive soil: weak, moderate, strong.
- ⁵ Pocket penetrometer, torevane, in situ density, etc.

GSE
 DRILLING CONTRACTOR
 LOGGED BY: Clay Kilmer
 DATE: 9-26-05
 JOB NO.
 FILE NAME:



DRAFT

BOREHOLE LOG GA05-08

SITE NAME AND LOCATION: name and location	DRILLING METHOD: Hollow Stem Auger	BORING NO.
		SHEET 1 of 2
	SAMPLING METHOD: SPT, Shelby	DRILLING
		START FINISH
NORTHING	EASTING:	DATE
DATUM: amsl	ELEVATION:	DATE
	WATER LEVEL	
	TIME	
	DATE	
	CASING DEPTH	
DRILL RIG:	SURFACE CONDITIONS: Road Bed in natural material	
ANGLE: 90	BEARING: -	
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.		
1 waste base fill		

DEPTH IN METERS (ELEVATION)	BLOW/ 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ³	% FINES ³	COLOR	CONSISTENCY/ CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	9	0			Fill, Sand, silty, gravelly, dark reddish brown sl. moist						5/12 Soft L	PH: 7.0	
10	4	100			Fill, sand, silty, gravelly, dark brown, sl moist						7.5/12 Soft L	PH: 6.0	
15	6	0			Fill, sand, gravelly, silty, light brown, sl. moist						7.5/12 Firm N	PH 5.0	
20	6	0			Fill, Sand, gravelly, Silty, reddish yellow sl. moist						5/12 hard N	PH: 4.5	
25	9	60											

Notes:

¹ Percent > 3inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR

LOGGED BY:

DATE:

JOB NO.

FILE NAME:

992-3010

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET 2 of 2

NORTHING
DATUM: amsl

EASTING:
ELEVATION:

WATER LEVEL									
TIME									
DATE									
CASING DEPTH									

DRILLING
START FINISH

DATE 9-20 DATE

DRILL RIG:
ANGLE: 90
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

BEARING: -

SURFACE CONDITIONS:

DEPTH IN METERS (ELEVATION)	BLOW 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY/ CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS
25	9	0	fill		Gravel, sandy, silty, pink	75				fin	NP	PH: 4.0	
29	11	100			Sl. moist	75				fin	NP	PH: 4.0	
30	10		fill		Gravel, sandy, silty,	54				hard	LP	PH: 4.0	
	18				reddish brown, saturated	54							
	28		ptg										
35	30				Gravel, sandy, silty,	75				hard	NP	PH: 4.0	
					moist, reddish yellow	42							
					TD:35	6/6							

- Notes:
- ¹ Percent > 3inch.
 - ² Sum of gravel, sand, and fines = 100%
 - ³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.
 - ⁴ For noncohesive soil: weak, moderate, strong.
 - ⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR GSI

LOGGED BY: Clay Kilmer

DATE: 9-20-05

JOB NO.

FILE NAME:



DRAFT

BOREHOLE LOG GA-05-09

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET

1 of 2

DRILLING

START FINISH

NORTHING
DATUM: arnsiEASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DATE

DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCl reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5					Fill, waste rock, gravel, sand, silt								
10													
15													
20	19	100			gravel, sandy, silty pale brown, st. moist					gray fine	firm	L	PH _v 4.0
	21	100											
	500	100											

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

GSI

DRILLING CONTRACTOR

LOGGED BY: Clay Kimer

DATE: 9-21-05

JOB NO.

FILE NAME:



DRAFT

BOREHOLE LOG GA-05-10

SITE NAME AND LOCATION: name and location

GA-05-10

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET 1 of 2

NORTHING
DATUM: amslEASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DRILLING

START FINISH

DATE DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY/ CEMENTATION ³	PLASTICITY (no. l. m. n)	OTHER TESTS ⁴
0	6	100			Fill, silty, clayey sandy loam stonewall base, strong brown, silty					10% 1/6			L PH 5.5
5	11	100			Fill sand, silty, med, angular dark brown					10% 3/4			M PH 5.0
10	3	100			Fill, roots rock, sand, gravelly, coarse, angular PLS salination - - - tools plating out in copper					10% 7/6			N PH 4.0
15	4	70			Drilling hard sand, gravelly, coarse, sub rounded					7.5% 6/3			N PH 4.0
20	16	5			Gravel, sandy, coarse pink					7.5% 6/3			N
25	32	100								7.5% 7/4			

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR Golder Associates SW

LOGGED BY: Clay Kilmer

DATE: 8-20-05

JOB NO.

FILE NAME:



DRAFT

BOREHOLE LOG GA-05-11

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET 2 of 2

NORTHING
DATUM: amslEASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DRILLING
START FINISH

DATE DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN METERS (ELEVATION)	BLOW/ 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY/ CEMENTATION ³	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
25	6 10 14	0 50 100			waste rock, gravel, sand, silt, brown, 5% moist					7.5 6/3	firm	L	PH 4.5
30	33 50	0 20 100			waste rock, gravel, sand, silt, brown, saturated					7.5 6/3	firm	N	PH 4.0
35	17 54	0 25 100			Sand, silty, brown fine gravel, light grey moist					10gr 7/2	hard	N	PH 4.0
40					Sand, gravelly, light grey TD: 40					10gr 7/2	hard	N	PH 5.5-6.0

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR GSI

LOGGED BY Clay Kilgus

DATE: 9-20-05

JOB NO.

FILE NAME:



DRAFT

BOREHOLE LOG GA-05-12

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger		BORING NO.	
		SAMPLING METHOD: SPT, Shelby		SHEET 1062	
		WATER LEVEL		DRILLING	
		TIME		START FINISH	
NORTHING		EASTING:		DATE	
DATUM: amsl		ELEVATION:		DATE	
DRILL RIG:		SURFACE CONDITIONS: Road bed in fill			
ANGLE: 90		BEARING: -			
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.					

DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ /CEMENTATION ⁴	PLASTICITY (np, l. m. h.)	OTHER TESTS ⁵
5	10 80 5	0 70 100			Sandy, gravelly, silty Reddish brown					5/8 3/3	Firm	L	PH: 7.0
10	8 7	0 50 100			Sand, gravelly, silty dark reddish brown sl. moist					5/8 3/3	Firm	N	PH: 6.5
15	3 2 5	0 50 80			Sand, gravelly, silty brown, sl. moist					7.5 4 5/4	Hard	N	PH: 5.0
20	11 10 11	0 80 100			Sand, silty, gravelly Reddish brown, sl. moist					5/8 5/3	Soft	I	PH: 4.5
25					Waste rock, gravel, sandy,					2.5 4 3/6	Firm	NP	PH: 5.0

Notes:

- Percent > 3inch.
- Sum of gravel, sand, and fines = 100%
- For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.
- For noncohesive soil: weak, moderate, strong.
- Pocket penetrometer, torevane, in situ density, etc.

GS

DRILLING CONTRACTOR

LOGGED BY: Clay Silver

DATE: 9-23-05

JOB NO.

FILE NAME



DRAFT

BOREHOLE LOG GA-05-12

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger		BORING NO.	
				SHEET 2062	
		SAMPLING METHOD: SPT, Shelby		DRILLING	
				START FINISH	
NORTHING		EASTING:		WATER LEVEL	
DATUM: amsl		ELEVATION:		TIME	
				DATE	
				CASING DEPTH	
DRILL RIG:		SURFACE CONDITIONS:		DATE	
ANGLE: 90				DATE	
BEARING: -					
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.					

DEPTH IN METERS (ELEVATION)	BLOW 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ³	% FINES ³	COLOR	CONSISTENCY ⁴ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS
25	22	0	fill		Waste rock gravel,					2.5	hard	NP	PH: 5.0
	23	40	Gal		Sand, silty, red					4/6			
	34	100			Gravel, rounded					5/6			
			ftg		27' out of gravel -								
					drilling much harder								
30	35	0			Sandy, gravelly, silty					1/6	hard	NP	PH: 4.0
	36	50			brownish yellow, moist					4/6			
		100											
35	50	0			Sand, silty, gravelly,					1/6	hard	NP	PH: 4.0
		8			brownish yellow, moist					4/6			
		100											
					TD: 35 ft								

Notes:

¹ Percent > 3in.

² Sum of gravel, sand, and fines = 100%

³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.

⁴ For noncohesive soil: weak, moderate, strong.

⁵ Pocket penetrometer, torevane, in situ density, etc.

GSI

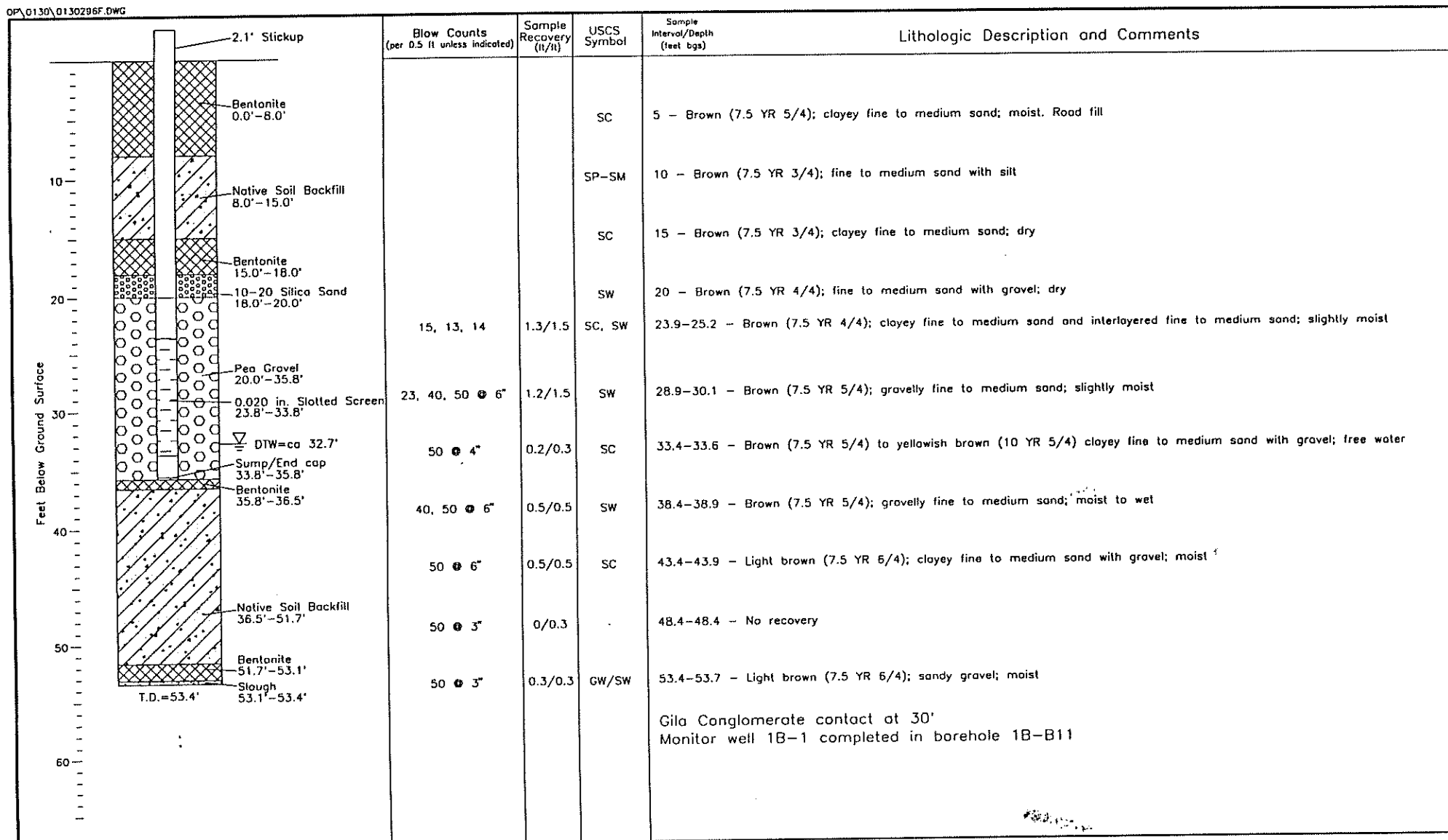
DRILLING CONTRACTOR

LOGGED BY: Clay/Titus

DATE: 9-23-05

JOB NO.

FILE NAME:



Explanation

	Native backfill		Slough
	Bentonite seal		Screen
	Gravel pack	ca=	Approximately
	Silica sand		

Geologist: AP
 Driller: Layne Environmental Drilling, Inc.
 Date Completed: 12-8-96
 Well Diameter: 4.0 in.
 Casing Material: SCH 40 PVC
 Drilling Method: Hollow-stem auger

Sampler Type: Split spoon and cuttings
 Bit Diameter: 8.0 in.
 Total Drill Depth: 53.4 ft.
 Screen Interval: 23.8 ft to 33.8 ft
 Screen Type: 0.020 in. Slotted



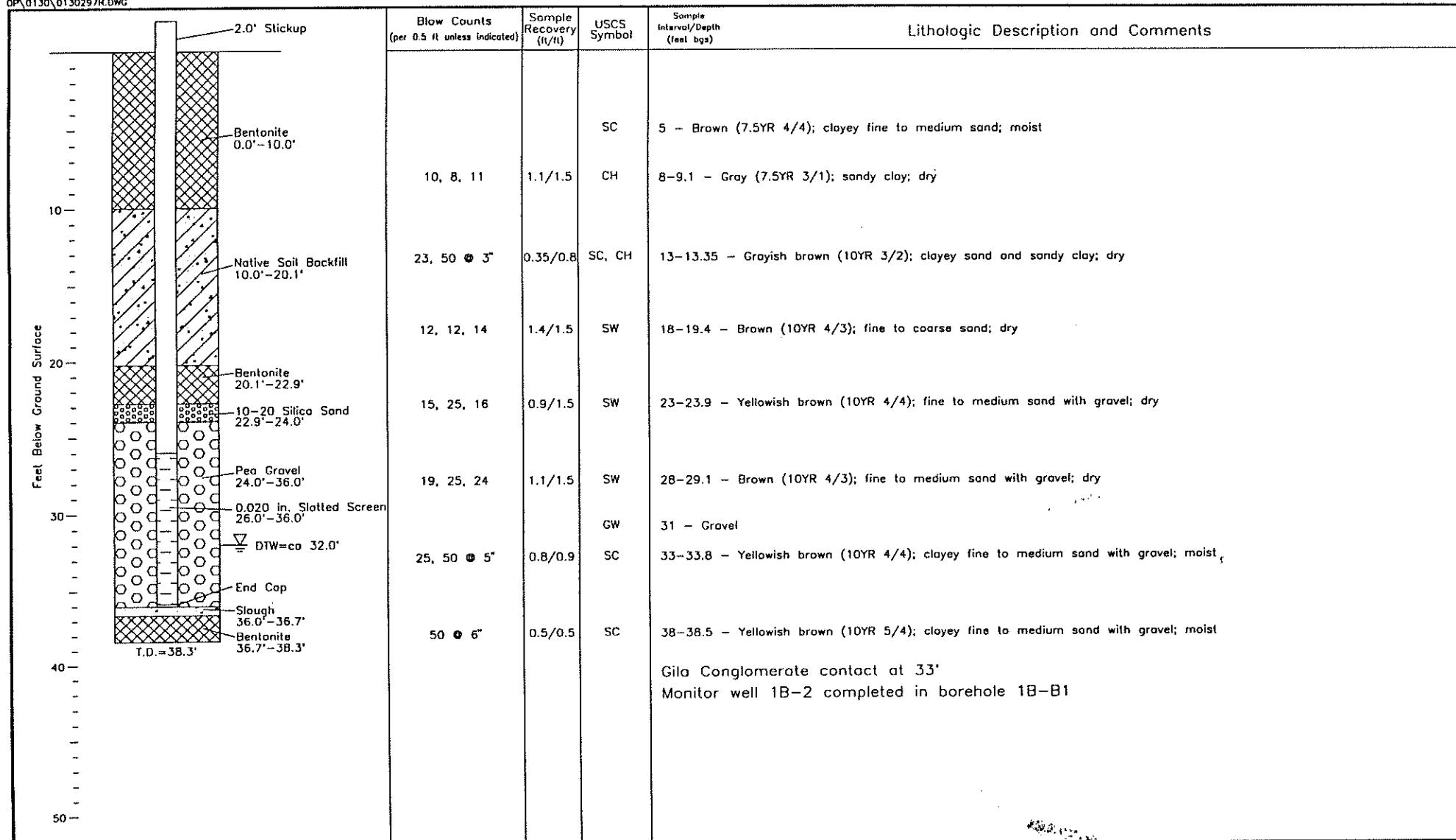
DANIEL B. STEPHENS & ASSOCIATES, INC.

PHELPS DODGE TYRONE, INC.
 Tyrone, New Mexico

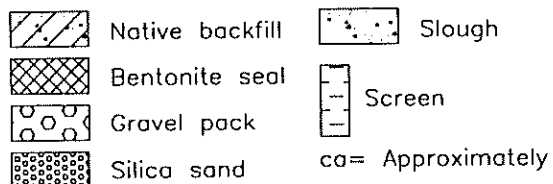
No. 1B Stockpile

Transect No. 1

WELL LOG: 1B-1

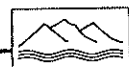


Explanation



Geologist: AP
 Driller: Layne Environmental Drilling, Inc.
 Date Completed: 12-8-96
 Well Diameter: 4.0 in.
 Casing Material: SCH 40 PVC
 Drilling Method: Hollow-stem auger

Sampler Type: Split spoon and cuttings
 Bit Diameter: 10.5 in.
 Total Drill Depth: 38.3 ft.
 Screen Interval: 26.0 ft to 36.0 ft
 Screen Type: 0.020 in. Slotted



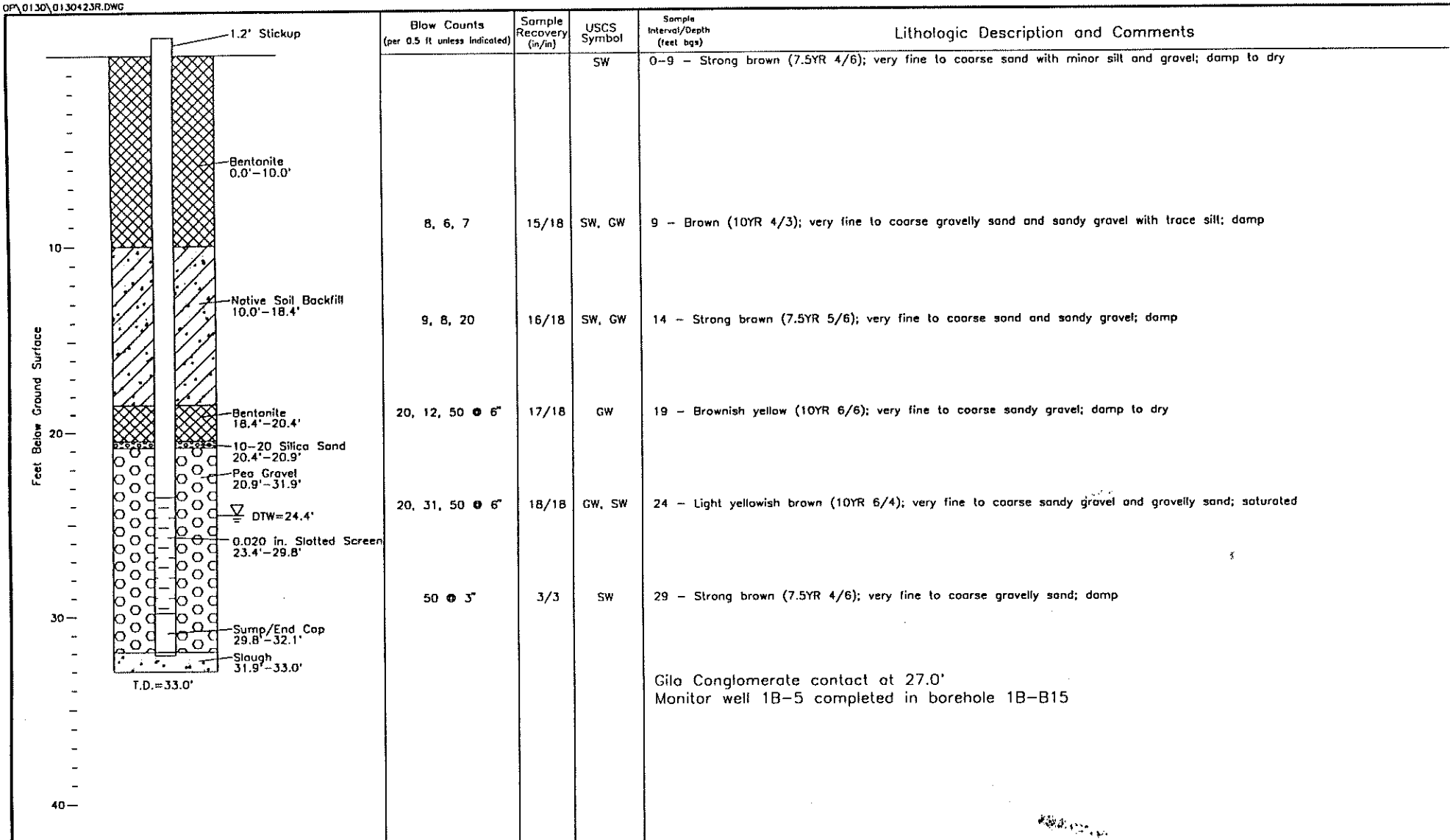
DANIEL B. STEPHENS & ASSOCIATES, INC.
 1-3-97 JN 0130

PHELPS DODGE TYRONE, INC.
 Tyrone, New Mexico

No. 1B Stockpile

Transect No. 1

WELL LOG: 1B-2

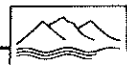


Explanation

	Native Backfill		Slough
	Bentonite Seal		Screen
	Gravel Pack		
	Silica Sand		

Hydrologist: BC
Driller: Layne Environmental Drilling, Inc.
Date Completed: 2-22-97
Well Diameter: 4.0 in.
Casing Material: SCH 40 PVC
Drilling Method: Hollow-stem auger

Sampler Type: Split spoon and cuttings
Bit Diameter: 10.5 in.
Total Drill Depth: 33.0 ft
Screened Interval: 23.4 ft - 29.8 ft
Screen Type: 0.020 in. Slotted



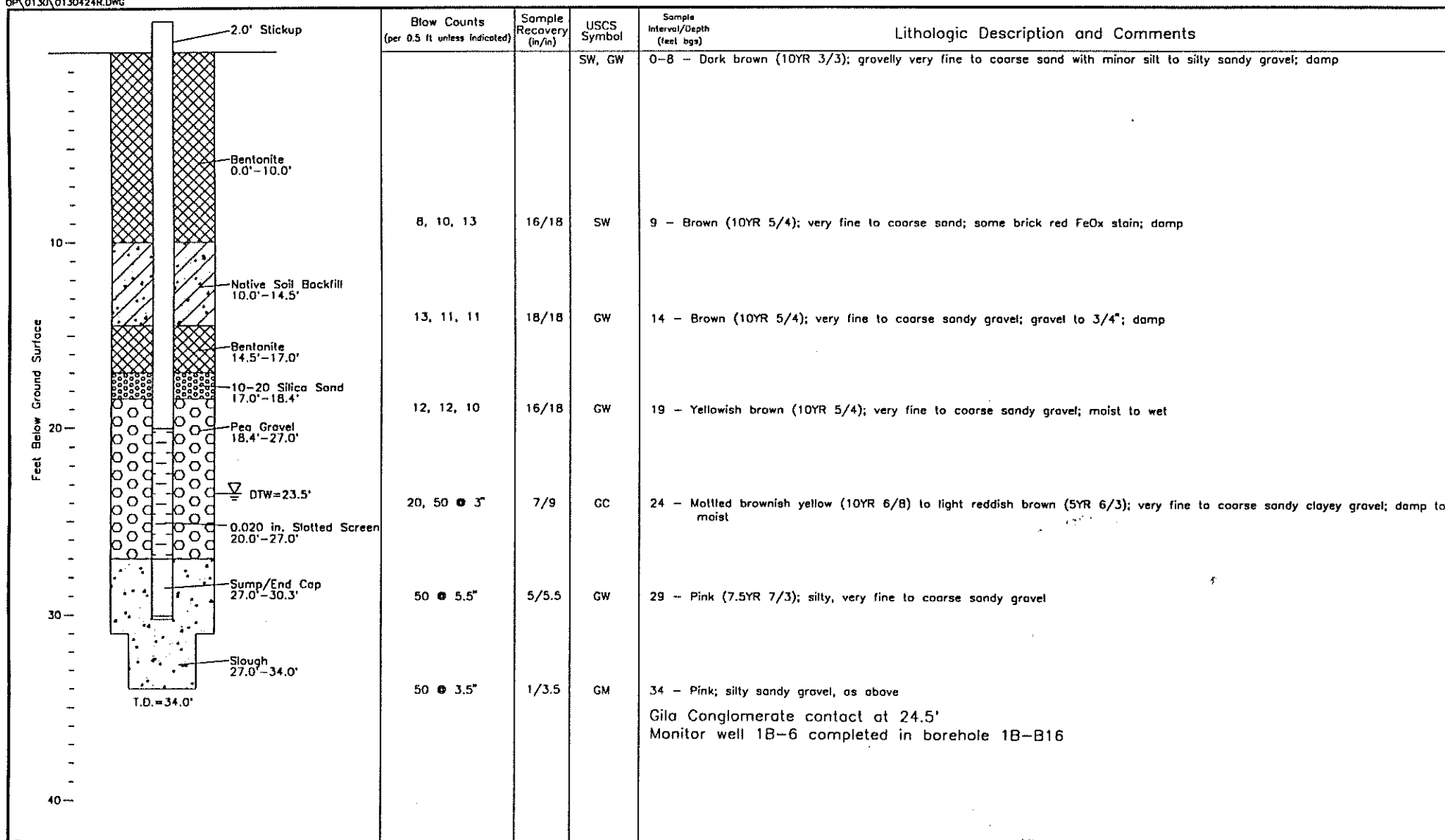
DANIEL B. STEPHENS & ASSOCIATES, INC.
1-15-97 JUN 0130

PHELPS DODGE TYRONE, INC.
Tyrone, New Mexico

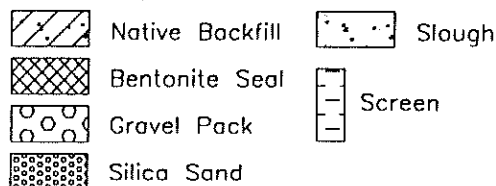
No. 1B Stockpile

Transect No. 2

Well Log: 1B-5

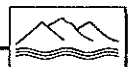


Explanation



Hydrologist: BC
 Driller: Layne Environmental Drilling, Inc.
 Date Completed: 2-22-97
 Well Diameter: 4.0 in.
 Casing Material: SCH 40 PVC
 Drilling Method: Hollow-stem auger

Sampler Type: Split spoon and cuttings
 Bit Diameter: 10.5 in. at 0.0 ft - 31.0 ft
 8.5 in. at 31.0 ft - 34.0 ft
 Total Drill Depth: 34.0 ft
 Screened Interval: 20.0 ft - 27.0 ft
 Screen Type: 0.020 in. Slotted



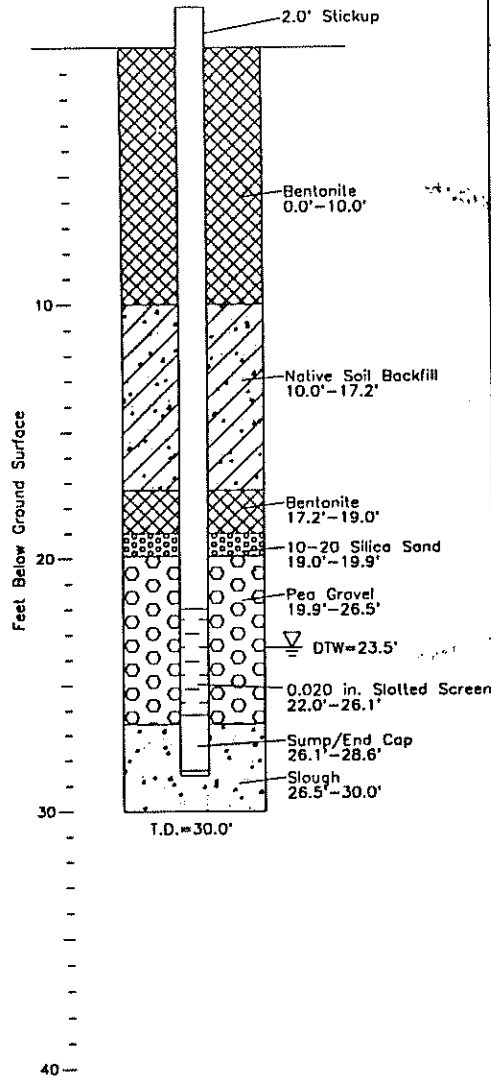
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 4-2-97 JN 0130

PHELPS DODGE TYRONE, INC.
 Tyrone, New Mexico

No. 1B Stockpile

Transect No. 2

Well Log: 1B-6



Blow Counts (per 0.5 ft unless indicated)	Sample Recovery (in/in)	USCS Symbol	Sample Interval/Depth (feet bgs)	Lithologic Description and Comments
		SM	0-7	Dark brown (7.5YR 3/3); silty, very fine to coarse sand with gravel layers; damp
		CH	7-9	Yellowish red (5YR 4/6); very fine to fine sandy clay; medium to high plasticity; damp to moist
		SM	9-13	Brown (7.5YR 4/3); silty, very fine to medium coarse sand with minor clay and layers of gravel; damp
		SW	13-19	Brown (7.5YR 4/3); gravelly, very fine to coarse sand with trace silt to well graded sand; damp
22, 28, 28	13/18	GW	19	Strong brown (7.5YR 4/6); very fine to medium sandy gravel; damp
12, 50 @ 5"	8/11	GW GM to SW	24 24.5	Strong brown (7.5YR 4/4); very fine to coarse sandy gravel; saturated Reddish yellow (7.5YR 6/6); silty, very fine to coarse sandy gravel to gravelly sand; damp
50 @ 6"	6/6	GM	29	Reddish yellow (7.5YR 6/6); silty, very fine to coarse sandy gravel; damp
Gila Conglomerate contact at 24.5'				
Monitor well 1B-7 completed in borehole 1B-B20				

Explanation

	Native Backfill		Slough
	Bentonite Seal		Screen
	Gravel Pack		
	Silica Sand		

Hydrologist: BC
Driller: Layne Environmental Drilling, Inc.
Date Completed: 2-24-97
Well Diameter: 4.0 in.
Casing Material: SCH 40 PVC
Drilling Method: Hollow-stem auger

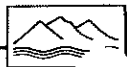
Sampler Type: Split spoon and cuttings
Bit Diameter: 10.5 in.
Total Drill Depth: 30.0 ft
Screened Interval: 22.0 ft - 26.1 ft
Screen Type: 0.020 in. Slotted

PHELPS DODGE TYRONE, INC.
Tyrone, New Mexico

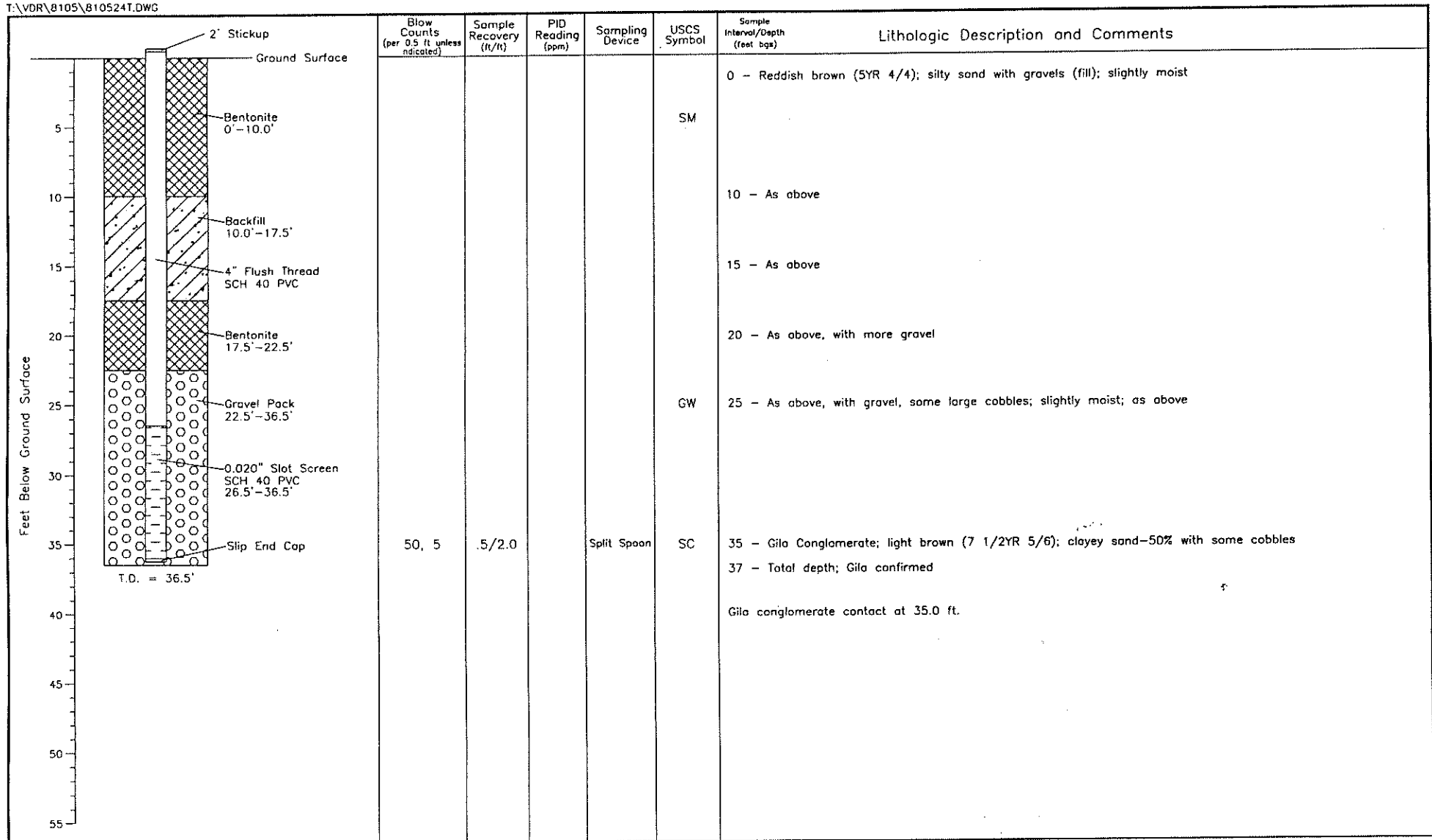
No. 1B Stockpile

Transect No. 2

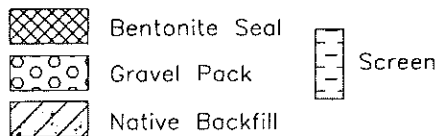
Well Log: 1B-7



DANIEL B. STEPHENS & ASSOCIATES, INC.
4-2-97 JN 0130



Explanation



Geologists: M. Koffler
 Driller: Alliance Environmental Inc.
 Date Completed: 12-21-98
 Drilling Method: Hollow-stem auger

Bit Diameter: 11.5 in.
 Total Drill Depth: 36.5 ft

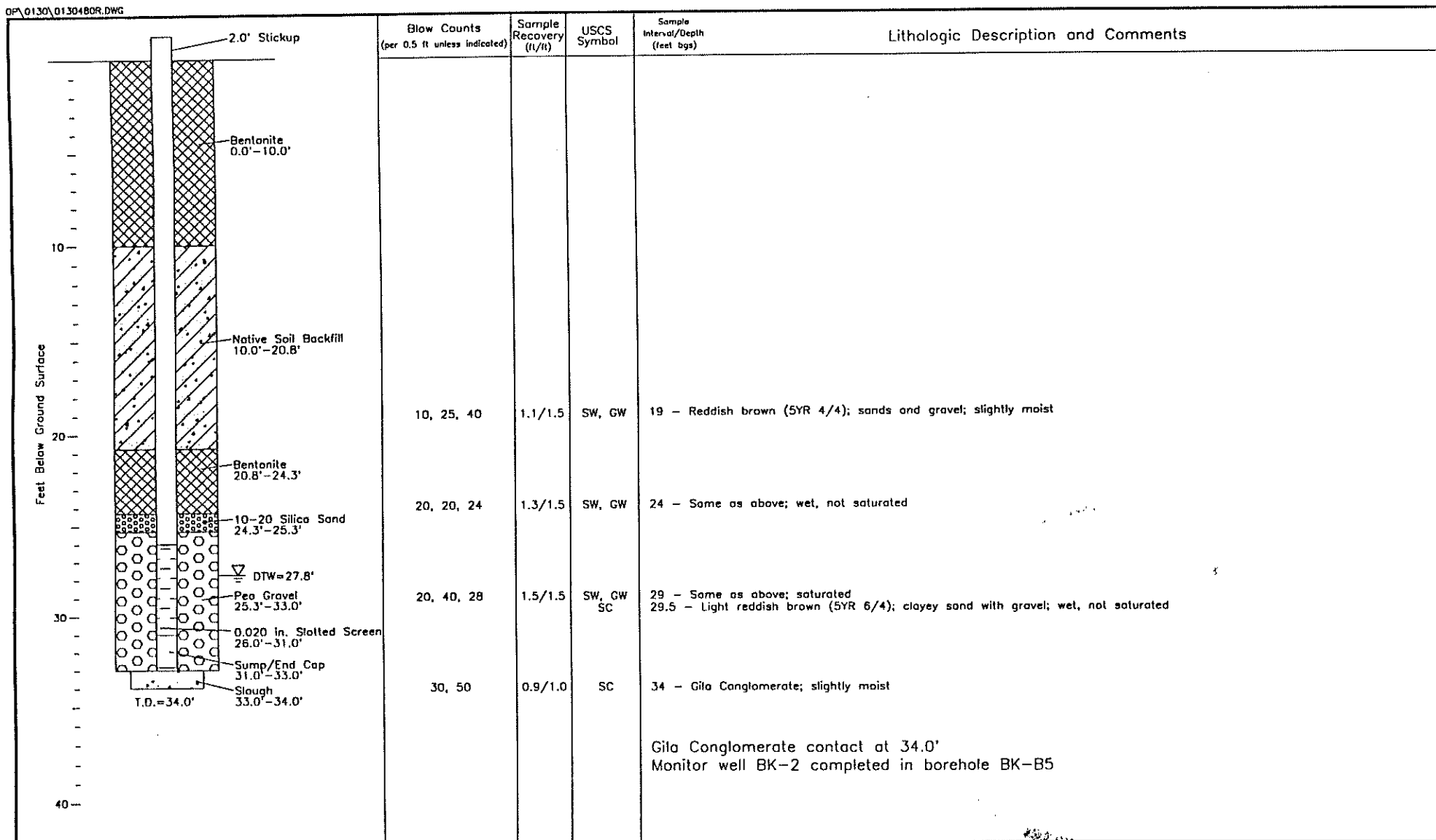


Daniel B. Stephens & Associates, Inc.
 5-24-99 JN 8105

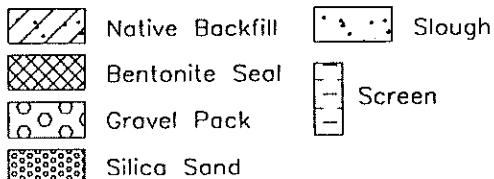
PHELPS DODGE TYRONE, INC.
 Tyrone, New Mexico

NO. 1B STOCKPILE

Monitor Well: IB-9

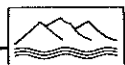


Explanation



Hydrologist: DB
 Driller: Layne Environmental Drilling, Inc.
 Date Completed: 2-10-97
 Well Diameter: 4.0 in.
 Casing Material: SCH 40 PVC
 Drilling Method: Hollow-stem auger

Sampler Type: Split spoon
 Bit Diameter: 10.5 in. at 0.0 ft - 33.0 ft
 8.5 in. at 33.0 ft - 34.0 ft
 Total Drill Depth: 34.0 ft
 Screened Interval: 26.0 ft - 31.0 ft
 Screen Type: 0.020 in. Slotted



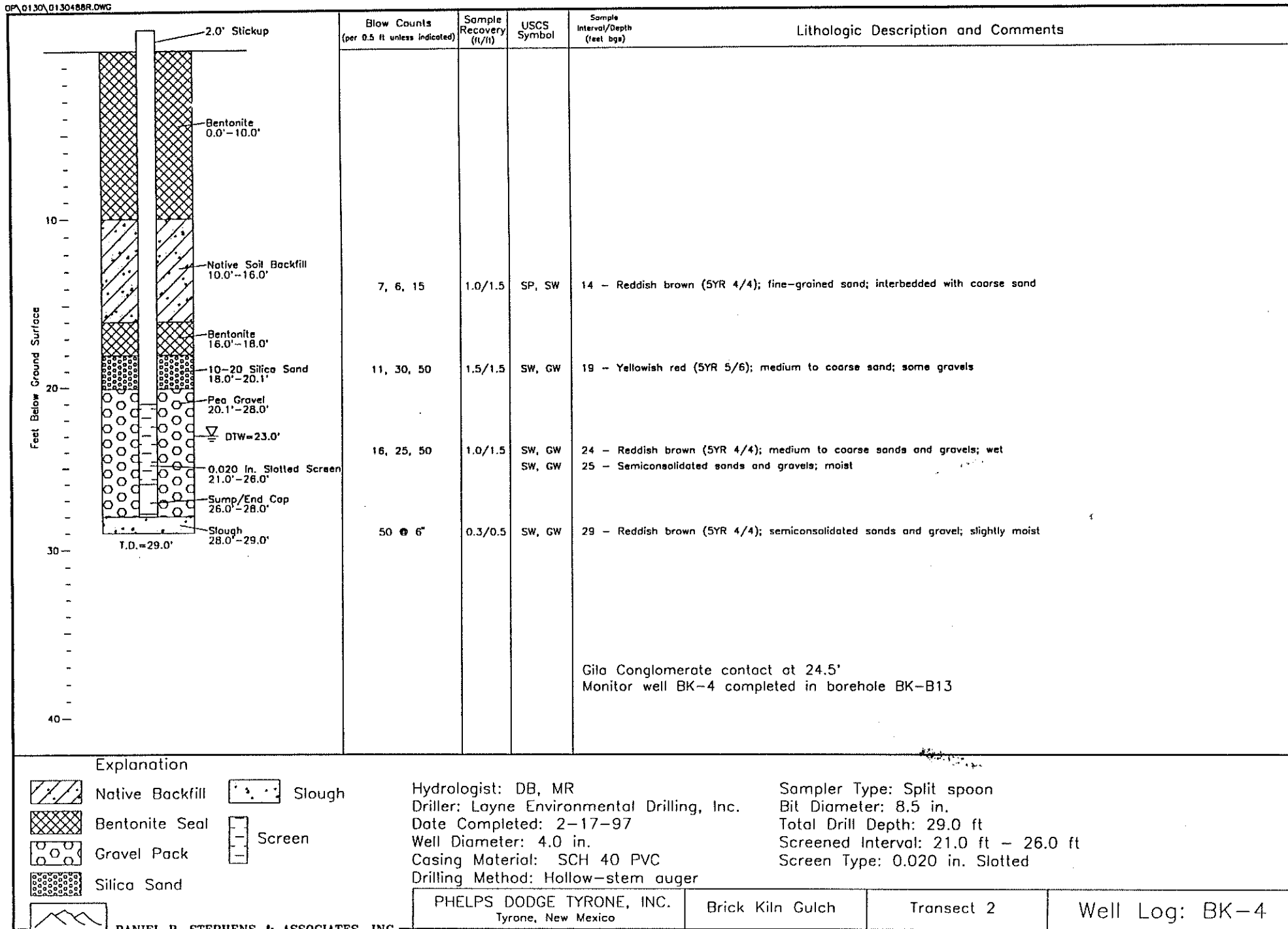
DANIEL B. STEPHENS & ASSOCIATES, INC.
 4-16-97 JN 0130

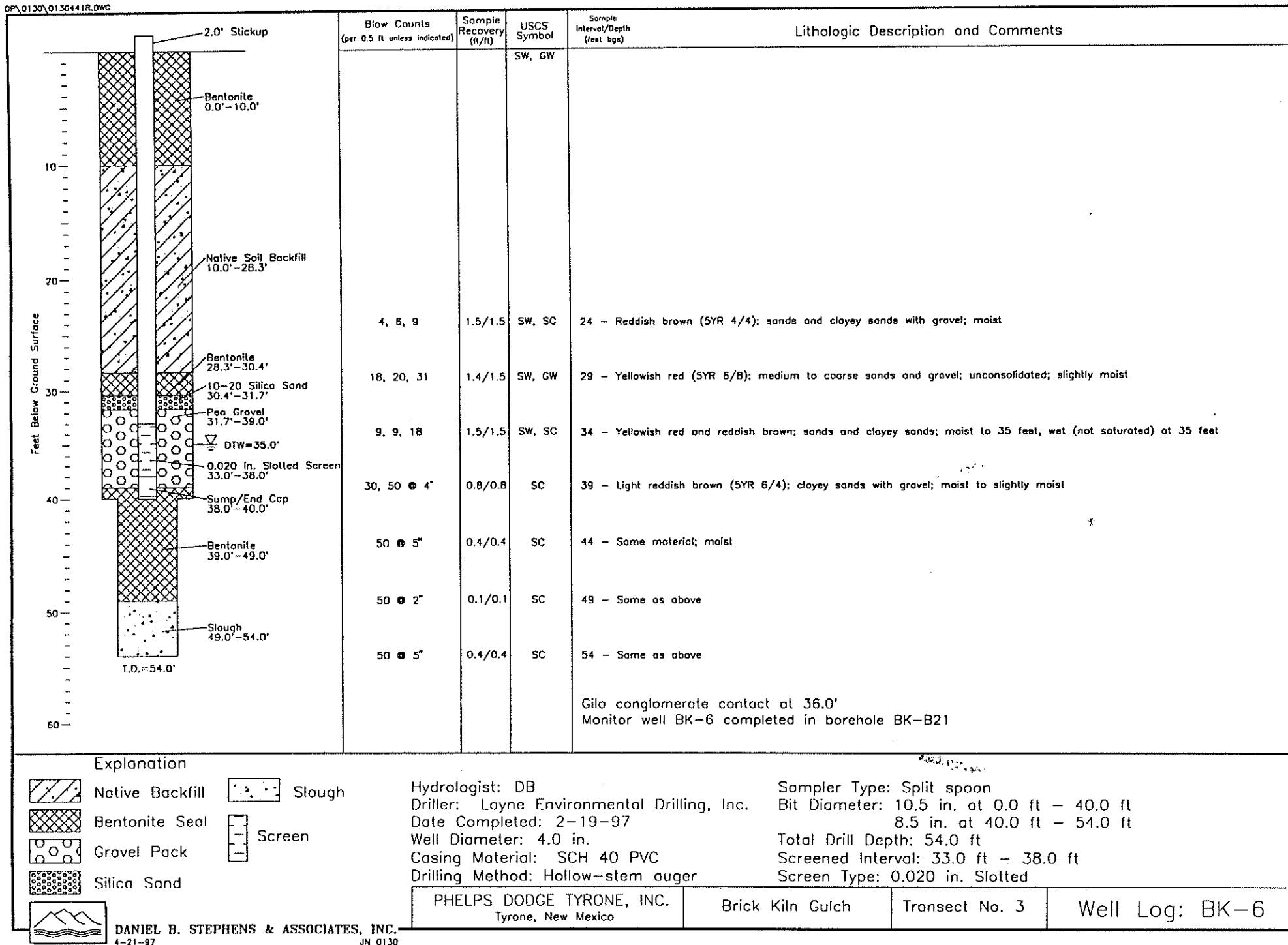
PHELPS DODGE TYRONE, INC.
 Tyrone, New Mexico

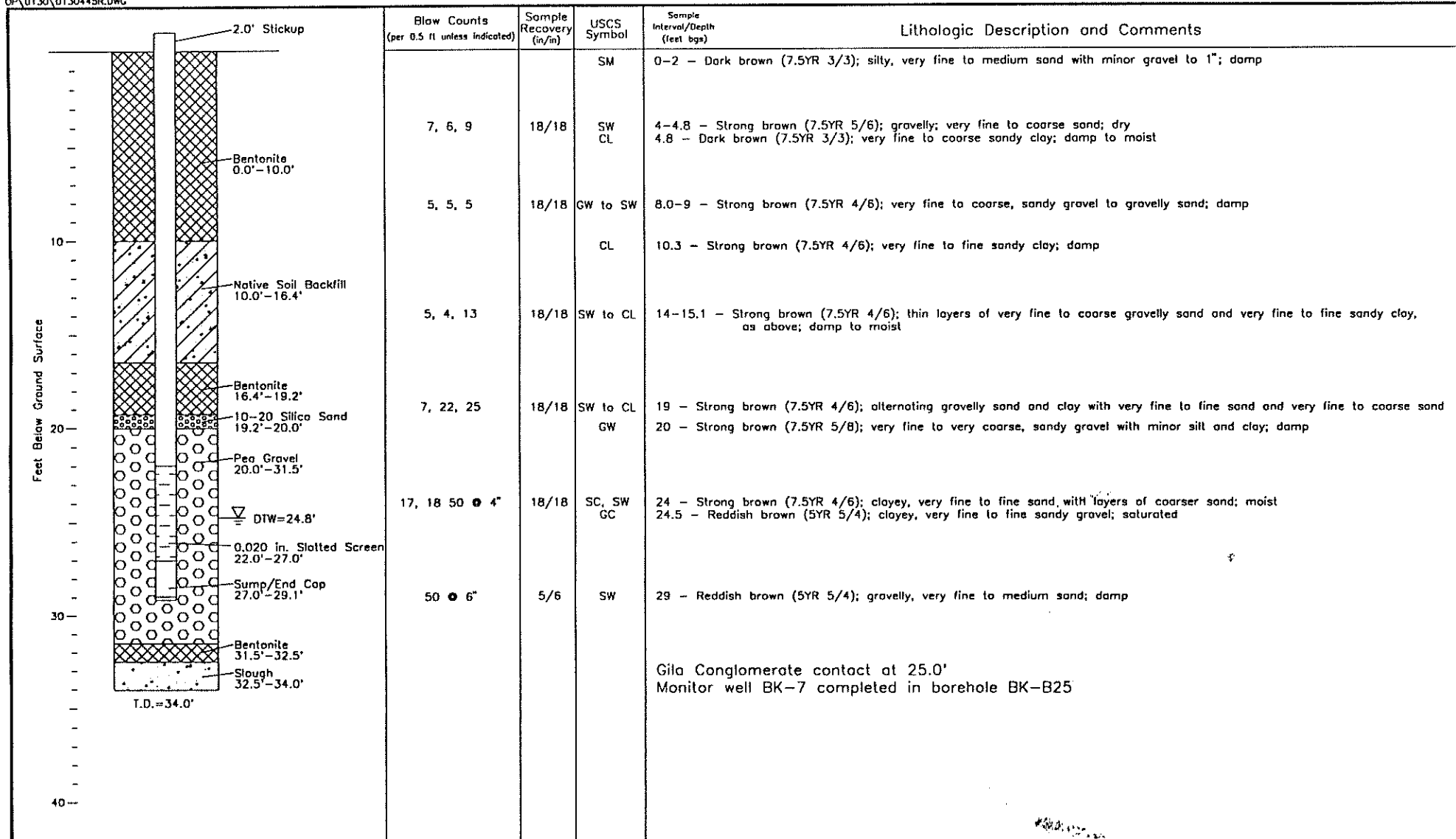
Brick Kiln Gulch

Transect 1

Well Log: BK-2





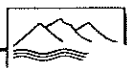


Explanation

- Native Backfill
- Bentonite Seal
- Gravel Pack
- Silica Sand
- Slough
- Screen

Hydrologist: BC
 Driller: Layne Environmental Drilling, Inc.
 Date Completed: 2-26-97
 Well Diameter: 4.0 in.
 Casing Material: SCH 40 PVC
 Drilling Method: Hollow-stem auger

Sampler Type: Split spoon and cuttings
 Bit Diameter: 8.5 in.
 Total Drill Depth: 34.0 ft
 Screened Interval: 22.0 ft - 27.0 ft
 Screen Type: 0.020 in. Slotted



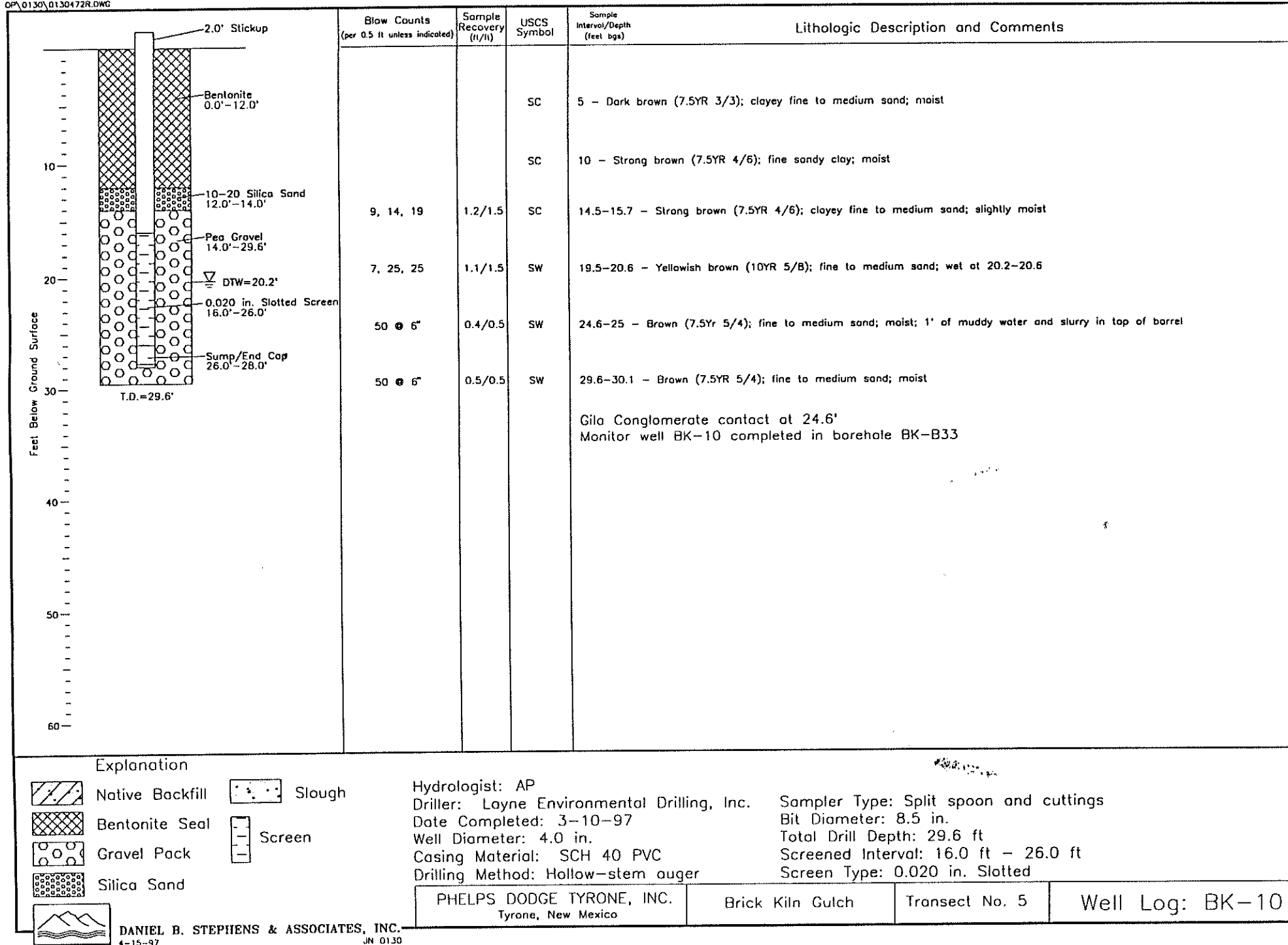
DANIEL B. STEPHENS & ASSOCIATES, INC.
 1-21-97 JN 0130

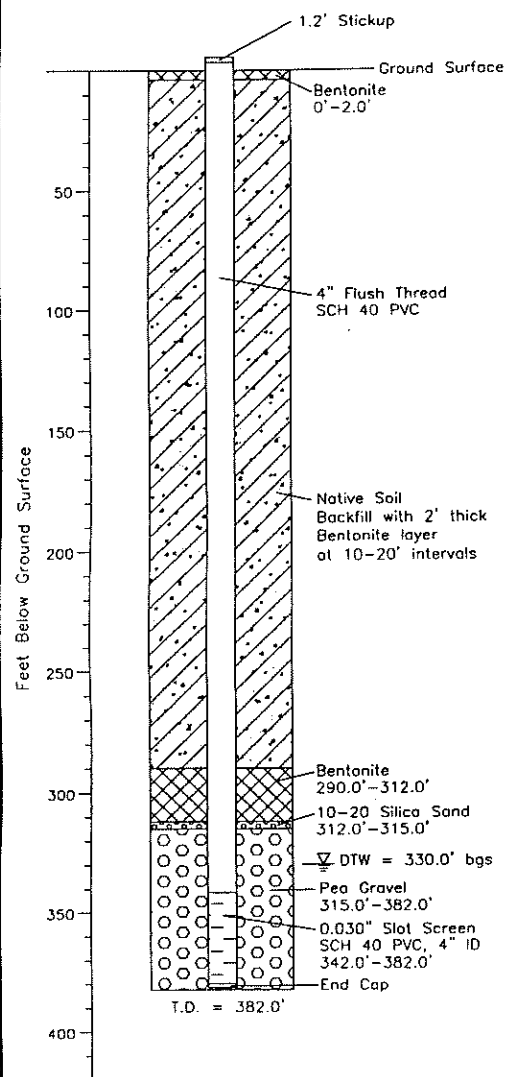
PHELPS DODGE TYRONE, INC.
 Tyrone, New Mexico

Brick Kiln Gulch

Transect No. 4

Well Log: BK-7



Sample
Interval/Depth
(feet bgs)

Lithologic Description and Comments

0.0-255 QTz - Gila Conglomerate

255-333 Dark brown; fractured / "weathered" rock

333-382 Light red; "weathered" rock

Note: Lithologic description based on driller's notes.

Explanation



Bentonite Seal



Gravel Pack



Native Backfill



Screen



Silica Sand

Hydrologist: Rick Lawrence
 Driller: Aaron Roberts
 Date Completed: 5-18-99
 Drilling Method: Cable Tool

Bit Diameter: 8.0 in. O.D.
 Total Drill Depth: 382.0'
 Sampler Type: Cuttings

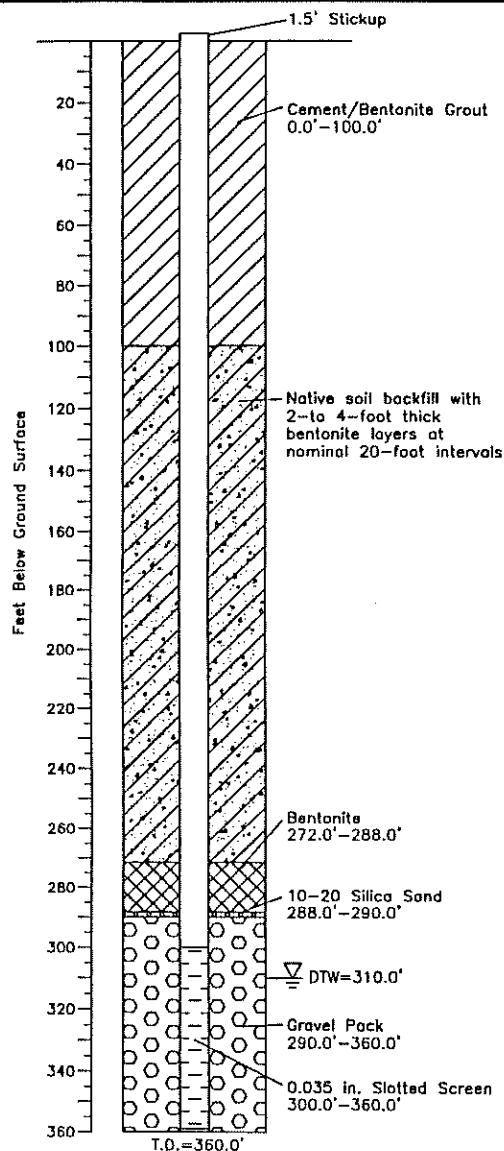


Daniel B. Stephens & Associates, Inc.
 11-29-99 JN 9096

PHELPS DODGE TYRONE, INC.
 Tyrone, New Mexico

TYRONE EAST SIDE No. 1 STOCKPILE AREA

Well Schematic: MB-40



Soil Chemical Parameters

Depth	pH	Electrical Conductivity at 25°C (µmhos/cm)
0-10	7.72	325
10-20	7.76	201
20-30	7.79	155
30-40	7.82	182
40-50	7.93	140
50-60	8.22	132
60-70	8.1	112
70-80	8.25	84
80-90	8.17	106
90-100	8.19	124
100-110	8.34	106
110-120	8.18	104
120-130	8.09	108
130-140	8.12	116
140-150	8.4	89
150-160	8.13	122
160-170	8.22	122
170-180	8.11	133
180-190	8.11	150
190-200	8.18	118
200-210	8.18	130
210-220	8.06	140
220-230	8.31	114
230-240	8.22	101
240-250	8.11	120
250-260	8.11	129
260-270	8.1	130
270-280	8.04	123
280-290	8.23	176
290-300	8.17	212
300-310	7.99	270
310-320	7.62	231
320-330	8.19	393
330-340	7.67	1086
340-350	7.55	1119
350-360	7.35	1626

Comments and Lithology

Lithologic description based on driller's log

Gila conglomerate - 0 - 298

0-230 - Reddish brown conglomerate abundant cobbles

230-298 - Light brown conglomerate with boulders

Bedrock - 298-360 - (no lithologic data available at this time)

298-330 - Light brown hard rock

330-360 - Dark brown soft rock

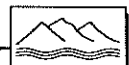
Note: Paste pH and electrical conductivity were measured
on drill cuttings collected at 10-ft intervals

Explanation

- Native backfill
- Bentonite seal
- Gravel pack
- Silica sand
- Cement/Bentonite Grout
- Screen

Hydrologists: RSP
Driller: Aaron Roberts
Date Completed: 8-15-97
Well Diameter: 4.0 in.
Casing Material: SCH 40 PVC
Drilling Method: Cable Tool

Bit Diameter: 8.0 in.
Total Drill Depth: 360.0 ft
Screened Interval: 300.0 ft to 360.0 ft
Screen Type: 0.035" Slotted



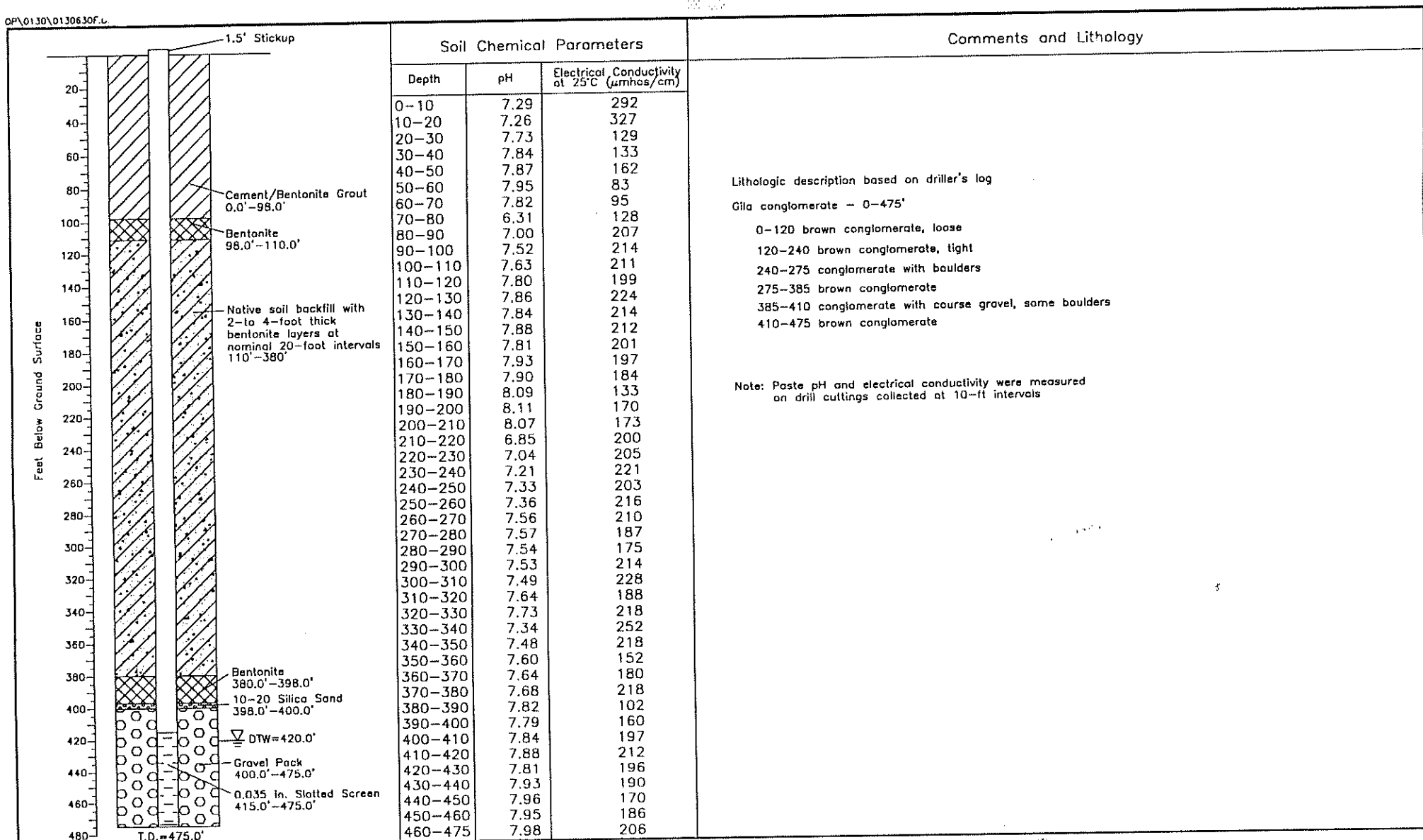
DANIEL B. STEPHENS & ASSOCIATES, INC.
11-7-97 JN 0130

PHELPS DODGE TYRONE, INC.
Tyrone, New Mexico

East Side Area

No. 1 Stockpile

WELL LOG: MB-34



Explanation



Native backfill

Cement/
Bentonite Grout

Bentonite seal



Screen



Gravel pack



Silica sand

Hydrologists: RSP

Driller: Aaron Roberts

Date Completed: 12-17-97

Well Diameter: 4.0 in.

Casing Material: SCH 40 PVC

Drilling Method: Cable Tool

Bit Diameter: 8.0 in.

Total Drill Depth: 475.0 ft

Screened Interval: 415.0 ft to 475.0 ft

Screen Type: 0.035" Slotted

 PHELPS DODGE TYRONE, INC.
 Tyrone, New Mexico

East Side Area

No. 1B Stockpile

WELL LOG: MB-36

 DANIEL B. STEPHENS & ASSOCIATES, INC.
 2-27-98 JN 0130

3A Leach Stockpile Liquefaction Evaluation
Qal SPT Data, All Sampled Intervals
Corrected N Value (As per Youd et al., 1996 and 1998 NCEER Workshops)

Saturation Assumed below 20 ft

Qal moist unit weight 120 PCF
Qal sat unit weight 133 PCF

Borehole dia correction 1.05 (Cb) Youd et al
Rod length corr 0.8 (Cr) Youd et al
Energy Ratio 1 (Ce) Youd et al
Sampling Method Corr 1 (Cs) Youd et al
Correction Product 0.84 (Less Ovb corr, Column J)
Max surface Accel 0.14 g USGS Unified Hazard Tool
3A Leach Site Class (760 m/sec)
30% Amplification

Percent Fines 10 %
Max Quake Magnitude 6.7 URS Seismicity Study
Mag Scale Factor 1.31 Table 3, Idriss, Column 3 (after Youd and Noble 1997)
Regrade Burial Depth 0 Feet

Surcharge Pressure 0 PSF at surface

Hole	Depth to Water (bgs)	Sample Depth	Material	Blows/ft (uncorr)	Depth below gws	u (pcf)	Total Stress	Effective Stress	CSR (Youd eqn 1)	CRR (LIQFAC Lookup)	FS (Youd eqn 23)
7-2	20	5	Qal	17	0	0	600	600	0.090	0.5699	NL
7-2	20	10	Qal	9	0	0	1200	1200	0.089	0.1900	NL
7-3	20	5	Qal	18	0	0	600	600	0.090	0.8253	NL
7-3	20	10	Qal	13	0	0	1200	1200	0.089	0.2646	NL
7-3	20	15	Qal	11	0	0	1800	1800	0.088	0.1900	NL
7-3	20	20	Qal	5	0	0	2400	2400	0.087	0.1048	1.6
7-3	20	25	Qal	18	5	312	3065	2753	0.095	0.2358	3.2
7-3	20	30	Qal	25	10	624	3730	3106	0.102	0.2948	3.8
7-4	20	5	Qal	17	0	0	600	600	0.090	0.5699	NL
7-4	20	10	Qal	28	0	0	1200	1200	0.089	2.6200	NL
7-4	20	15	Qal	13	0	0	1800	1800	0.088	0.2188	NL
7-4	20	20	Qal	14	0	0	2400	2400	0.087	0.2031	3.1
7-4	20	25	Qal	18	5	312	3065	2753	0.095	0.2358	3.2
7-5	20	5	Qal	25	0	0	600	600	0.090	2.6200	NL
7-5	20	10	Qal	26	0	0	1200	1200	0.089	2.6200	NL
7-5	20	15	Qal	14	0	0	1800	1800	0.088	0.2358	NL
7-5	20	20	Qal	18	0	0	2400	2400	0.087	0.2502	3.8
7-5	20	25	Qal	32	5	312	3065	2753	0.095	0.4467	6.1
7-6	20	5	Qal	20	0	0	600	600	0.090	2.6200	NL
7-6	20	10	Qal	57	0	0	1200	1200	0.089	2.6200	NL
7-8	20	5	Qal	29	0	0	600	600	0.090	2.6200	NL
7-8	20	10	Qal	38	0	0	1200	1200	0.089	2.6200	NL
7-8	20	15	Qal	63	0	0	1800	1800	0.088	2.6200	NL
7-8	20	20	Qal	14	0	0	2400	2400	0.087	0.2031	3.1
7-8	20	25	Qal	25	5	312	3065	2753	0.095	0.3157	4.3
8-2	20	5	Qal	10	0	0	600	600	0.090	0.2646	NL
8-2	20	10	Qal	46	0	0	1200	1200	0.089	2.6200	NL
8-3	20	10	Qal	7	0	0	1200	1200	0.089	0.1598	NL
8-3	20	15	Qal	13	0	0	1800	1800	0.088	0.2188	NL

Hole	Depth to Water (bgs)	Sample Depth	Material	Blows/ft (uncorr)	Depth below gws	u (pcf)	Total Stress	Effective Stress	CSR (Youd eqn 1)	CRR (LIQFAC Lookup)	FS (Youd eqn 23)
8-3	20	20	Qal	16	0	0	2400	2400	0.087	0.2358	3.6
8-4	20	5	Qal	8	0	0	600	600	0.090	0.2188	NL
8-4	20	10	Qal	9	0	0	1200	1200	0.089	0.1900	NL
8-4	20	15	Qal	27	0	0	1800	1800	0.088	0.4913	NL
8-4	20	20	Qal	40	0	0	2400	2400	0.087	2.6200	NL
8-5	20	5	Qal	11	0	0	600	600	0.090	0.2843	NL
8-5	20	10	Qal	33	0	0	1200	1200	0.089	2.6200	NL
8-8	20	5	Qal	4	0	0	600	600	0.090	0.1310	NL
8-8	20	10	Qal	50	0	0	1200	1200	0.089	2.6200	NL
8-8	20	15	Qal	26	0	0	1800	1800	0.088	0.4913	NL
8-8	20	20	Qal	47	0	0	2400	2400	0.087	2.6200	NL
8-9	20	5	Qal	28	0	0	600	600	0.090	2.6200	NL
8-9	20	10	Qal	38	0	0	1200	1200	0.089	2.6200	NL
8-9	20	15	Qal	31	0	0	1800	1800	0.088	1.3100	NL
8-9	20	20	Qal	37	0	0	2400	2400	0.087	2.6200	39.6
8-10	20	5	Qal	23	0	0	600	600	0.090	2.6200	NL
8-10	20	10	Qal	70	0	0	1200	1200	0.089	2.6200	NL
8-10	20	15	Qal	21	0	0	1800	1800	0.088	0.3367	NL
8-10	20	20	Qal	50	0	0	2400	2400	0.087	2.6200	NL
9-1	20	5	Qal	21	0	0	600	600	0.090	2.6200	NL
9-1	20	10	Qal	27	0	0	1200	1200	0.089	2.6200	NL
9-1	20	15	Qal	49	0	0	1800	1800	0.088	2.6200	NL
9-1	20	20	Qal	15	0	0	2400	2400	0.087	0.2188	3.3
9-1	20	25	Qal	69	5	312	3065	2753	0.095	2.6200	NL
10-2	20	5	Qal	32	0	0	600	600	0.090	2.6200	NL
10-3	20	5	Qal	29	0	0	600	600	0.090	2.6200	NL
10-3	20	10	Qal	23	0	0	1200	1200	0.089	0.5699	NL
10-4	20	5	Qal	17	0	0	600	600	0.090	0.5699	NL
10-4	20	10	Qal	21	0	0	1200	1200	0.089	0.4467	NL
10-4	20	15	Qal	22	0	0	1800	1800	0.088	0.3563	NL
10-4	20	20	Qal	24	0	0	2400	2400	0.087	0.3367	5.1
10-4	20	25	Qal	37	5	312	3065	2753	0.095	1.0480	14.4
10-4	20	30	Qal	41	10	624	3730	3106	0.102	1.3100	16.9
10-5	20	5	Qal	20	0	0	600	600	0.090	2.6200	NL
10-5	20	10	Qal	28	0	0	1200	1200	0.089	2.6200	NL
10-5	20	15	Qal	14	0	0	1800	1800	0.088	0.2358	NL
10-5	20	20	Qal	26	0	0	2400	2400	0.087	0.3563	5.4
10-5	20	25	Qal	46	5	312	3065	2753	0.095	2.6200	NL
10-5	20	30	Qal	75	10	624	3730	3106	0.102	2.6200	NL
10-6	20	5	Qal	19	0	0	600	600	0.090	1.3100	NL
10-6	20	10	Qal	17	0	0	1200	1200	0.089	0.3367	NL
10-6	20	15	Qal	23	0	0	1800	1800	0.088	0.3825	NL
10-6	20	20	Qal	25	0	0	2400	2400	0.087	0.3563	5.4
10-6	20	25	Qal	55	5	312	3065	2753	0.095	2.6200	NL
10-6	20	30	Qal	51	10	624	3730	3106	0.102	2.6200	NL
10-7	20	5	Qal	55	0	0	600	600	0.090	2.6200	NL
10-8	20	5	Qal	19	0	0	600	600	0.090	1.3100	NL
10-11	20	5	Qal	25	0	0	600	600	0.090	2.6200	NL
10-11	20	10	Qal	18	0	0	1200	1200	0.089	0.3563	NL

Hole	Depth to Water (bgs)	Sample Depth	Material	Blows/ft (uncorr)	Depth below gws	u (pcf)	Total Stress	Effective Stress	CSR (Youd eqn 1)	CRR (LIQFAC Lookup)	FS (Youd eqn 23)
10-11	20	15	Qal	20	0	0	1800	1800	0.088	0.3157	NL
11-14	20	5	Qal	14	0	0	600	600	0.090	0.3563	NL
11-14	20	10	Qal	46	0	0	1200	1200	0.089	2.6200	NL
11-14	20	15	Qal	11	0	0	1800	1800	0.088	0.1900	NL



DRAFT

BOREHOLE LOG

7-1

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SHEET

SAMPLING METHOD: SPT, Shelby

DRILLING

START FINISH

NORTHING
DATUM: amslEASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DATE DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
24 37 50	100				Fill in 7 Canyon Reworked Gila/Qual								
28 38 47	100		SP		Sand, silty, gravelly reddish yellow, dry	15	60	25	7.5 1/2 7/6	hard	NP		pH: 8.0
10 22 19 37			SP		Sand, gravelly, cobbly, silty, reddish yellow, dry	25	60	15	7.5 1/2 7/6	hard	NP		pH: 8.0 pH: 7.0
30 34 35 39													
20 21 44 48			SP		Sand, silty, gravelly, pink, dry	15	55	30	7.5 1/2 7/6	hard	NP		pH: 7.0
24 50					Sand 145 Above								
30 40 41 36			SP		Sand, gravelly, silty strong brown, moist	40	40	20	7.5 1/2 7/6	hard	1		pH: 7.0
36 43 50			SP		Sand, gravelly, silty strong brown, moist TD: 35 ft	25	60	15	7.5 1/2 7/6	hard	NP		pH: 7.0
40													

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR: GSI

LOGGED BY: 15/11/02

DATE:

7-11-06

JOB NO.

FILE NAME:

Tyreac 34



**Golder
Associates**

DRAFT

BOREHOLE LOG 7-2

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET

NORTHING
DATUM: amsl

EASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DRILLING
START FINISH

DATE DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	5 5 12	100	SP		Sand, gravelly, silty, brown, sl moist	15	60	25	7.5 42 4/4	Soft	1	PH: 8.0	
10	3 4 5		SP		Sand, silty, gravelly brown, sl. moist	5	60	25	7.5 42 4/4	Soft	1	PH: 7.0	
15	14 21 33		Qal Plg		Sand, silty, gravelly Reddish yellow, sl. moist	15	60	25	7.5 42 4/6	firm	NP	PH: 7.5	
20	20 32 39		SP		Sand, gravelly, reddish yellow, sl moist TD: 20 ft	25	70	5	7.5 42 6/6	hard	NP	PH: 7.0	
25													

Notes:

¹ Percent > 3inch.

² Sum of gravel, sand, and fines = 100%

³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.

⁴ For noncohesive soil: weak, moderate, strong.

⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR GSI

LOGGED BY: Kilmene

DATE: 7-12-06

JOB NO.

FILE NAME: Tysons 3A



DRAFT

BOREHOLE LOG 7-3 Sheet 1

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger		BORING NO.	
hole spudded @ surveyed L.S. -0.54		SAMPLING METHOD: SPT, Shelby		SHEET 1 of 2	
				DRILLING	
				START FINISH	
NORTHING		WATER LEVEL		DATE	
DATUM: amsl		TIME		DATE	
EASTING:		DATE			
ELEVATION:		CASING DEPTH			
DRILL RIG:		SURFACE CONDITIONS:			
ANGLE: 90					
BEARING: -					
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.					

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	11 9	60%	SP		Fill, reworked gal, fly								
10	6 5 7	74%	SP		Sand, gravelly, silty reddish yellow, clay	35	50	15	75 42 1/8	fine	NP	PH: 7.0	
15	4 5 6	50%	SP		Sand, silty, gravelly, strong brown, silty moist	10	65	25	75 42 5/6	fine	1	PH: 7.0	
20	3 2 3	90%	SP		Sand, as above	10	65	25	75 42 5/6	fine	1	PH: 7.0	
25					Sand, silty, gravelly dark brown	10	60	30	75 42 3/2	fine	NP	PH: 7.0	

Notes:

¹ Percent > 3in.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR QSZ

LOGGED BY: KILMER

DATE: 7-12-06

JOB NO.

FILE NAME: 14R02 3A



DRAFT

BOREHOLE LOG

7-3 Sheet
2

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger		BORING NO.			
				SHEET 2062			
		SAMPLING METHOD: SPT, Shelby		DRILLING			
				START FINISH			
NORTHING DATUM: amsl		EASTING: ELEVATION:		WATER LEVEL		DATE	
				TIME		DATE	
				DATE		DATE	
				CASING DEPTH		DATE	
DRILL RIG: ANGLE: 90 SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.		BEARING: -		SURFACE CONDITIONS:			

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
25	5 7 11	80%	SP		Sand, AS Above								
			SP		Sand, gravelly, silty, reddish yellow, v. moist	20	65	15	7.5 72 7/6	firm	np	PH: 7.0	
30	17 18 17	90%	Qap Qty		Sand, gravelly, silty, reddish yellow, moist drilling hard	25	65	10	7.5 72 7/6	hard	1	PH: 4.0	
35	24 24	70	SP		Sand, gravelly, silty, reddish yellow, moist v. hard drilling	75	65	10	7.5 72 6/6	hard	1	PH: 5.0	
40					TD: 35 ft hole moist @ bottom - 36.5 ft								

Notes:

¹ Percent > 3inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR GSI

LOGGED BY: K/llwsh

DATE: 7-12-06

JOB NO.

FILE NAME: Tynewe 312



DRAFT

BOREHOLE LOG

7-4 Sheet
1

SITE NAME AND LOCATION: name and location	DRILLING METHOD:	Hollow Stem Auger	BORING NO.
			SHEET 1 of 2
	SAMPLING METHOD:	SPT, Shelby	DRILLING
			START FINISH
			DATE DATE
NORTHING	EASTING:		
DATUM: amsl	ELEVATION:		
DRILL RIG:	BEARING: -		
ANGLE: 90			
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.		SURFACE CONDITIONS:	

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY/ ³ CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	7 7 15	100%	SP		fill, resurfaced gravel, fly								
					sandy, silty, gravelly, brown dry waste rock large rock 7' - refusal skidded rig 3' west	20	60	20	7.5 7.2 3 1/2	Soft	np	PH: 7.5	
10	20 19 9	0	GP		waste rock fragments in cuttings								
15	8 7 6	30	SP		sand, gravelly, silty brown, dry (waste rock fragments)	25	60	15	7.5 7.2 5/4	firm	NP	PH: 7.0	
20	13 12 12	20	GP		sand, gravelly, cobbly, silty, sl. moist (waste rock fragments)	30	50	20	7.5 7.2 5/4	firm	1	PH: 7.0	
25													

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR

LOGGED BY:

DATE:

JOB NO.

FILE NAME:



**Golder
Associates**

DRAFT

BOREHOLE LOG

7-4 Sheet 2

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger		BORING NO.	
				SHEET	
		SAMPLING METHOD: SPT, Shelby			
				DRILLING	
NORTHING		EASTING:		START	
DATUM: amsl		ELEVATION:		FINISH	
		WATER LEVEL			
		TIME			
		DATE		DATE	
		CASING DEPTH			
DRILL RIG:		SURFACE CONDITIONS:			
ANGLE: 90					
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.					

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
25	4 6 8	40%	SP		Sand, As Above Sand, silty, gravelly dark brown, sl. moist	25	55	20	75 42 1/4	firm	np	PH: 6.5	
30	5 7 11	70%	SP Pal Ftg		Sand, gravelly, silty, light yellowish brown, moist, Drilling hard	25	65	10	10 72 6/4	firm	1	PH: 6.5	
35	14 54	90%										PH: 6.0	
40	37 50	95%	SP		Gravelly sand/silt light yellow brown, slightly moist	40	50	10	100 5/6	Firm	NP	PH: 4.0	

Notes:

¹ Percent > 3inch.

² Sum of gravel, sand, and fines = 100%

³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.

⁴ For noncohesive soil: weak, moderate, strong.

⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR GSI

LOGGED BY: KIMMEL

DATE: 7-12-06

JOB NO.

FILE NAME: Tyrene 3A



**Golder
Associates**

DRAFT

BOREHOLE LOG

7-5 Sheet
1

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger		BORING NO.	
Spilled @ -0.75 below SURVEYED point		SAMPLING METHOD: SPT, Shelby		SHEET	
				DRILLING	
		WATER LEVEL		START	
		TIME		FINISH	
NORTHING		EASTING:		DATE	
DATUM: amsl		ELEVATION:		DATE	
DRILL RIG:		SURFACE CONDITIONS:			
ANGLE: 90		BEARING: -			
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.					

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	7 11 14	0	SP		fill, mixed Gal, Ptg waste rock								
10	14 12 14	60%	SP		fill, mixed Gal, Ptg waste rock, wood							PH: 5.0	
15	8 7 7	50%	SP		fill, mixed Gal, Ptg waste rock							PH: 7.0	
20	10 8 8	30% (rock)	GP		cobbly, gravelly							PH: 8.0	
25	16 16		Gal										

Notes:

¹ Percent > 3inch.

² Sum of gravel, sand, and fines = 100%

³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.

⁴ For noncohesive soil: weak, moderate, strong.

⁵ Pocket penetrometer, torevane, in situ density, etc.

GSI

DRILLING CONTRACTOR

LOGGED BY: Kilmer/Schindler

DATE: 7-12-06

FILE NO.

3A

THANE

FILE NAME:



DRAFT

BOREHOLE LOG

7-5 Sheet
2

SITE NAME AND LOCATION: name and location	DRILLING METHOD: Hollow Stem Auger	BORING NO.
		SHEET
	SAMPLING METHOD: SPT, Shelby	
		DRILLING
		START FINISH
NORTHING	EASTING:	
DATUM: amsl	ELEVATION:	
	WATER LEVEL	
	TIME	
	DATE	
	CASING DEPTH	
DRILL RIG:	SURFACE CONDITIONS:	
ANGLE: 90		
BEARING: -		
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.		

DEPTH IN FEET (ELEVATION)	BLOW/ 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
25	16 16 16	90%	SP Qc Qtg		Sand, gravelly, silty pink, dry -Drilling hand-		30	50	20	75 42 74	hard	NP	PH: 7.5
30	16 23 50+		SP		Sand, gravelly, silty brown, very sl. moist TD: 30 ft		30	60	10	75 42 74	hard	NP	PH: 6.5

GSI

LOGGED BY: K. H. W. / Schindler

DRILLING CONTRACTOR

DATE: 7-12-06

JOB NO.

FILE NAME: Tyronne 3A

Notes:

¹ Percent > 3in.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.



**Golder
Associates**

DRAFT

BOREHOLE LOG

7-6

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger		BORING NO.	
NORTHING DATUM: amsl		EASTING: ELEVATION:		SHEET	
				SAMPLING METHOD: SPT, Shelby	
				DRILLING	
				START	FINISH
WATER LEVEL					
TIME					
DATE					
CASING DEPTH					
DATE					
DATE					
DRILL RIG:		SURFACE CONDITIONS:			
ANGLE: 90					
BEARING: -					
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.					

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ^{3/} CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	8 9 11	100%	SP		fill mixed Gal, Qty, wash rock								
10	17 28 29	80%	SP		Sand, gravelly, silty pink, dry	25	65	10	75 42 1/3	hard	NP	PH: 8.0	
15	34 504	50%	SP		Sand, gravelly, silty pinkish grey, v. sl. moist (wash rock fragments)	25	65	10	75 42 1/2	hard	NP	PH: 8.0	
20	37 504	50%	SP		Drilling hard	25	60	15	75 42 1/6	u. hard	NP	PH: 7.0	
25	37 504	50%	SP		Sand, gravelly, silty reddish yellow. sl. moist	25	60	15	75 42 1/6	u. hard	NP	PH: 7.0	
25					TD: 20 ft								

Notes:

¹ Percent > 3 inch.

² Sum of gravel, sand, and fines = 100%

³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.

⁴ For noncohesive soil: weak, moderate, strong.

⁵ Pocket penetrometer, torevane, in situ density, etc.

GSD

DRILLING CONTRACTOR

LOGGED BY: KILMER

DATE: 7-12-06

JOB NO.

FILE NAME: Tyronne 3A



**Golder
Associates**

DRAFT

BOREHOLE LOG

7-8

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger	BORING NO.
			SHEET
		SAMPLING METHOD: SPT, Shelby	
			DRILLING
			START FINISH
NORTHING	EASTING:	WATER LEVEL	
DATUM: amsl	ELEVATION:	TIME	
		DATE	
		CASING DEPTH	
DRILL RIG:		SURFACE CONDITIONS:	
ANGLE: 90			
BEARING: -			
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.			

DEPTH IN FEET (ELEVATION)	BLOW/ 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
5	13 9 20	100	SP	13-20	Sand, silty, gravelly Strong brown, silty moist	25	50	25	75 42 3/4	firm	1	PH: 8.0	
5	14 24	100	SP	14-24	Sand, silty, gravelly Dark brown, moist plastic	15	50	30	75 42 3/4	firm	M	PH: 8.0	
10	22 25 38	100	SP	22-38	Sand, gravelly, silty v. moist to saturated (soil) very dark grey to brown, organic	20	60	20	75 42 3/4	firm	1	PH: 4.0	
15	6 8	100	SP	6-8	Sand, coarse, gravelly sl. silty, brown, saturated, friable				75 42 3/4	soft	NP	PH: 4.0	
20	8 9 16		SP	8-16	Sand, as above TD: 20 feet				75 42 3/4	soft	NP	PH: 4.0	

Notes:

¹ Percent > 3in.

² Sum of gravel, sand, and fines = 100%

³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.

⁴ For noncohesive soil: weak, moderate, strong.

⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR
GSI

LOGGED BY: K. H. H.

DATE: 7-11-05

JOB NO.

FILE NAME: Tysons 344



DRAFT

BOREHOLE LOG 8-1

SITE NAME AND LOCATION: name and location	DRILLING METHOD: Hollow Stem Auger	BORING NO.
		SHEET 1 of 1
	SAMPLING METHOD: SPT, Shelby	DRILLING
		START FINISH
NORTHING	EASTING:	
DATUM: amsl	ELEVATION:	
DRILL RIG:	WATER LEVEL	
ANGLE: 90	TIME	
BEARING: -	DATE	
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.	CASING DEPTH	DATE DATE
	SURFACE CONDITIONS:	

DEPTH IN FEET (ELEVATION)	BLOW/ 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ^{3/} CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
5	13 32 39		SP		Weathered Qtz @ Surface, sandy silty loam, Sand, gravelly, silty, reddish yellow, dry	25	60	15	7.5 4 6	hard	np	PH: 6.5	
10	27 36 37		SP		Sand, gravelly TD = 10 ft							PH: 8.0	
15													

Notes:

¹ Percent > 3inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR GSI

LOGGED BY: K. Silver

DATE: 7-13-06

JOB NO.

FILE NAME: TYPON 3A



DRAFT

BOREHOLE LOG 8-2

SITE NAME AND LOCATION: name and location	DRILLING METHOD: Hollow Stem Auger	BORING NO.
		SHEET
	SAMPLING METHOD: SPT, Shelby	DRILLING
		START FINISH
		DATE DATE
NORTHING DATUM: amsl	EASTING: ELEVATION:	
DRILL RIG: ANGLE: 90	BEARING: -	
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.		
SURFACE CONDITIONS:		

DEPTH IN FEET (ELEVATION)	BLOW/ 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
					Soil, sandy silty loam								PH: 7.0
5	21 25 21		SP		Sand, silty, gravelly, brown, dry (organic)	15	60	25	7.5 42 5/2	firm	NP		PH: 8.0
10	4 5 5		SP Pal		Sand, gravelly, silty Strong brown, sl. Moist	20	60	20	7.5 42 5/6	soft	NP		PH: 5.0
15	37 50+		SP Pty		Sand, gravelly, silty, brown, v. sl. moist TD: 15 ft	30	60	10	7.5 42 5/4	hard	NP		PH: 7.0

Notes:

¹ Percent > 3inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR GSI

LOGGED BY: KIMEN

DATE: 7-13-06

JOB NO.

FILE NAME: Tyrene 3A



DRAFT

BOREHOLE LOG

8-3 Sheet
1 of 2

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET 1 of 2

NORTHING
DATUM: amslEASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DRILLING
START FINISH

DATE DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
1			ML		Tailing, sand, white saturated		100	0	10 4R 8 1/2	weak	M	5.0	
5	20%	Split spoon sank with weight of hammer		10.5-11.5	Tailing Qal		25	60	15	7.5 4R 4/3	firm	NP	PH: 4.0
10	60%		SP	15.5-16.5	silty sand gravelly, light brown, sl moist		20	60	20	7.5 4R 4/3	hard	NP	PH 5.0
15	30%		SP	20.5-21.5	sand, silty, gravelly light brown, sl moist		15	65	20	7.5 4R 5/4	hard	I	PH: 4.5
20	60%		SP	25.5-26.5	sand, silty, gravelly brown, sl moist								PH: 5.5
25	80%				Drilling hard								PH: 4.0

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR GSI

LOGGED BY: Kilmer

DATE: 7-13-06

JOB NO.

FILE NAME: Tyonez 3A



DRAFT

BOREHOLE LOG 8-3

Sheet
2 of 2

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger		BORING NO.	
		SAMPLING METHOD: SPT, Shelby		SHEET 2 of 2	
NORTHING DATUM: amsl		EASTING: ELEVATION:		DRILLING	
				START	FINISH
DRILL RIG: ANGLE: 90 SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.		WATER LEVEL		DATE	
		TIME		DATE	
BEARING: -		CASING DEPTH		DATE	
		SURFACE CONDITIONS:		DATE	

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
25	23 38 50	30%	Qal SP	25-26.5	Drilling Hand Sand, gravelly, silty strong brown, sl. moist	20	55	15	75 42 5/8	hand	np	PH: 4.0	
30	25 25 46	100	SP	30.5-31.5	TD: Sand, gravelly, silty strong brown, sl. moist TD: 30 ft	25	60	15	75 42 5/8	hand	1	PH 7.0	

DRILLING CONTRACTOR

LOGGED BY:

DATE:

JOB NO.

FILE NAME:

Notes:

¹ Percent > 3in.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soft-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.



**Golder
Associates**

DRAFT

BOREHOLE LOG

8-4 Sheet
1 of 2

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET

NORTHING
DATUM: amsl

EASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DRILLING

START FINISH

DATE DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
			ML		Tailing, sand, silty, saturated	0	80	20	75 42 8/6	soft	h	PH: 4.0	
5	5 4 4	100%	Qal SW		Silt, sandy, gravelly loam, dark brown, sl. moist, friable	25	65	10	75 42 4/3	soft	NP	PH: 8.0	
10	3 4 5	100%	SP		Sand, silty, gravelly dark brown, sl. moist	15	60	25	75 42 4/3	firm	1	PH: 8.0	
15	9 13 14	100%	SW		Sand, coarse, gravelly, silty, brown, friable, moist	15	70	15	75 42 5/4	firm	NP	PH: 7.0	
20	16 19 21		GP Qal Qty		Gravel, sandy, silty, strong brown, moist Drilling very hard	50	30	20	75 42 4/6	hard	NP	PH: 4.0	
25													

DRILLING CONTRACTOR

LOGGED BY: KIMSEL

DATE: 7-13-06

JOB NO.

FILE NAME: Tyron 3A

Notes:

¹ Percent > 3in.

² Sum of gravel, sand, and fines = 100%

³ For cohesive soil: soft, firm, hard, v. hard.

⁴ For noncohesive soil: weak, moderate, strong.

⁵ Pocket penetrometer, torevane, in situ density, etc.



**Golder
Associates**

DRAFT

BOREHOLE LOG

8-11 Sheet
2 of 2

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO.

NORTHING
DATUM: amsl

EASTING:
ELEVATION:

SAMPLING METHOD: SPT, Shelby

SHEET

DRILLING
START FINISH

WATER LEVEL

TIME

DATE

CASING DEPTH

DATE DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
25	9 17 25		SP		Sand, gravelly, silty strong brown, sl. moist TD: 25-4		20	65	15	7.5 4R 3/4	V. hard	np	PH: 8.0

DRILLING CONTRACTOR GSI

LOGGED BY: Kilmer

DATE: 7-13-06

JOB NO.

FILE NAME: Tyronc 3A

Notes:

¹ Percent > 3inch.

² Sum of gravel, sand, and fines = 100%

³ For cohesive soil: soft-v. soft, soft, firm, hard, v. hard.

⁴ For noncohesive soil: weak, moderate, strong.

⁵ Pocket penetrometer, torevane, in situ density, etc.



DRAFT

BOREHOLE LOG 8-5

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET

NORTHING
DATUM: amslEASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DRILLING

START FINISH

DATE DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
			ML		Tailing, sand, moist, pink		50	20	75 42 9/4	soft	M	PH: 4.0	
5	4 4 7		Pal SP		Soil on fly, gravelly sandy loam, dark brown, dry		35	40	25 42 4/3	soft		PH: 4.0 PH: 8.0	
10	22 18 25		GP Pal fly		gravel, sandy, silty reddish yellow, dry		45	35	20 42 7/8	hard	NP	PH: 7.0	
15	30 54		GP		gravel, as above, dry TD: 15 ft		45	35	20 42 7/8	hard	NP	PH: 7.0	

DRILLING CONTRACTOR
GSI

LOGGED BY: Kimer

DATE: 7-14-06

JOB NO.

FILE NAME: Tyrone 3A

Notes:

¹ Percent > 3/16 in.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.



DRAFT

BOREHOLE LOG 8-6

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger		BORING NO.					
				SHEET					
		SAMPLING METHOD: SPT, Shelby							
				DRILLING					
NORTHING DATUM: amsl		EASTING: ELEVATION:		WATER LEVEL		START		FINISH	
				TIME					
				DATE					
				CASING DEPTH					
DRILL RIG: ANGLE: 90		BEARING: -		SURFACE CONDITIONS:					
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.									

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
					Soil, sandy, gravelly loam, dark brown (Soil developed on qtz)					7.5 1/4			PH: 7.0
5	12 17 19	100%	GP		gravel, sandy, silty brown, organic, dry	40	40	20		7.5 4/2 5/3	Firm	np	PH: 7.0
10	38 50+		GP		gravel, sandy, silty, reddish yellow, silty moist TD: 10-ft	40	35	25		7.5 4/2 5/3	hard	np	

GSI
DRILLING CONTRACTOR

LOGGED BY: K. W. W.

DATE: 7-14-06

JOB NO.

FILE NAME: Typens 3A

Notes:

- ¹ Percent > 3 inch.
- ² Sum of gravel, sand, and fines = 100%
- ³ For cohesive soil: soft, v. soft, firm, hard, v. hard.
- ⁴ For noncohesive soil: weak, moderate, strong.
- ⁵ Pocket penetrometer, torevane, in situ density, etc.



DRAFT

BOREHOLE LOG 8-7

SITE NAME AND LOCATION: name and location

Spudbed 12' east of (stake) location
(Surveyed Elev. - 4.5 ft)

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET

NORTHING
DATUM: amslEASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DRILLING
START FINISH

DATE DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
5	22 50%	40%	GP		Soil, sandy, gravelly loam, dark brown (soil developed on Gtg) Gravel, sandy, silty, pink, dry TD: 5ft		40	35	25	7.5 42 1/3	hard	np	PH: 8.0 PH: 8.0

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR GSI

LOGGED BY: Kilmer

DATE: 7-16-06

JOB NO.

FILE NAME: Tysons 34



**Golder
Associates**

DRAFT

BOREHOLE LOG 8-8

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger	BORING NO.	
NORTHING DATUM: amsl		EASTING: ELEVATION:	SHEET 1 of 1	
			SAMPLING METHOD: SPT, Shelby	DRILLING
			START	FINISH
			DATE	DATE
DRILL RIG: ANGLE: 90 BEARING: - SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.		SURFACE CONDITIONS:		

DEPTH IN FEET (ELEVATION)	BLOW 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
5	10 4 2 2	70%	SP		fill, cultural debris constant							PH 8.0	
10	16 5 4 7	60%	SP		Sand, coarse, silty, gravelly, strong brown sl moist	20	60	20	7.5 7.5 7.5	firm	L	PH: 8.0	
15	9 13 13	65%	SP		Sand, silty, gravelly, dark grey, vsl. moist	15	65	20	7.5 4.2 4.2	hard	NP	PH: 8.0	
20	16 22 25	100%	SP		Sand, gravelly, silty, strong brown, sl. moist	15	60	25	7.5 4.2 5.6	firm	NP	PH: 8.0	
			SP		Sand, gravelly, silty sl. moist dark brown TD: 20 ft				7.5 4.2 3.2			PH: 7.0	

Notes:

¹ Percent > 3inch.

² Sum of gravel, sand, and fines = 100%

³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.

⁴ For noncohesive soil: weak, moderate, strong.

⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR GSI

LOGGED BY: Kimer

DATE: 7-11-06

JOB NO.

FILE NAME: Tyrene 3A



**Golder
Associates**

DRAFT

BOREHOLE LOG 8-9

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger		BORING NO.	
		SAMPLING METHOD: SPT, Shelby		SHEET	
				DRILLING	
				START FINISH	
NORTHING		EASTING:		WATER LEVEL	
DATUM: amsl		ELEVATION:		TIME	
				DATE	
				CASING DEPTH	
DRILL RIG:		SURFACE CONDITIONS:			
ANGLE: 90		BEARING: -			
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.					

DEPTH IN FEET (ELEVATION)	BLOW 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	25 15 13	10 (Rock)	SP	5-10 grab	Real sand sand, gravelly, silty brown, sl. moist	30	50	20	75 10 5/4	hard	1	PH: 8.0	
10	18 14 19	100%	SP	5-10 grab	Sand, aa, 1-inch layer of darker organic	30	50	20	75 10 5/4	firm	1	PH: 7.0	
15	11 17 14		SP		Sand, silty, minor gravel strong brown, sl. moist	10	70	30	75 10 5/6	firm	1	PH: 7.0	
					Sand, gravelly, silty friable brown, sl. moist	20	65	15	75 10 5/4	firm	NP	PH: 7.0	
20	7 20 17		SP		Sand, silty, gravelly dark brown, moist Buried soil, roots TD: 20 ft	15	60	25	75 10 3/2	firm	1	PH: 7.0	

Notes:

¹ Percent > 3 inch.

² Sum of gravel, sand, and fines = 100%

³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.

⁴ For noncohesive soil: weak, moderate, strong.

⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR GSI

LOGGED BY: K. H. M. S. R.

DATE: 7-1-06

JOB NO.

FILE NAME: Tyne 30



**Golder
Associates**

DRAFT

BOREHOLE LOG 8-10

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger		BORING NO.			
				SHEET			
		SAMPLING METHOD: SPT, Shelby					
				DRILLING			
NORTHING DATUM: amsl		EASTING: ELEVATION:		START		FINISH	
				WATER LEVEL			
				TIME			
				DATE			
				CASING DEPTH			
DRILL RIG:		SURFACE CONDITIONS:					
ANGLE: 90		BEARING: -					
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.							

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (mp, l, m, h)	OTHER TESTS ⁵
5	26 100	100	SP	5-gallon grab sample	ROAD way fill Sand, gravelly, silty, reddish grey (low sl. moist)	20	50	30	75 42 5/6	firm	1	PH: 8.0	
10	17 39 31	100	SP	5-gallon grab sample	Sand, gravelly, silty VERY dark grey, sl. moist	20	60	20	75 42 13/1	firm	1	PH: 7.0	
15	5 102 11		SP		Sand, gravelly, silty, light brown, friable sl. moist	25	65	10	75 42 5/4	firm MP		PH: 8.0	
20	30 50		SP		Sand, gravelly, silty very dark grey, sl. moist	25	65	10	75 42 13/1	firm MP		PH: 7.0	
					Sand, gravelly, silty, strong brown	20	60	20	75 42 5/6	firm	1	PH: 7.0	
					TD: 20 ft								

- Notes:
- ¹ Percent > 3 inch.
 - ² Sum of gravel, sand, and fines = 100%
 - ³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.
 - ⁴ For noncohesive soil: weak, moderate, strong.
 - ⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR
GSI

LOGGED BY: C. M. M. S. A.
DATE: 7-11-06

JOB NO.
FILE NAME: Tyners 30A



DRAFT

BOREHOLE LOG

9-1

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger		BORING NO.			
				SHEET 1 of 1			
		SAMPLING METHOD: SPT, Shelby		DRILLING			
				START	FINISH		
NORTHING DATUM: amsl		EASTING: ELEVATION:		WATER LEVEL			
				TIME			
				DATE			
				CASING DEPTH			
DRILL RIG: ANGLE: 90		BEARING: -		DATE		DATE	
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.		SURFACE CONDITIONS:					

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
5	20 12 9	100	SP	56.5 Brass Tube 0-5 Split sample	Sil, gravelly loam, silty sandy, pink sand, silty, gravelly sl. moist yellowish red	80	20	20	7.5 4.2 8 1/4	firm	NP	pH: 7.0	
	12 13 7	100	SP	56.5 Brass Tube 5-10 Split sample	sand, gravelly, silty, light brown, sl. moist	15	50	35	7.5 4.2 6 1/8	firm	1	pH: 7.0	
						25	50	25	7.5 4.2 6 1/4	firm	1	pH: 7.0	
10	8 33 26	100	SP	56.5 Brass Tube 10-11.5 Split sample	sand, gravelly, sandy, very dark greyish brown, v. moist (scic)	15	65	20	10 4.2 3 1/2	firm hard	M	pH: 4.0	
15	9 40 31	80	SP		sand, gravelly, as above, significant wood - tree roots saturated in lowest portion	15	65	20	10 4.2 3 1/2	hard	M	pH: 4.0	
20	17 25 44		SP		sand, gravelly, silty, pink, sl moist TD: 20-ft	15	70	15	5.2 7 1/3	hard	NP	pH: 7.0	

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

GSI

DRILLING CONTRACTOR

LOGGED BY: Kri/mse

DATE: 7-10-06

JOB NO.

FILE NAME: Tyronse 3A



**Golder
Associates**

DRAFT

BOREHOLE LOG 10-2

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET

NORTHING
DATUM: amsl

EASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DRILLING
START FINISH

DATE DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN FEET (ELEVATION)	BLOW/ 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
5	12 15 17		SP		Soil, sandy silty loam, dark brown	5	70	25	75 42 5/4	Soft	1		
					Sand, silty, gravelly brown	10	65	25	75 42 5/4	Firm	np	PH: 7.0	
				Q _{cal} Q _{tg}	Drilling hard				75 42 5/4				
10	24 37 37		SP		Sand, gravelly, silty, reddish yellow, 5% moist	20	70	10	75 42 5/4	V. hard	np	PH: 7.0	
15													
20					TD: 10 ft								

Notes:

¹ Percent > 3inch.

² Sum of gravel, sand, and fines = 100%

³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.

⁴ For noncohesive soil: weak, moderate, strong.

⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR *AST*

LOGGED BY: *Kimberly*

DATE: 7-17-06

JOB NO.

FILE NAME: *Tyrene 3A*



DRAFT

BOREHOLE LOG 10-3

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET

NORTHING
DATUM: amslEASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DRILLING
START FINISH

DATE DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
5	13 18 17	54%	SP		Soil, Sandy gravelly silty, brown, dk brown organic Sand, silty gravelly, dk brown, dry		15	60	25	7.5 7.2 7.4 7.5 7.2 6.4	firm	np	PH: 6.5
10	11 12	64%	SP		Sand, gravelly, silty Strong brown, dry		25	60	15	7.5 7.2 5.6	hard	np	PH: 7.0
15	17 23 28	85%	GP		Gravelly, cobbly, sand, pink v. sl. moist Drilling hard	10	60	20	10	7.5 7.2 7.4	hard	np	PH: 7.0
20	38 54		SP		sand, silty, strong brown, sl. moist TD: 20 ft	5	75	20		7.5 7.2 5.6	hard	np	PH: 7.0
25													

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soft, v. soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, corevane, in situ density, etc.

DRILLING CONTRACTOR GSI

LOGGED BY: H. H. H.

DATE: 7-17-06

JOB NO.

FILE NAME: Tyronne 3A



DRAFT

BOREHOLE LOG

10-4 sheet
10fz

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET

NORTHING
DATUM: amslEASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DRILLING
START FINISH

DATE DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (pp, l, m, h)	OTHER TESTS ⁵
5	6 50	30%	GP		Soil sandy, gravelly, brown								
10	9 9 12	60%	SP		Gravel, sandy, silty, brown, dry	50	30	20	7.5 1/2 5/4	firm	NP	PH: 7.0	
15	7 11 4	30%	SP		Sand, silty, gravelly, light brown, dry	15	65	20	7.5 1/2 5/4	firm	NP	PH: 7.0	
20	13 11 13	60%	SP		Sand, silty, gravelly, light brown, dry	15	65	20	7.5 1/2 5/3	firm	NP	PH: 7.0	
25					Sand, gravelly, silty, pink, dry	20	70	10	7.5 1/2 2/3	firm	NP	PH: 7.5	

Notes:

¹ Percent > 3inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR GSI

LOGGED BY: GSI/MSH

DATE: 7-17-06

JOB NO.

FILE NAME: 10fz 3A



DRAFT

BOREHOLE LOG 10-4

Sheet
2 of 2

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SHEET

SAMPLING METHOD: SPT, Shelby

DRILLING

START FINISH

NORTHING
DATUM: amslEASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DATE

DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
25	7 17 20	30	SP		Sand, silty, gravelly, yellowish red, dry	20	60	20	5/6	hard	np	PH: 7.0	
30	12 21 20	60%	GP		Gravel, sandy, silty, yellowish red, sl. moist	65	25	10	5/6	hard	np	PH: 5.0	
35	50		GP		Gravel, as above	65	25	10	5/6	hard	np	5.5	
40					TD: 35 ft								

DRILLING CONTRACTOR GSC

LOGGED BY: K. W. S.

DATE:

JOB NO.

FILE NAME:

Tysons 364

7-17-06

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.



DRAFT

BOREHOLE LOG 16-5 *Sheet 1 of 2*

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger		BORING NO.	
				SHEET	
NORTHING		EASTING:		DRILLING	
DATUM: amsl		ELEVATION:		START FINISH	
		WATER LEVEL			
		TIME			
		DATE		DATE DATE	
		CASING DEPTH			
DRILL RIG:		SURFACE CONDITIONS:			
ANGLE: 90					
BEARING: -					
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.					

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
5	7 11 9	70%	SP		Soil, sandy gravelly loam, sand, gravelly, silty brown, dry	25	60	15	75 4 5/4	7.5 9/4	soft	np	PH: 6.5
10	13 14 16	30%	SP		Sandy gravelly, silty light brown, dry	20	65	15	75 4 6/4	7.5	firm	np	PH: 6.5
15	8 2 8	75%	SP		Sand, silty, gravelly brown, friable, sl. moist	15	65	25	75 4 3/4	7.5	firm	np	8.0
20	11 11 5	0%			rock in sampler end								
25													

- Notes:
- ¹ Percent > 3 inch.
 - ² Sum of gravel, sand, and fines = 100%
 - ³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.
 - ⁴ For noncohesive soil: weak, moderate, strong.
 - ⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR

LOGGED BY:

DATE:

JOB NO.

FILE NAME:



DRAFT

BOREHOLE LOG 10-5

Sheet
2 of 2

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET

NORTHING
DATUM: amslEASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DRILLING
START FINISH

DATE DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
25	11 19 27	75%	SP		Sand, gravelly, reddish yellow, dry	30	55	15	7.5 4 5/8	firm	NP	PH: 7.0	
30	37 43 32	40%	SP		Sand, silty, gravelly, strong brown, sl. moist	15	60	25	7.5 4 5/8	hard	NP	PH: 4.5	
35	23 36 40	100%	SP	QAL Otg	Sand, silty, gravelly, strong brown, moist -Drilling hard-	15	55	30	7.5 4 5/8	firm	I	PH: 4.0	
40	15 46 504		SP		Sand, gravelly, silty, yellowish red, sl. moist TD: 40 ft	25	50	25	5 4 5/8	hard	NP	PH: 6.0	

DRILLING CONTRACTOR

GSI

LOGGED BY:

Kilmer

DATE:

7-14-06

JOB NO.

FILE NAME:

Tyacne 3A

Notes:

¹ Percent > 3inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.



DRAFT

BOREHOLE LOG 10-6 *Sheet 1 of 2*

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger		BORING NO.					
				SHEET					
		SAMPLING METHOD: SPT, Shelby							
				DRILLING					
NORTHING DATUM: amsl		EASTING: ELEVATION:		WATER LEVEL		START		FINISH	
				TIME					
				DATE					
				CASING DEPTH					
DRILL RIG: ANGLE: 90		BEARING: -		SURFACE CONDITIONS:					
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.									

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
					Soil, gravelly sandy loam								
5	8 11	30%	SP		Sand, gravelly, silty reddish yellow, dry	25	60	15	75 42 5/8	SOFT	NP	PH: 7.0	
10	8 13 16	30%	SP		Sand, gravelly, silty reddish yellow, dry	25	60	15	75 42 5/8	FIRM	NP	PH: 7.0	
15	9 9 14	60%	SP		Sand, silty, gravelly reddish yellow, dry	15	65	20	75 42 5/8	FIRM	NP	PH: 7.0	
20	8 11 14	60%	SP		Sand, silty, gravelly reddish yellow, dry	15	65	20	75 42 5/8	FIRM	NP	PH: 7.0	
25	18 23 27												

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.DRILLING CONTRACTOR *GST*LOGGED BY: *KJ/mc*DATE: *7-14-06*

JOB NO.

FILE NAME: *TyKone 34*



**Golder
Associates**

DRAFT

BOREHOLE LOG

10-6 Sheet
2 of 2

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger		BORING NO.	
				SHEET	
		SAMPLING METHOD: SPT, Shelby			
				DRILLING	
NORTHING		EASTING:		START	
DATUM: amsl		ELEVATION:		FINISH	
		WATER LEVEL			
		TIME			
		DATE			
		CASING DEPTH			
DRILL RIG:		SURFACE CONDITIONS:			
ANGLE: 90					
BEARING: -					
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.					

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
25	18 28 27	60	SP		Sandy, gravelly, silty, Reddish yellow, dry	25	55	20	7.5 yr 5/8	hard	NP	7.0	
30	22 25 21	60%	SP Qal Qtz		Sand, gravelly, silty, Strong brown, sil. moist Drilling hard -	30	50	20	7.5 yr 5/8	hard	NP	PH: 4.0	
35	504	30%	GP		Gravel, sandy, strong brown TD: 35 ft	50	30	20	7.5 yr 5/6	v. hard	NP	PH: 4.0	

DRILLING CONTRACTOR

LOGGED BY:

DATE:

JOB NO.

FILE NAME:

Notes:

¹ Percent > 3 inch.

² Sum of gravel, sand, and fines = 100%

³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.

⁴ For noncohesive soil: weak, moderate, strong.

⁵ Pocket penetrometer, torevane, in situ density, etc.



DRAFT

BOREHOLE LOG

10-7

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET

NORTHING
DATUM: amslEASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DRILLING
START FINISH

DATE DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN FEET (ELEVATION)	BLOW/ 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
5	15 30 25	100%	Qar SP		Soil, sandy gravelly loam Sand, gravelly, silty, organic, dark brown, clay		30	50	20	7.5 12 4/3	v. hard	np	6.5
10	39 50+		SP		Sand, gravelly, reddish yellow, silty, moist, friable					7.5 12 4/3	v. hard	np	7.5
15													
20													
25													

DRILLING CONTRACTOR
GSI

LOGGED BY: Kimsok

DATE: 7-14-06

JOB NO.

FILE NAME: Tyee 3A

Notes:

¹ Percent > 3inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.



**Golder
Associates**

DRAFT

BOREHOLE LOG 10-8

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger		BORING NO.			
				SHEET			
		SAMPLING METHOD: SPT, Shelby					
				DRILLING			
NORTHING DATUM: amsl		EASTING: ELEVATION:		START		FINISH	
				DATE		DATE	
DRILL RIG: ANGLE: 90 BEARING: - SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.				SURFACE CONDITIONS:			

DEPTH IN FEET (ELEVATION)	BLOW/ 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (pp, l, m, h)	OTHER TESTS ⁴
5	8 11	50%	SP		Soil, sandy gravelly loam								
10	19 23 50+	70%	GP Qal Qty		Sand, silty, gravelly brown, dry, friable,	15	65	20	75 40 5/2	soft	np	PH: 7.5	
15	50+	20%	SP		Gravel, sandy, silty, Strong brown, dry	45	30	25	75 40 5/6	hard	np	PH: 8.0	
20					Sand, gravelly, silty reddish yellow, sl. moist, friable	25	45	30	75 70 5/8	hard	np	PH: 7.0	
					TD: 15 ft								

Notes:

¹ Percent > 3 inch.

² Sum of gravel, sand, and fines = 100%

³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.

⁴ For noncohesive soil: weak, moderate, strong.

⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR GSI

LOGGED BY: KILMER

DATE: 7-11-06

JOB NO. Tyconz 3A
FILE NAME:



**Golder
Associates**

DRAFT

BOREHOLE LOG 10-11

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger		BORING NO.	
				SHEET 1061	
		SAMPLING METHOD: SPT, Shelby		DRILLING	
				START FINISH	
NORTHING		EASTING:		DATE	
DATUM: amsl		ELEVATION:		DATE	
DRILL RIG:		SURFACE CONDITIONS:			
ANGLE: 90					
BEARING: -					
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.					

DEPTH IN METERS (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
5	10 100	50%	SP	SP	Road grade, sandy gravel, silty, reddish yellow	40	35	25	54 7/6	Soft	NP	PH 7.0	
5	80	50%	SP	SP	sand, gravelly, silty reddish yellow	20	50	30	52 6/6	Soft	1	PH 7.0	
10	79 11	100	SP	SP	Sand, gravelly, silty st moist, dark brown	15	50	35	7.5 42 3/2	firm	M	PH 7.0	
15	18 22 50+	100	SP	SP	Drilling hard - Gila P.B. Sand, as above	15	55	30	7.5 42 3/2	hard	L	PH 7.0	
20	9 10 17	20%	GP	GP	Gravel, sandy reddish yellow clay TD: 20-ft	40	35	25	75 42 4/6	firm	NP	PH 7.0	

Notes:

- Percent > 3 inch.
- Sum of gravel, sand, and fines = 100%
- For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.
- For noncohesive soil: weak, moderate, strong.
- Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR
GSI

LOGGED BY: Kimer
DATE: 7-10-05

JOB NO.
FILE NAME: Tyeone 3A



**Golder
Associates**

DRAFT

BOREHOLE LOG 11-1

3A Soil boring

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO. 11-1

SAMPLING METHOD: SPT, Shelby

SHEET

NORTHING
DATUM: amsl

EASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DRILLING

START FINISH

DATE 7/18 DATE 7/18

SURFACE CONDITIONS:

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

DEPTH IN FEET (ELEVATION)	BLOW/ 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ^{3/} CEMENTATION ⁴	PLASTICITY (mp, l, m, h)	OTHER TESTS ⁴
5	35	50%	SP/gm		silly sandy gravel dry (7.5 PR 7/6) soft, reddish brn, pH=7.5	15	50	35	7.5 4/2 7/6	NC	NP	pH=7.5	
6.5					Drilled 6.5' into Gila. From Gila								
15					Gila from surface								

Notes:

¹ Percent > 3inch.

² Sum of gravel, sand, and fines = 100%

³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.

⁴ For noncohesive soil: weak, moderate, strong.

⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR Geomechanics Southwest Inc.

LOGGED BY: Steven Schindler

DATE: 7/18/2006

JOB NO. 013-1595-002

FILE NAME:



**Golder
Associates**

DRAFT

BOREHOLE LOG 11-2

3A Boring

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET

NORTHING
DATUM: amsl

EASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

SURFACE CONDITIONS:

DRILLING

START

FINISH

7/12

7/12

DATE

DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
------------------------------	------------	----------	--------	------------	---	-------------------------	-----------------------	---------------------	----------------------	-------	--	-----------------------------	--------------------------

5	50	30%	SP		Sandy silty gravel, slightly moist, (P/S 4/1R 5/16) PH=7.5-8.0 slightly Firm, slightly cemented	15	50	35	7.5	11R 5/16		NP	
6.5													
10					Drilled 5' into Gila from surface								
15													

Notes:

¹ Percent > 3inch.

² Sum of gravel, sand, and fines = 100%

³ For cohesive soil: soft, firm, hard, v. hard.

⁴ For noncohesive soil: weak, moderate, strong.

⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR GST

LOGGED BY: Steven Schindler

DATE: 7/10/2000

JOB NO. 013-1595-002

FILE NAME:



DRAFT

BOREHOLE LOG 11-3

SITE NAME AND LOCATION: name and location	DRILLING METHOD: Hollow Stem Auger	BORING NO.
		SHEET
	SAMPLING METHOD: SPT, Shelby	DRILLING
		START FINISH
		DATE DATE
NORTHING DATUM: amsl	EASTING: ELEVATION:	
DRILL RIG: ANGLE: 90	BEARING: -	
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.		
SURFACE CONDITIONS:		

DEPTH IN FEET (ELEVATION)	BLOW/ 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, L, m, h)	OTHER TESTS ⁴
5													
6													
7													
8													
9													
10													
11													
12													
13													
14													
15													

Notes:

¹ Percent > 3inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR GSI

LOGGED BY: Steven Schindler

DATE: 7/18/2006

JOB NO. 013-1595-002

FILE NAME:



**Golder
Associates**

DRAFT

BOREHOLE LOG 11-4

3A soil borings

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger		BORING NO.	
		SAMPLING METHOD: SPT, Shelby		SHEET	
		WATER LEVEL		DRILLING	
		TIME		START FINISH	
NORTHING		EASTING:		DATE	
DATUM: amsl		ELEVATION:		DATE	
DRILL RIG:		CASING DEPTH		DATE	
ANGLE: 90		SURFACE CONDITIONS:		DATE	
BEARING: -					
SAMPLER: 2.0 In. OD Split Spoon, 140 lb hammer.					

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ^{3/} CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	25	70%	SP		6 in 3 6'								
6	40				sandy silty gravel soft	10	45	45					
7	50				poorly cemented, silty to sub					(7.5 4/18)	np		pH 5.5
8					rd gravel, dry, 7.5 4/12 6/6					6/6	poor		
9					pH 5.5								
10	34	80%	SP		sandy silty gravel,	10	45	45		7.5 4/18	slight L		pH 7.5
11	37				slight to moderate cemented,					6/6			
12	40				slightly moist, firm,								
13					(7.5 4R 6/6) pH 7.5								
15	29	80%	SP		sandy silty gravel	5	50	45		7.5 4/18	moder L		pH 7.5
16.5	50				slightly moist, soft to friable					6/6			
17.5					7.5 4/18 6/6 pH 7.5								
20	47	30%	SP		21.5'								
21.5					same as above								
					pH: 7.0								pH 7.0

Notes:

¹ Percent > 3 inch.

² Sum of gravel, sand, and fines = 100%

³ For cohesive soil: soft-v. soft, soft, firm, hard, v. hard.

⁴ For noncohesive soil: weak, moderate, strong.

⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR GSD

LOGGED BY: Steven Schindler

DATE: 7/18/00

JOB NO. 013-1595-002

FILE NAME:



DRAFT

BOREHOLE LOG 11-5

3A soil Boring

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET

NORTHING
DATUM: amslEASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DRILLING

START FINISH

DATE 7/18 DATE 7/18

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
3'	20	80%	SP		sandy silt, gravel, dry, Firm 7.54IR 6/6 pH=7.0.	10	10	40	50	7.54IR 6/6	slight	np	pH 7.0
4'	36												
5'	50												
6'													
7'													
8'	48	80%	SP		as above, Firm, sandy silty gravel, dry, (7.54IR 5/6) pH 7.0	10	10	40	50			np	pH 7.0
13'	40	40%											
14.5'	50												
18'													
23'													

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, corevane, in situ density, etc.

DRILLING CONTRACTOR GSI

LOGGED BY: Steven Schindler

DATE: 7/18/2006

JOB NO. 013-1595-002

FILE NAME:



**Golder
Associates**

DRAFT

BOREHOLE LOG 11-6

3A Soil Boring

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET

Grass samples

DRILLING

NORTHING

EASTING:

DATUM: amsl

ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

START FINISH

DATE 7/3 DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL: (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
------------------------------	------------	----------	--------	------------	--	-------------------------	-----------------------	---------------------	----------------------	-------	--	-----------------------------	--------------------------

0'					Soil, brown								
5'	10 14 14	80%	SP		Sandy silty gravel soft, 7.5 4R (7/6) pH 7.0, dry	10	50	40	7.5 4R 7/6	soft	np	pH 7.0	
10'	29 36 56	60%	SP		Gravelly sandy silt (10 4R 6/6) dry, firm, pH = 6.0 more gravels, & cementation	20	40	40	10 4R 6/6	Firm	np	pH 6.0	
15'	50	20%	SP/ GM										
20'					TD 16.5' Gravelly silty sand, (10 4R 6/6) Hard, dry, pH 6.0	25	35	40	10 4R 6/6	Hard	np	pH 6.0	

Notes:

¹ Percent > 3/16 in.

² Sum of gravel, sand, and fines = 100%

³ For cohesive soil: soft-v. soft, soft, firm, hard, v. hard.

⁴ For noncohesive soil: weak, moderate, strong.

⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR **GSI**

LOGGED BY: Steven Schindler

DATE: 7/18/2006

JOB NO. 013-1595-002

FILE NAME:



**Golder
Associates**

DRAFT

BOREHOLE LOG

11-7

SITE NAME AND LOCATION: name and location

Spotted on fill (+2.5 ft) above
surveyed point

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET

NORTHING

DATUM: amsl

EASTING:

ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DRILLING

START FINISH

DATE DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN FEET (ELEVATION)	BLOW/ 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
					Soil, sandy gravelly, silty loam								
5	9 10 13	70%	SP		sand, gravelly, silty reddish yellow, dry	25	60	15	7.5 1/2 5/8	firm	np		PH: 6.5
10	14 15 20	50%	SP		sand, silty, gravelly reddish yellow, dry	15	50	25	7.5 1/2 6/8	firm	np		PH: 7.0
15	6 17 35	80%	GP		Gravel, sandy, silty yellowish red, dry	55	25	20	5.4 3/8	hard	np		PH: 6.5
20	19 32 32	70%	GP		Gravel, sandy, as above	55	25	20	5.4 3/8	hard	np		PH: 6.5
				Pal Phg	— peeling Hard								
25	54 60	60%	SP		sand, silty, gravelly, pink U.S.I. moist	10	85	25	7.5 1/2 3/4	hard	np		PH: 7.0



**Golder
Associates**

DRAFT

BOREHOLE LOG

11-8 Sheet
1 of 2

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET

NORTHING
DATUM: amsl

EASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DRILLING

START FINISH

DATE DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ^{3/4} CEMENTATION ⁴	PLASTICITY (U_p , L , m , h)	OTHER TESTS ⁵
5	7 11 15	100%	SP		Soil, sandy silty loam dark brown								
10	9 11 11	75%	SP		Sand, silty, gravelly reddish yellow, dry	15	65	20	7.5 42 6/6	firm	np	PH: 6.5	
15	10 12 14	95%	SP		Sand, gravelly, silty, strong brown, dry	25	60	15	7.5 42 5/6	hard	np	PH: 6.5	
20	14 32 24	80%	SP		Sand, silty, strong brown, dry	10	65	25	7.5 42 5/8	hard	np	PH: 7.0	
25	14 32 24	80%	SP		Gravel, sandy, silty, pinkish grey, sl. moist	40	35	25	7.5 42 7/2			PH: 7.0	

Notes:

¹ Percent > 3 inch.

² Sum of gravel, sand, and fines = 100%

³ For cohesive soil: soft, soft, firm, hard, v. hard.

⁴ For noncohesive soil: weak, moderate, strong.

⁵ Pocket penetrometer, torvane, in situ density, etc.

DRILLING CONTRACTOR **GSI**

LOGGED BY: **K. M. J.**

DATE: **7-19-06**

JOB NO.

FILE NAME: **Hydrom 312**



**Golder
Associates**

DRAFT

BOREHOLE LOG

11-8 *Sheet*
2012

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET

NORTHING
DATUM: amsl

EASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DRILLING

START FINISH

DATE DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁴
25	23 22 18	100	SP		Sand, silty, gravelly, reddish yellow, friable, v. sl. moist	15	65	20	7.5 42 6/8	hand	np	PH: 7.0	
30	20 28 35	70%	SP		Sand, silty, gravelly reddish yellow, friable sl. moist	15	65	20	7.5 42 7/8	hand	np	PH: 7.0	
35	50+5%	rock	GP		Gravel, sandy, silty Reddish yellow, dry	50	30	20	7.5 42 6/8	hand	np	PH: 7.0	
40	50+20%	SP			Sand, gravelly, silty Reddish yellow, dry TD: 40 ft	20	65	15	7.5 42 6/8	hand	np	PH: 7.0	

GSI

DRILLING CONTRACTOR

Kimber

LOGGED BY:

7-19-06

DATE:

JOB NO.

FILE NAME: *Tyrens 3A*

Notes:

¹ Percent > 3inch.

² Sum of gravel, sand, and fines = 100%

³ For cohesive soil: soft, firm, hard, v. hard.

⁴ For noncohesive soil: weak, moderate, strong.

⁵ Pocket penetrometer, corevane, in situ density, etc.



**Golder
Associates**

DRAFT

BOREHOLE LOG 11-9 Sheet 1 of 2

SITE NAME AND LOCATION: name and location <i>Spadston Hill 2' above sunny point</i>		DRILLING METHOD: <i>Hollow Stem Auger</i>		BORING NO.	
NORTHING DATUM: amsl		SAMPLING METHOD: <i>SPT, Shelby</i>		SHEET	
EASTING: ELEVATION:		WATER LEVEL		DRILLING START FINISH	
		TIME		DATE	
		DATE		DATE	
DRILL RIG: ANGLE: 90 SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.		SURFACE CONDITIONS:			
BEARING: -					

DEPTH IN FEET (ELEVATION)	BLOW 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ^{3/} CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	9 8 9	60%	SP		Soil, sandy loam, clay brown								
10	11 22 23	60%	SP		Sand, silty, gravelly reddish yellow, dry	15	65	20	75 42 5/6	firm	np	PH: 7.0	
15	8 18 19	60%	SP		Sand, silty, gravelly reddish yellow, dry	15	65	20	75 42 5/6	firm	np	PH: 7.0	
20	18 24 25	50%	SP		Sand, silty, gravelly reddish yellow	15	60	25	75 42 5/6	hard	np.	PH: 7.0	
25					Drilling hard								

- Notes:
- Percent > 3/16 in.
 - Sum of gravel, sand, and fines = 100%
 - For cohesive soil: soft, firm, hard, v. hard.
 - For noncohesive soil: weak, moderate, strong.
 - Pocket penetrometer, corevane, in situ density, etc.

DRILLING CONTRACTOR *GSI*

LOGGED BY: *Kilmer*
DATE: *7-19-06*

JOB NO.
FILE NAME: *tysons 3A*



**Golder
Associates**

DRAFT

BOREHOLE LOG

11-9 sheet
2 of 2

SITE NAME AND LOCATION: name and location

DRILLING METHOD: Hollow Stem Auger

BORING NO.

SAMPLING METHOD: SPT, Shelby

SHEET

NORTHING
DATUM: amsl

EASTING:
ELEVATION:

WATER LEVEL

TIME

DATE

CASING DEPTH

DRILLING
START FINISH

DATE DATE

DRILL RIG:

ANGLE: 90

BEARING: -

SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.

SURFACE CONDITIONS:

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
25	42 50%	60%	SP		Sand; silty, gravelly light brown, dry TD: 25-66		10	65	25	2.5 4/4	hard	np.	PH: 7.0
30													
35													

GSI

DRILLING CONTRACTOR

LOGGED BY: K. J. M. S. R.

DATE: 7-19-06

JOB NO.

FILE NAME: 149000 3A

Notes:

¹ Percent > 3 inch.

² Sum of gravel, sand, and fines = 100%

³ For cohesive soil: soft, firm, hard, v. hard.

⁴ For noncohesive soil: weak, moderate, strong.

⁵ Pocket penetrometer, torevane, in situ density, etc.



BOREHOLE LOG

11-10

SITE NAME AND LOCATION: name and location	DRILLING METHOD: Hollow Stem Auger							BORING NO.	
	SAMPLING METHOD: SPT, Shelby							SHEET	
								DRILLING	
								START	FINISH
NORTHING	EASTING:	WATER LEVEL							
DATUM: amsl	ELEVATION:	TIME							
		DATE							
		CASING DEPTH							
DRILL RIG:		SURFACE CONDITIONS:							
ANGLE: 90									
BEARING: -									
SAMPLER: 2.0 In. OD Split Spoon, 140 lb hammer.									

[illegible]¹ Percent > 3inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.

⁴ For noncohesive soil: weak, moderate, strong.

⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR

LOGGED BY: Kimsek

DATE: 7-19-06

JOB NO.

FILE NAME: Tyrene 314



DRAFT

BOREHOLE LOG 11-12

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger		BORING NO.			
				SHEET			
		SAMPLING METHOD: SPT, Shelby					
				DRILLING			
NORTHING DATUM: amsl		EASTING: ELEVATION:		START		FINISH	
				WATER LEVEL			
				TIME			
				DATE			
DRILL RIG: ANGLE: 90 SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.		SURFACE CONDITIONS:		DATE		DATE	
				CASING DEPTH			

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	50%	20%	SP	Pal Qty	Spudded on weathered Gila outcrop, Soil, Sandy, silty loam, dk brown Sandy, silty, gravelly reddish yellow, friable, dry TD: 5 ft		15	75	10	7.5 yr 6%	hard	np.	PH: 7.0
10													
5													

Notes:

- ¹ Percent > 3 inch.
- ² Sum of gravel, sand, and fines = 100%
- ³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.
- ⁴ For noncohesive soil: weak, moderate, strong.
- ⁵ Pocket penetrometer, torevane, in situ density, etc.

GSI

DRILLING CONTRACTOR

LOGGED BY: K. H. M. S. R.

DATE: 7-19-06

JOB NO.

FILE NAME: Tyrens 3A



DRAFT

BOREHOLE LOG 11-13

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger		BORING NO.			
				SHEET			
		SAMPLING METHOD: SPT, Shelby					
				DRILLING			
NORTHING DATUM: amsl		EASTING: ELEVATION:		START		FINISH	
				WATER LEVEL			
				TIME			
				DATE			
DRILL RIG: ANGLE: 90 SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.		BEARING: -		CASING DEPTH		DATE	
				SURFACE CONDITIONS:			

DEPTH IN FEET (ELEVATION)	BLOW/ 6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
5	504	30%	SP	Par Qty	Spudded on weathered gila, soil, sandy, silty loam, dark brown								
10					Sand, gravelly, silty reddish yellow	25	60	15	7.5 42 6/6	hard	Np	PH: 7.0	
					TD: 5 ft								

Notes:

¹ Percent > 3inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soil-v. soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR

LOGGED BY:

DATE:

JOB NO.

FILE NAME:

GST

Kilmer

7-19-06

Tyreone 3A



DRAFT

BOREHOLE LOG 11-14

SITE NAME AND LOCATION: name and location		DRILLING METHOD: Hollow Stem Auger		BORING NO.	
		SAMPLING METHOD: SPT, Shelby		SHEET 1 of 1	
NORTHING DATUM: amsl		EASTING: ELEVATION:		DRILLING	
				START	
				FINISH	
				DATE	
DRILL RIG: ANGLE: 90		BEARING: -		DATE	
SAMPLER: 2.0 in. OD Split Spoon, 140 lb hammer.		SURFACE CONDITIONS:		DATE	

DEPTH IN FEET (ELEVATION)	BLOW/6 IN.	RECOVERY	SYMBOL	SAMPLE NO.	SAMPLE NUMBER AND DESCRIPTION OF MATERIAL (i.e., angularity, moisture, HCL reaction, cementation, max. particle size, gravel/cobble hardness, odor, interbeds, laminations)	% OVERSIZE ¹	% GRAVEL ²	% SAND ²	% FINES ²	COLOR	CONSISTENCY ³ / CEMENTATION ⁴	PLASTICITY (np, l, m, h)	OTHER TESTS ⁵
11	11	100%	GP	Gravel sample 0-5	Reddish brown, sandy gravelly loam, reddish brown	25	60	15	54 1/3	Soft	L	PH 7.0	
11	11	100%	GP	Gravel sample 5-10	gravel, sandy, silty, reddish brown, silty moist	45	30	25	54 1/4	Soft	M	PH 7.0	
5	5	60%	SP	Gravel sample 5-10	Sand, silty, gravelly, dark reddish brown v. silty moist	20	60	20	54 3/2	Firm	M	PH 7.0	
10	8	100%	SP	Gravel sample 10-15		10	75	15	54 1/4	Firm	L	PH 7.0	
15	4	100%	SP		Firm Drilling Top of Gravel?	10	75	15	54 3/4	Firm	L	PH 7.0	
20	38	50%	SP		Sandy gravelly, silty Reddish yellow, v. silty moist TD = 20 ft	10	75	15	54 1/2	Firm	N	PH 7.0	

Notes:

¹ Percent > 3 inch.² Sum of gravel, sand, and fines = 100%³ For cohesive soil: soft, soft, firm, hard, v. hard.⁴ For noncohesive soil: weak, moderate, strong.⁵ Pocket penetrometer, torevane, in situ density, etc.

DRILLING CONTRACTOR GSI

LOGGED BY: K. H. H. H.










DATE: 7-6-06

JOB NO.

FILE NAME: Tyone SP 3A

APPENDIX VI

Stability Output

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Strength Type	Cohesion (psf)	Phi (deg)	Water Surface	Hu Type	Hu	Ru
QTg - Gila Conglomerate		125	138	Mohr-Coulomb	1000	35	Water Surface	Custom	1	
pCg - Burro Mountain Granite		160	160	Mohr-Coulomb	48960	35	Water Surface	Custom	1	
Tqm - Quartz Monzonite + Intrusive Rocks		160	160	Mohr-Coulomb	96336	43	Water Surface	Custom	1	
Qa - Alluvium		125	138	Mohr-Coulomb	0	29	Piezometric Line 1	Custom	1	
Qa - Alluvium (liquefied)		125	138	Mohr-Coulomb	0	8	Piezometric Line 1	Custom	1	
Waste Rock		125	138	Mohr-Coulomb	1656	30.9	Piezometric Line 1	Custom	1	
Leached Ore		125	138	Mohr-Coulomb	288	35.5	Piezometric Line 1	Custom	1	
Structural Fill		125		Mohr-Coulomb	0	35.5	Piezometric Line 1	Custom	1	
Liner		125	138	Mohr-Coulomb	0	10	None			0



Tyrone Mine Closure Stockpile Stability - Soil and Rock Mass Material Properties

Analysis Description

Soil Legend

Figure

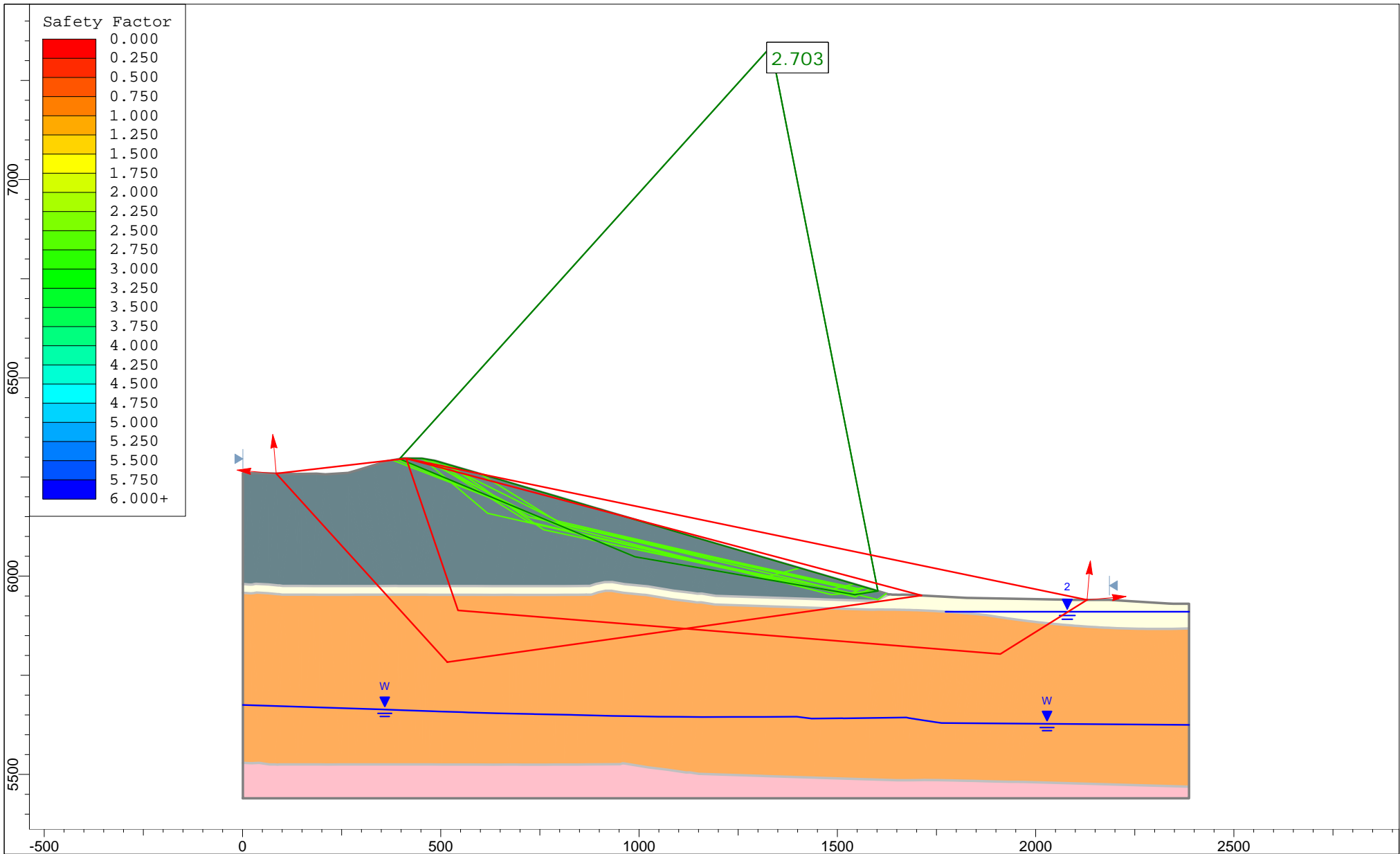
Scale

Company

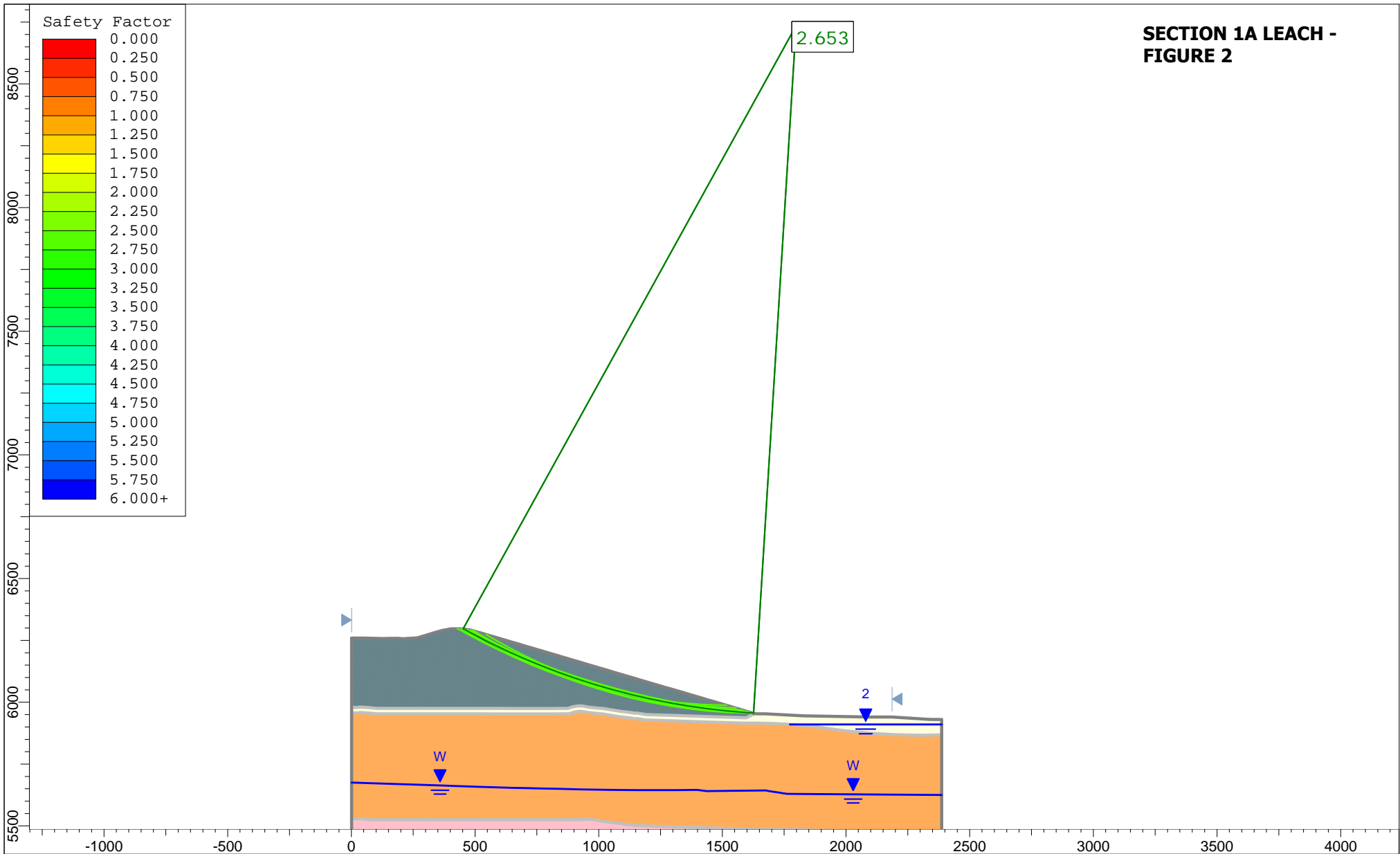
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Date

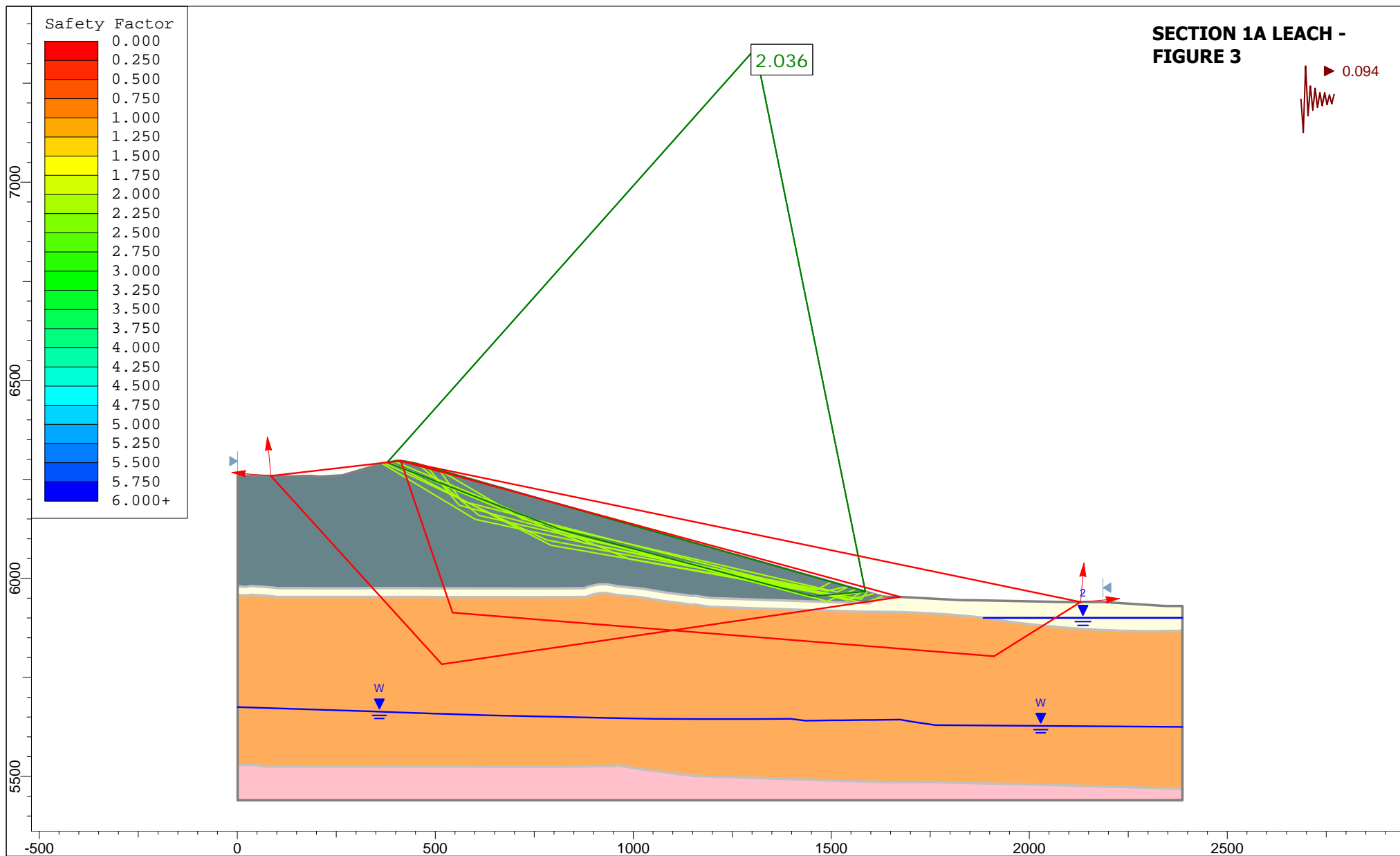
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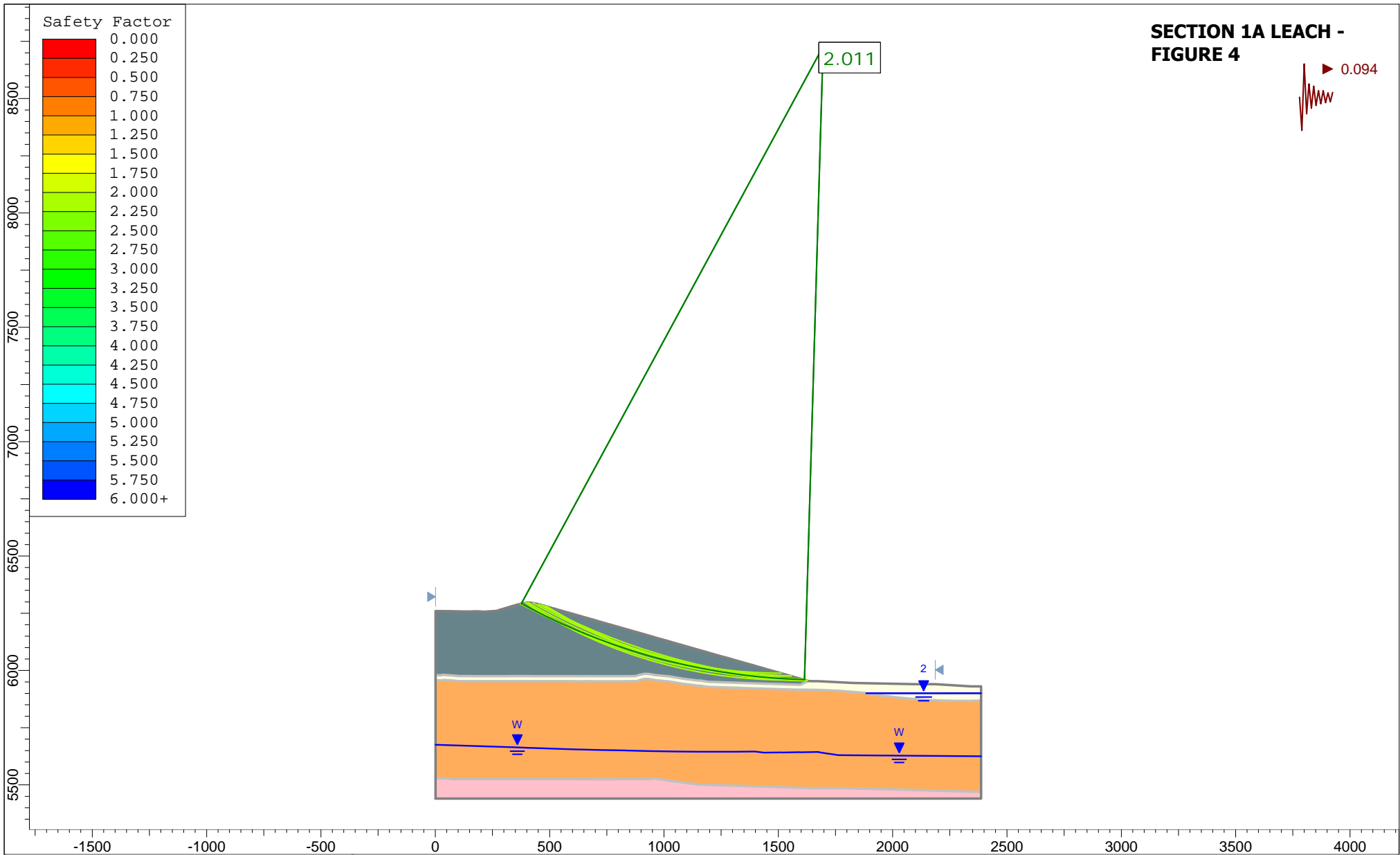
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Tyrone Mine Closure Stockpile Stability - Section 1A			
Analysis Description			
Static - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	1A.slmd		



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Tyrone Mine Closure Stockpile Stability - Section 1A LEACH			
Analysis Description			
Static - Circular Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
	1A.slmd		



Project			
Tyrone Mine Closure Stockpile Stability - Section 1A LEACH			
Analysis Description			
Seismic - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	1A.slmd		

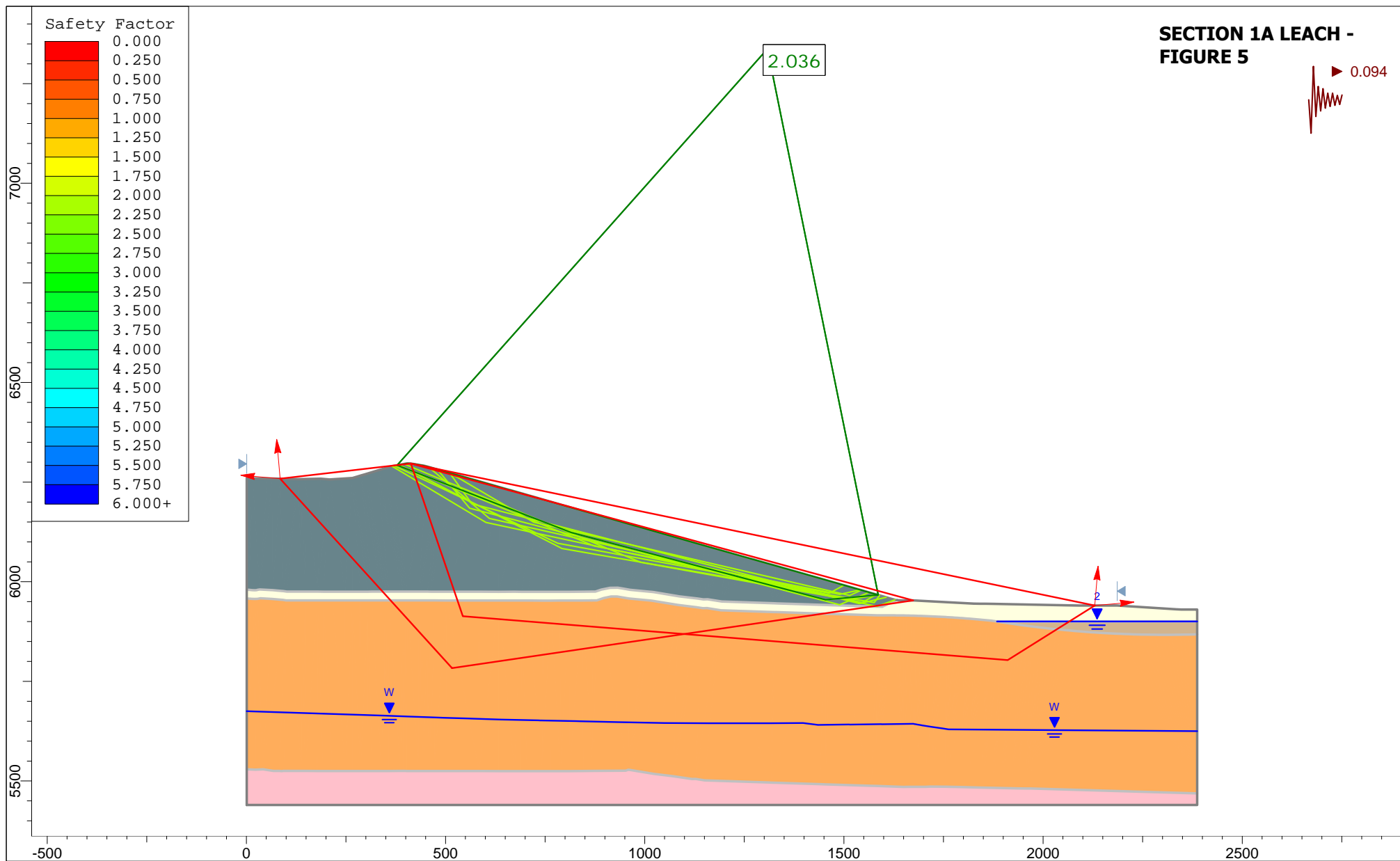


**SECTION 1A LEACH -
FIGURE 4**

0.094

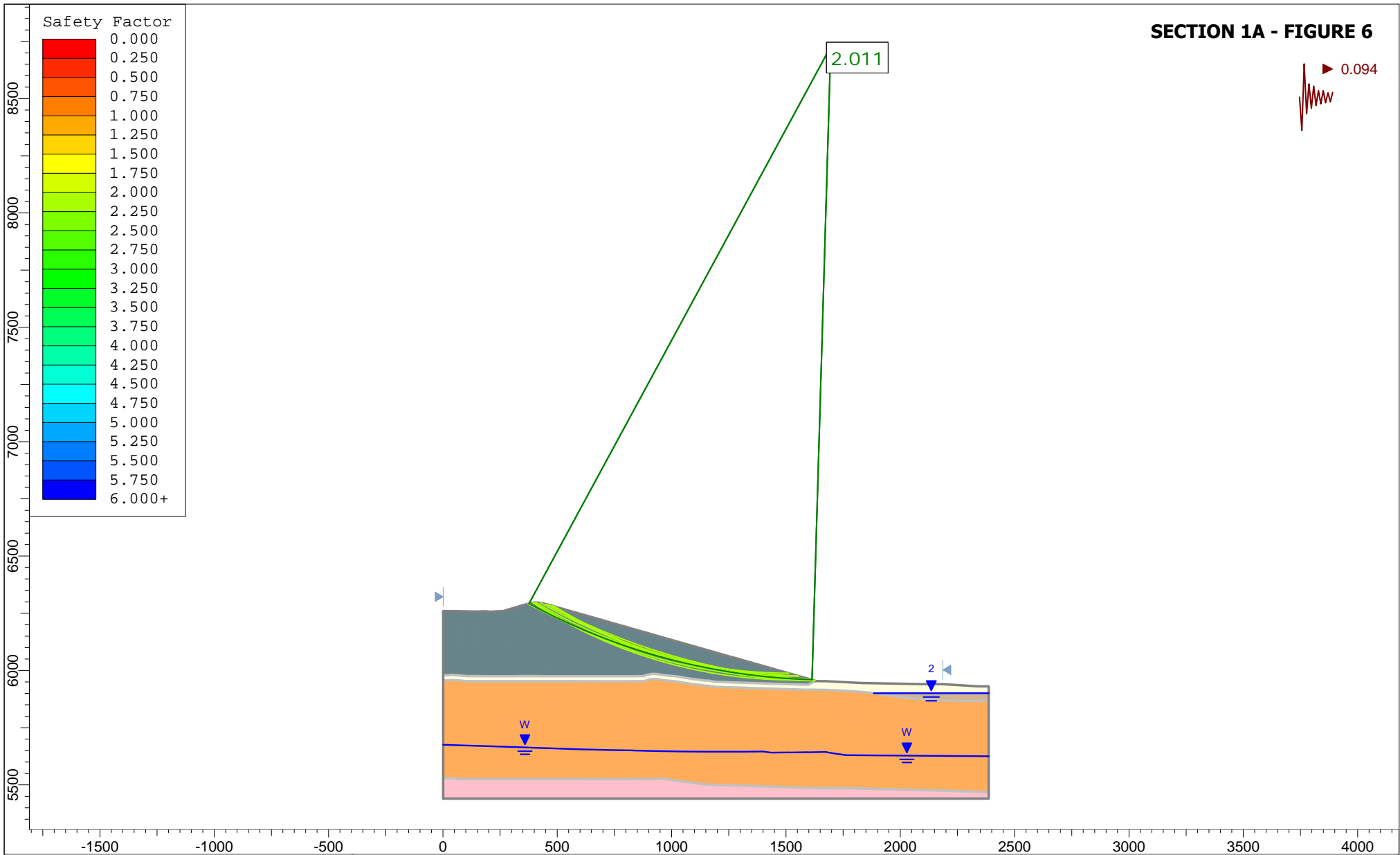


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Tyrone Mine Closure Stockpile Stability - Section 1A LEACH			
Analysis Description			
Seismic - Circular Failure (GLE / Morgenstern-Price)			
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Date	1/2/2019		File Name
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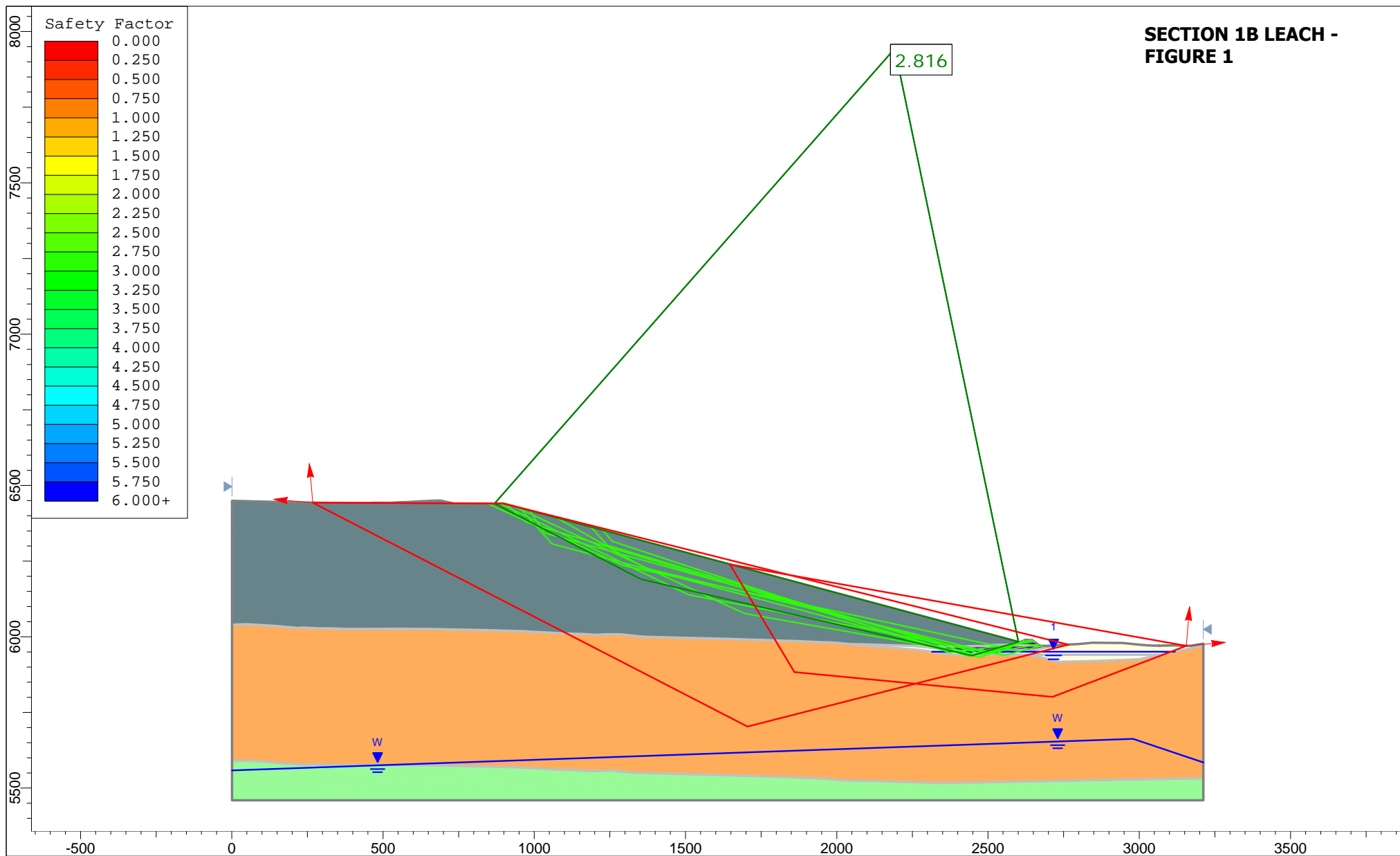


GOLDER

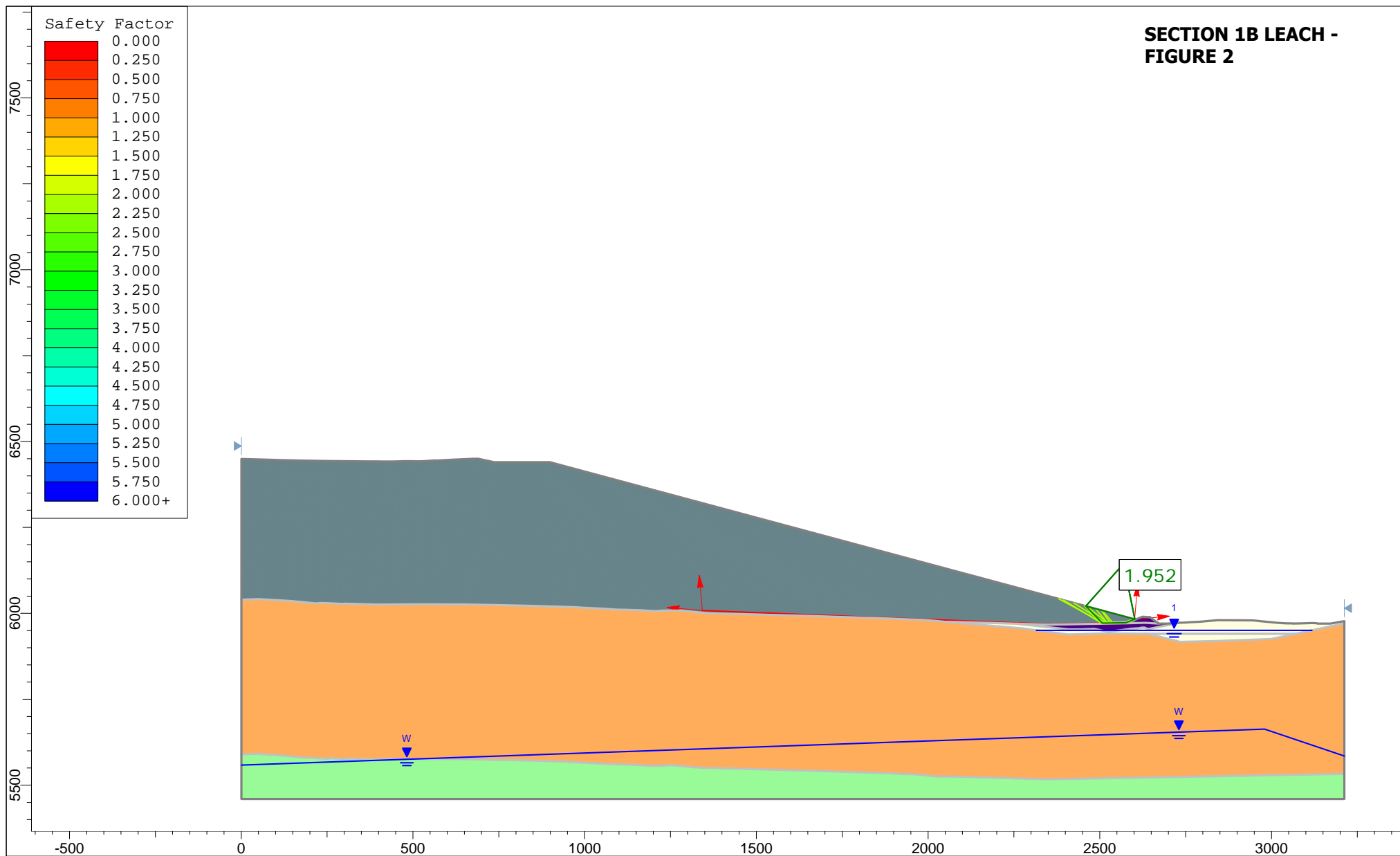
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Analysis Description		Seismic - Liquefied Qa - Block Failure (GLE / Morgenstern-Price)	
Figure	Scale	Company	
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Date	1/2/2019	File Name	
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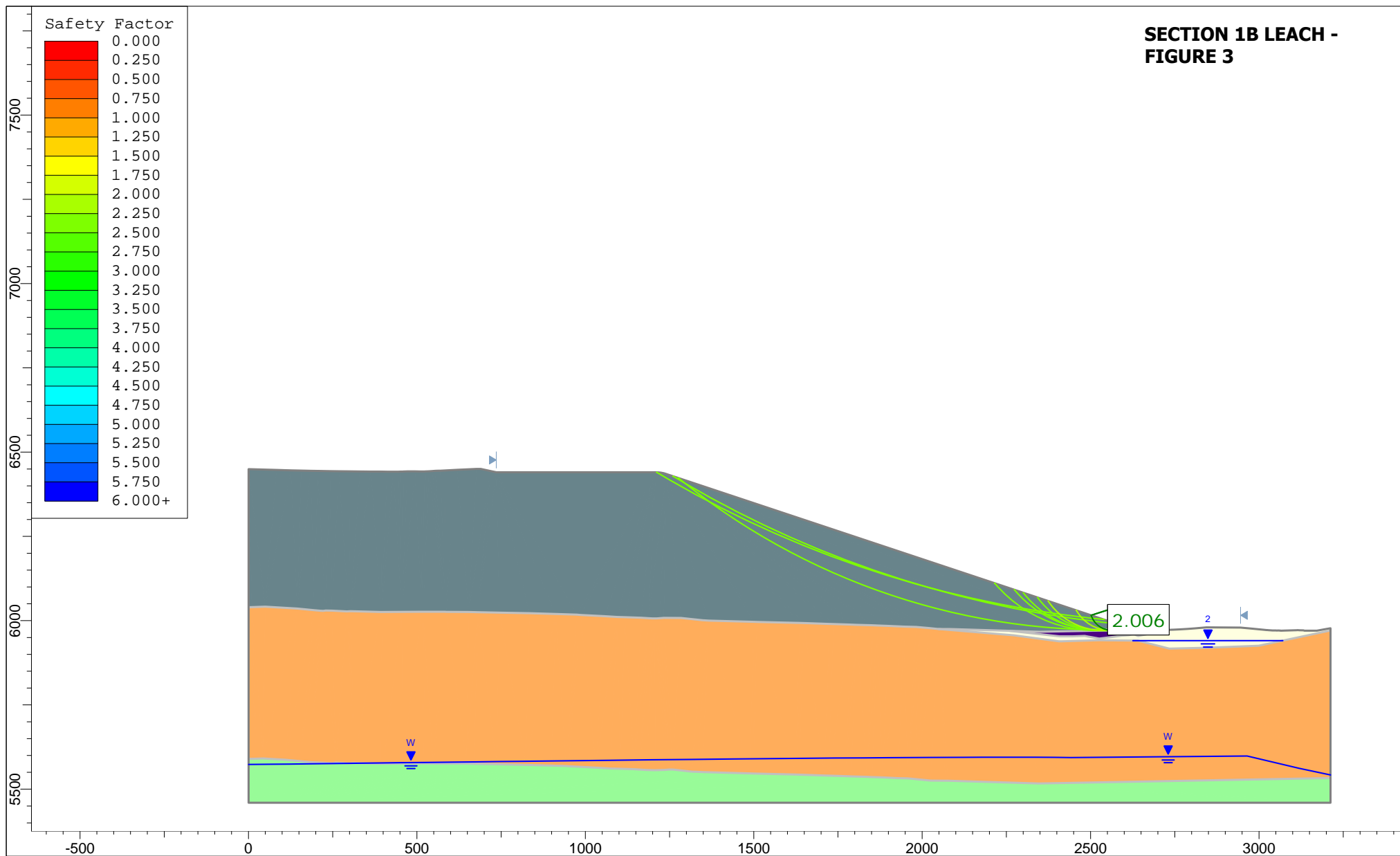
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Analysis Description				Seismic - Liquefied Qa - Circular Failure (GLE / Morgenstern-Price)	
Figure	Scale	1:6973	Company	Golder Associates	
Date	1/2/2019		File Name	1A.slmd	



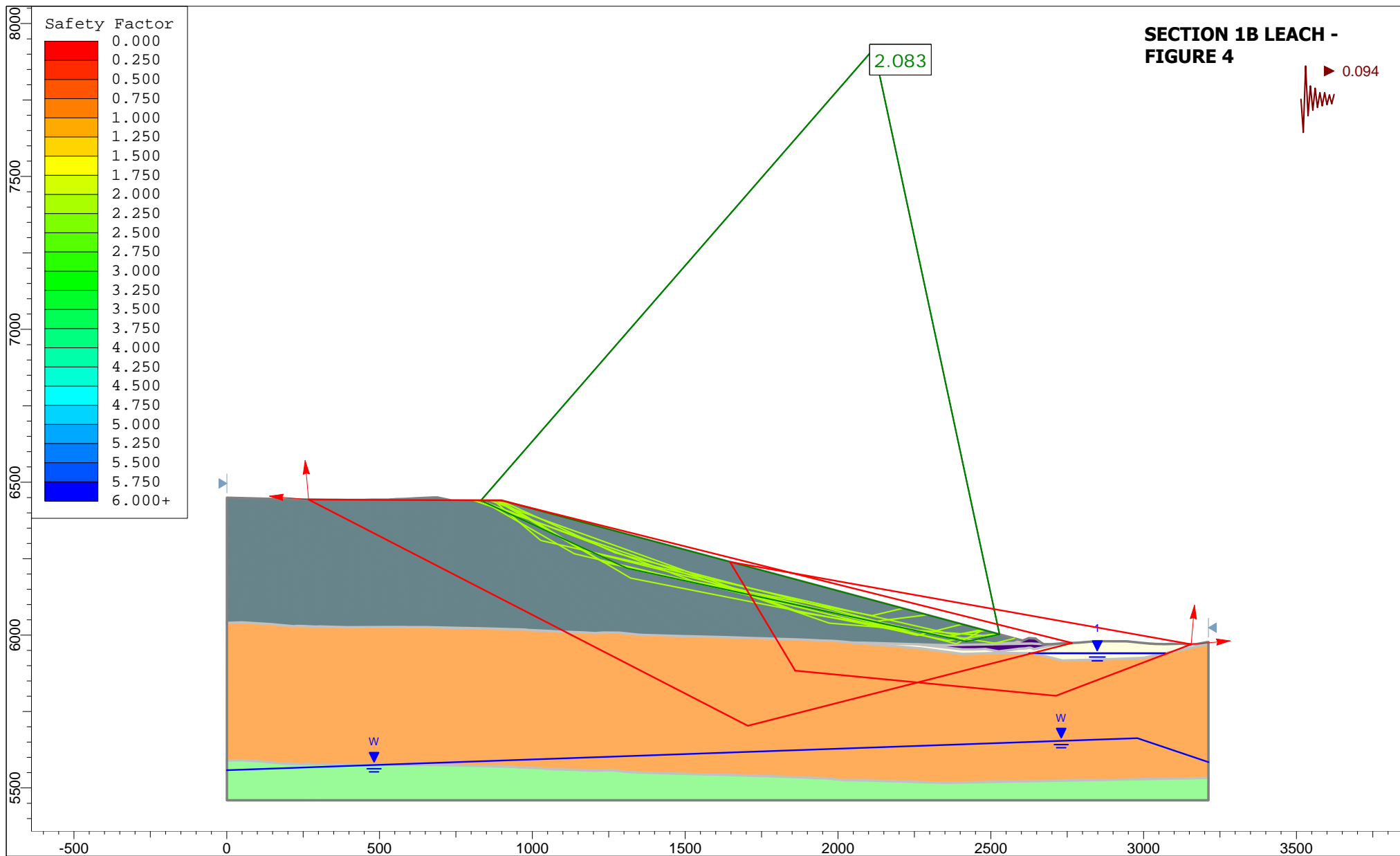
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Analysis Description			
Static - Block Failure (GLE / Morgenstern-Price)			
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Date	File Name		
1/2/2019	1B - Modified.slmd		



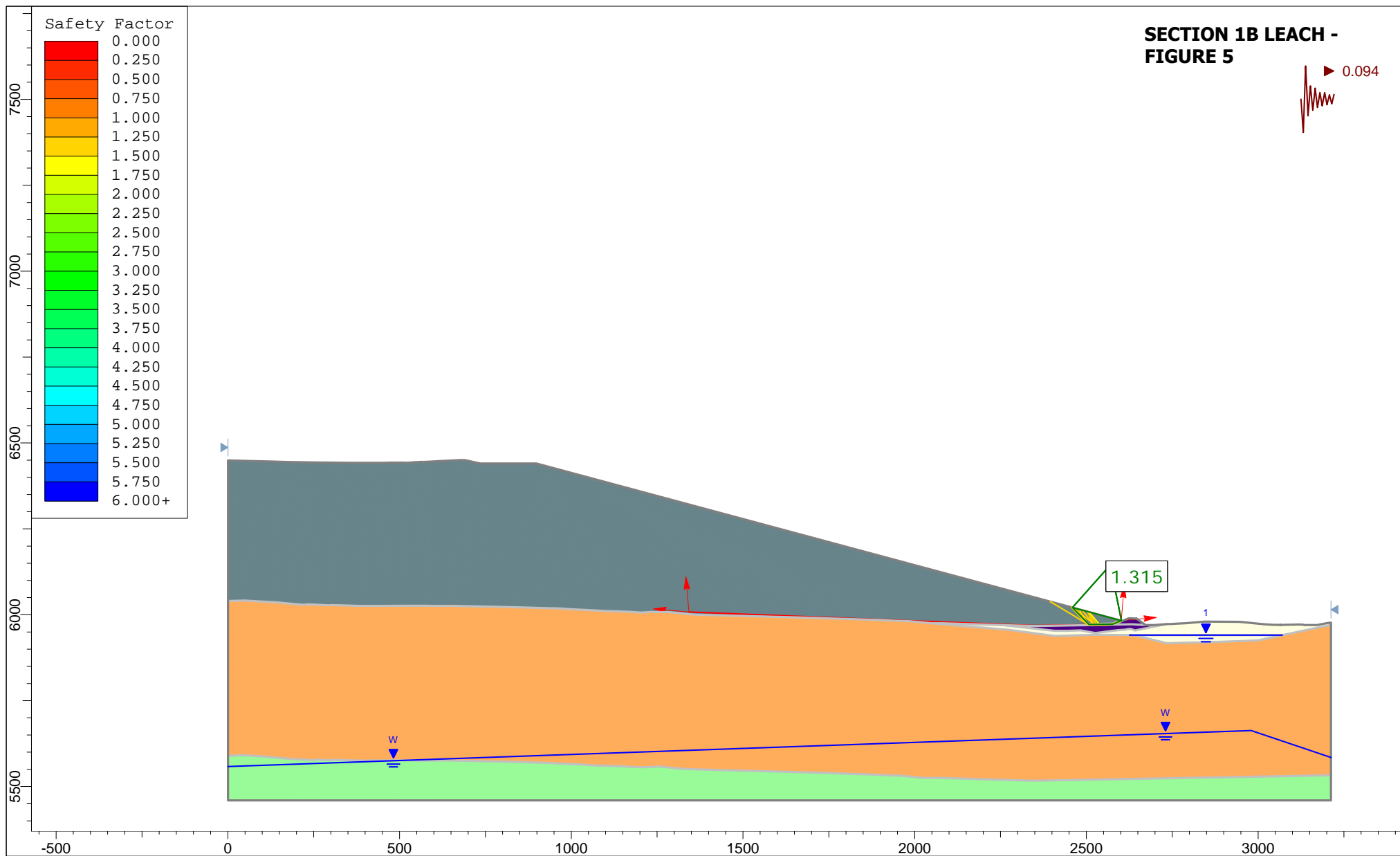
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Analysis Description			
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Date	File Name		
1/2/2019	1B - Modified.slmd		



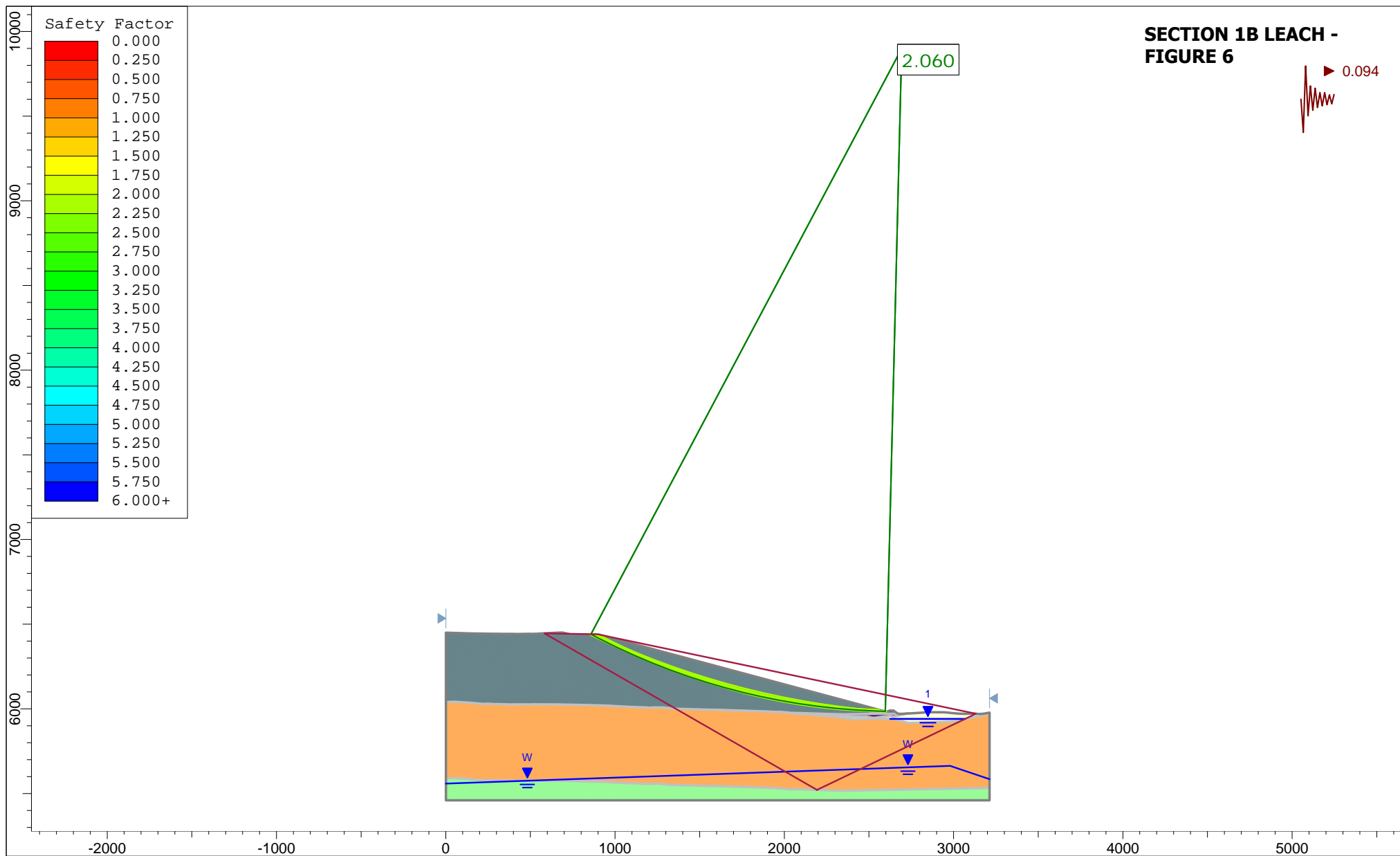
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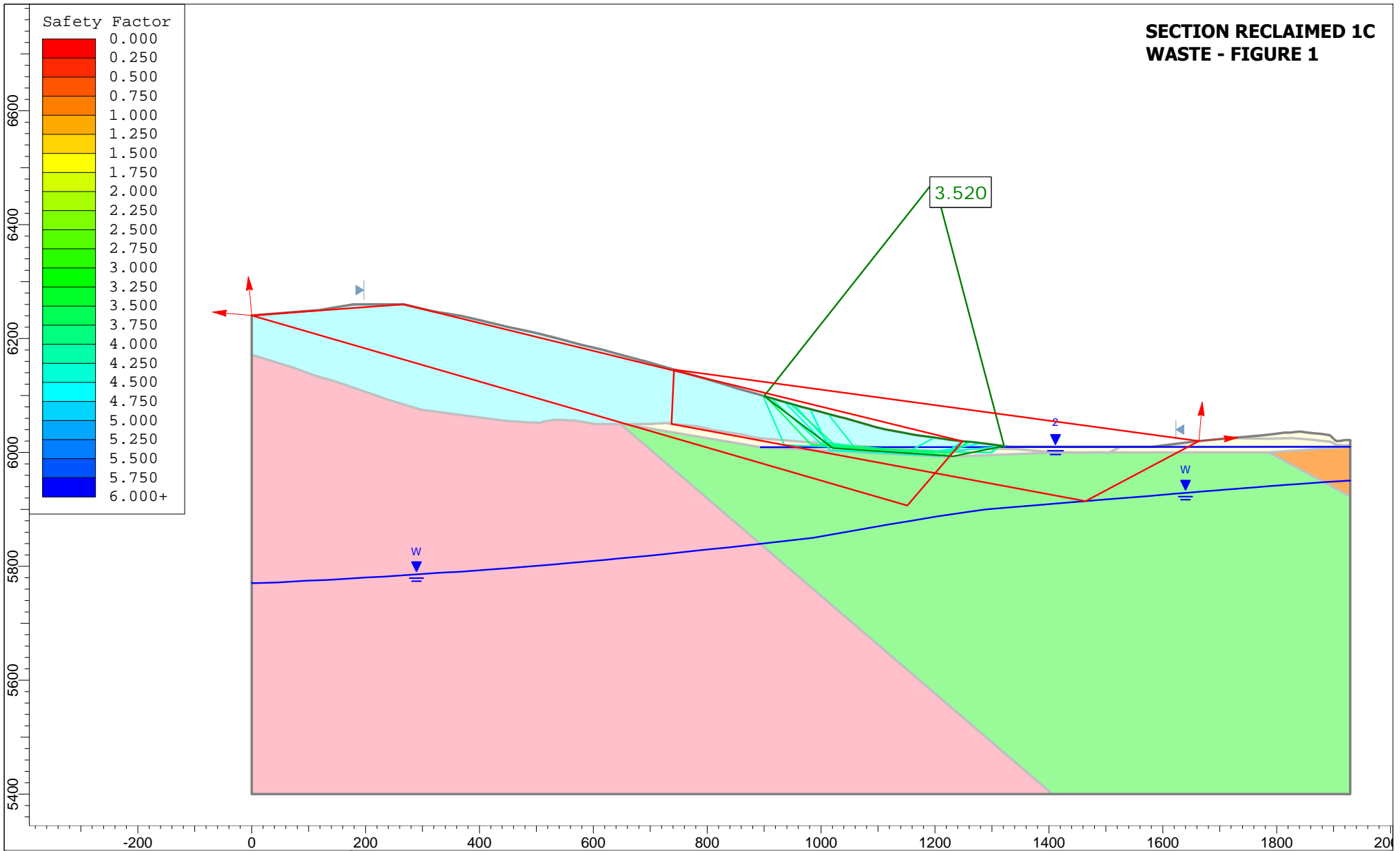
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1/2/2019	1B - Modified.slmd		



Project			
Tyrone Mine Closure Stockpile Stability - SECTION 1B LEACH			
Analysis Description			
Seismic - Block Liner Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	1B - Modified.slmd		

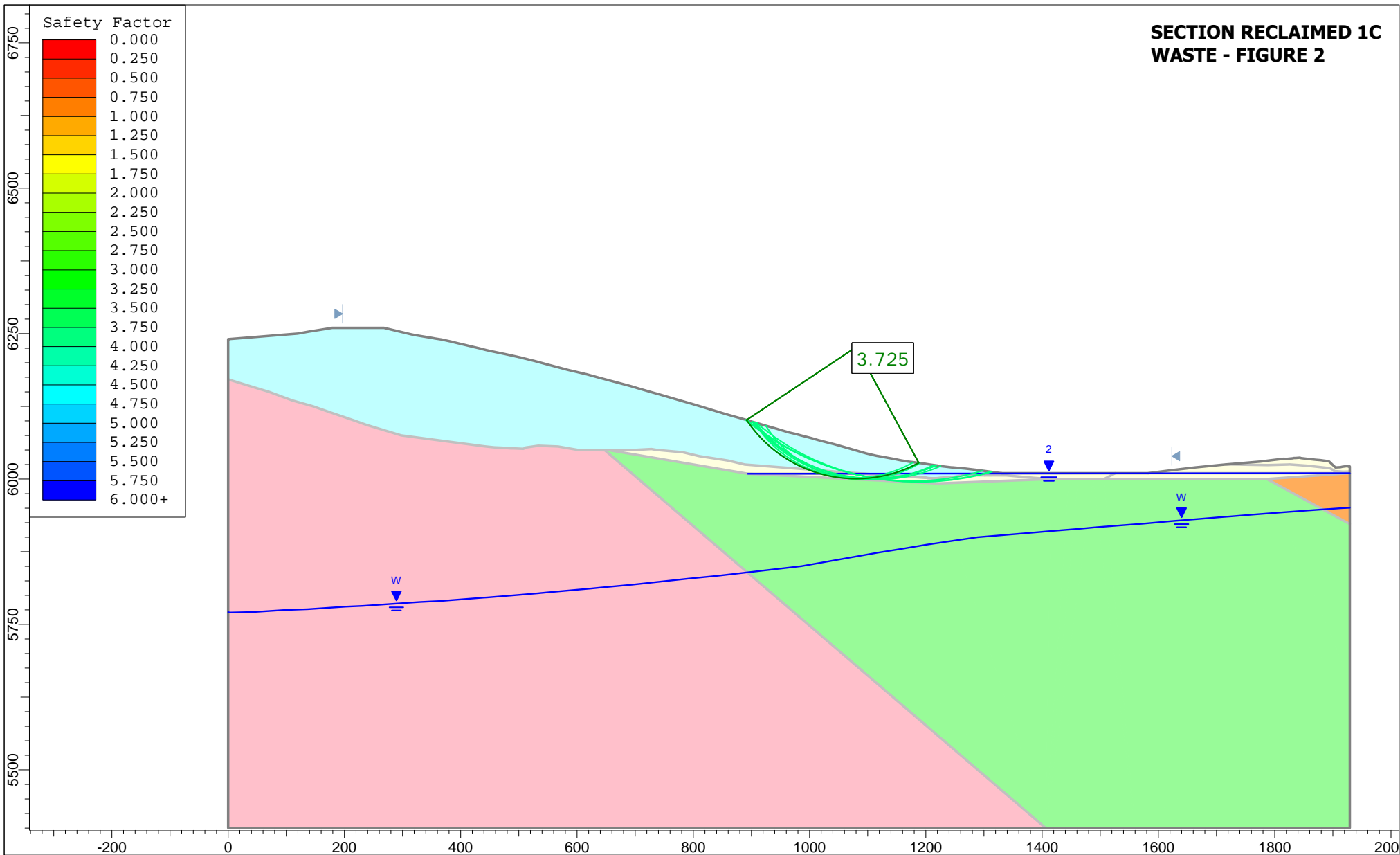


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Analysis Description			
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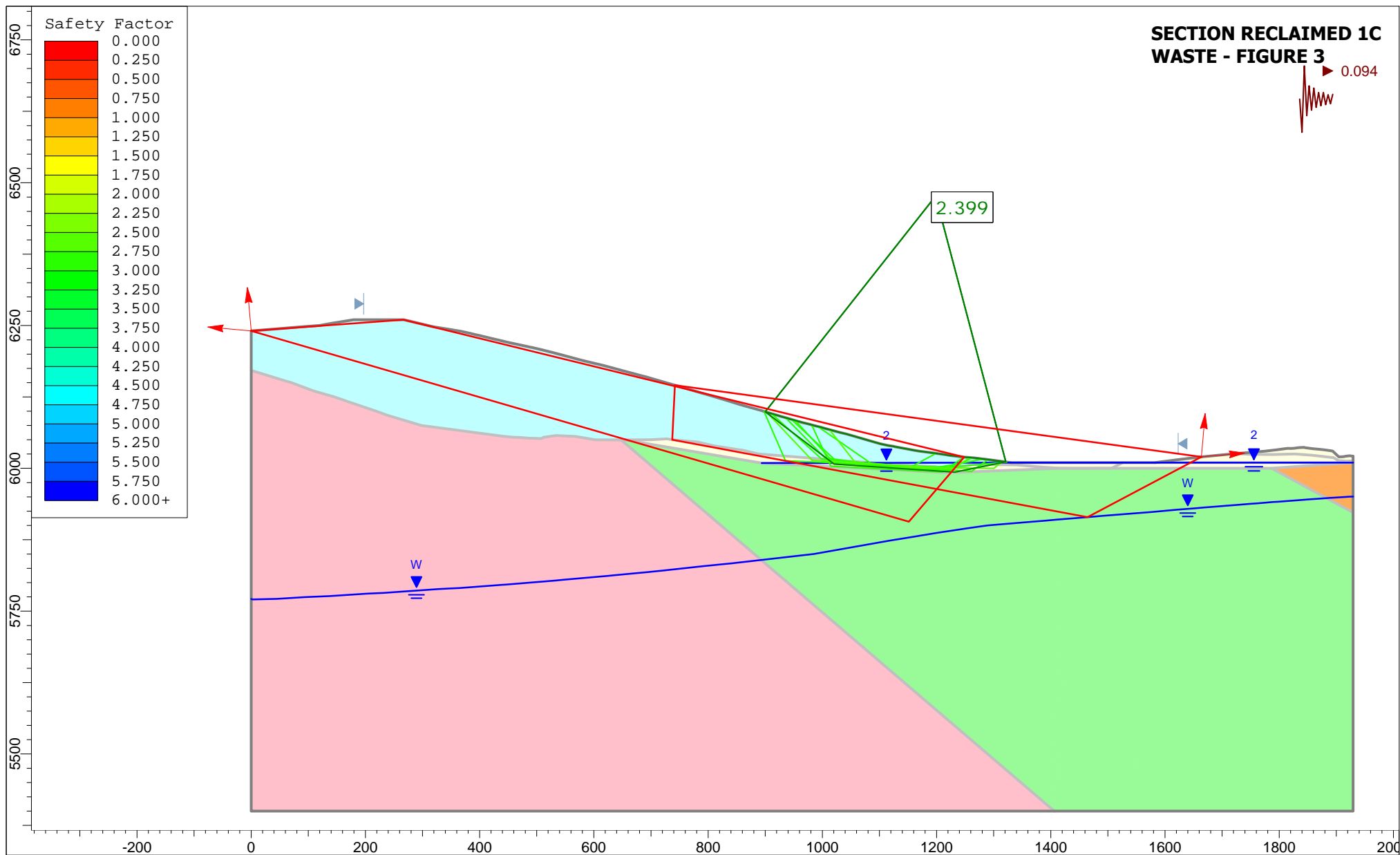


Project			
Tyrone Mine Closure Stockpile Stability - SECTION RECLAIMED 1C WASTE			
Analysis Description			
Static - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	1C.slmd		

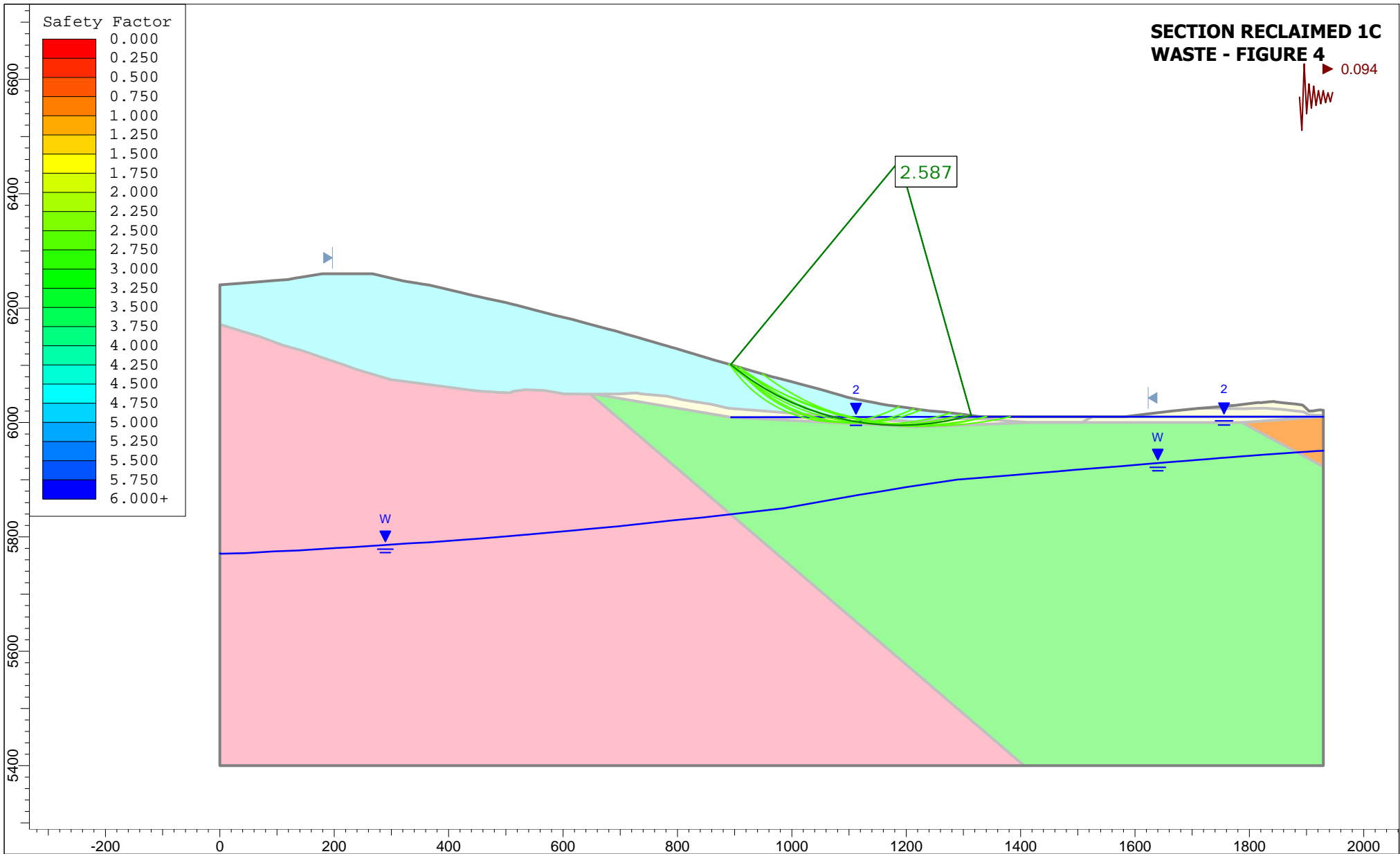
SECTION RECLAIMED 1C WASTE - FIGURE 2



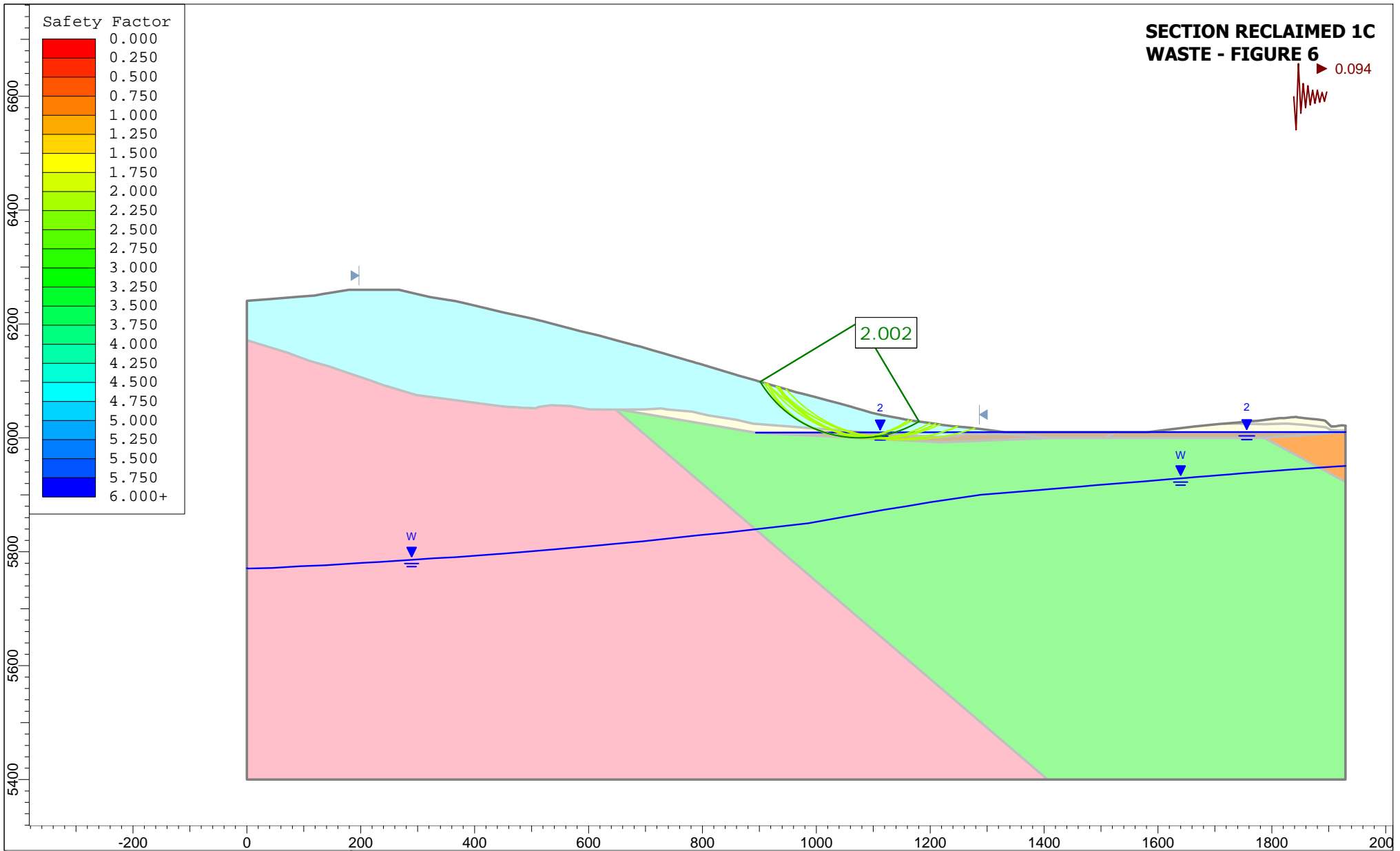
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Analysis Description				Static - Circular Failure (GLE / Morgenstern-Price)	
Figure	Scale	1:2742	Company	Golder Associates	
Date				File Name	1C.slmd



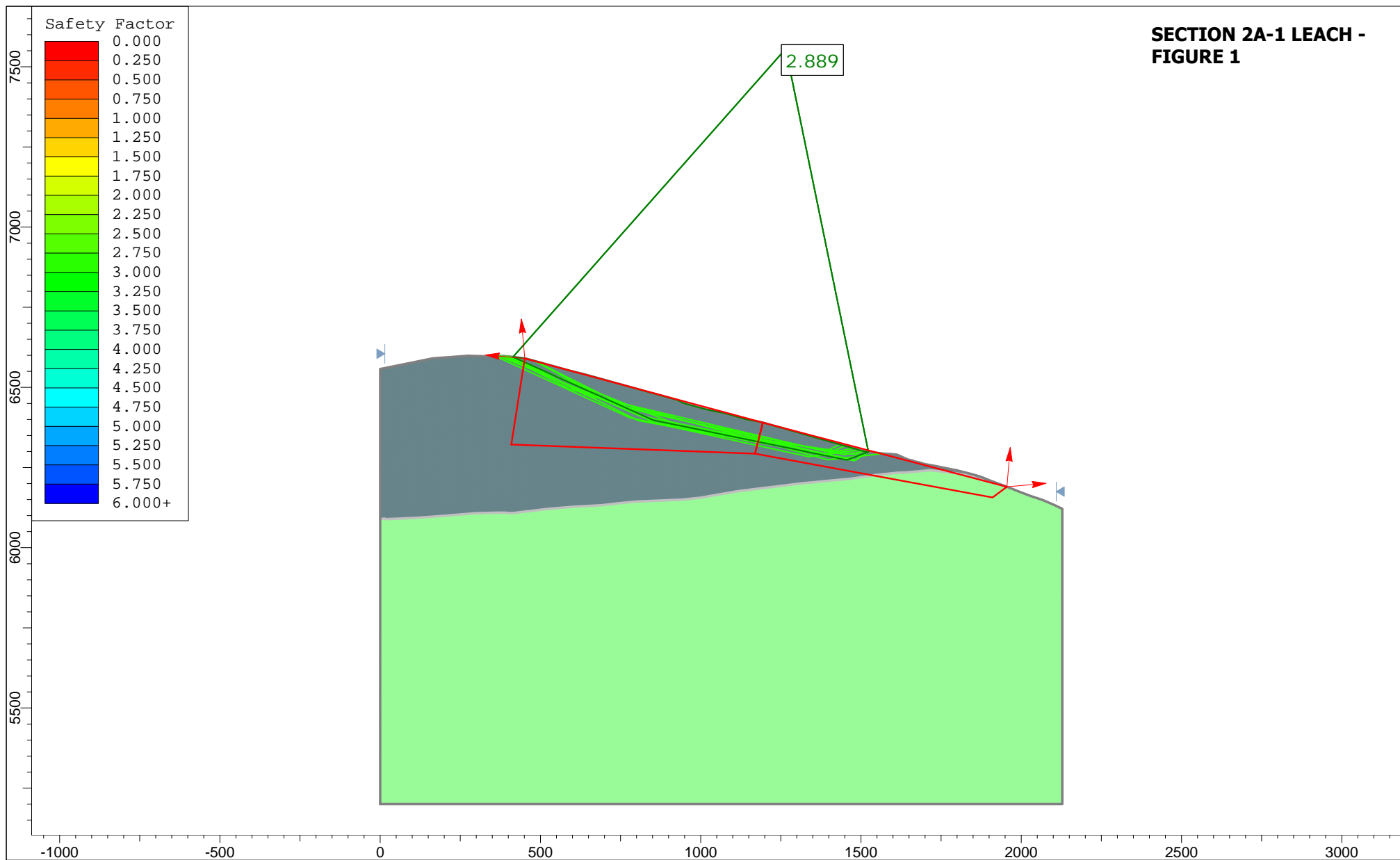
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Analysis Description			
Seismic - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	1C.slmd		



Project			
Tyrone Mine Closure Stockpile Stability - SECTION RECLAIMED 1C WASTE			
Analysis Description			
Seismic - Circular Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	1C.slmd		

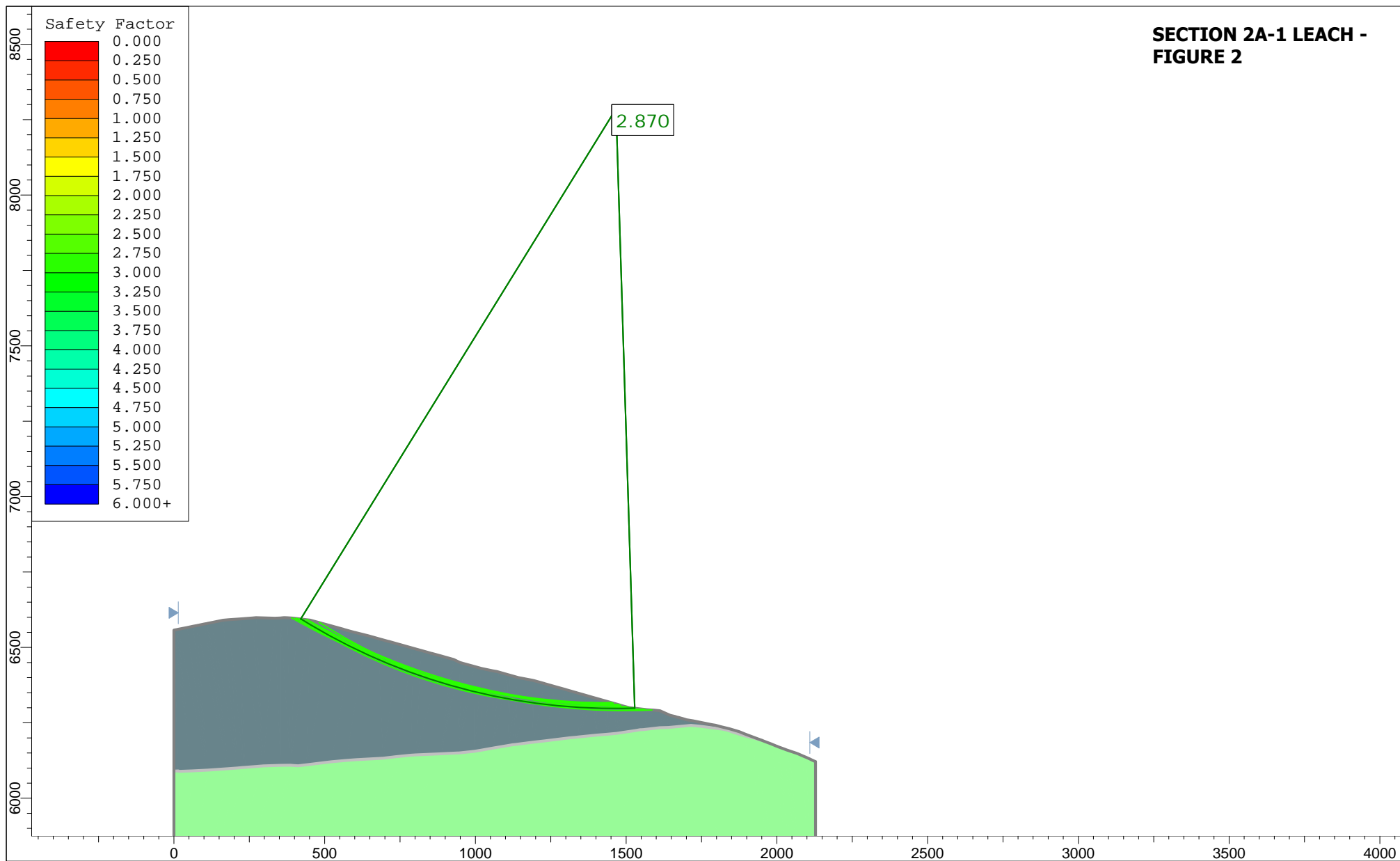


Project			
Tyrone Mine Closure Stockpile Stability - SECTION RECLAIMED 1C WASTE			
Analysis Description			
Seismic - Liquefied Qa - Circular Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
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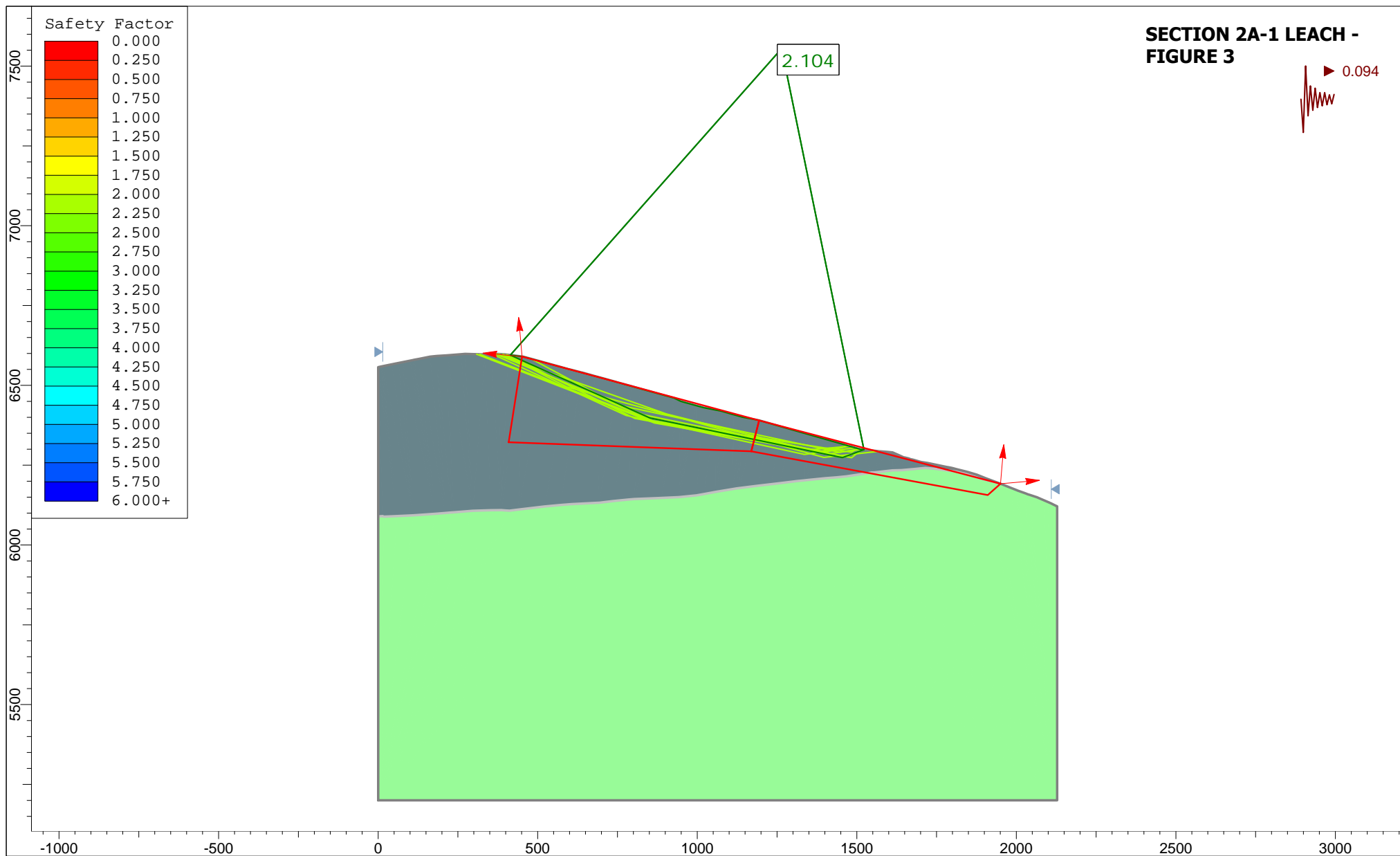


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Analysis Description		Static - Block Failure (GLE / Morgenstern-Price)	
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Date	1/2/2019	File Name	
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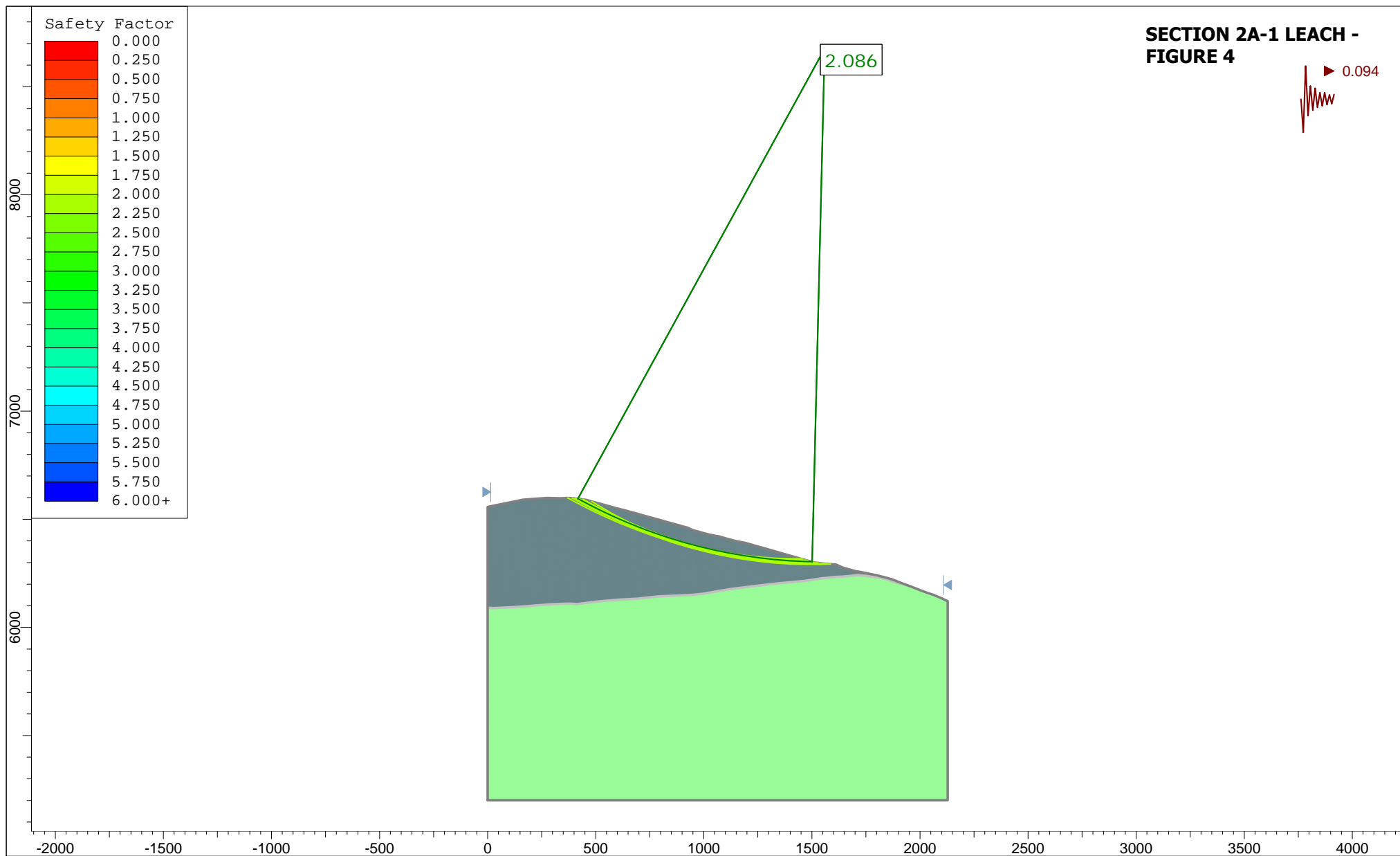
SECTION 2A-1 LEACH - FIGURE 2



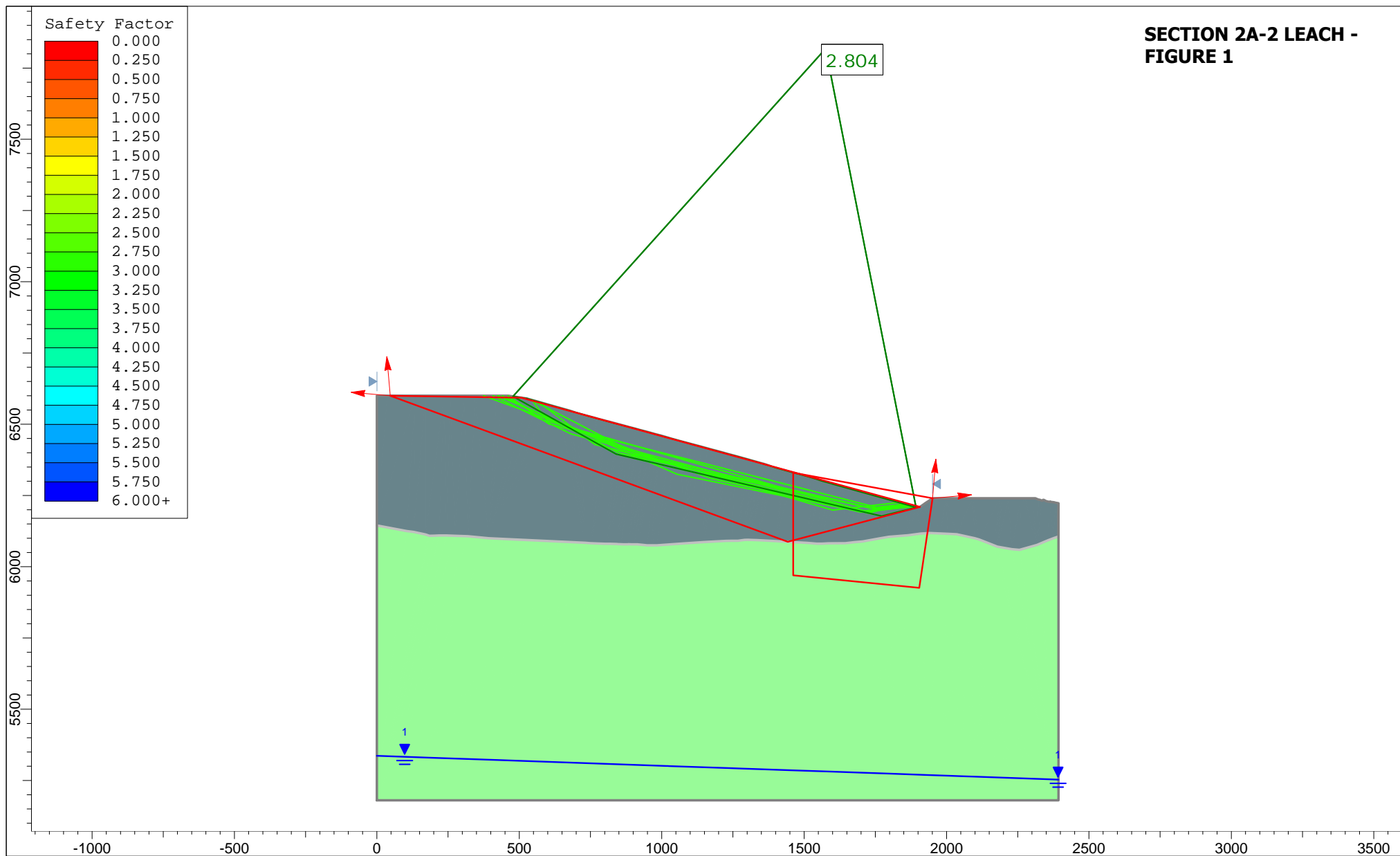
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Tyrone Mine Closure Stockpile Stability - SECTION 2A-1 LEACH		
Analysis Description		
Static - Circular Failure (GLE / Morgenstern-Price)		
Figure	Scale	Company
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Date	File Name	
	2A-1.slmd	



Project			
Tyrone Mine Closure Stockpile Stability - SECTION 2A-1 LEACH			
Analysis Description			
Seismic - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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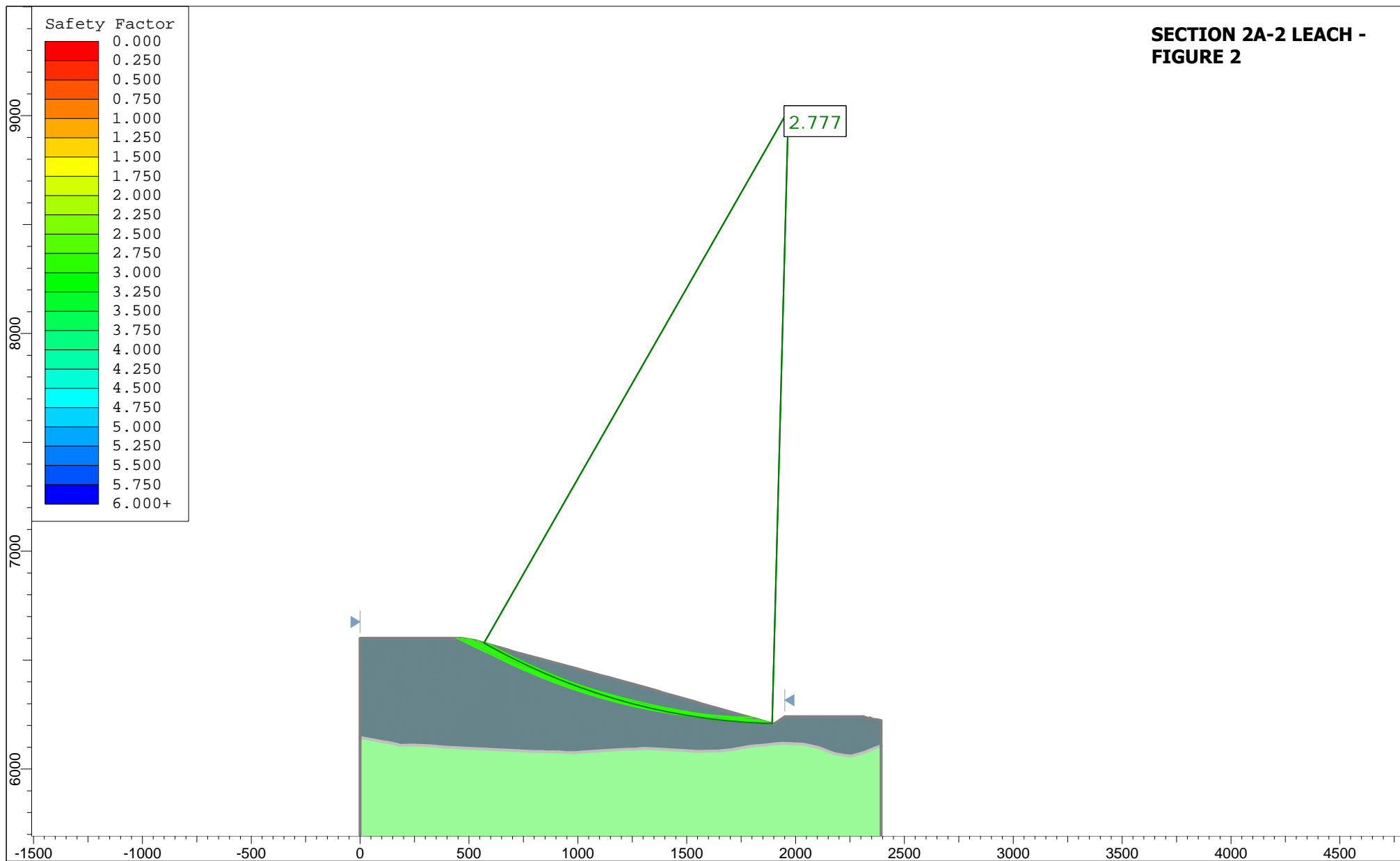


Project			
Tyrone Mine Closure Stockpile Stability - SECTION 2A-1 LEACH			
Analysis Description			
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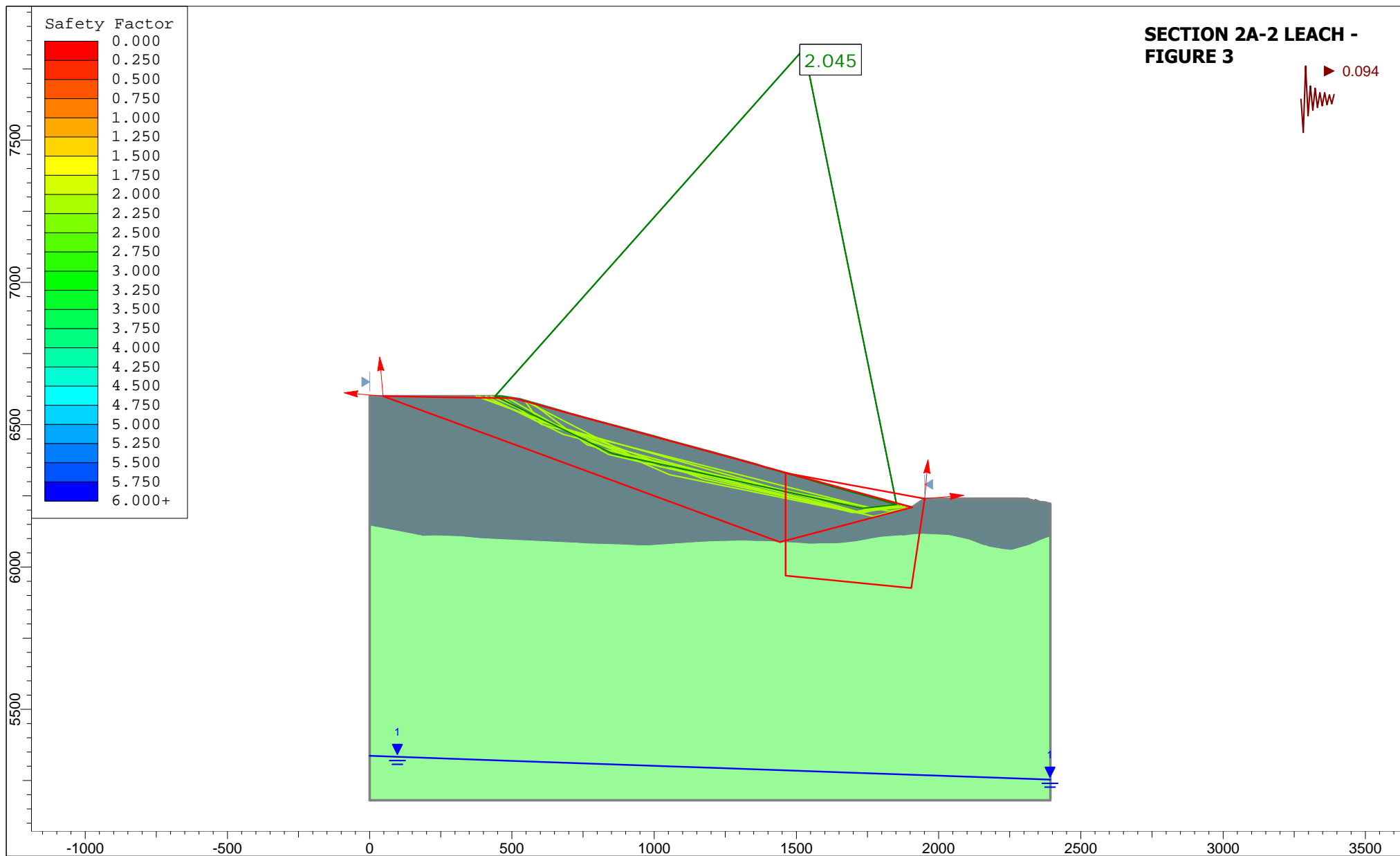


Project			
Tyrone Mine Closure Stockpile Stability			
Analysis Description			
Static - Block Failure (GLE / Morgenstern-Price)			
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Date	1/2/2019		File Name
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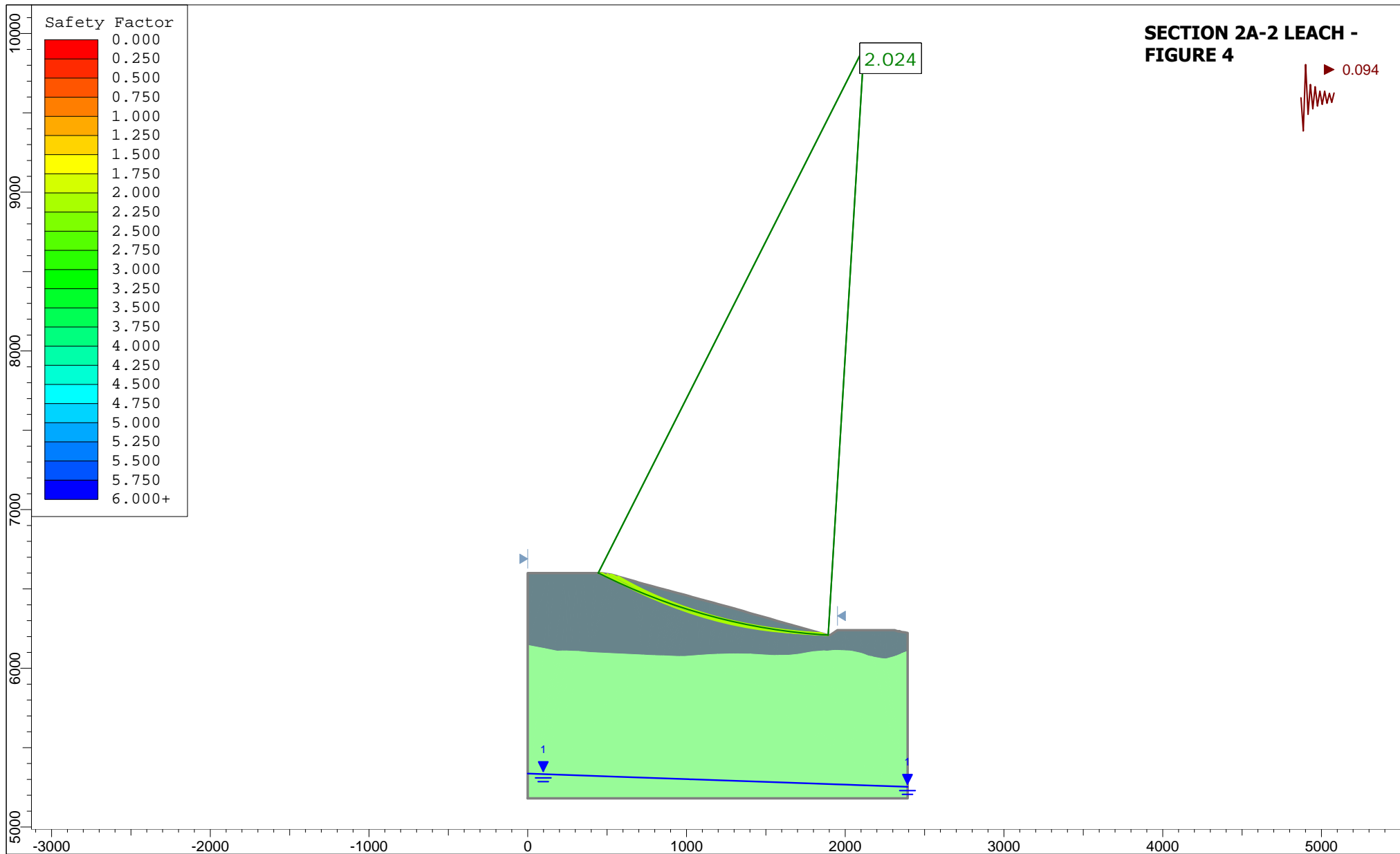
SECTION 2A-2 LEACH - FIGURE 2



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Analysis Description			Static - Circular Failure (GLE / Morgenstern-Price)		
Figure	Scale	1:7364	Company	Golder Associates	
Date			File Name	2A-2.slmd	



Project			Tyrone Mine Closure Stockpile Stability		
Analysis Description			Seismic - Block Failure (GLE / Morgenstern-Price)		
Figure	Scale	1:5602	Company	Golder Associates	
Date	1/2/2019		File Name	2A-2.slmd	



GOLDER

Project

Tyrone Mine Closure Stockpile Stability

Analysis Description

Seismic - Circular Failure (GLE / Morgenstern-Price)

Figure

Scale

1:10040

Company

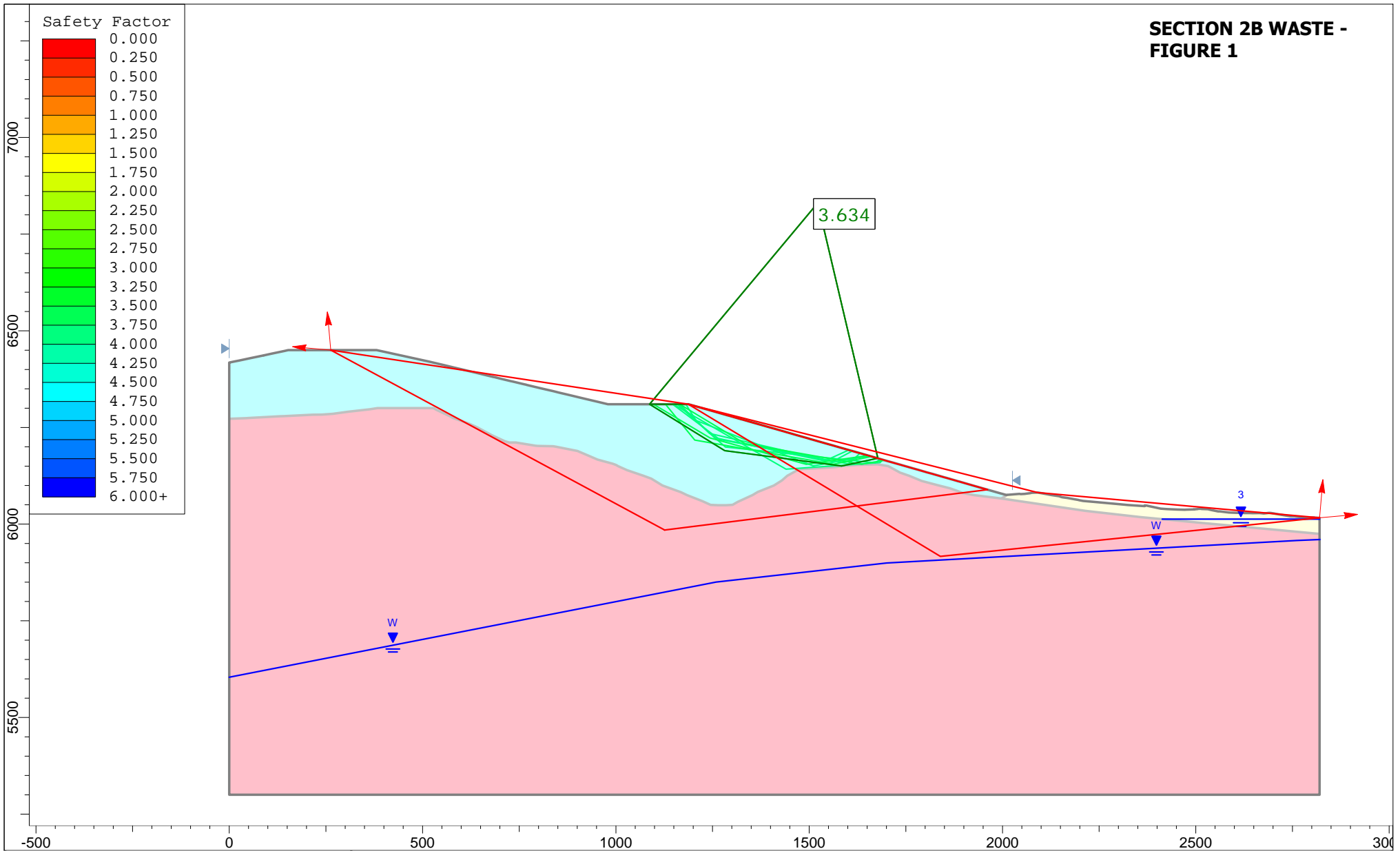
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Date

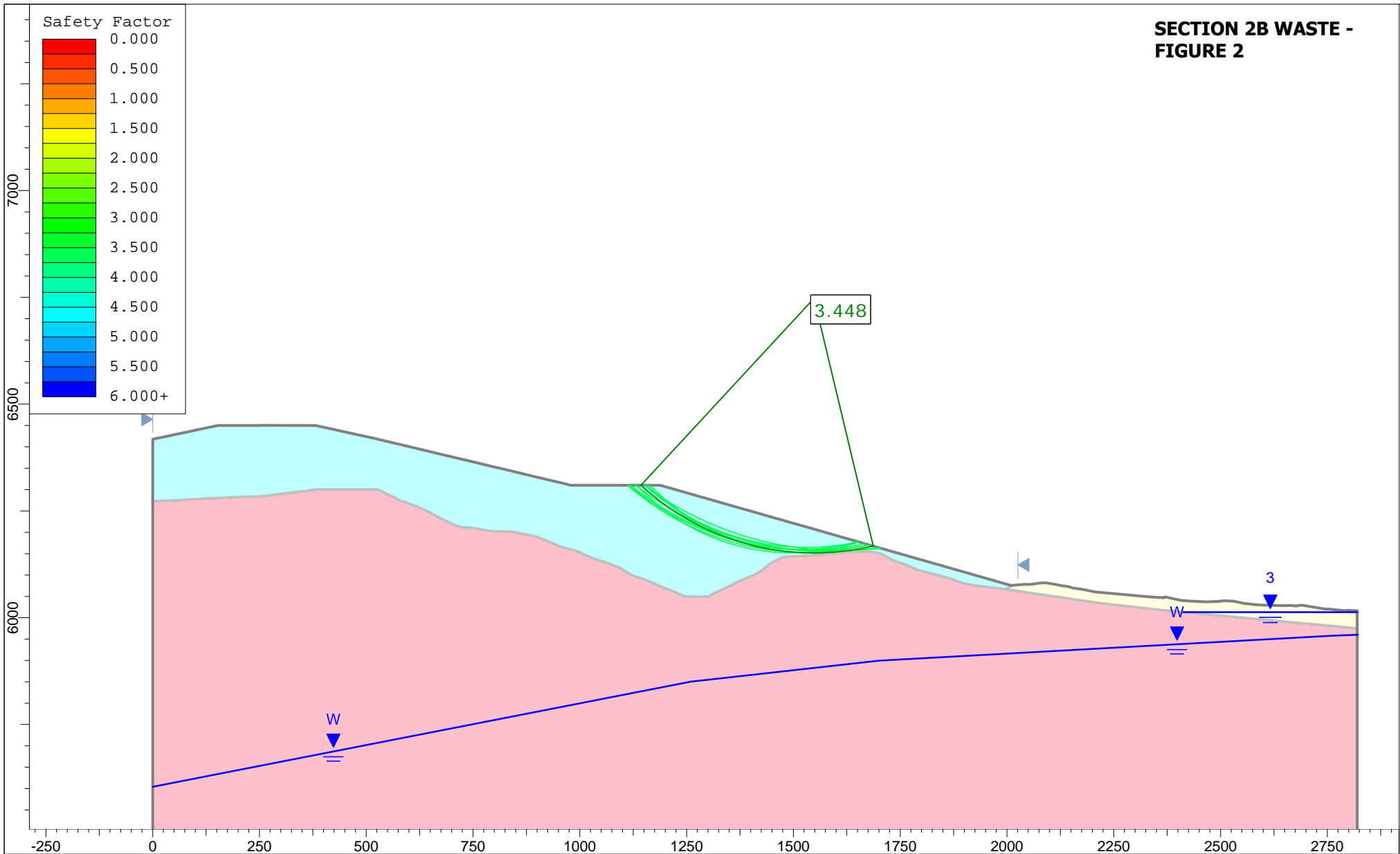
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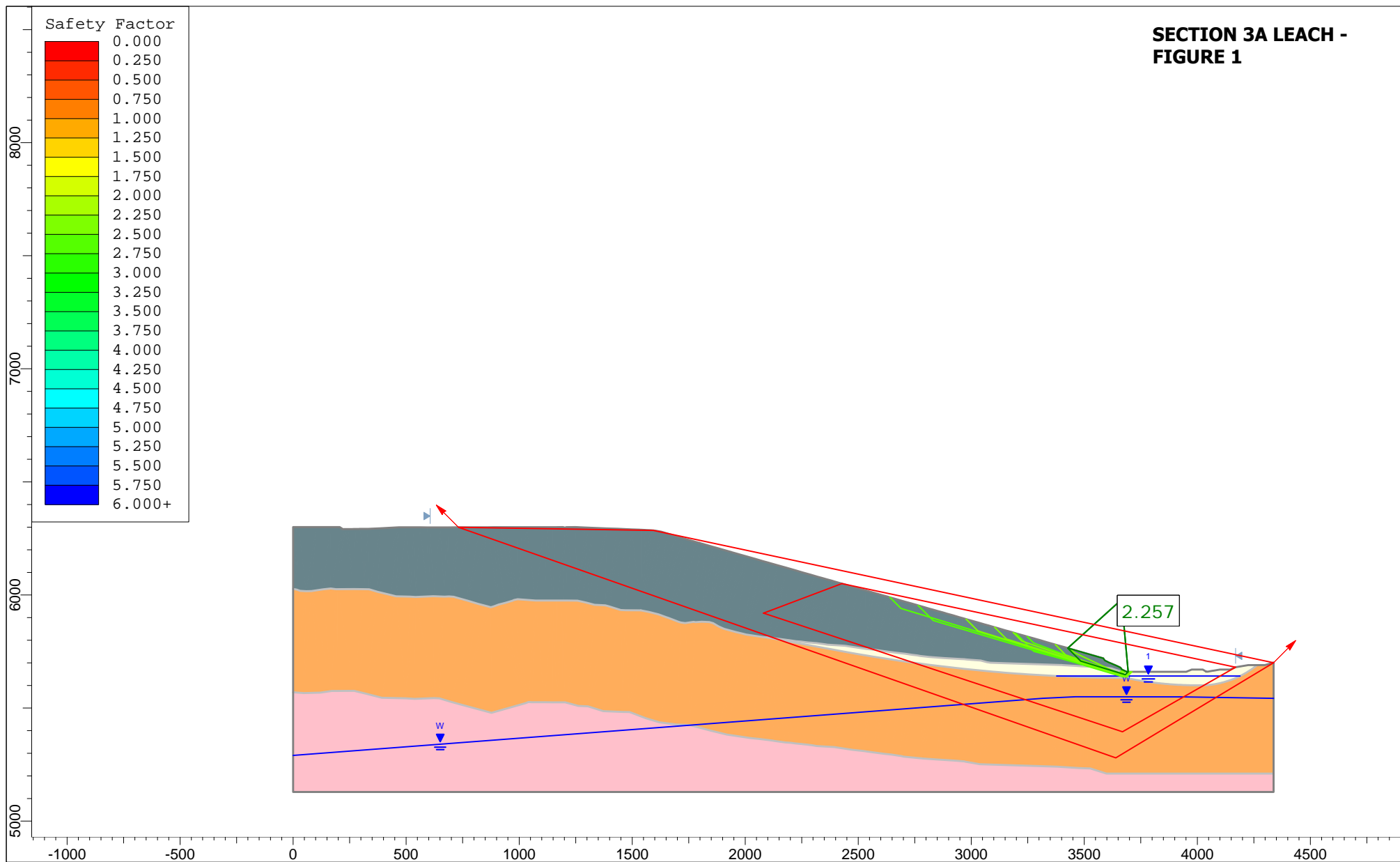


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Analysis Description				Static - Block Failure (GLE / Morgenstern-Price)	
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Date	1/2/2019		File Name	2B.slmd	



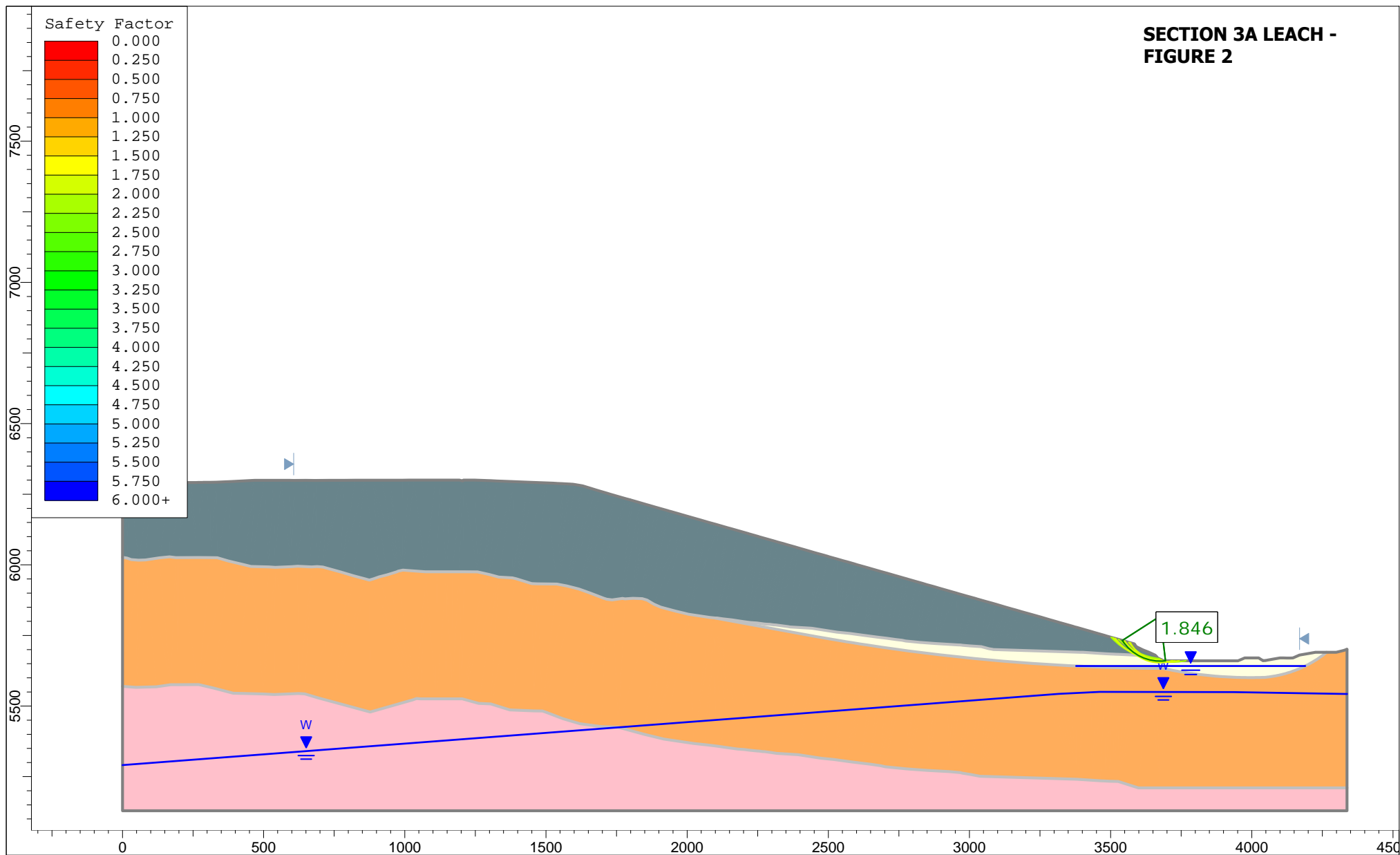
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Tyrone Mine Closure Stockpile Stability - SECTION 2B WASTE			
Analysis Description			
Static - Circular Failure (GLE / Morgenstern-Price)			
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Date	File Name		
	2B.slmd		

SECTION 3A LEACH - FIGURE 1

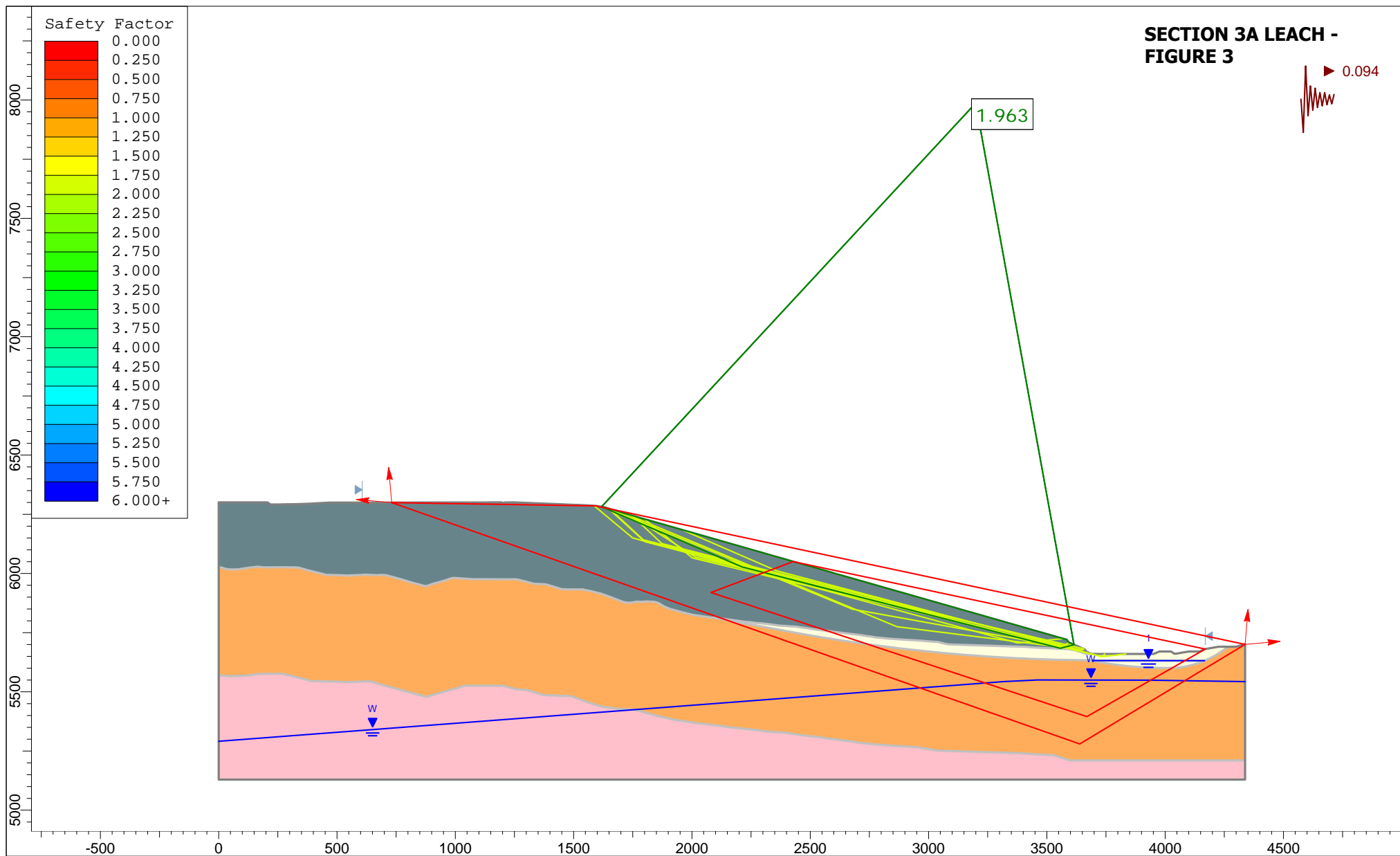


Project			Tyrone Mine Closure Stockpile Stability - SECTION 3A LEACH	
Analysis Description			Static - Block Failure (GLE / Morgenstern-Price)	
Figure	Scale	1:7099	Company	Golder Associates
Date	1/2/2019		File Name	3A.slmd

SECTION 3A LEACH - FIGURE 2

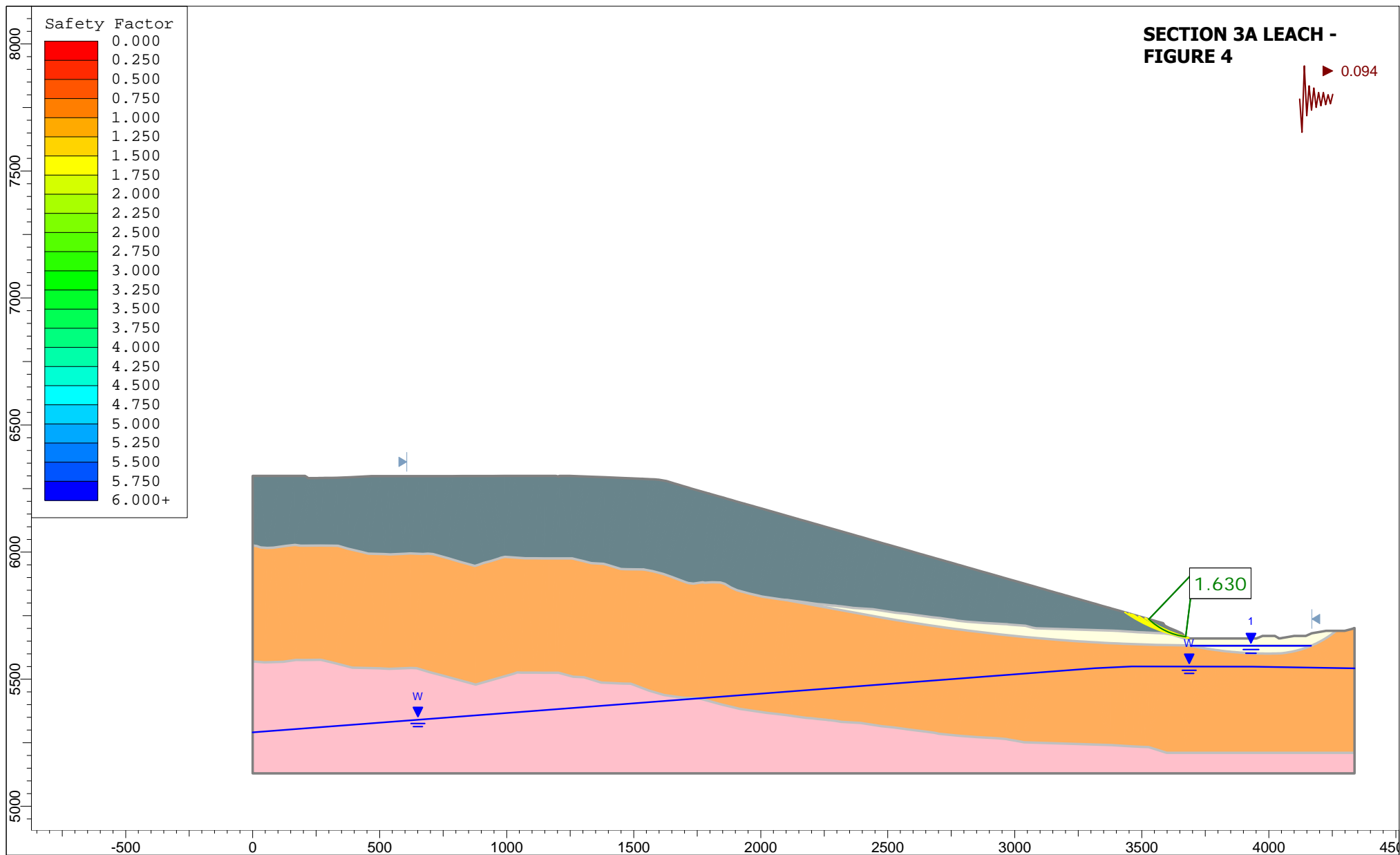


Project			
Tyrone Mine Closure Stockpile Stability - SECTION 3A LEACH			
Analysis Description			
Static - Circular Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date		File Name	
		3A.slmd	



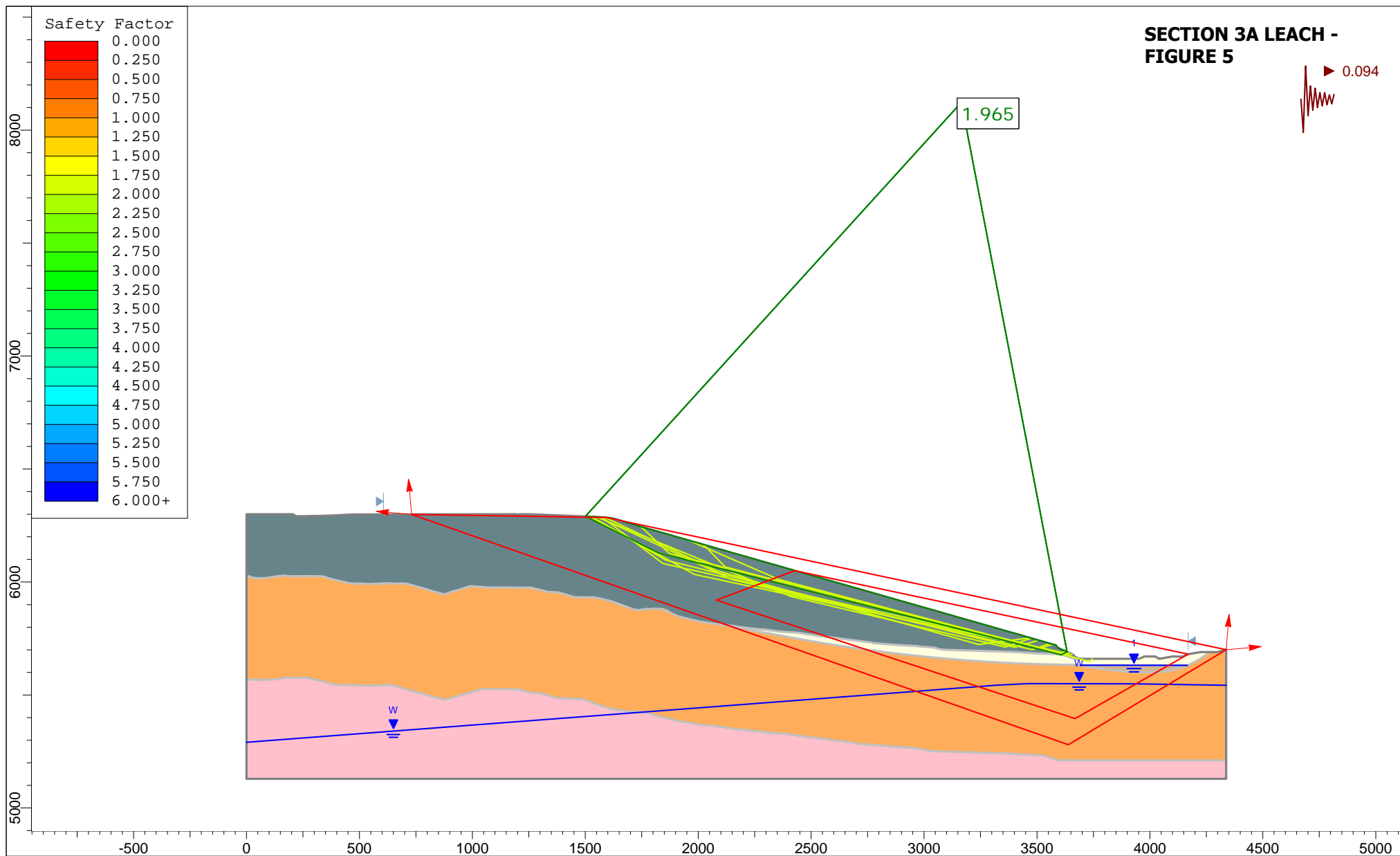
GOLDER

Project			Tyrone Mine Closure Stockpile Stability - SECTION 3A LEACH		
Analysis Description			Sesimic - Block Failure (GLE / Morgenstern-Price)		
Figure	Scale	1:6735	Company	Golder Associates	
Date	1/2/2019		File Name	3A.slmd	



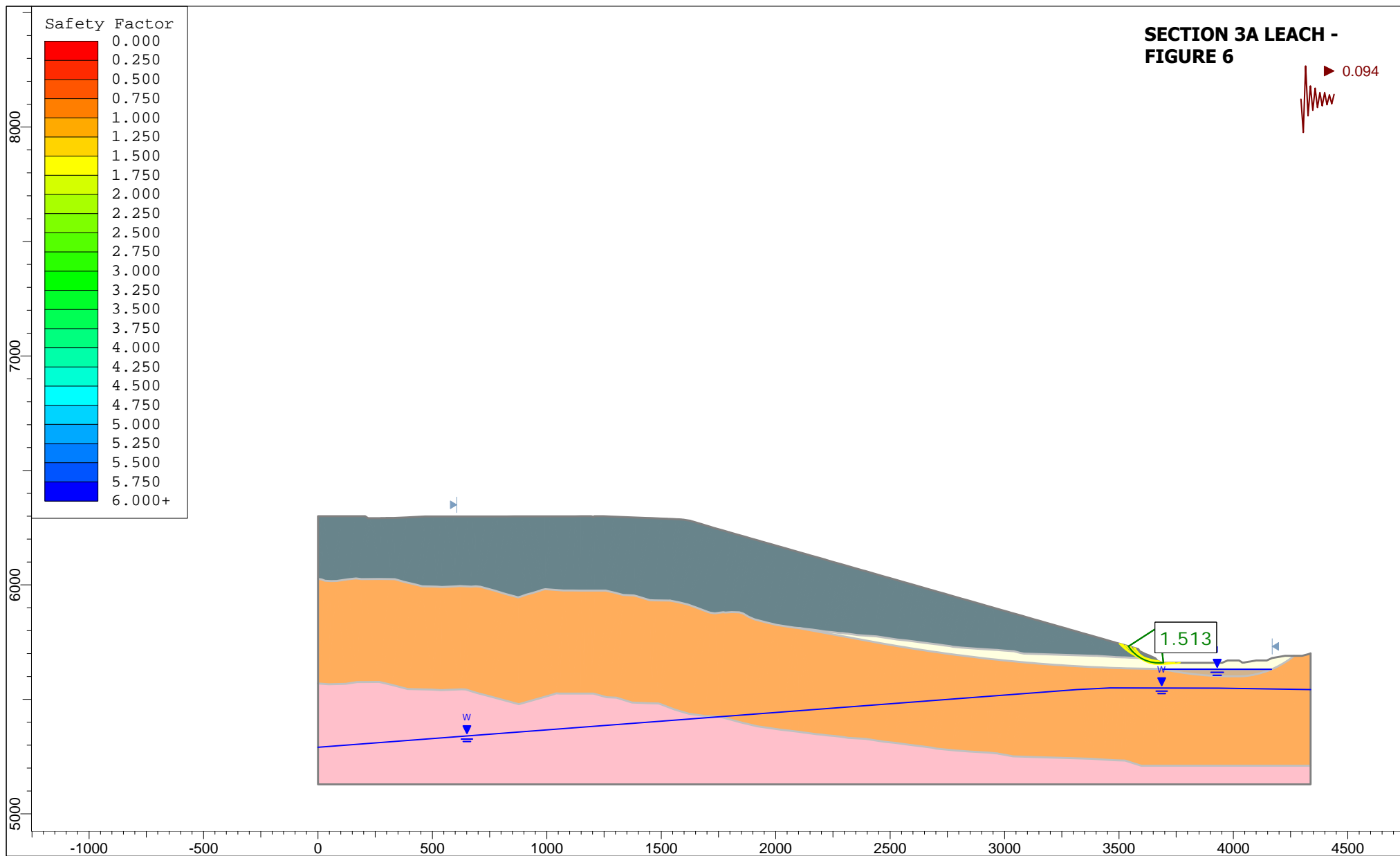
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Tyrone Mine Closure Stockpile Stability - SECTION 3A LEACH			
Analysis Description			
Sesimic - Circular Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
	1:6268	Golder Associates	
Date	1/2/2019	File Name	
		3A.slmd	



GOLDER

Project			
Tyrone Mine Closure Stockpile Stability - SECTION 3A LEACH			
Analysis Description			
Seismic - Liquefied Qa - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
	1:7056	Golder Associates	
Date	File Name		
1/2/2019	3A.slmd		



GOLDER

Project

Tyrone Mine Closure Stockpile Stability - SECTION 3A LEACH

Analysis Description

Seismic - Liquefied Qa - Circular Failure (GLE / Morgenstern-Price)

Figure

Scale

1:6964

Company

Golder Associates

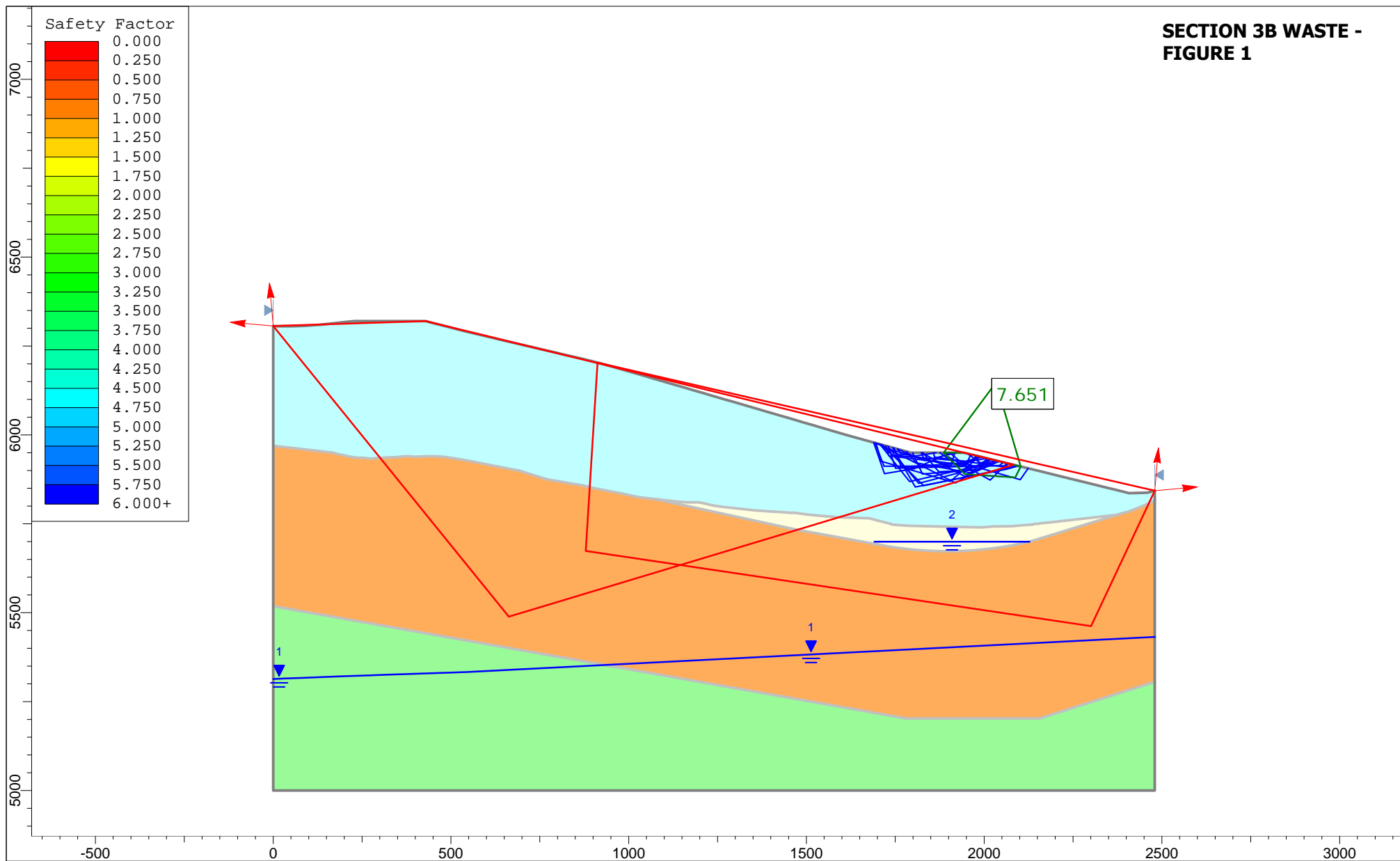
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1/2/2019

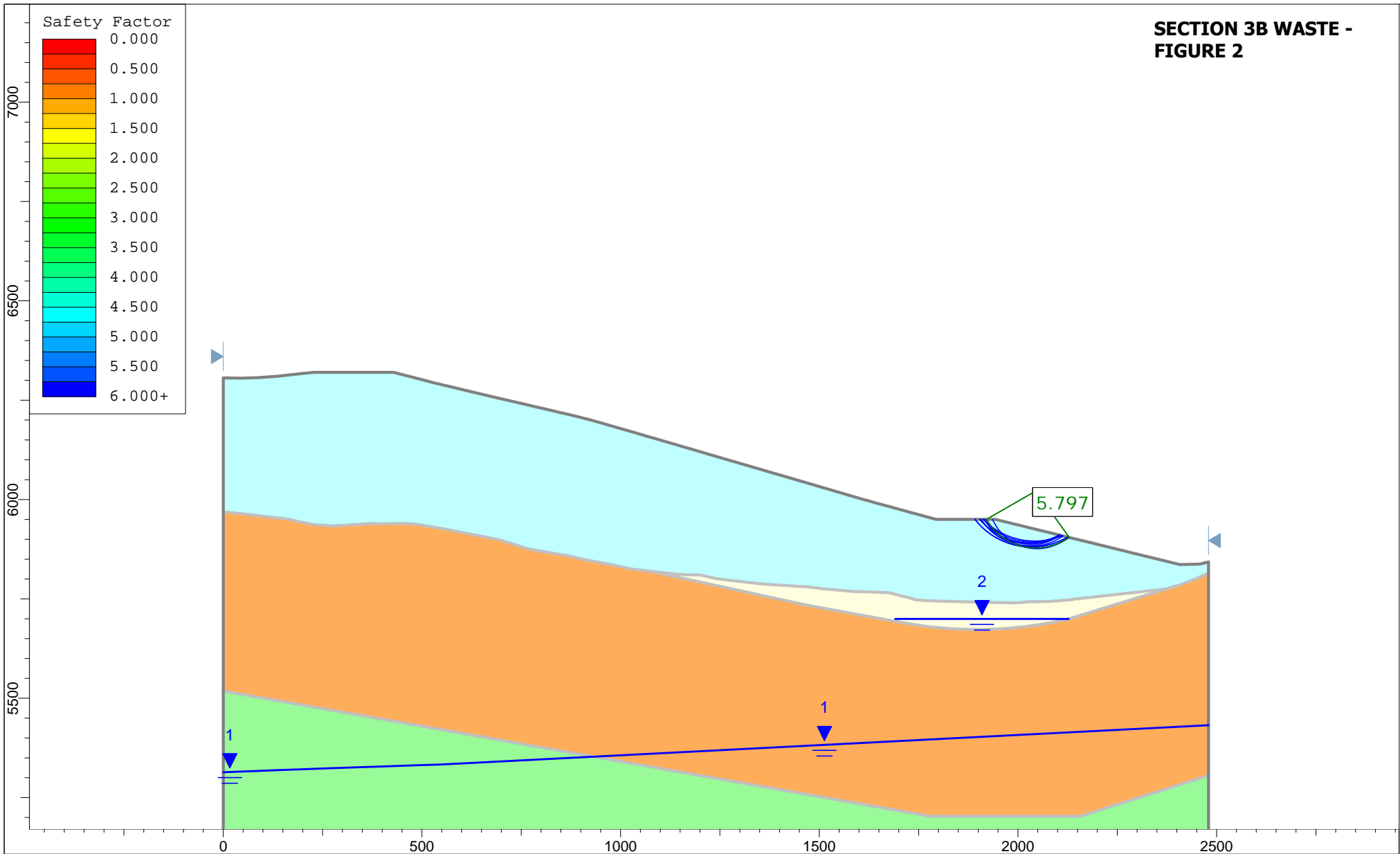
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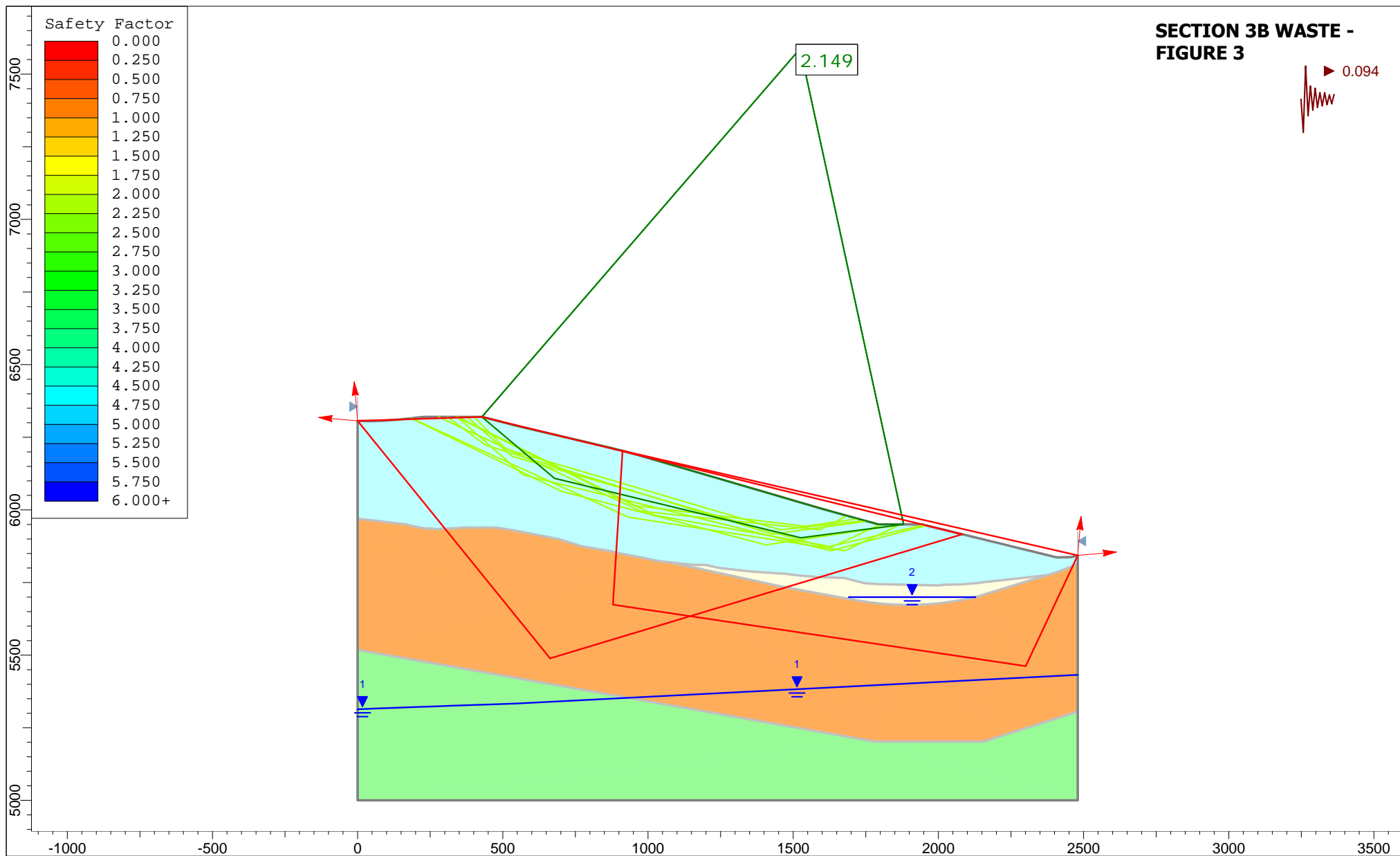
SECTION 3B WASTE - FIGURE 1



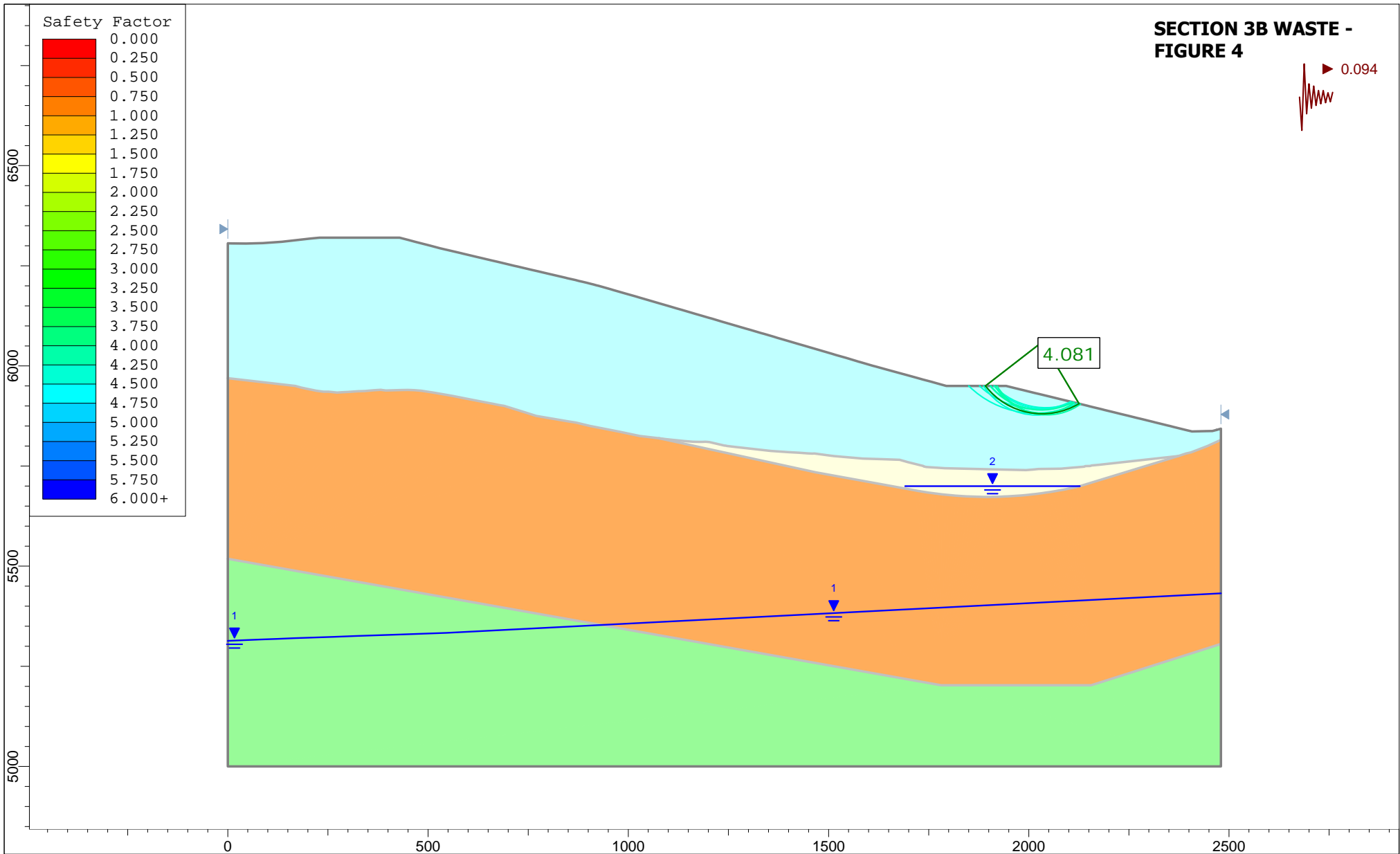
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Tyrone Mine Closure Stockpile Stability - SECTION 3B WASTE			
Analysis Description			
Static - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	3B.slmd		



 <small>SLIDEINTERPRET 8.014</small>	Project					Tyrone Mine Closure Stockpile Stability					
	Analysis Description					Static - Circular Failure (GLE / Morgenstern-Price)					
	Figure		Scale		1:4013		Company		Golder Associates		
	Date							File Name		3B.slmd	



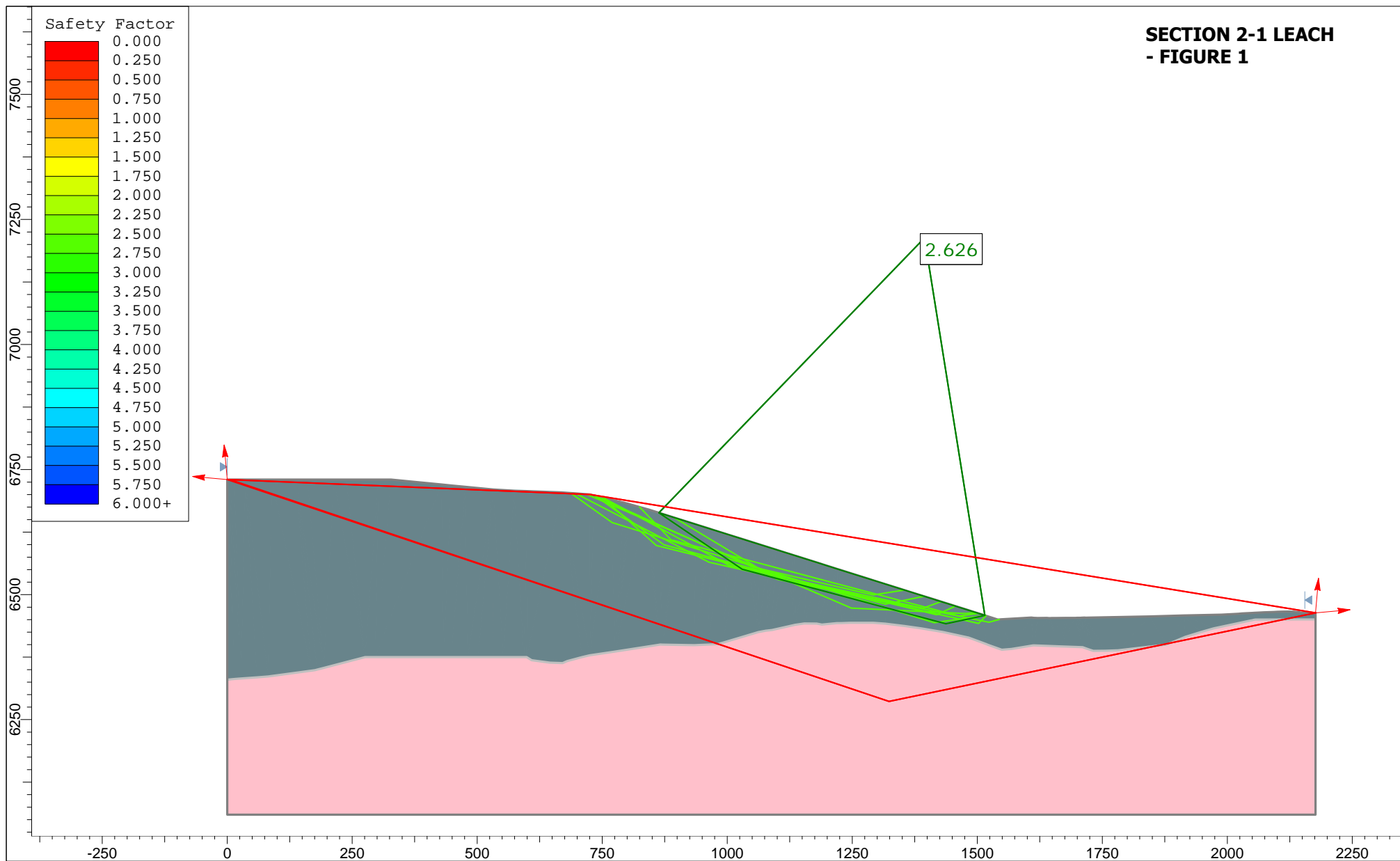
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Tyrone Mine Closure Stockpile Stability - SECTION 3B WASTE			
Analysis Description			
Seismic - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	3B.slmd		



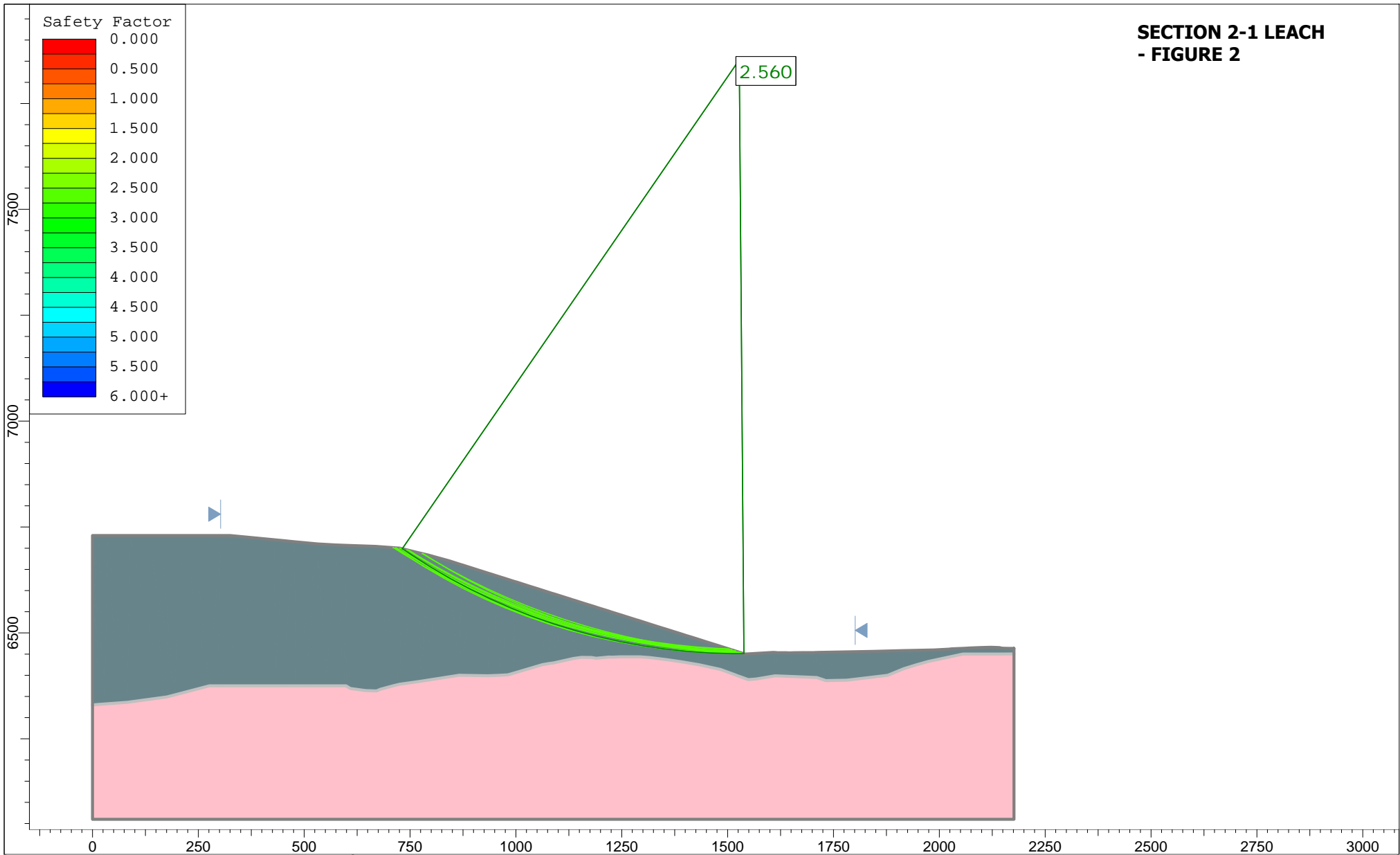
GOLDER

Project			
Tyrone Mine Closure Stockpile Stability - SECTION 3B WASTE			
Analysis Description			
Seismic - Circular Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	3B.slmd		

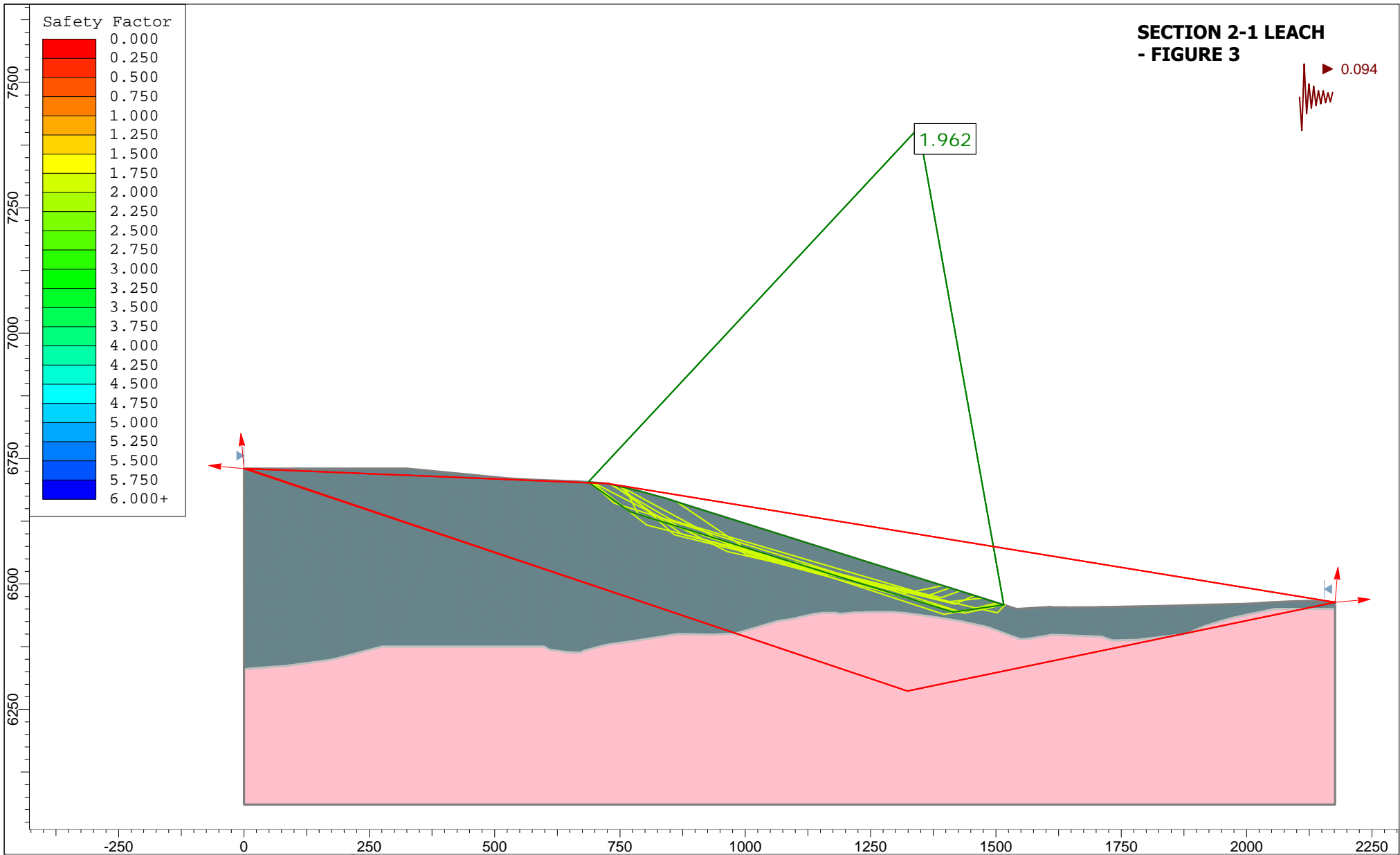
SECTION 2-1 LEACH - FIGURE 1



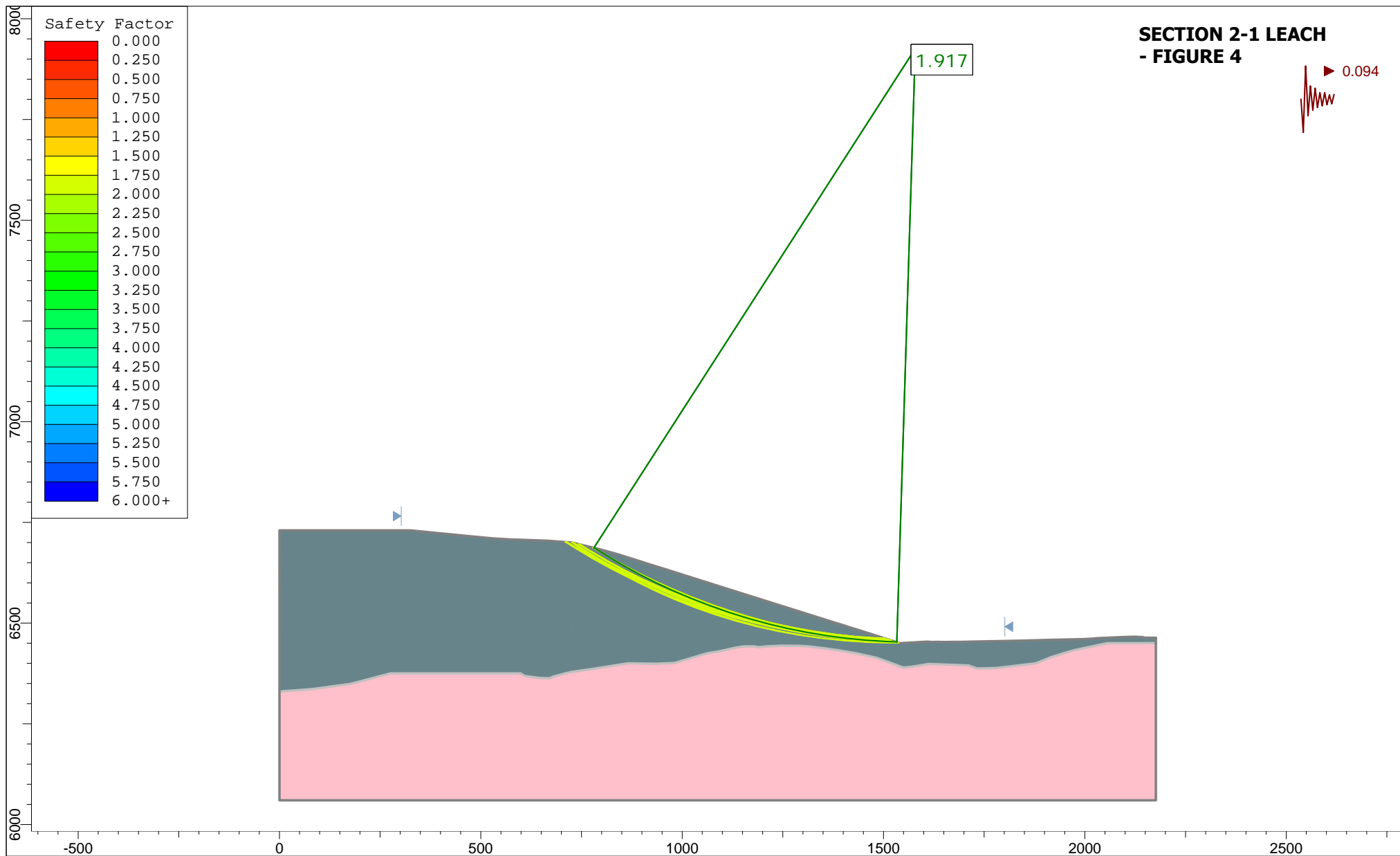
Project				Tyrone Mine Closure Stockpile Stability - SECTION 2 LEACH (Area 1)	
Analysis Description				Static - Block Failure (GLE / Morgenstern-Price)	
Figure	Scale	1:3206	Company	Golder Associates	
Date	1/2/2019		File Name	4C-1.slmd	



Project			Tyrone Mine Closure Stockpile Stability - SECTION 2 LEACH (Area 1)		
Analysis Description			Static - Circular Failure (GLE / Morgenstern-Price)		
Figure	Scale	1:3766	Company	Golder Associates	
Date			File Name	4C-1.slmd	



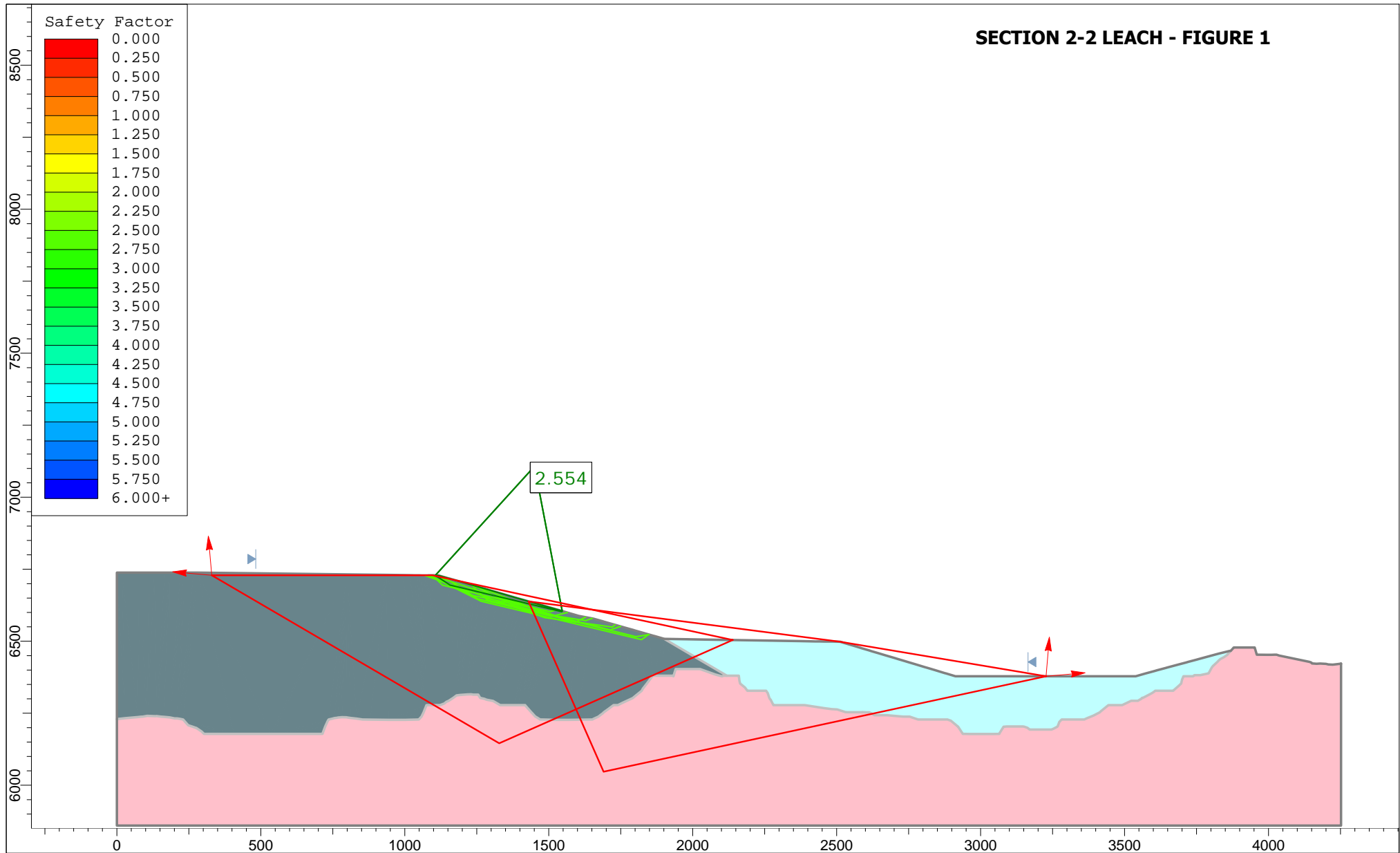
Project			
Tyrone Mine Closure Stockpile Stability - SECTION 2 LEACH (Area 1)			
Analysis Description			
Seismic - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	4C-1.slmd		



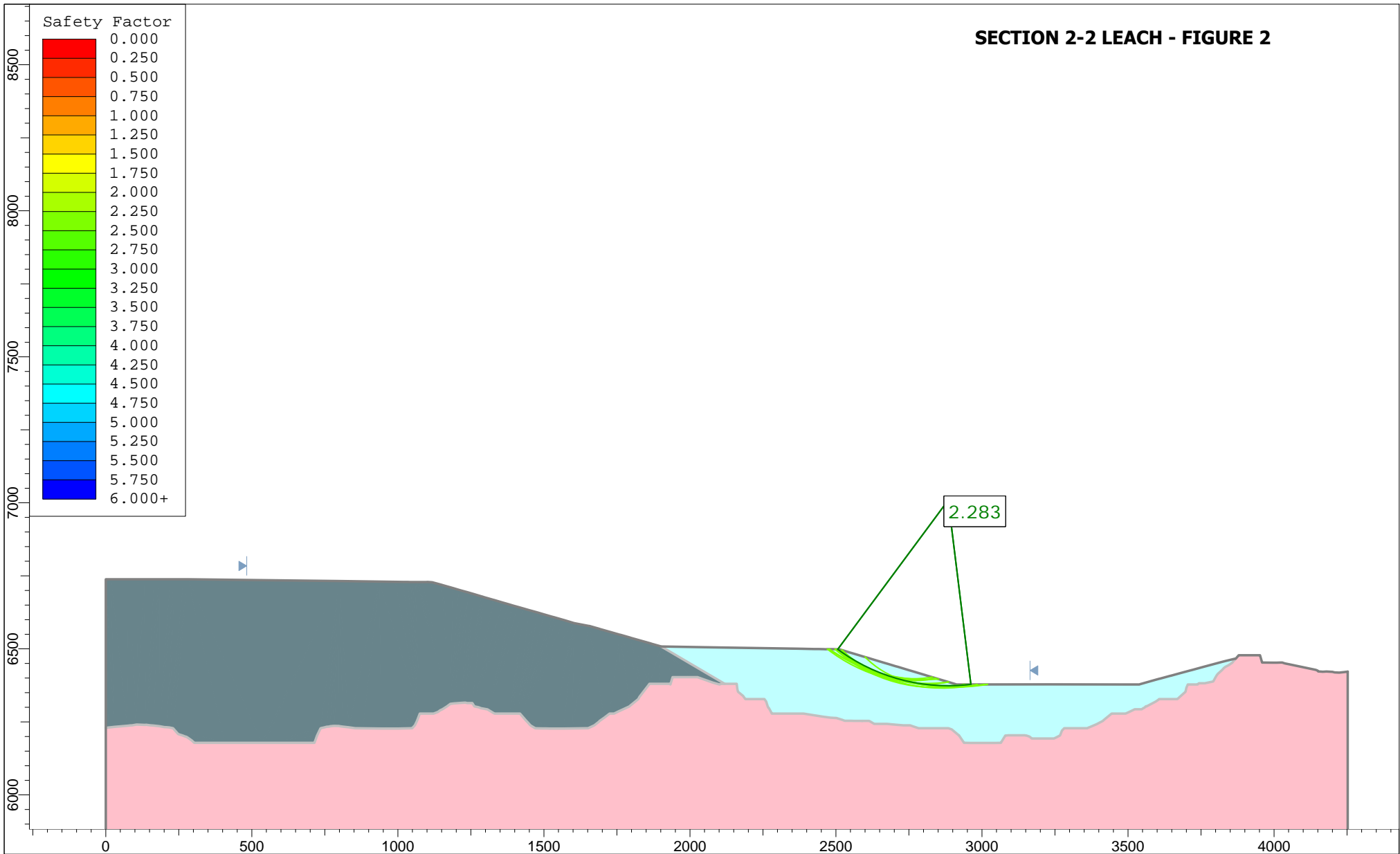
GOLDER

Project			
Tyrone Mine Closure Stockpile Stability - SECTION 2 LEACH (Area 1)			
Analysis Description			
Seismic - Circular Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	4C-1.slmd		

SECTION 2-2 LEACH - FIGURE 1

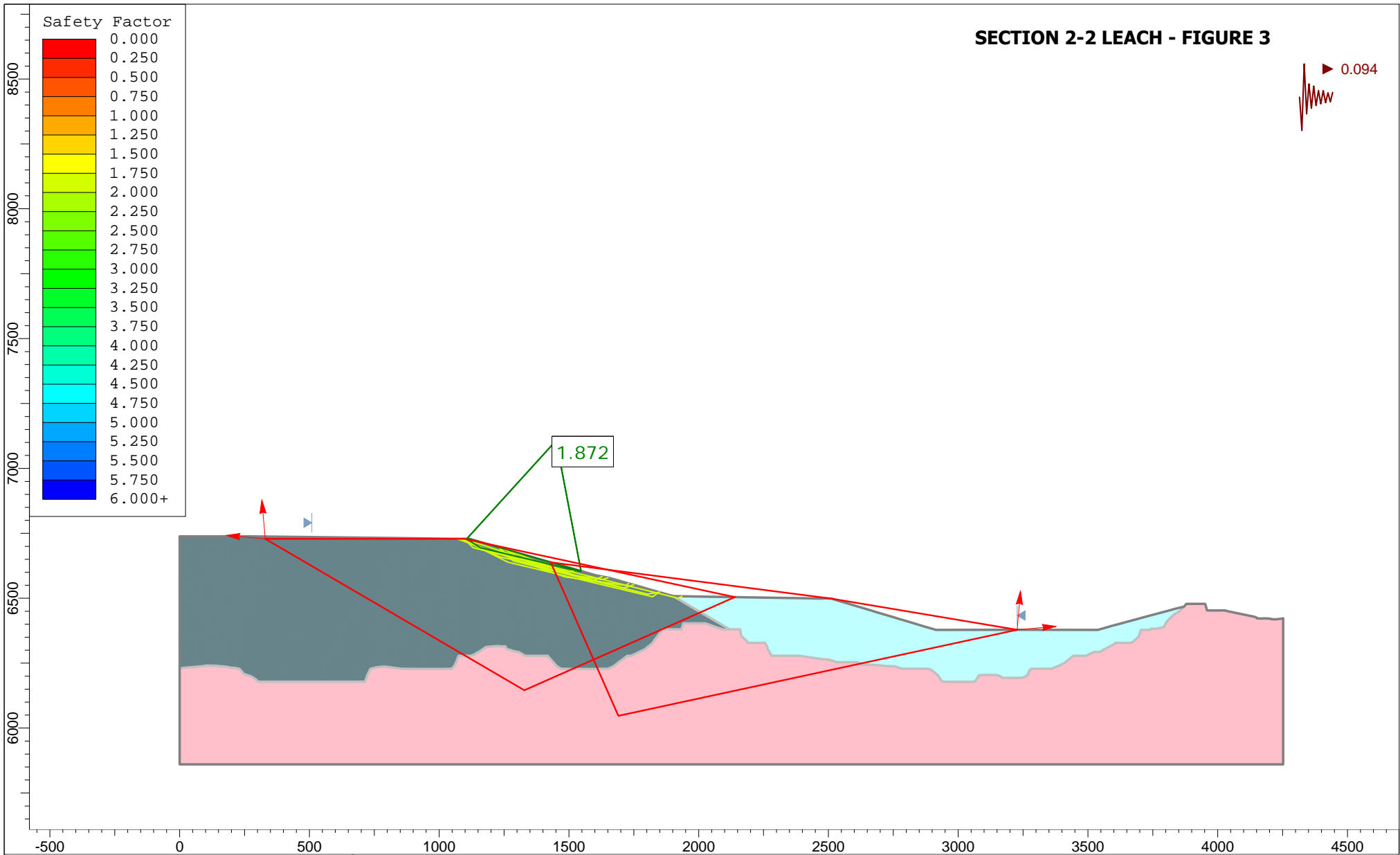


Project			
Tyrone Mine Closure Stockpile Stability - 2 Leach (Area 2) and San Salvador In-Pit			
Analysis Description			
Static - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
	1:5530	Golder Associates	
Date	File Name		
1/2/2019	SS.slmd		

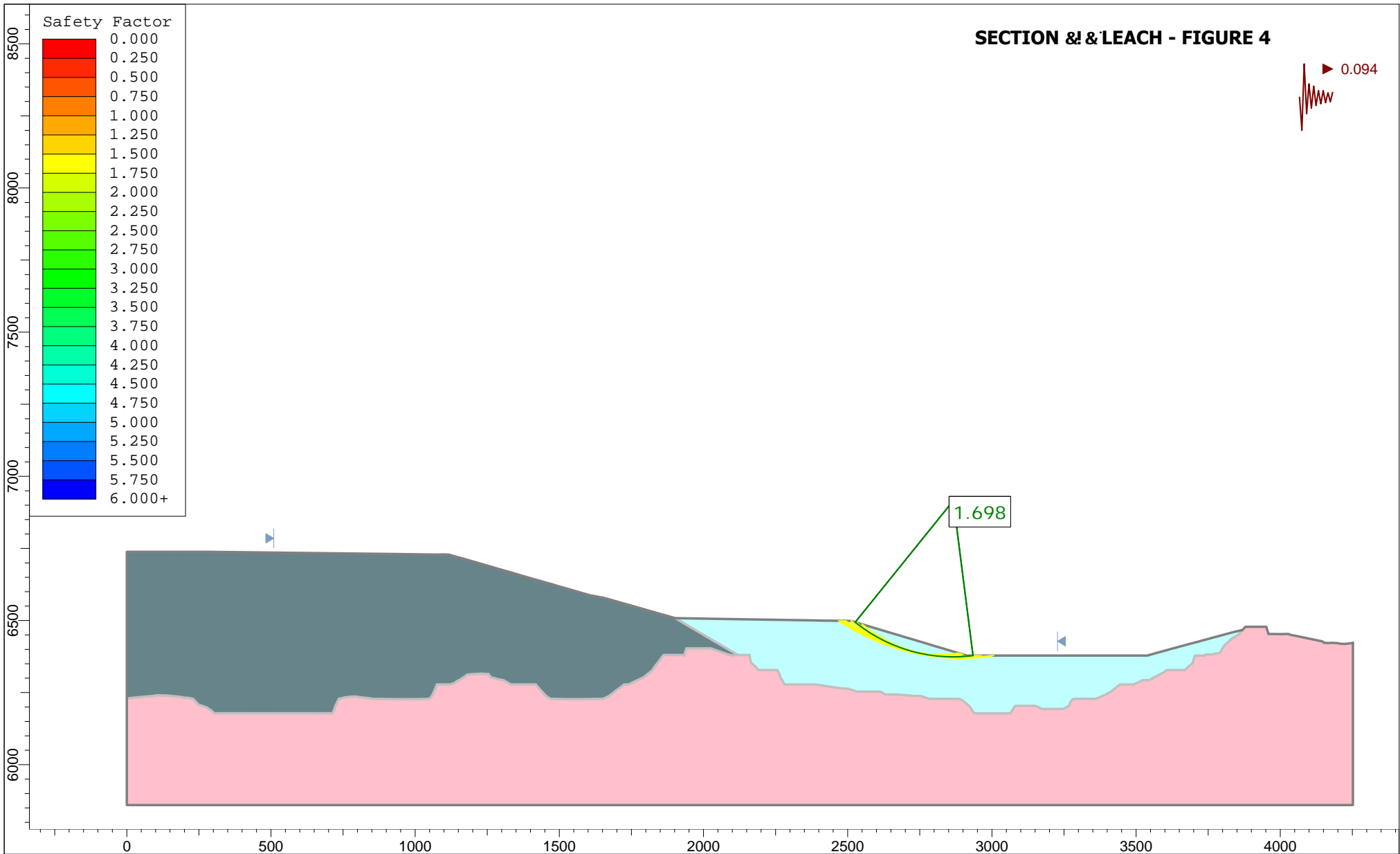


GOLDER

Project			Tyrone Mine Closure Stockpile Stability - 2 Leach (Area 2) and San Salvador In-Pit	
Analysis Description			Static - Circular Failure (GLE / Morgenstern-Price)	
Figure	Scale	1:5459	Company	Golder Associates
Date			File Name	SS.slmd



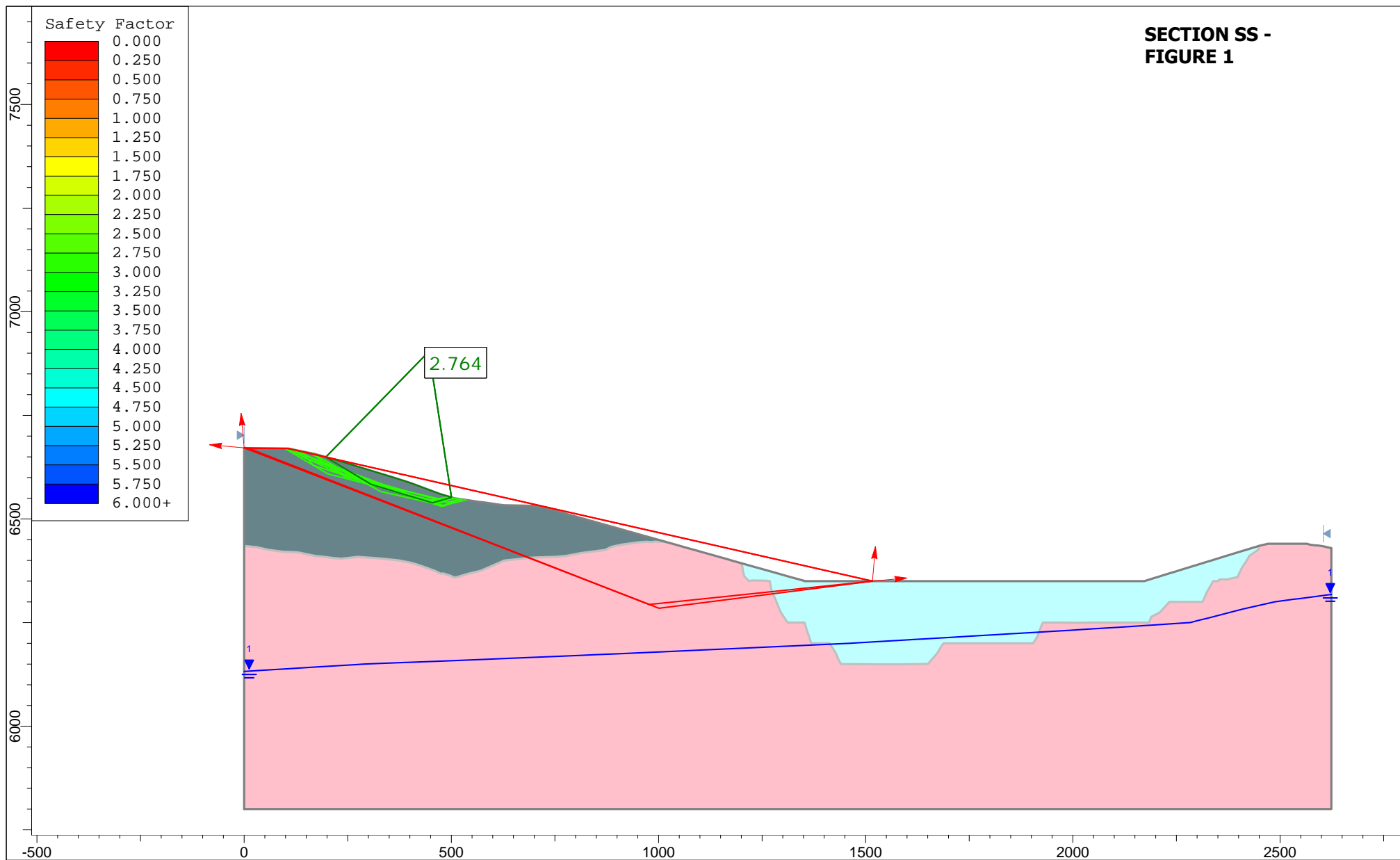
Project			
Tyrone Mine Closure Stockpile Stability - 2 Leach (Area 2) and San Salvador In-Pit			
Analysis Description			
Seismic - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	SS.slmd		



GOLDER

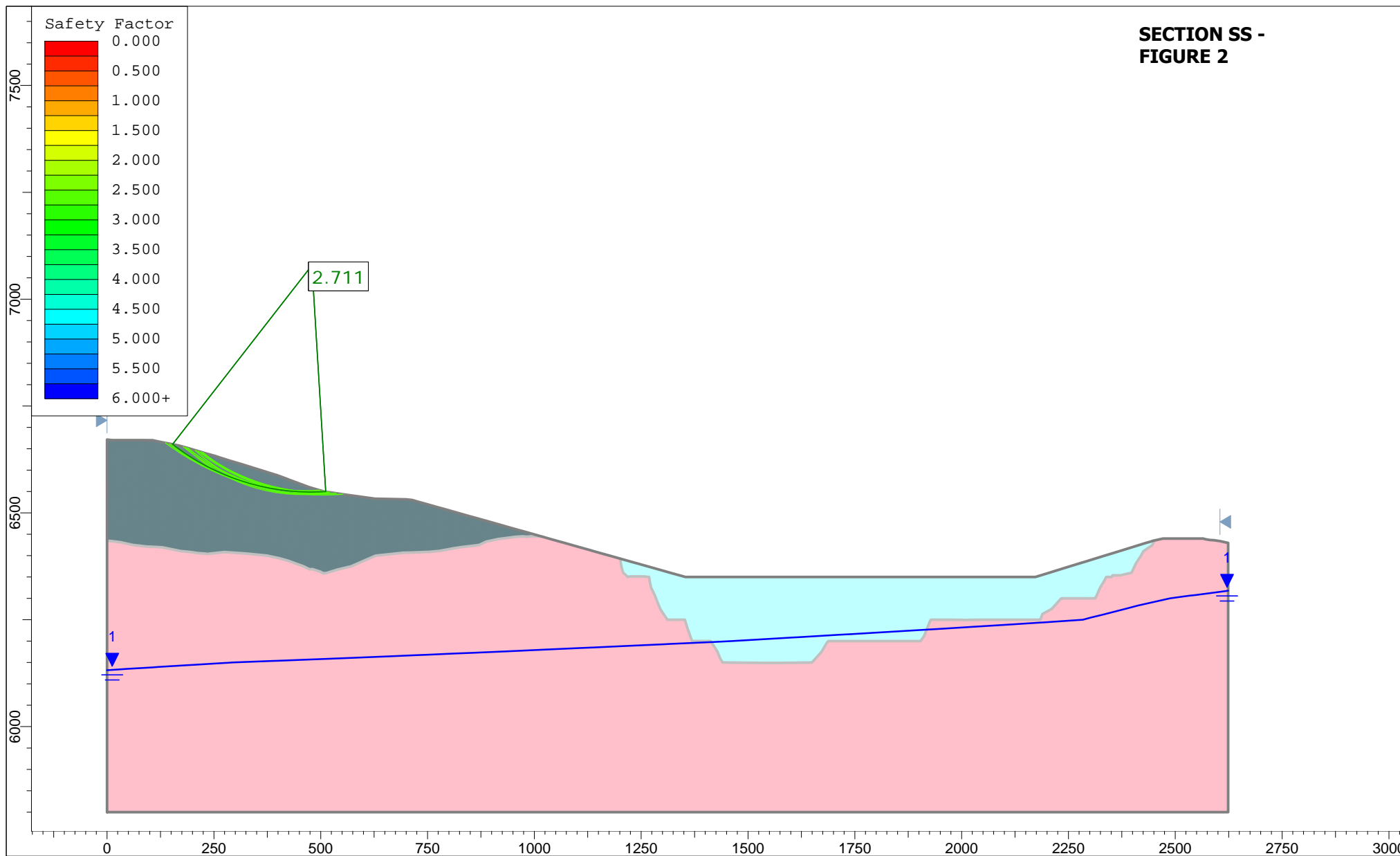
Project			Tyrone Mine Closure Stockpile Stability - 2 Leach (Area 2) and San Salvador In-Pit		
Analysis Description			Seismic - Circular Failure (GLE / Morgenstern-Price)		
Figure	Scale	1:5530	Company	Golder Associates	
Date	1/2/2019		File Name	SS.slmd	

SECTION SS - FIGURE 1

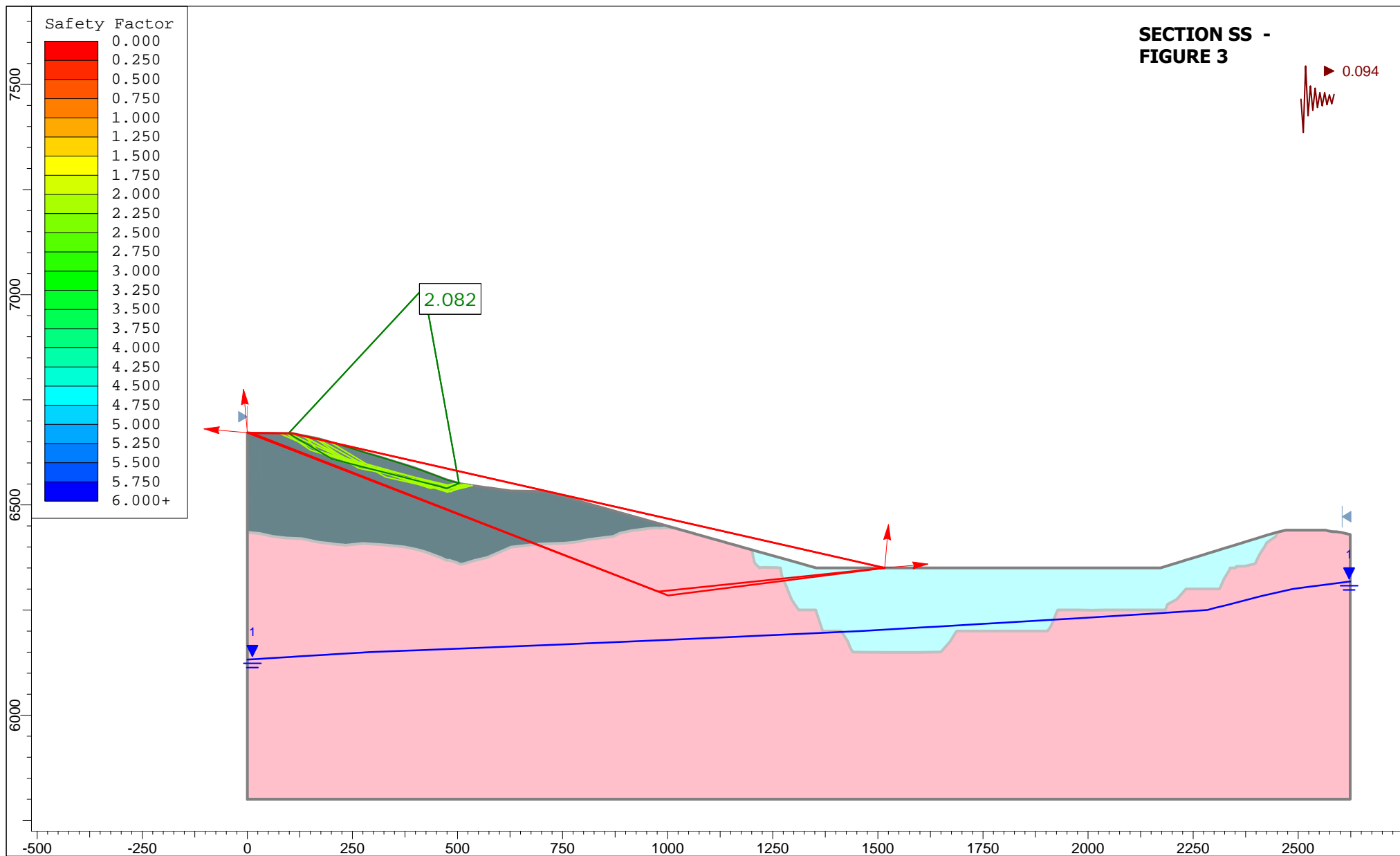


Project				Tyrone Mine Closure Stockpile Stability - San Salvador Waste Backfill (2 Leach Area 1)	
Analysis Description				Static - Block Failure (GLE / Morgenstern-Price)	
Figure	Scale	1:3865	Company	Golder Associates	
Date	1/2/2019		File Name	4C-2.slmd	

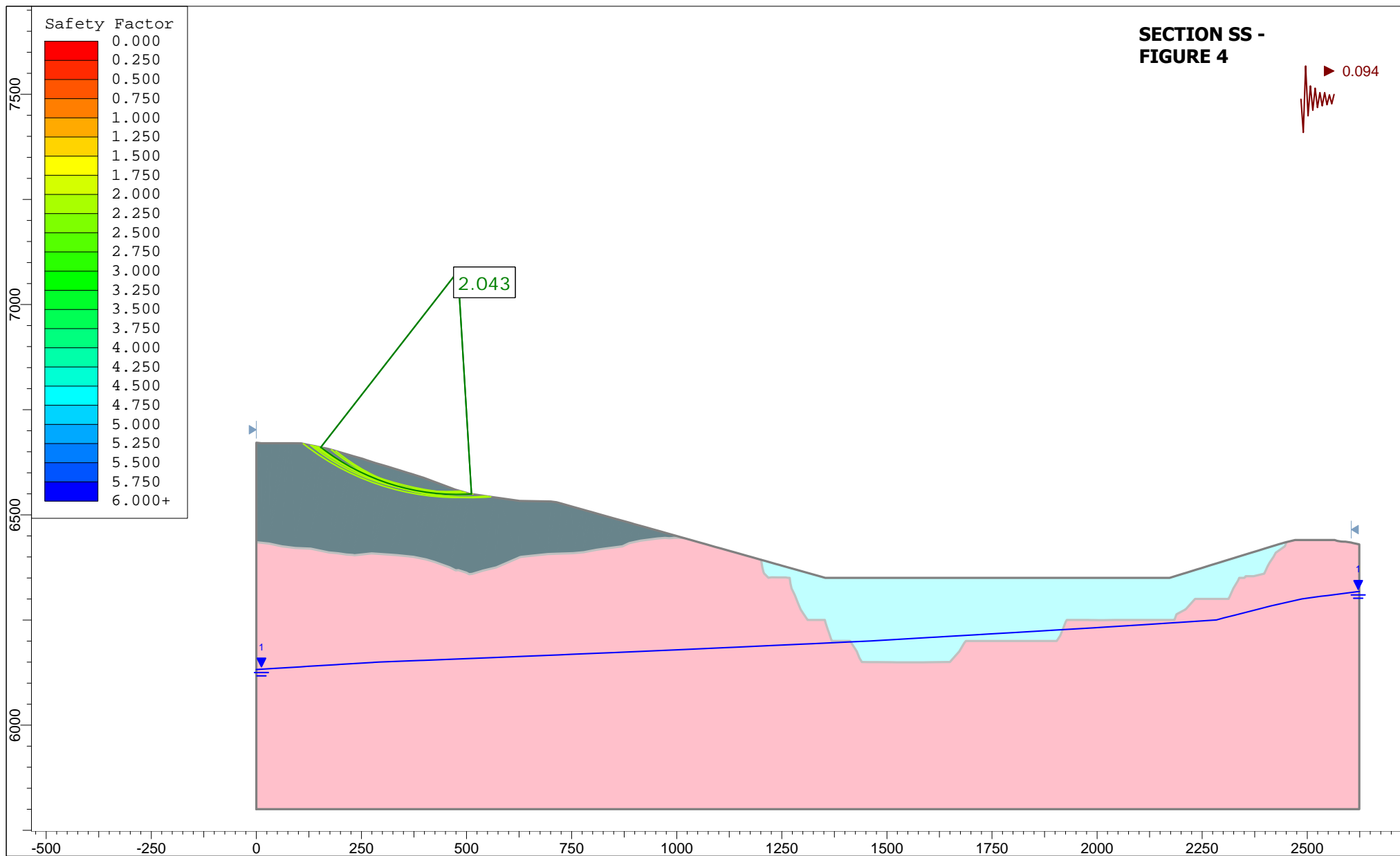
**SECTION SS -
FIGURE 2**



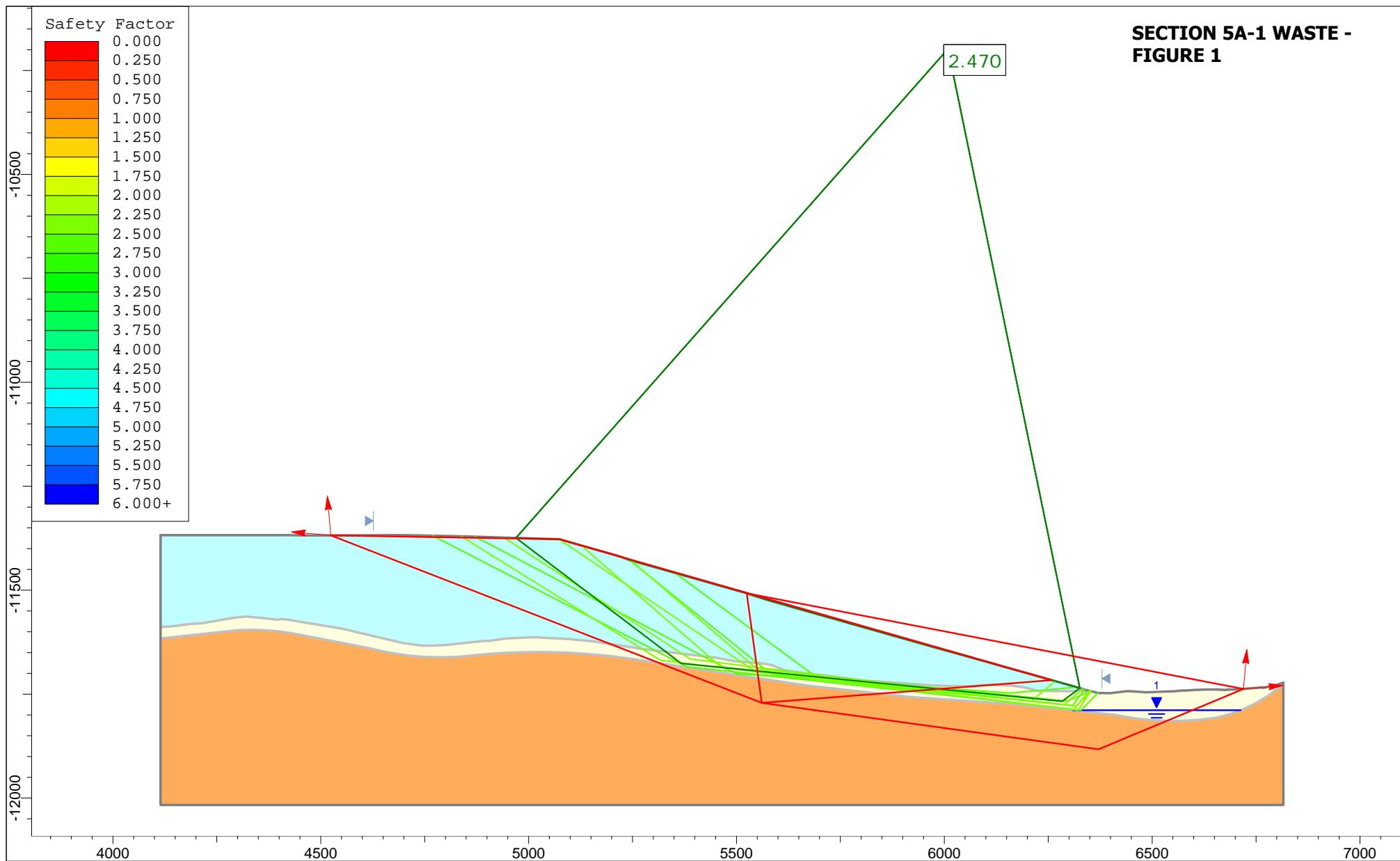
Project		Tyrone Mine Closure Stockpile Stability - San Salvador Waste Backfill (2 Leach Area 1)		
Analysis Description		Static - Circular Failure (GLE / Morgenstern-Price)		
Figure	Scale	1:3730	Company	Golder Associates
Date			File Name	4C-2.slmd



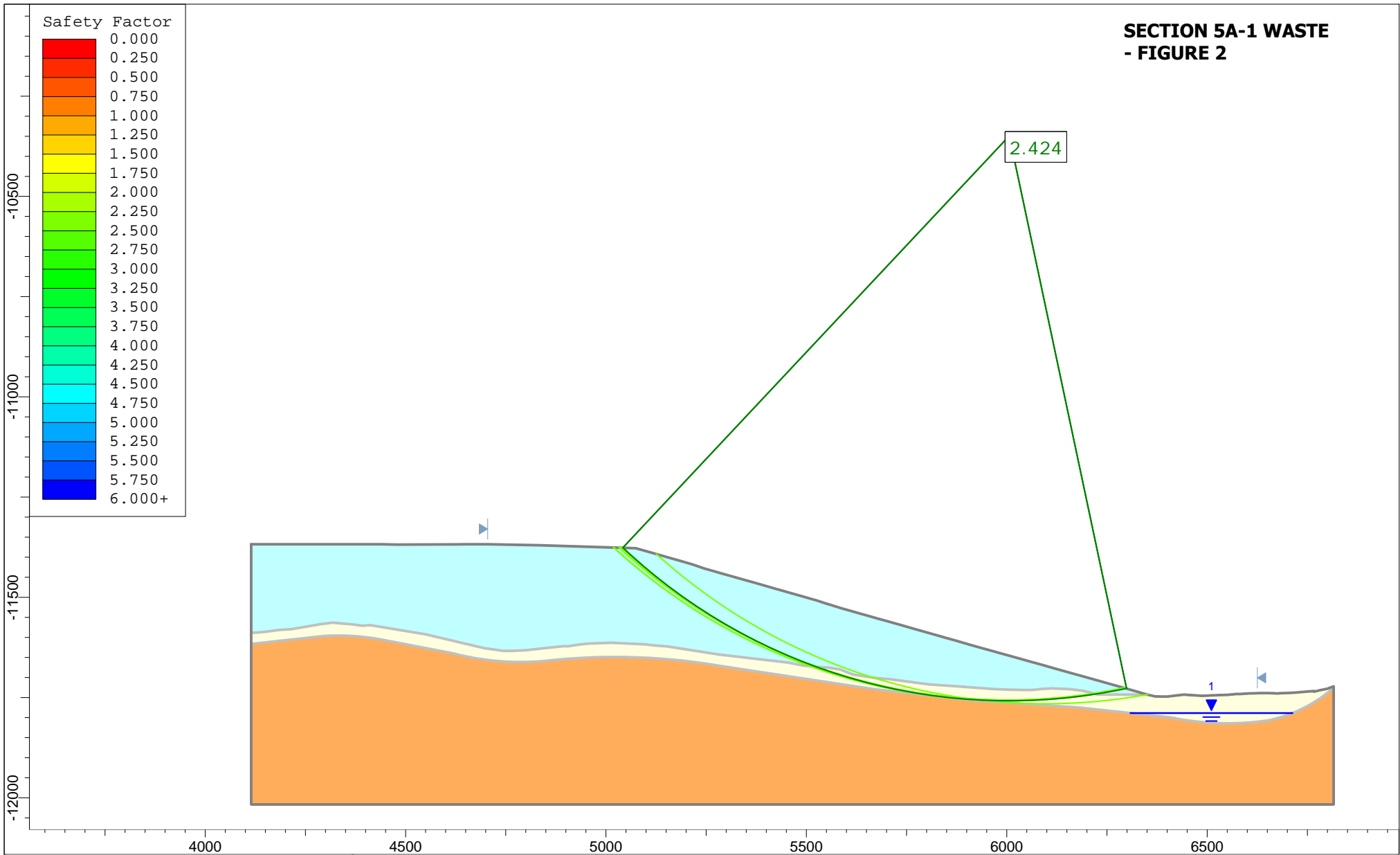
Project			
Tyrone Mine Closure Stockpile Stability - San Salvador Waste Backfill (2 Leach Area 1)			
Analysis Description			
Seismic - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
	1:3791	Golder Associates	
Date	File Name		
1/2/2019	4C-2.slmd		



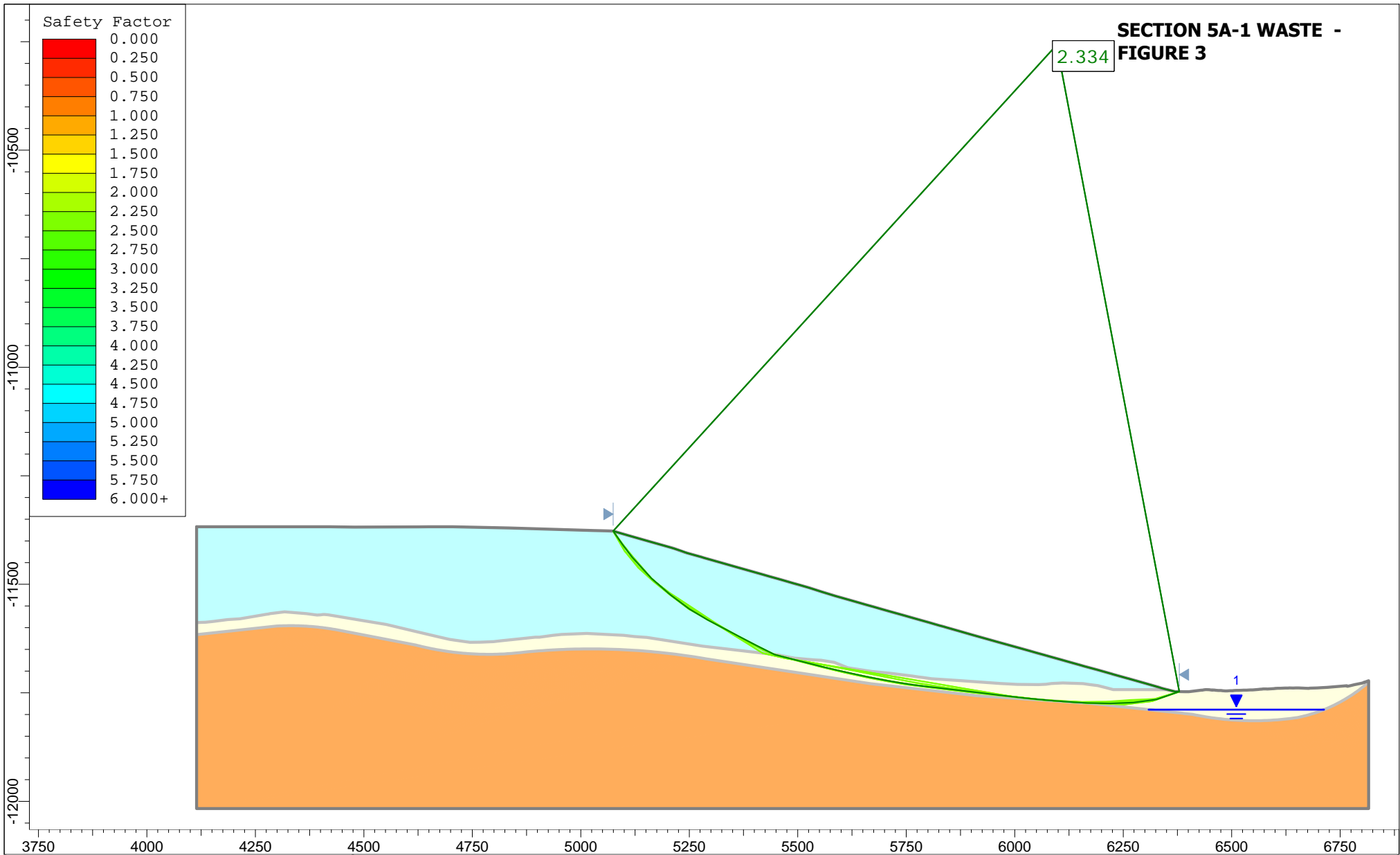
Project			
Tyrone Mine Closure Stockpile Stability - San Salvador Waste Backfill (2 Leach Area 1)			
Analysis Description			
Seismic - Circular Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
	1:3791	Golder Associates	
Date	File Name		
1/2/2019	4C-2.slmd		



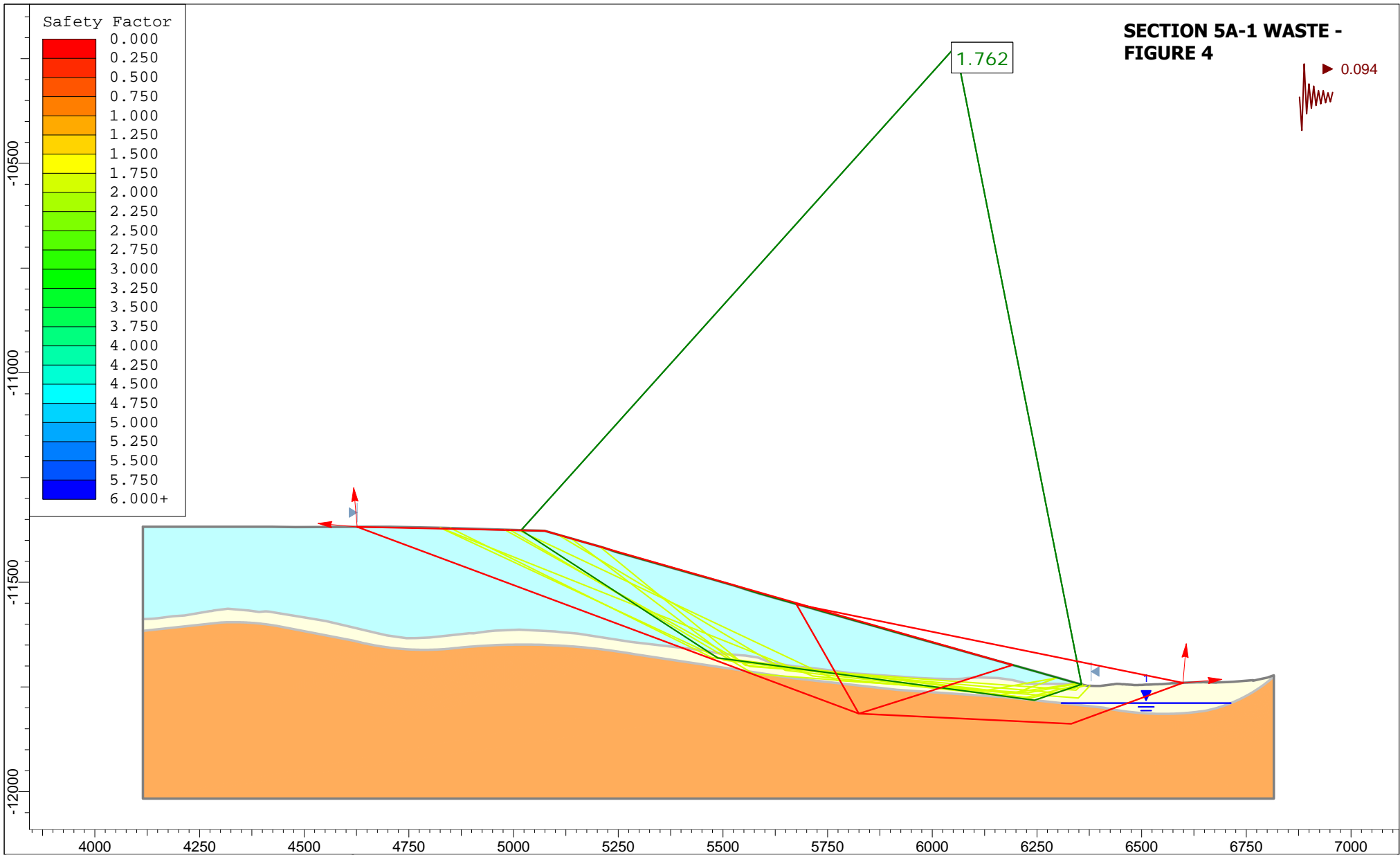
Project			Tyrone Closure Closeout Stability Analysis - SECTION 5A-1 WASTE		
Analysis Description			Static - Block Failure (GLE / Morgenstern-Price)		
Figure	Scale	1:3857	Company		
Date	4/8/2019, 10:08:36 AM		File Name	3c5a(2).slmd	



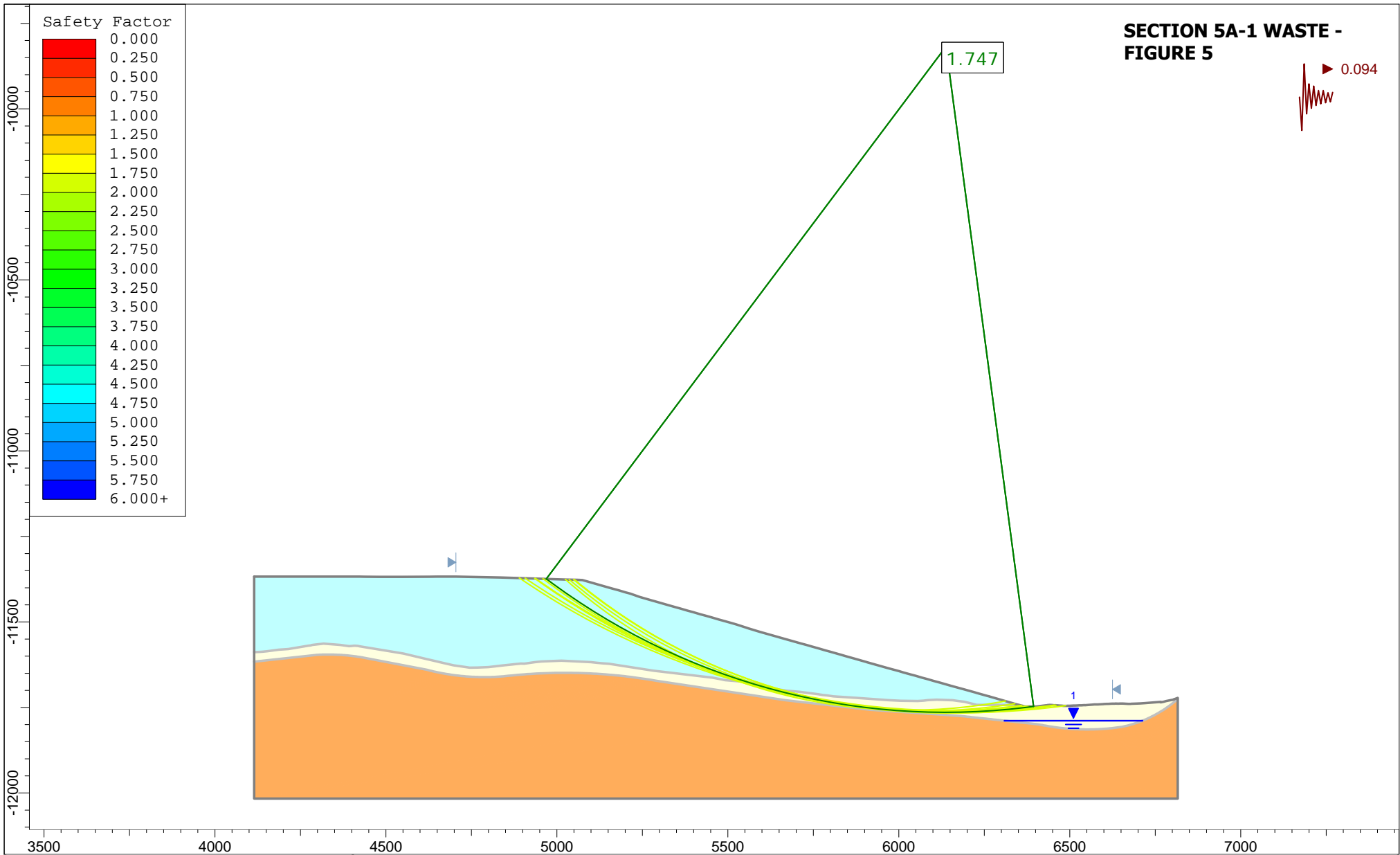
Project			Tyrone Mine Closure Stockpile Stability - SECTION 5A-1 WASTE	
Analysis Description			Static - Circular Failure (GLE / Morgenstern-Price)	
Figure	Scale	1:3978	Company	
Date	4/8/2019, 10:08:36 AM		File Name	
			3c5a(2).slmd	



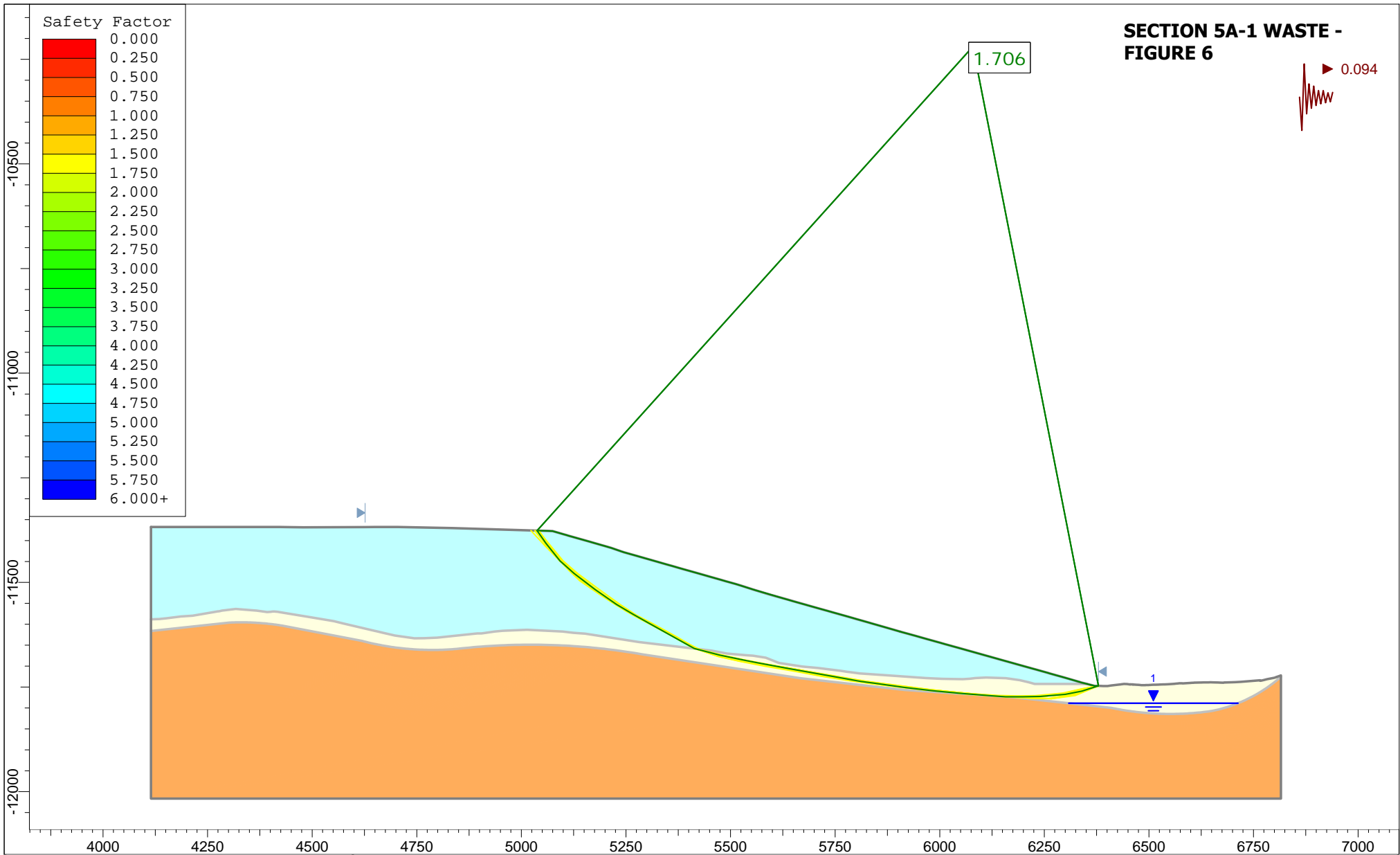
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Analysis Description			Static - Cuckoo Failure (GLE / Morgenstern-Price)		
Figure	Scale	1:3675	Company		
Date	4/8/2019, 10:08:36 AM		File Name	3c5a(2).slmd	



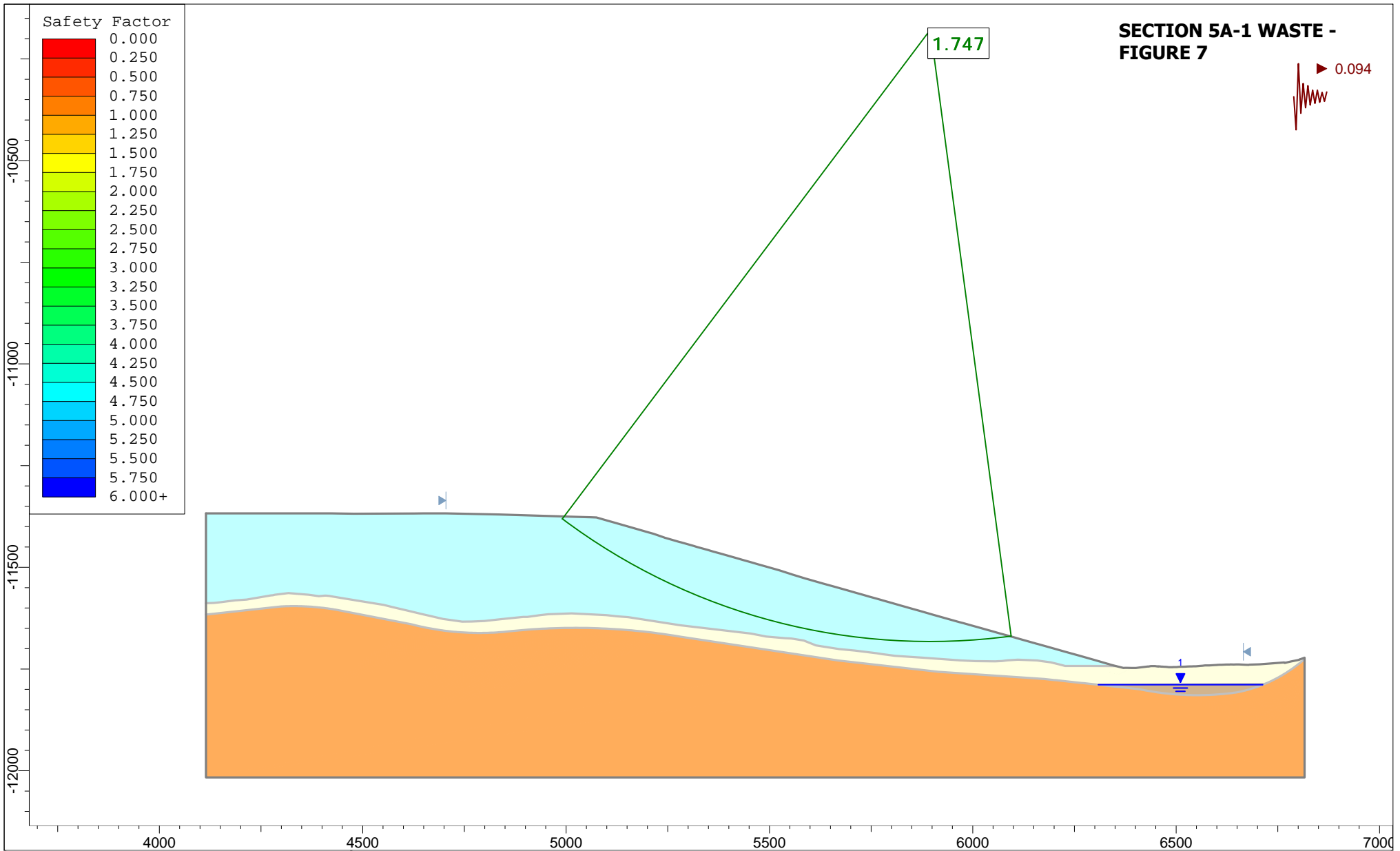
Project			Tyrone Closure Closeout Stability Analysis - SECTION 5A-1 WASTE		
Analysis Description			Seismic - Block Failure (GLE / Morgenstern-Price)		
Figure	Scale	1:3808	Company		
Date	4/8/2019, 10:08:36 AM		File Name		
			3c5a(2).slmd		



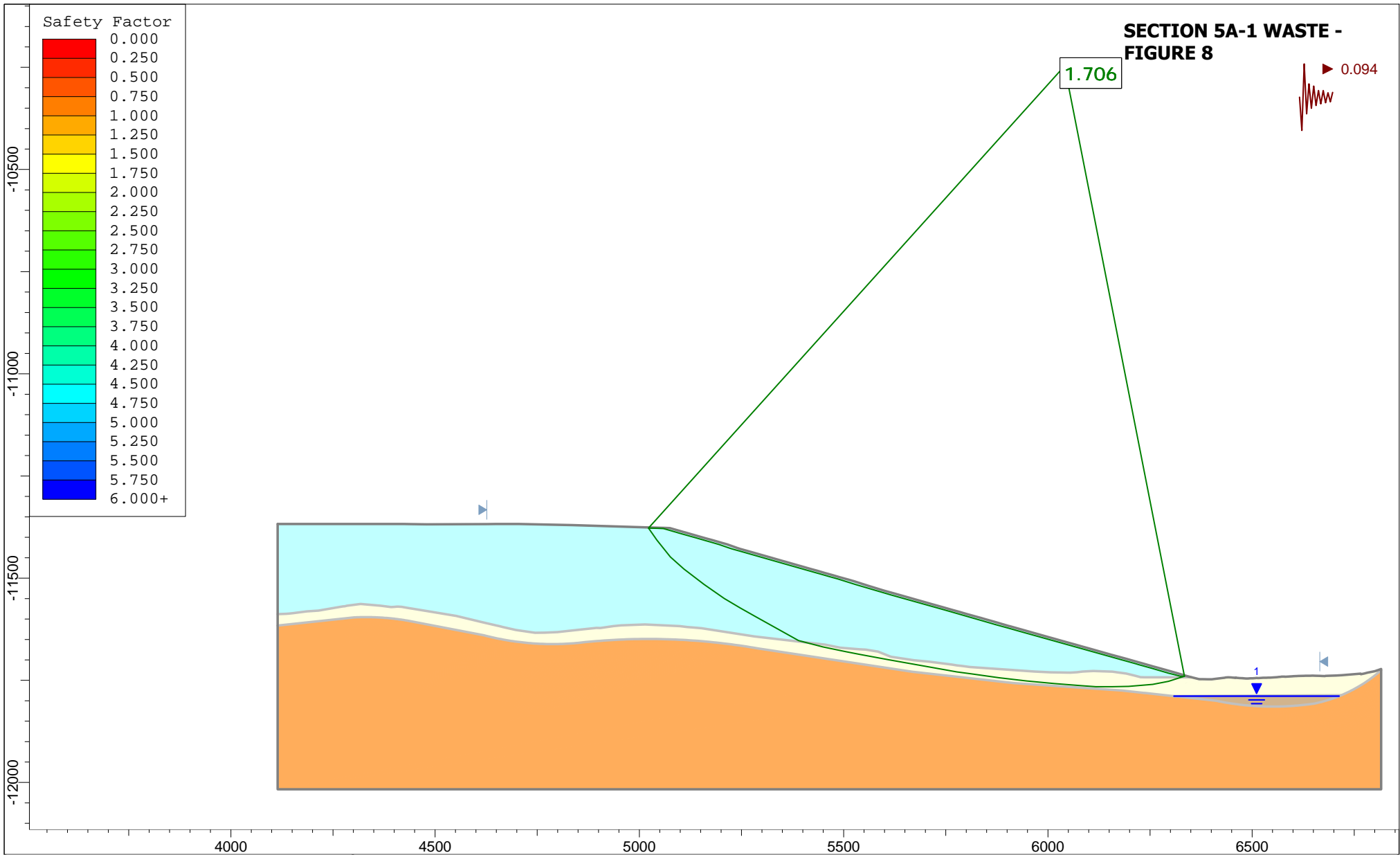
Project			Tyrone Closure Closeout Stability Analysis - SECTION 5A-1 WASTE		
Analysis Description			Seismic - Circular Failure (GLE / Morgenstern-Price)		
Figure	Scale	1:4663	Company		
Date	4/8/2019, 10:08:36 AM		File Name		
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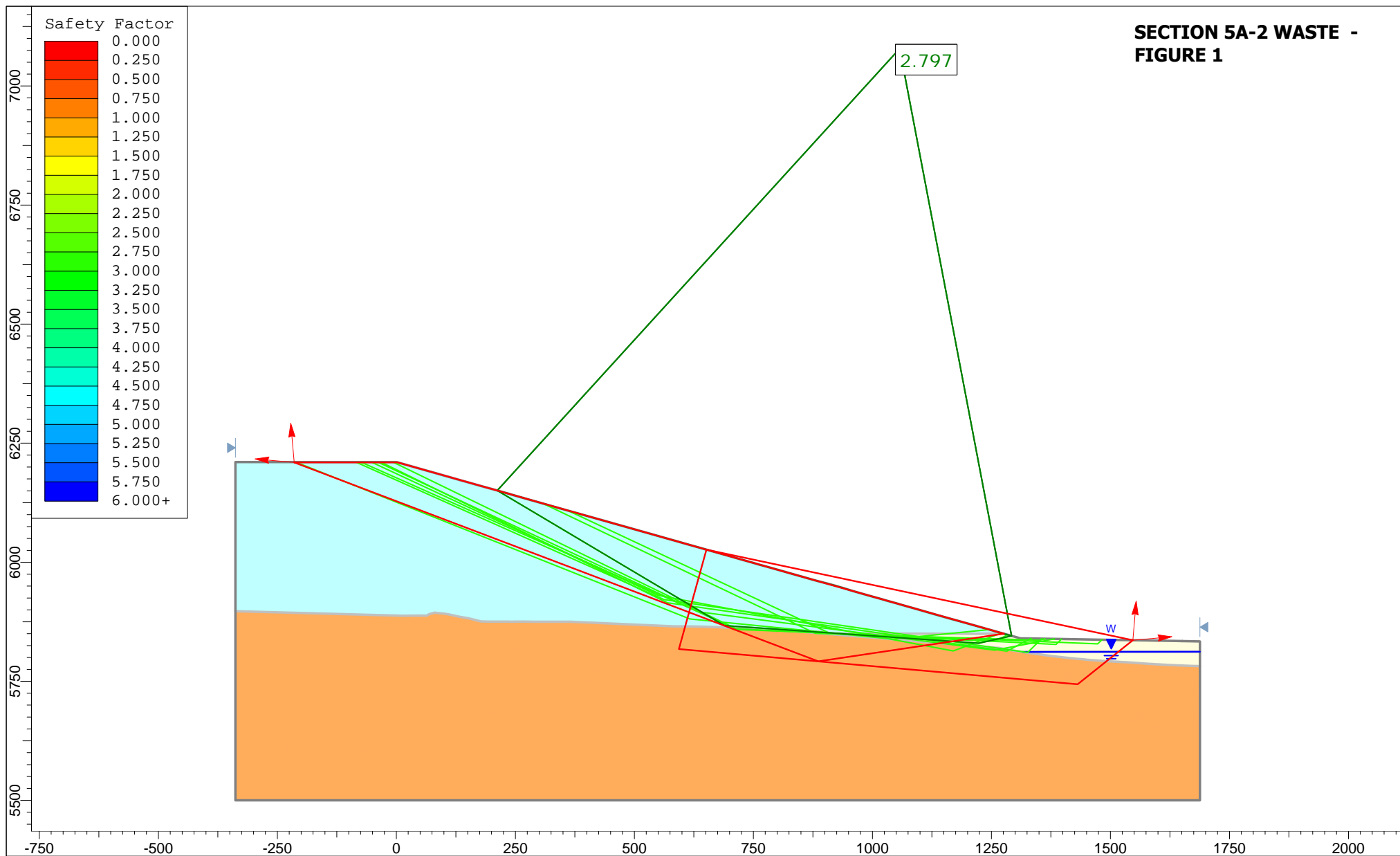
Project			Tyrone Closure Closeout Stability Anaylsis - SECTION 5A-1 WASTE		
Analysis Description			Seismic - Cuckoo Failure (GLE / Morgenstern-Price)		
Figure	Scale	1:3811	Company		
Date	4/8/2019, 10:08:36 AM		File Name	3c5a(2).slmd	



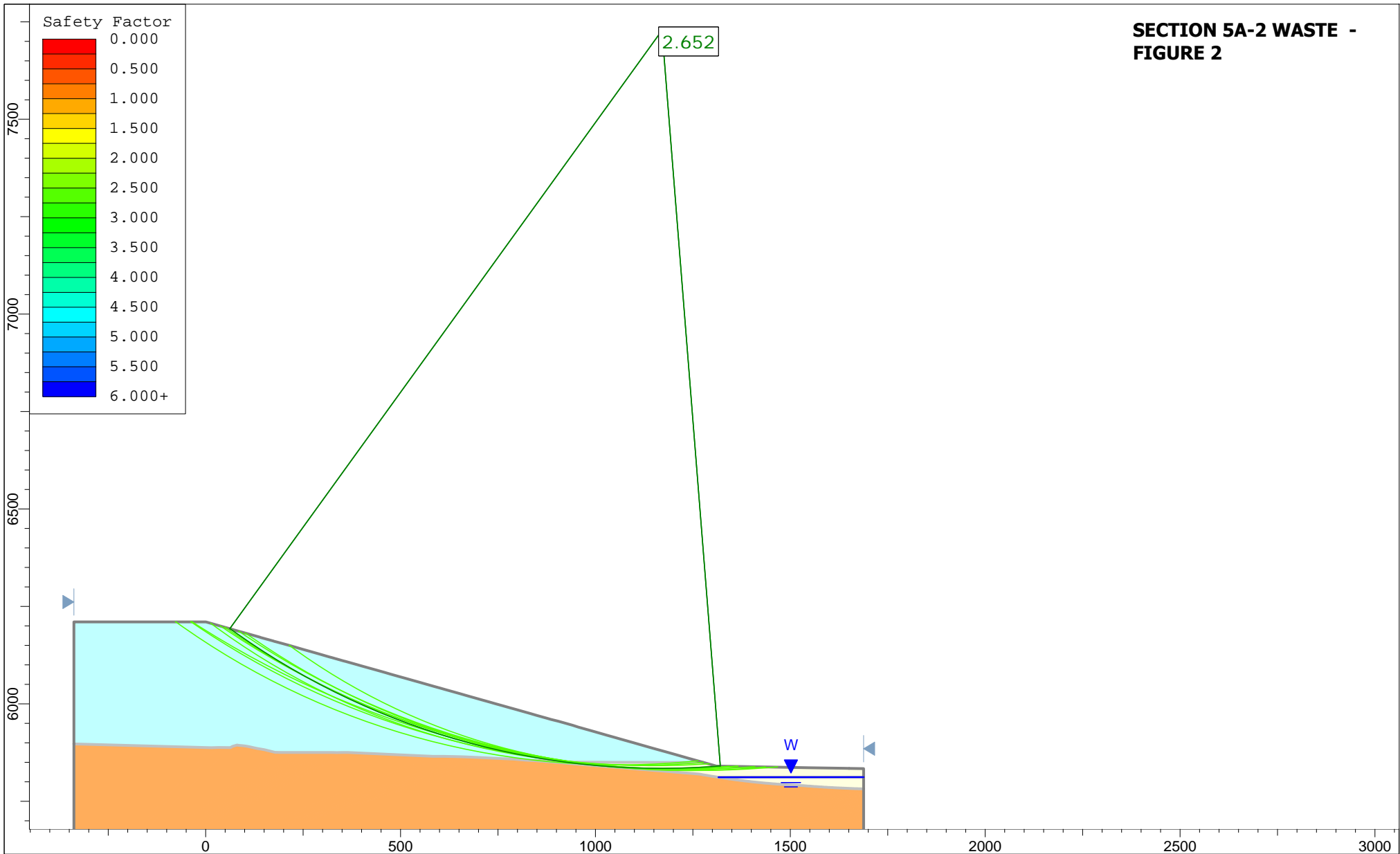
Project			Tyrone Closure Closeout Stability Anaylsis - SECTION 5A-1 WASTE	
Analysis Description			Seismic - Liquefied Qa - Circular Failure (GLE / Morgenstern-Price)	
Figure	Scale	1:3903	Company	
Date	4/8/2019, 10:08:36 AM		File Name	
			3c5a(2).slmd	



Project			Tyrone Closure Closeout Stability Anaylsis - SECTION 5A-1 WASTE	
Analysis Description			Seismic - Liquefied Qa - Cuckoo Failure (GLE / Morgenstern-Price)	
Figure	Scale	1:3903	Company	
Date	4/8/2019, 10:08:36 AM		File Name	
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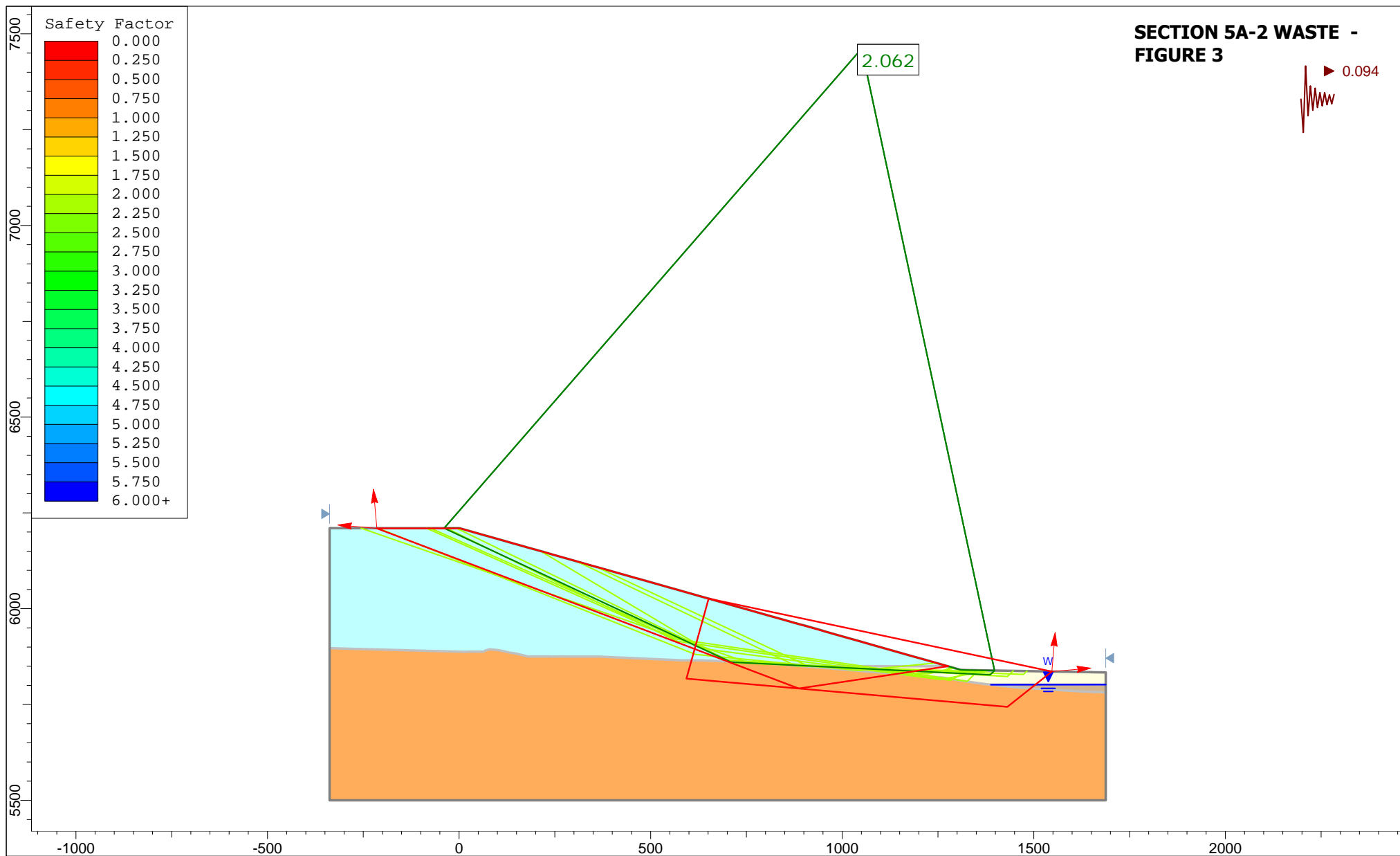
Project			
Tyrone Mine Closure Stockpile Stability - SECTION 5A-2 WASTE			
Analysis Description			
Static - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	5B.slmd		



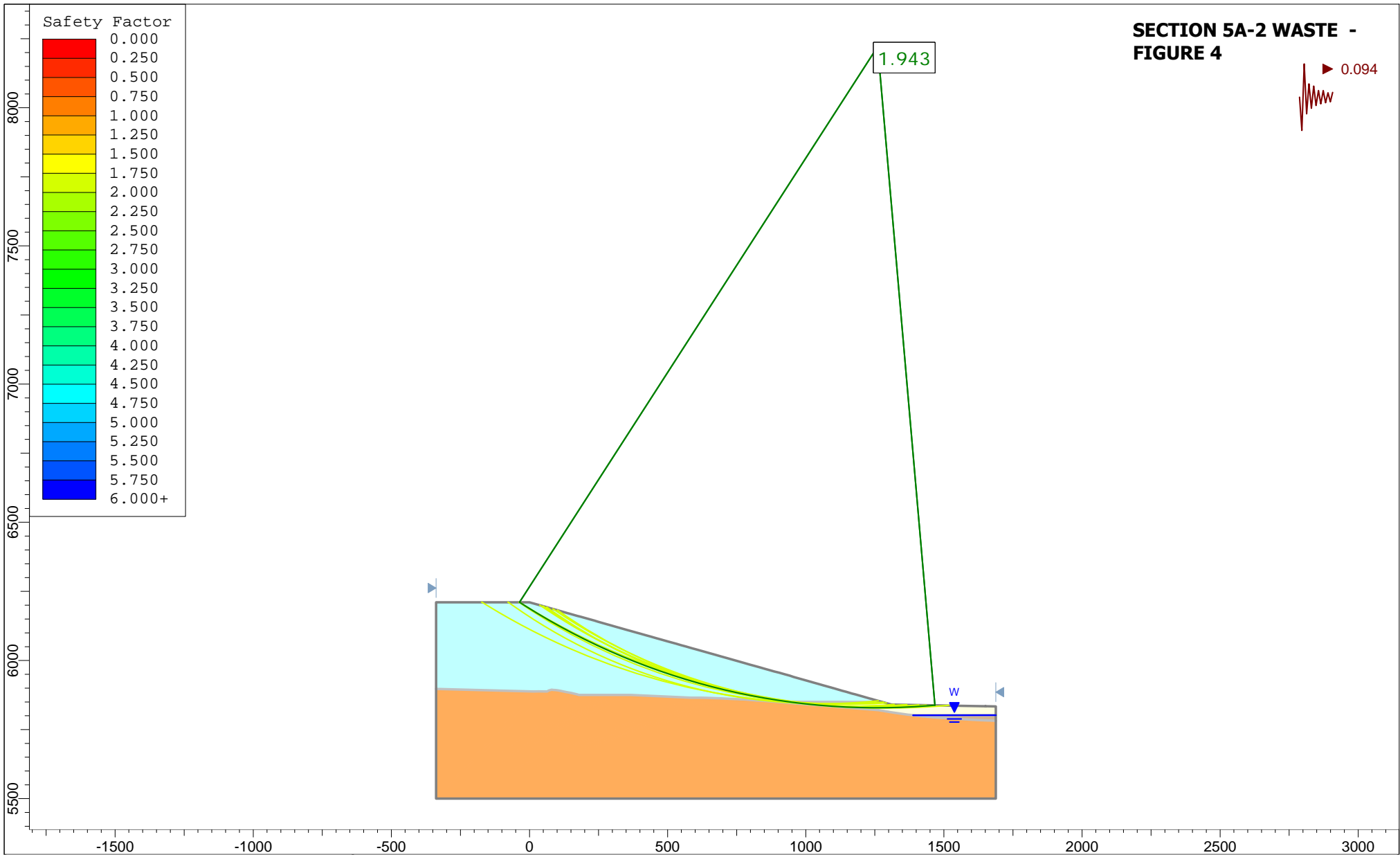
**SECTION 5A-2 WASTE -
FIGURE 2**



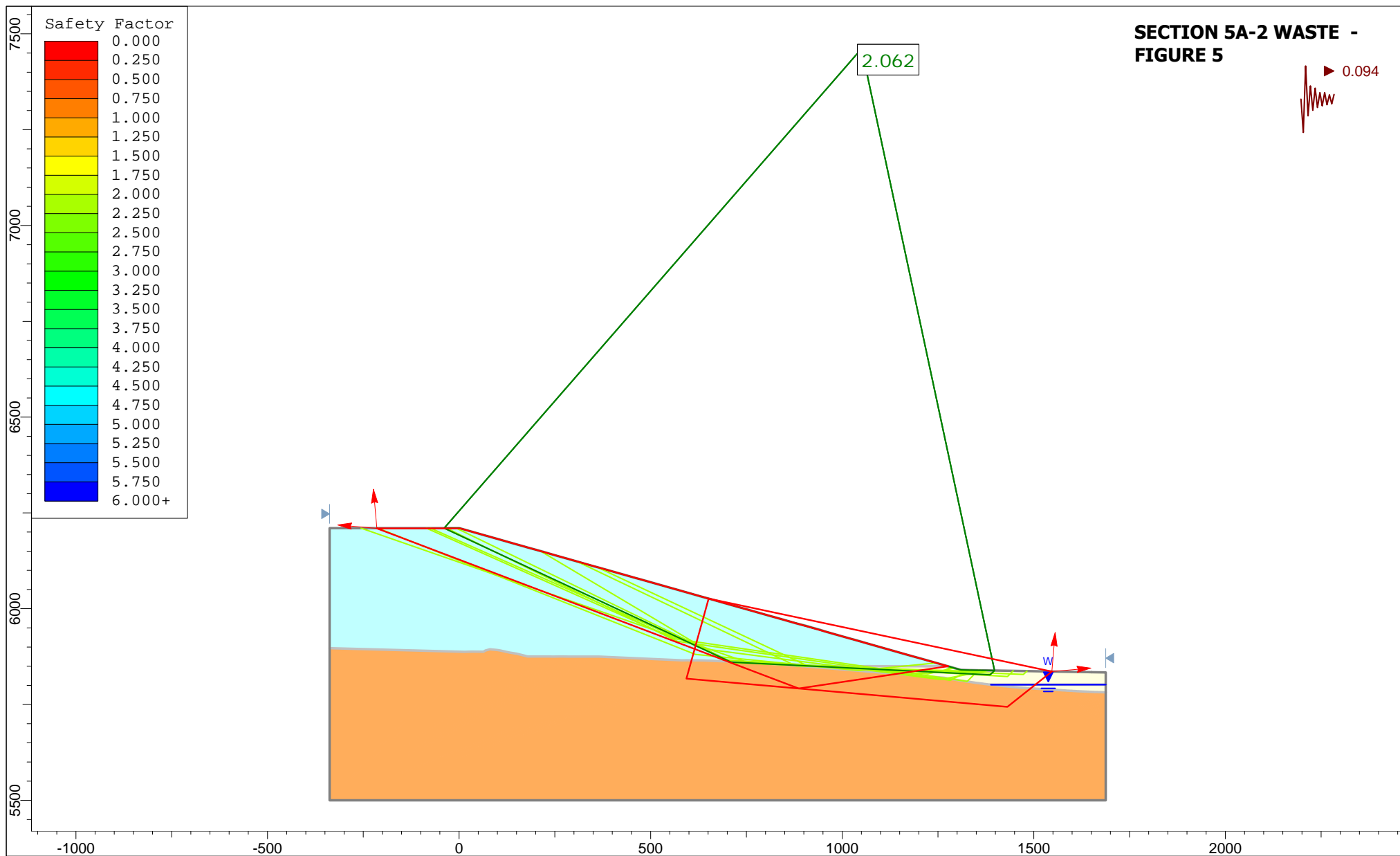
Project				Tyrone Mine Closure Stockpile Stability - SECTION 5A-2 WASTE	
Analysis Description				Static - Circular Failure (GLE / Morgenstern-Price)	
Figure	Scale	1:4091	Company	Golder Associates	
Date				File Name	5B.slmd



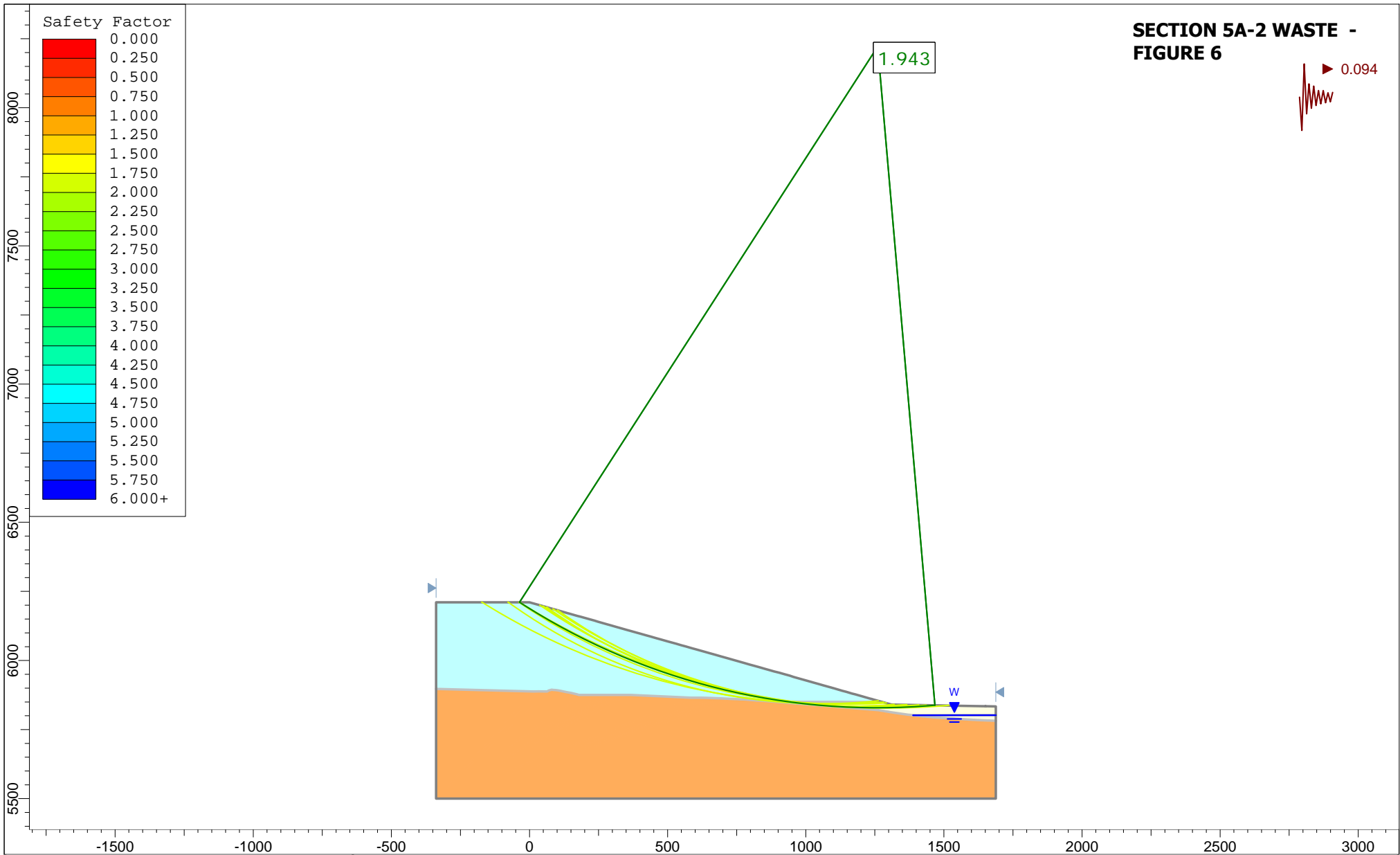
Project		Tyrone Mine Closure Stockpile Stability - SECTION 5A-2 WASTE	
Analysis Description		Seismic - Liquefied Qa - Block Failure (GLE / Morgenstern-Price)	
Figure	Scale	1:4160	Company Golder Associates
Date	1/2/2019		File Name 5B.slmd



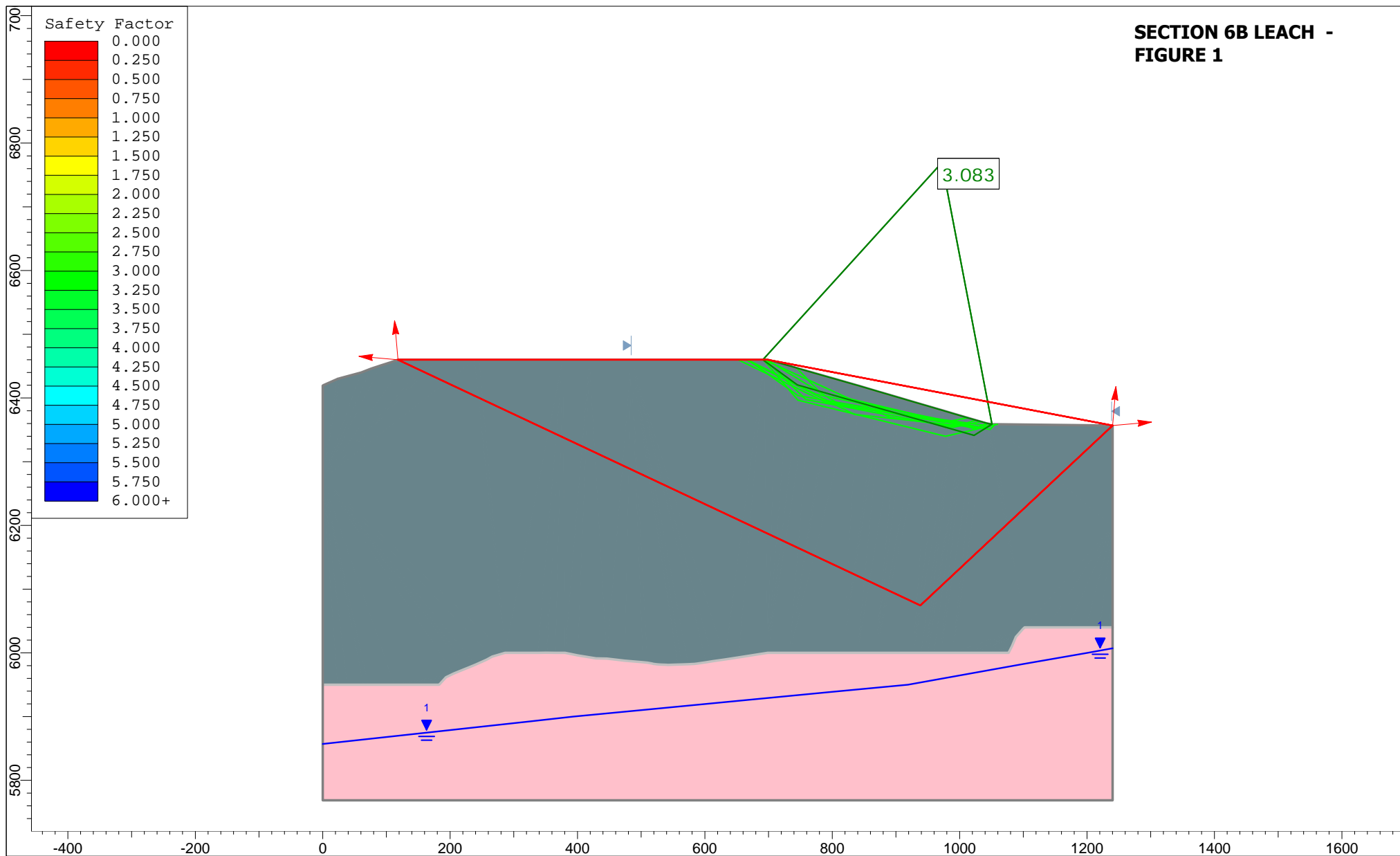
Project				Tyrone Mine Closure Stockpile Stability - SECTION 5A-2 WASTE	
Analysis Description				Seismic - Liquefied Qa - Circular Failure (GLE / Morgenstern-Price)	
Figure	Scale	1:5772	Company	Golder Associates	
Date	1/2/2019		File Name	5B.slmd	



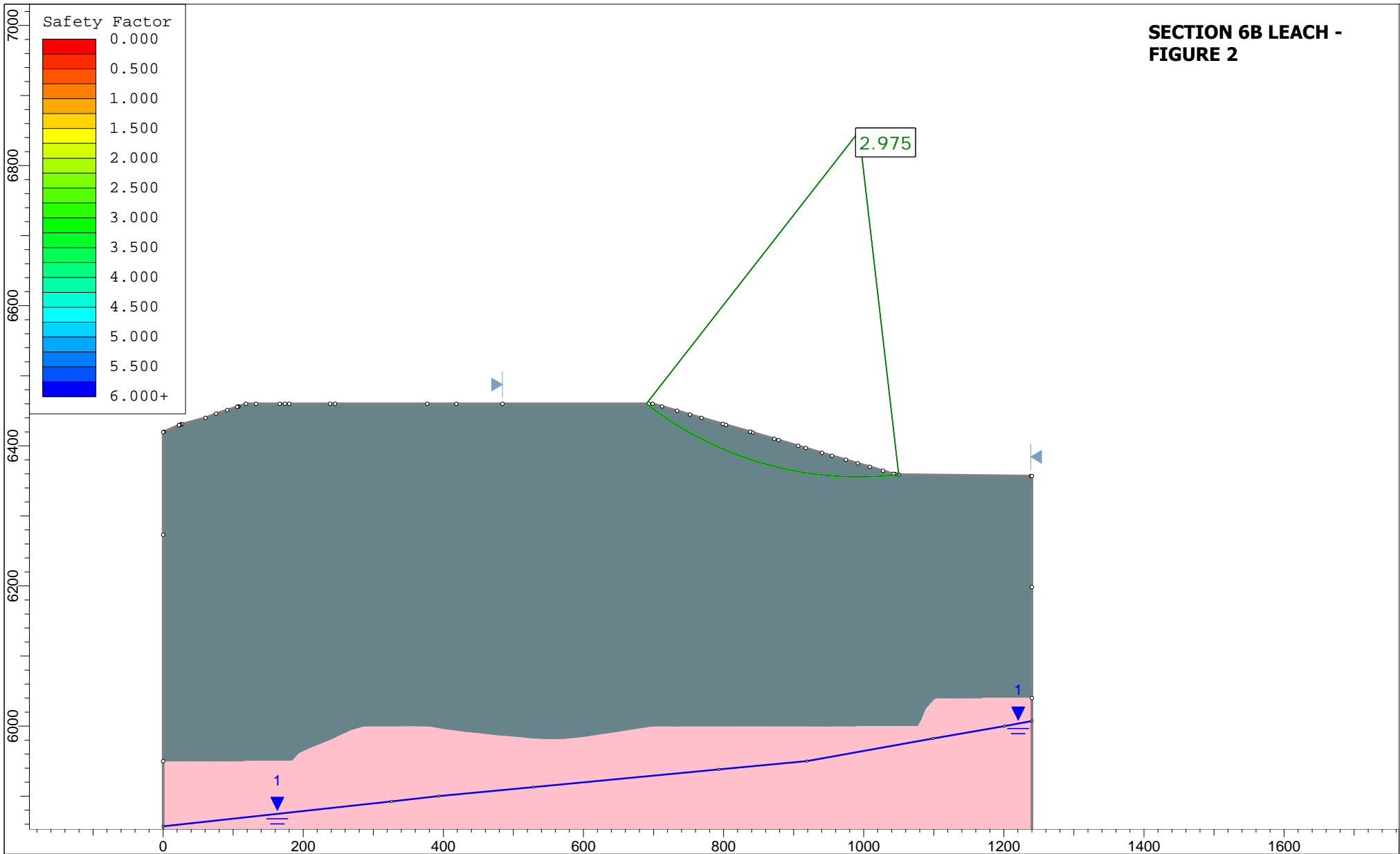
Project			
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Analysis Description			
Seismic - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	5B.slmd		



Project				Tyrone Mine Closure Stockpile Stability - SECTION 5A-2 WASTE	
Analysis Description				Seismic - Circular Failure (GLE / Morgenstern-Price)	
Figure	Scale	1:5772	Company	Golder Associates	
Date	1/2/2019		File Name	5B.slmd	



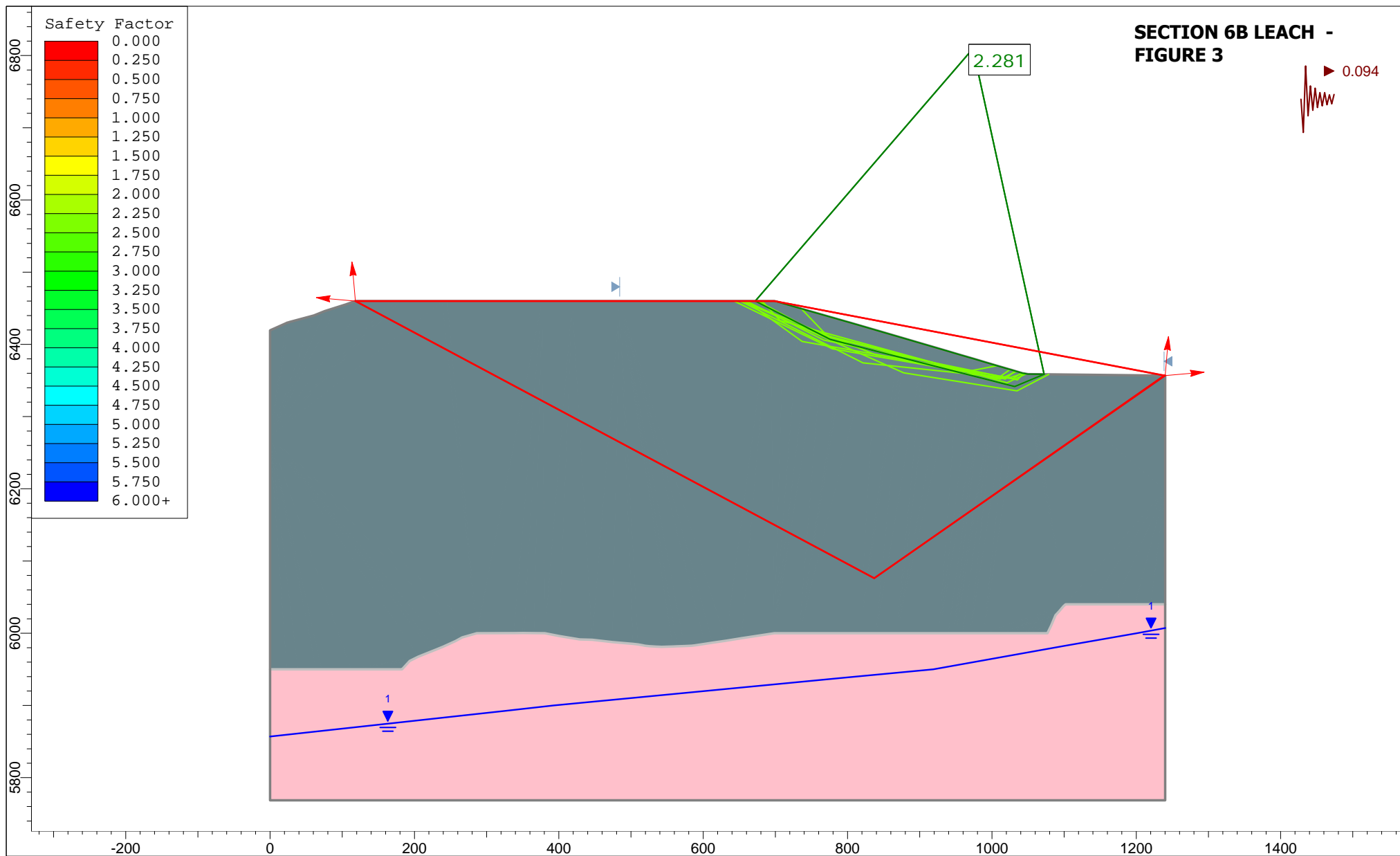
Project			
Tyrone Mine Closure Stockpile Stability - SECTION 6B LEACH			
Analysis Description			
Static - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	6B.slmd		



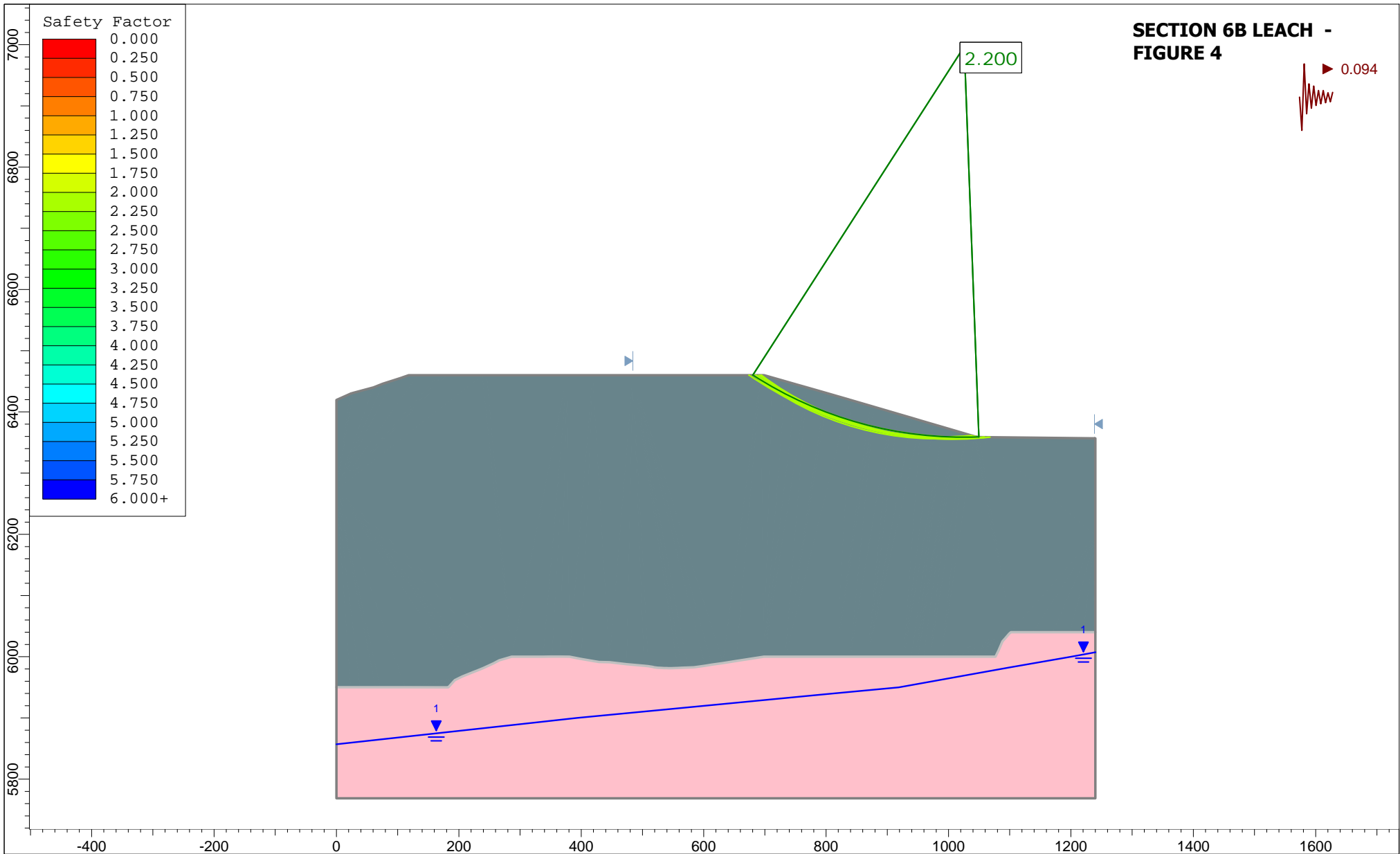
**SECTION 6B LEACH -
FIGURE 2**



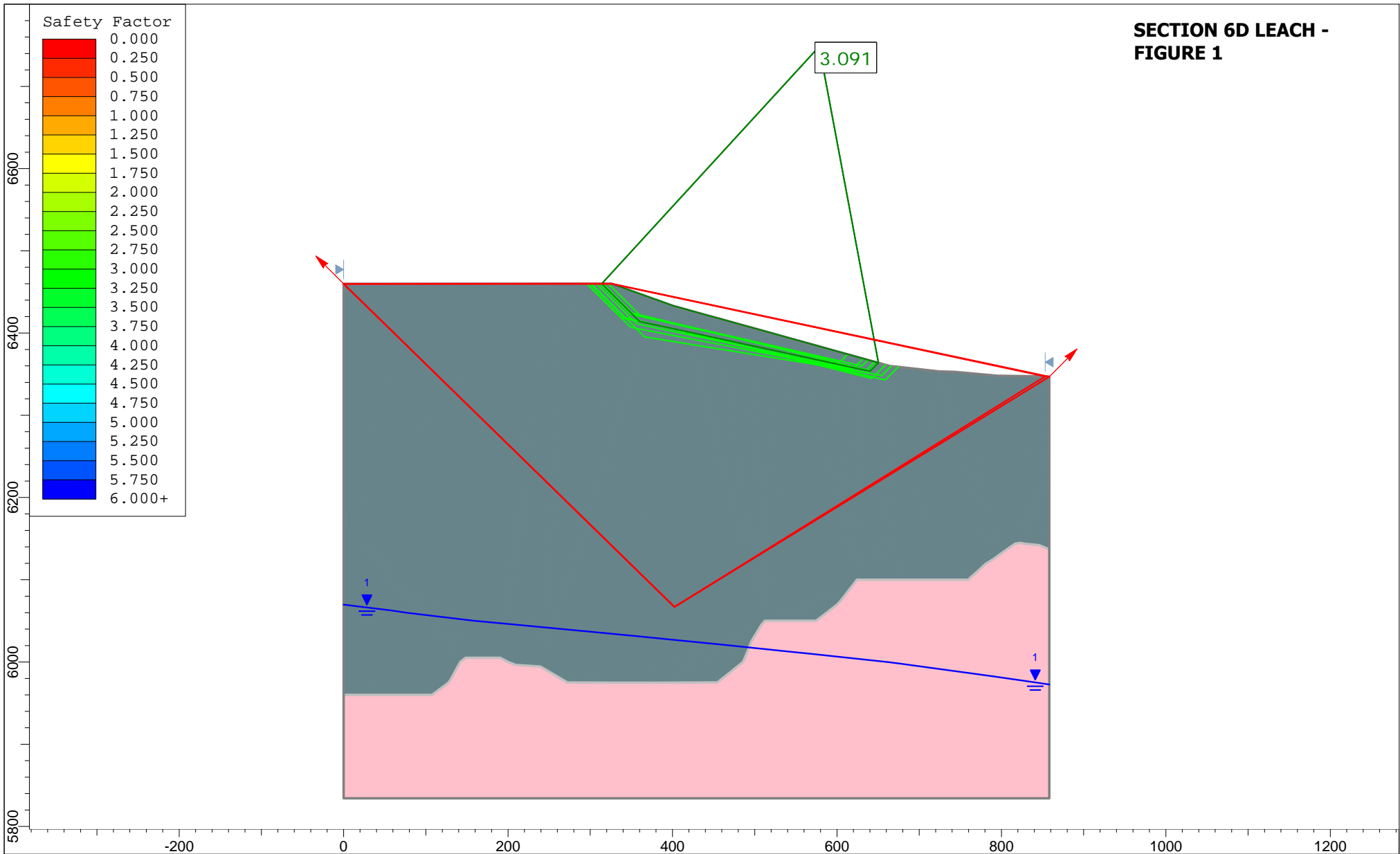
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Analysis Description		
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Figure	Scale	Company
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Date	File Name	
	6B.slmd	



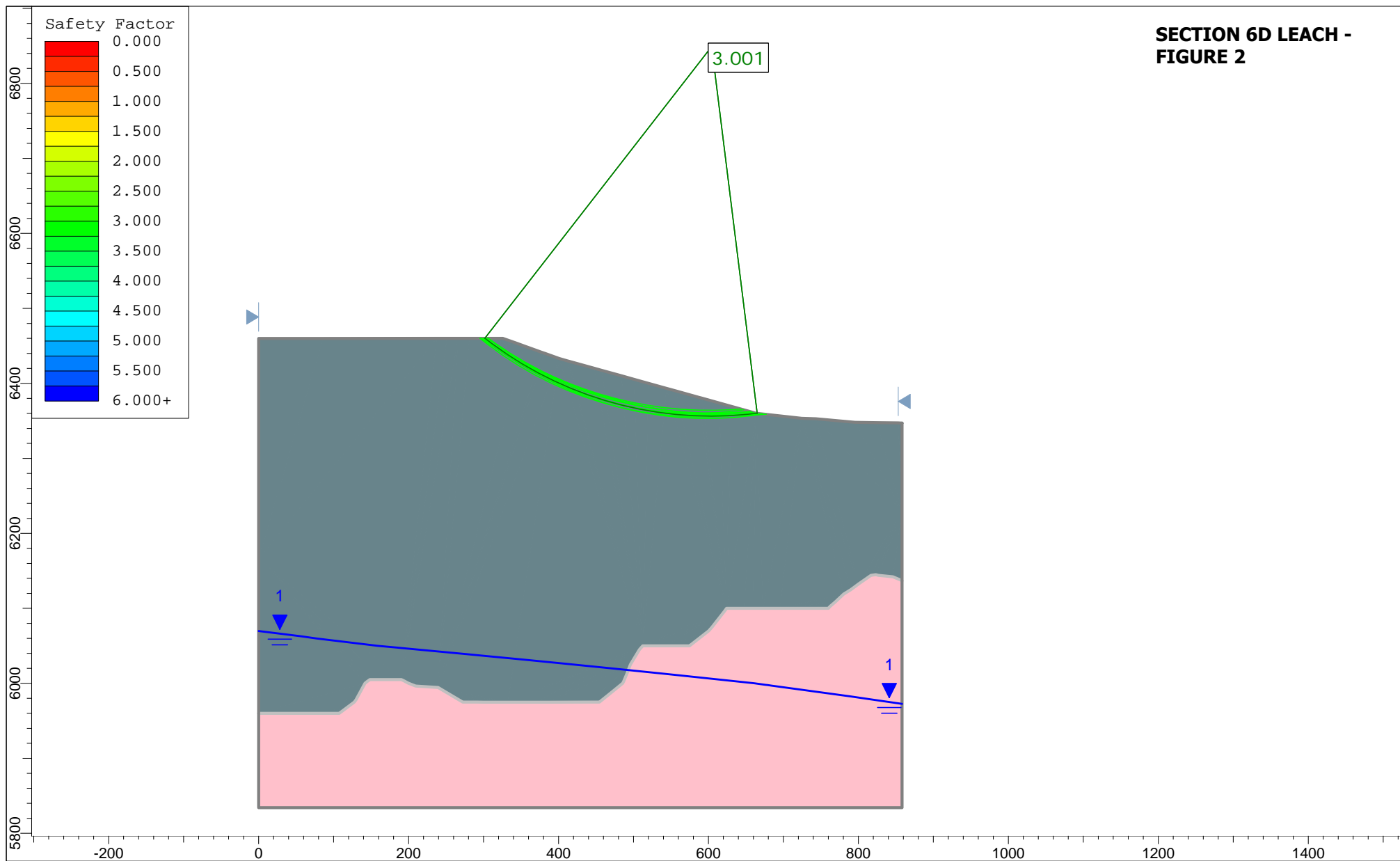
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Tyrone Mine Closure Stockpile Stability - SECTION 6B LEACH			
Analysis Description			
Seismic - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	6B.slmd		



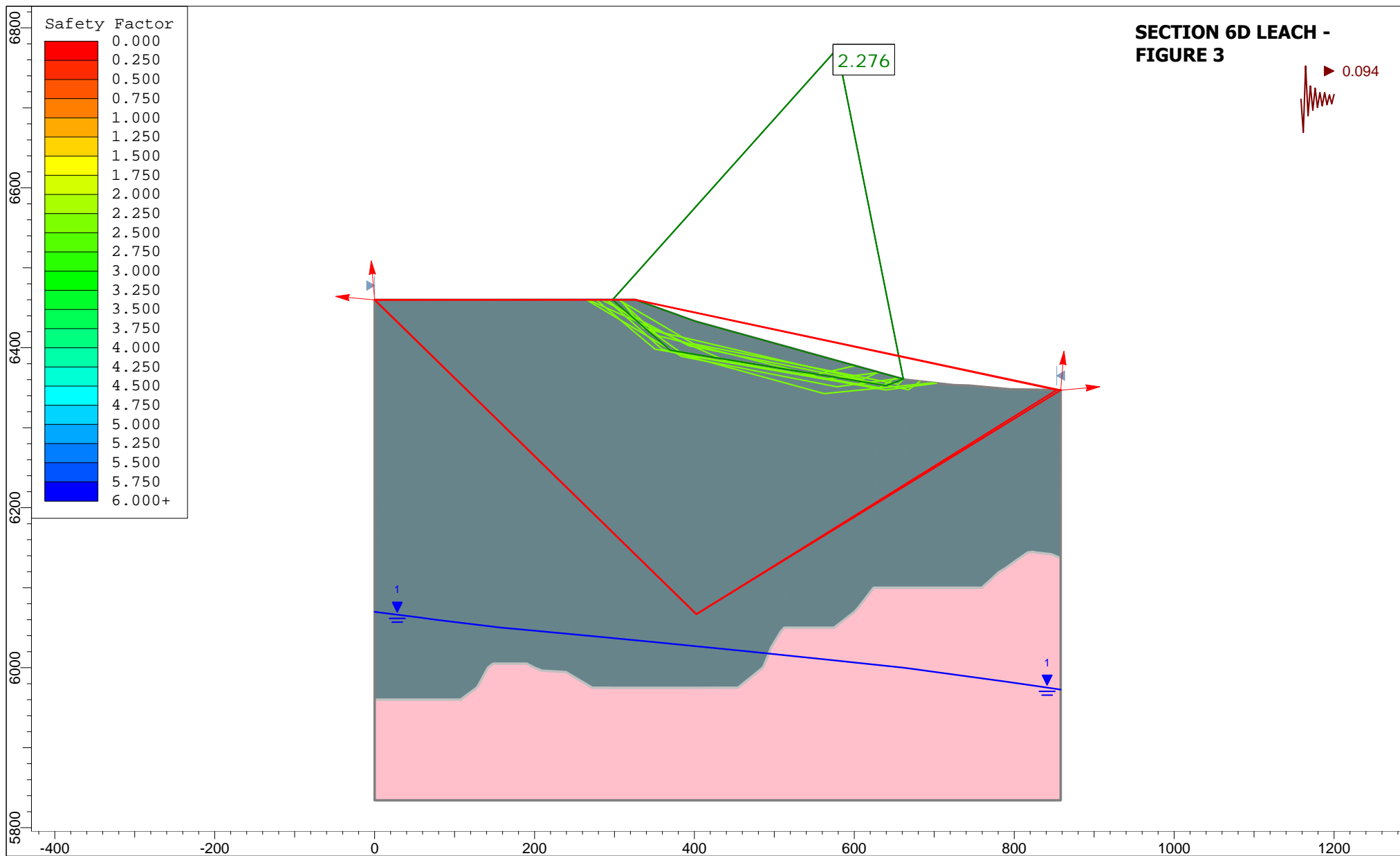
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Analysis Description			
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Figure	Scale	Company	
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Date	1/2/2019		File Name
			6B.slmd



Project			
Tyrone Mine Closure Stockpile Stability - SECTION 6D LEACH			
Analysis Description			
Static - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	6C.slmd		

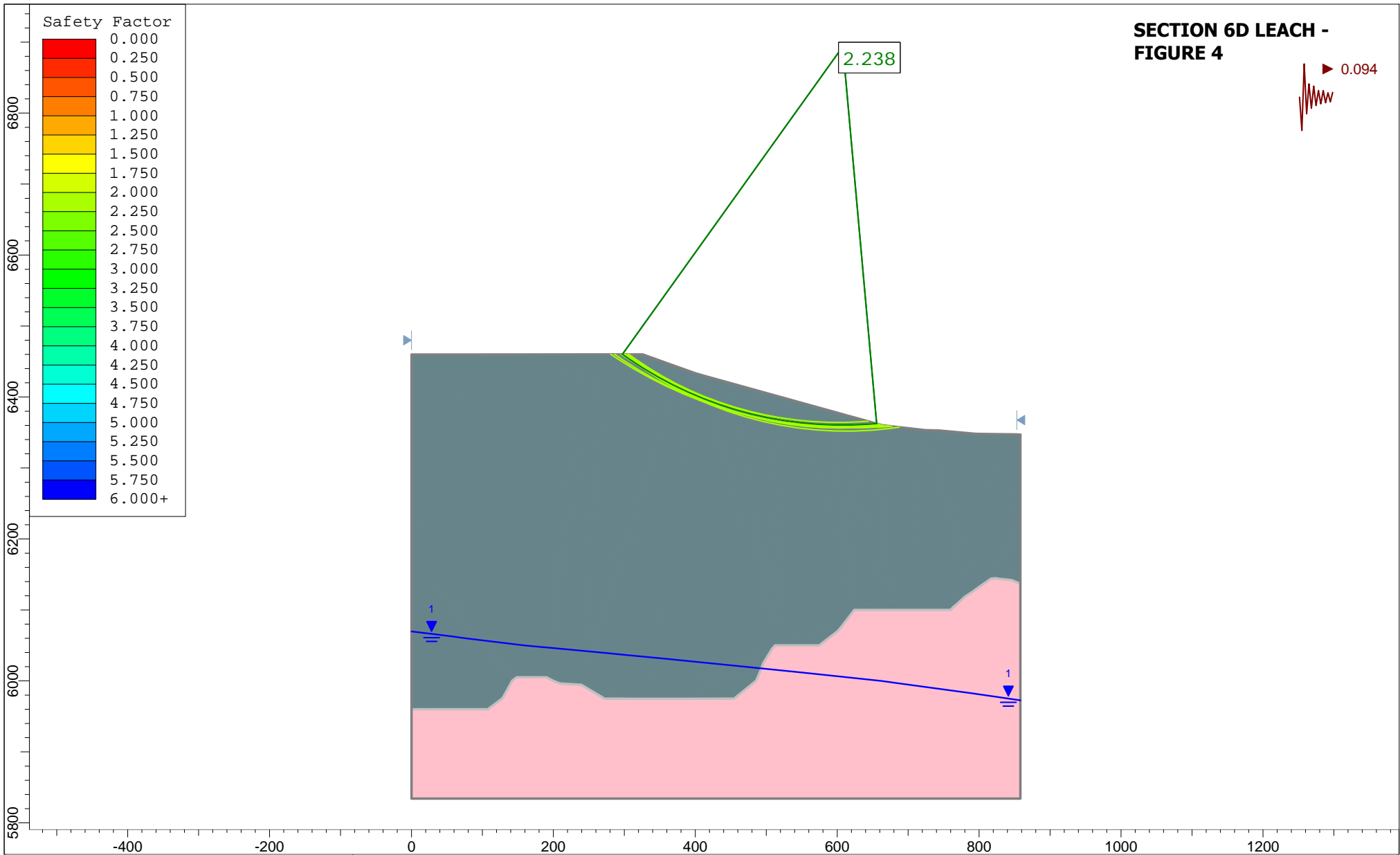


Project		
Tyrone Mine Closure Stockpile Stability - SECTION 6D LEACH		
Analysis Description		
Static - Circular Failure (GLE / Morgenstern-Price)		
Figure	Scale	Company
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Date	File Name	
	6C.slmd	



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Project			
Tyrone Mine Closure Stockpile Stability - SECTION 6D LEACH			
Analysis Description			
Seismic - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
	1:1993	Golder Associates	
Date	File Name		
1/2/2019	6C.slmd		



GOLDER

Project

Tyrone Mine Closure Stockpile Stability - Section 6D Leach

Analysis Description

Seismic - Circular Failure (GLE / Morgenstern-Price)

Figure

Scale

1:2247

Company

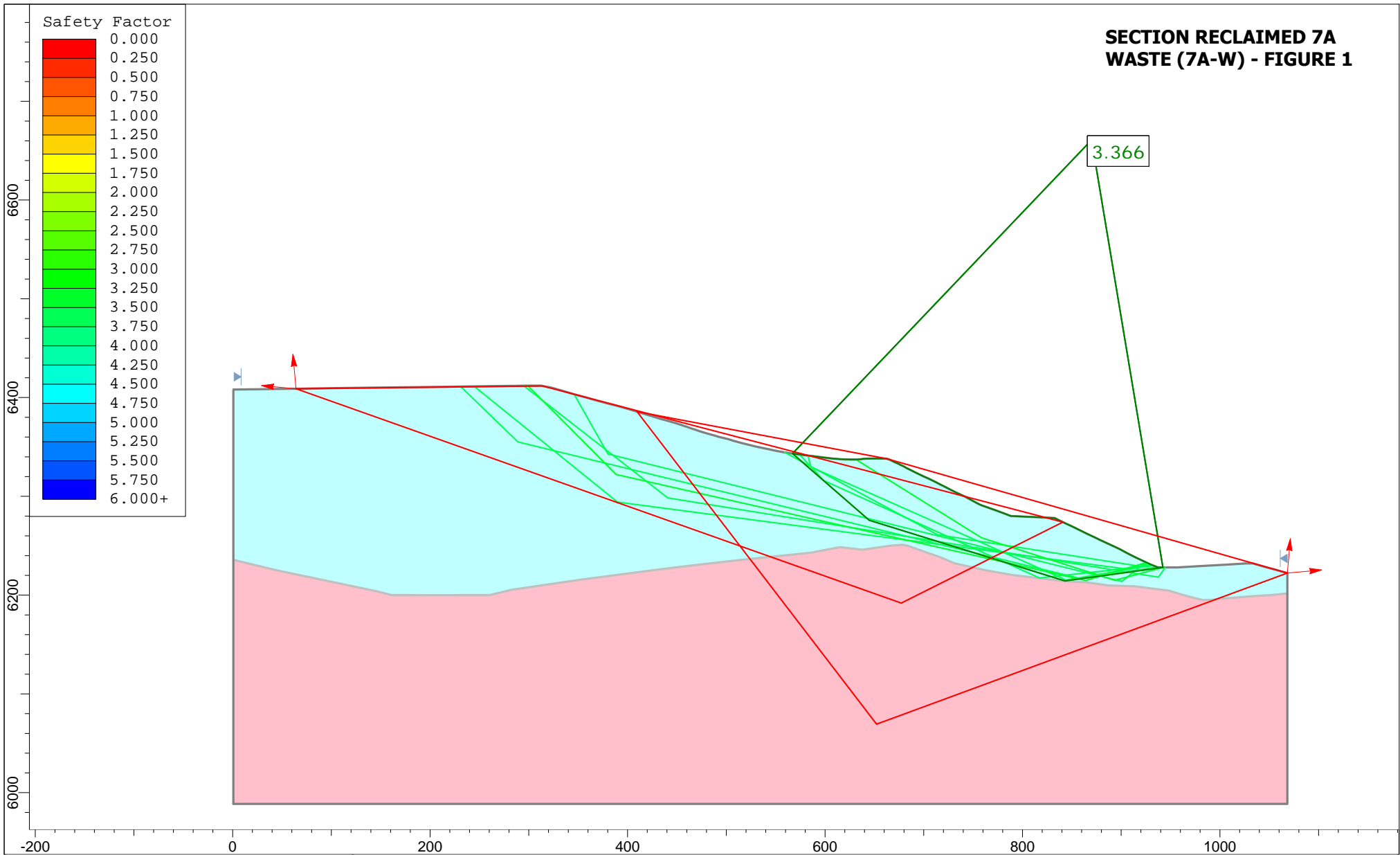
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Date

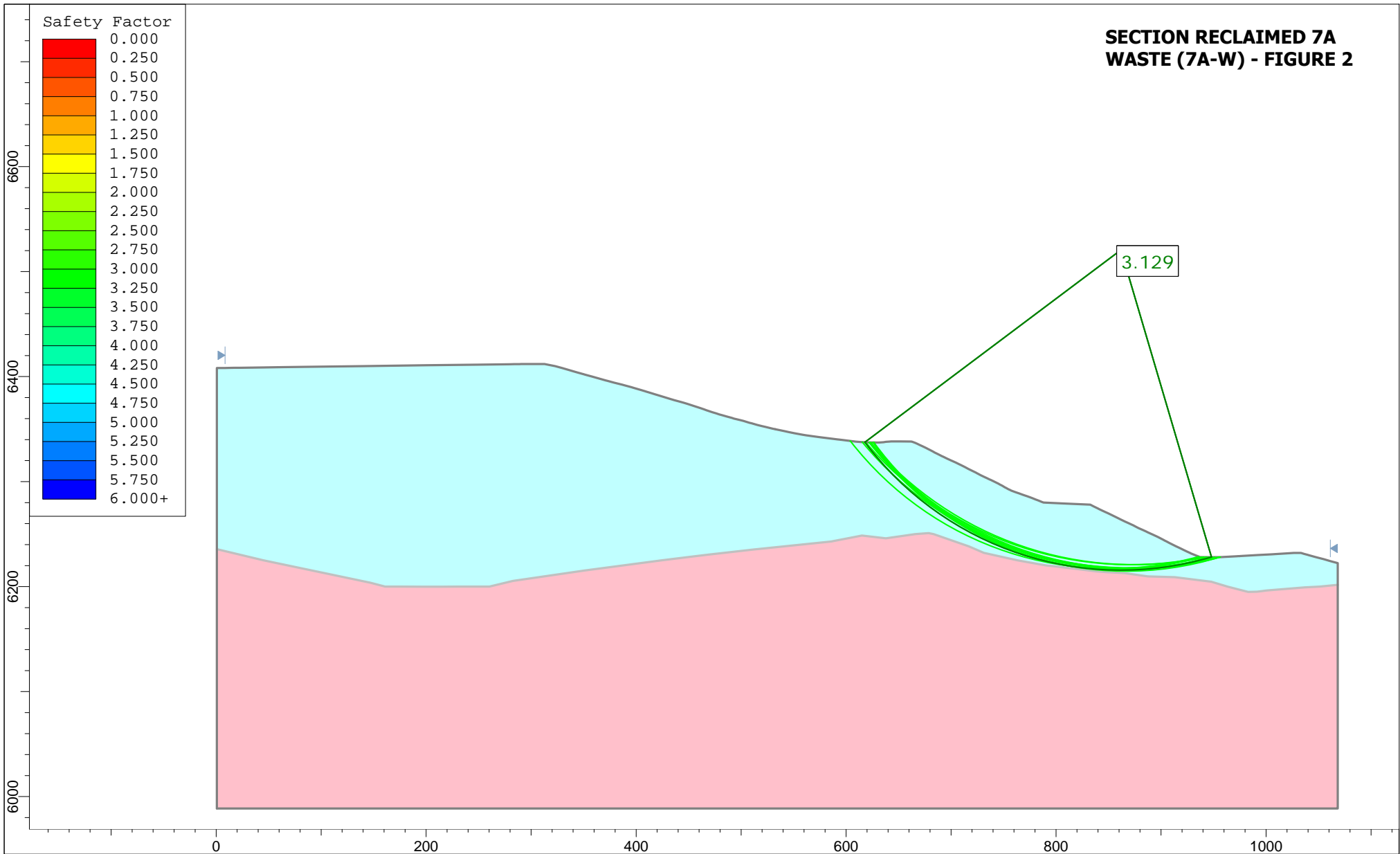
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File Name

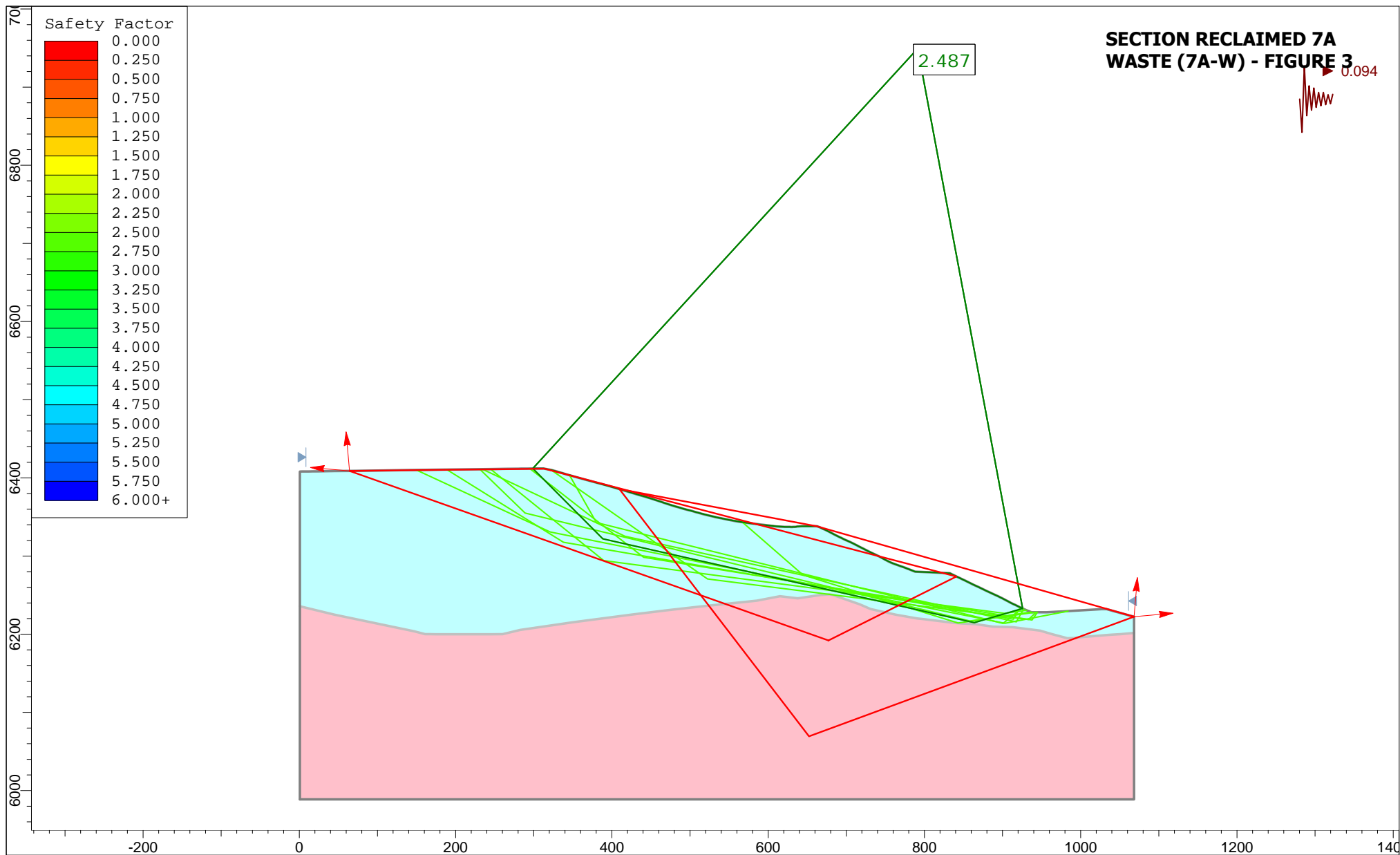
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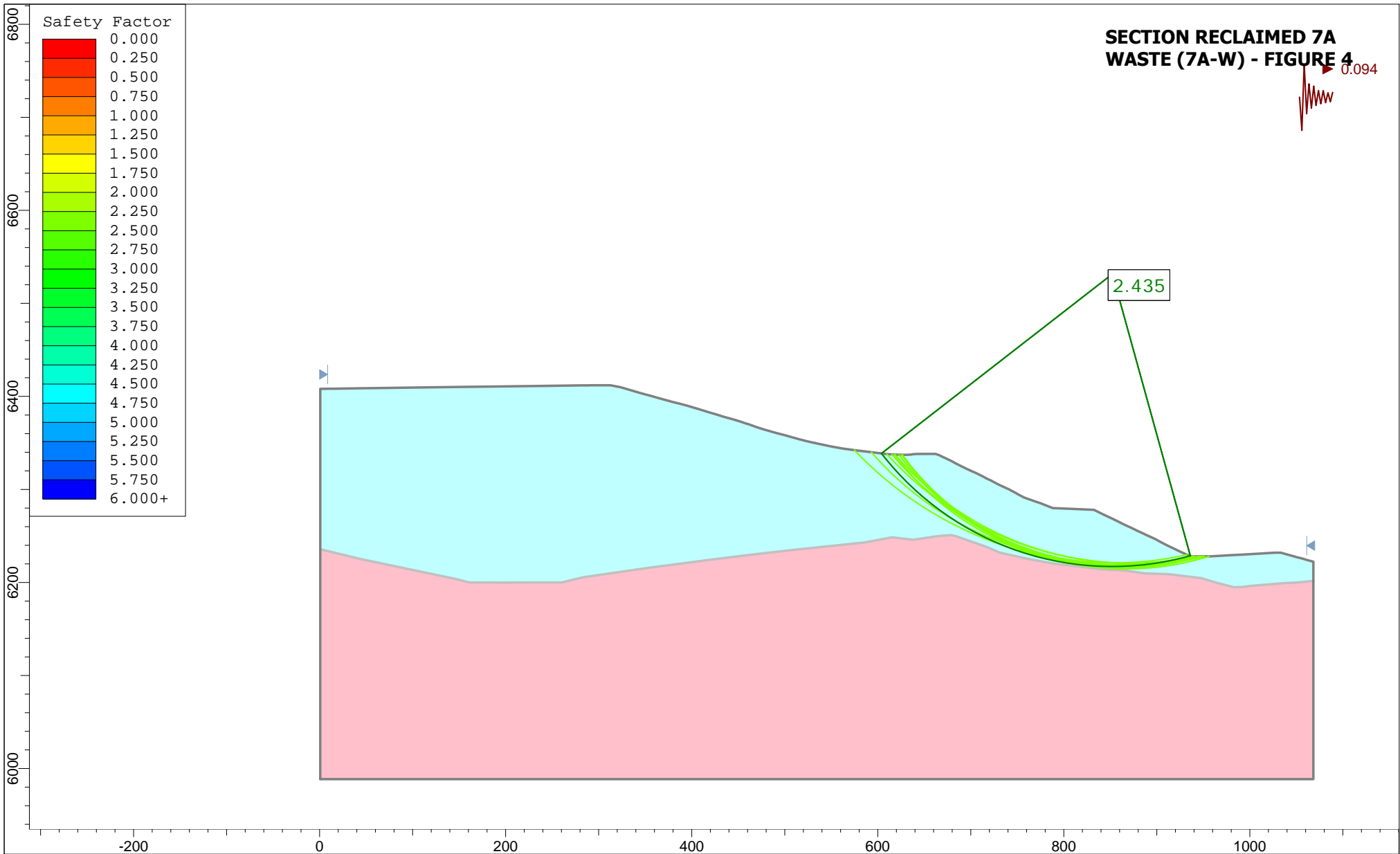
Project			
Tyrone Mine Closure Stockpile Stability - SECTION RECLAIMED 7A WASTE (7A-W)			
Analysis Description			
Static - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	7A west (2).slmd		



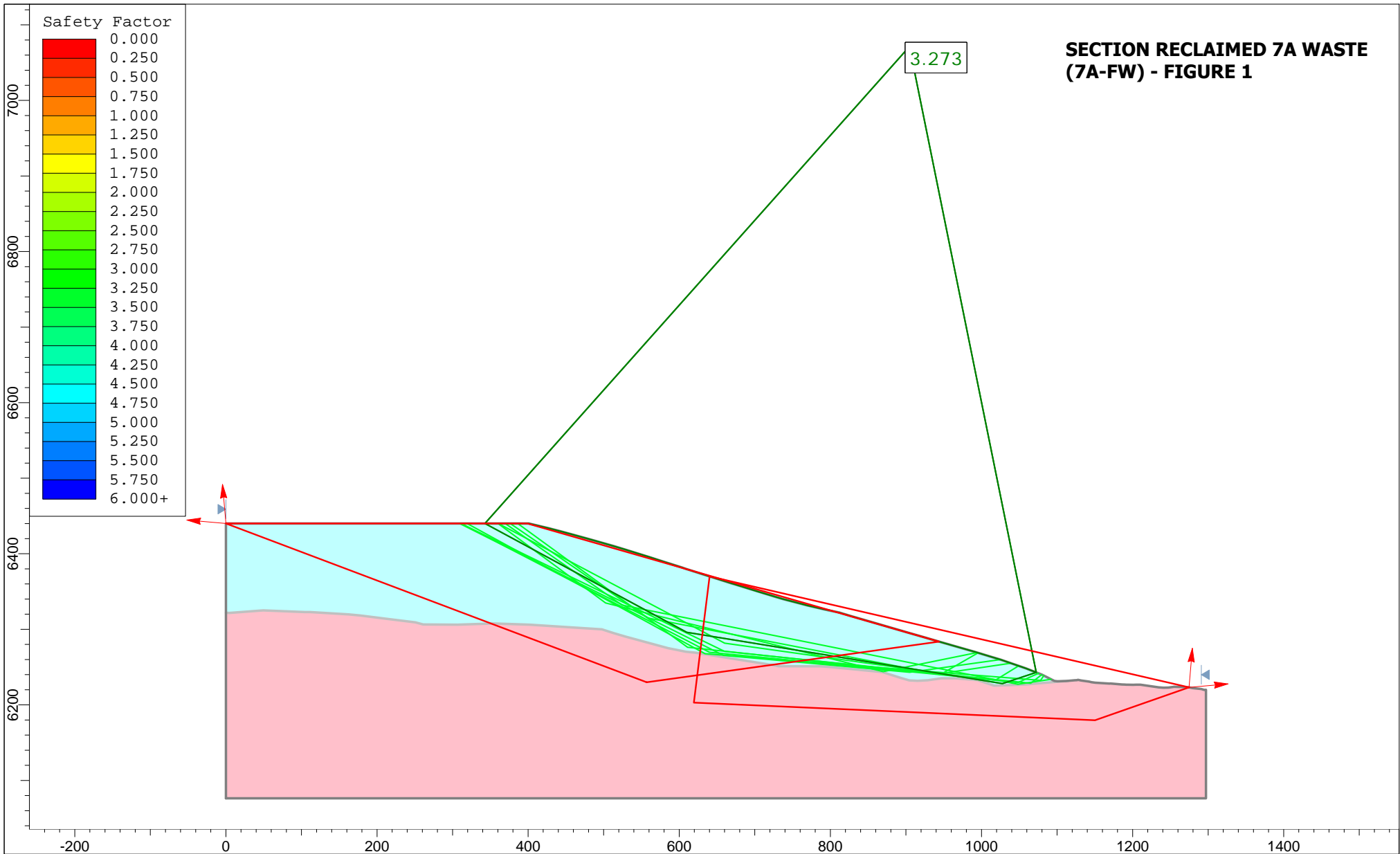
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Tyrone Mine Closure Stockpile Stability - SECTION RECLAIMED 7A WASTE (7A-W)			
Analysis Description			
Static - Circular Failure (GLE / Morgenstern-Price)			
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Date	File Name		
1/2/2019	7A west (2).slmd		



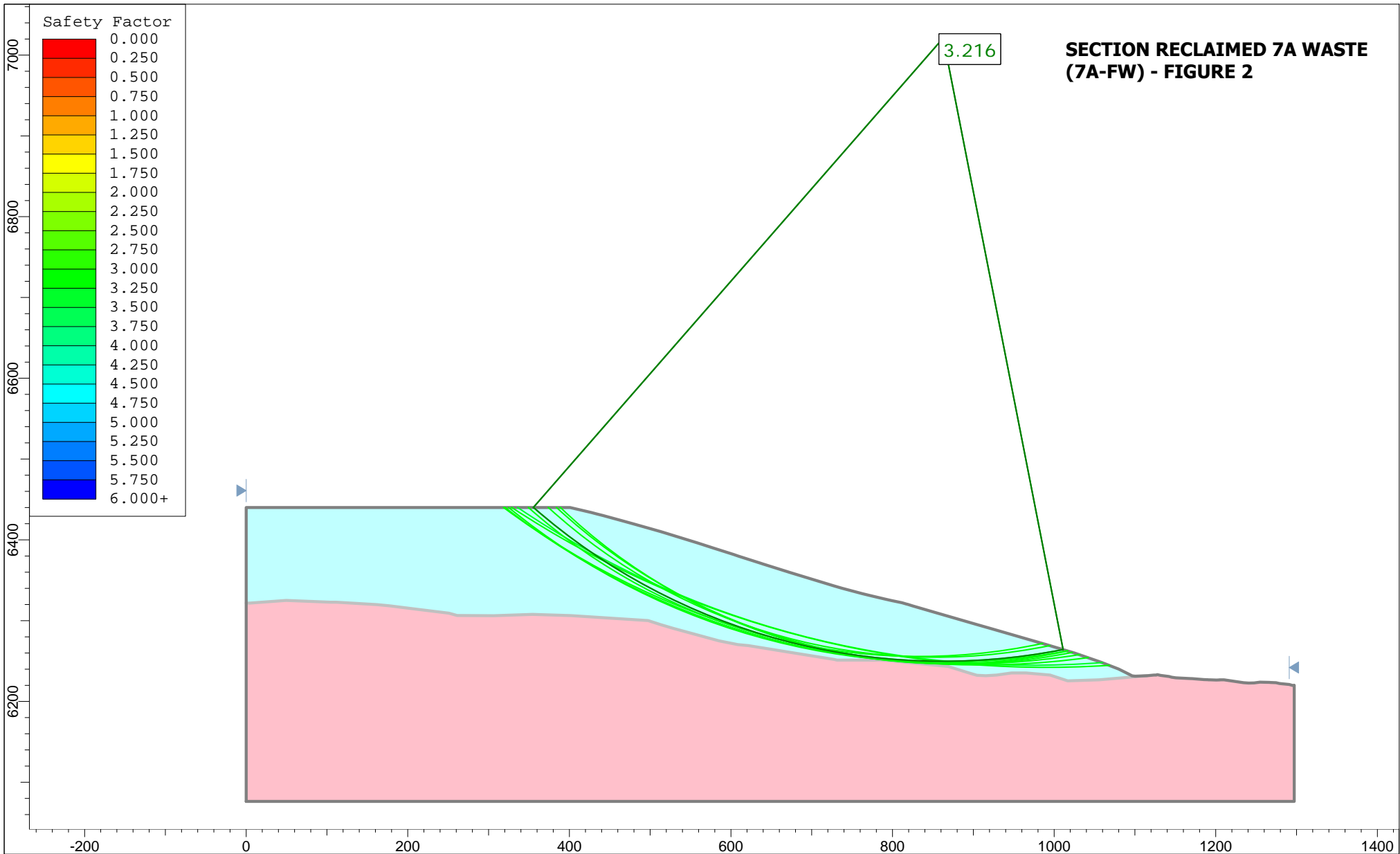
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Tyrone Mine Closure Stockpile Stability - SECTION RECLAIMED 7A WASTE (7A-W)			
Analysis Description			
Seismic - Block Failure (GLE / Morgenstern-Price)			
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Date	File Name		
1/2/2019	7A west (2).slmd		



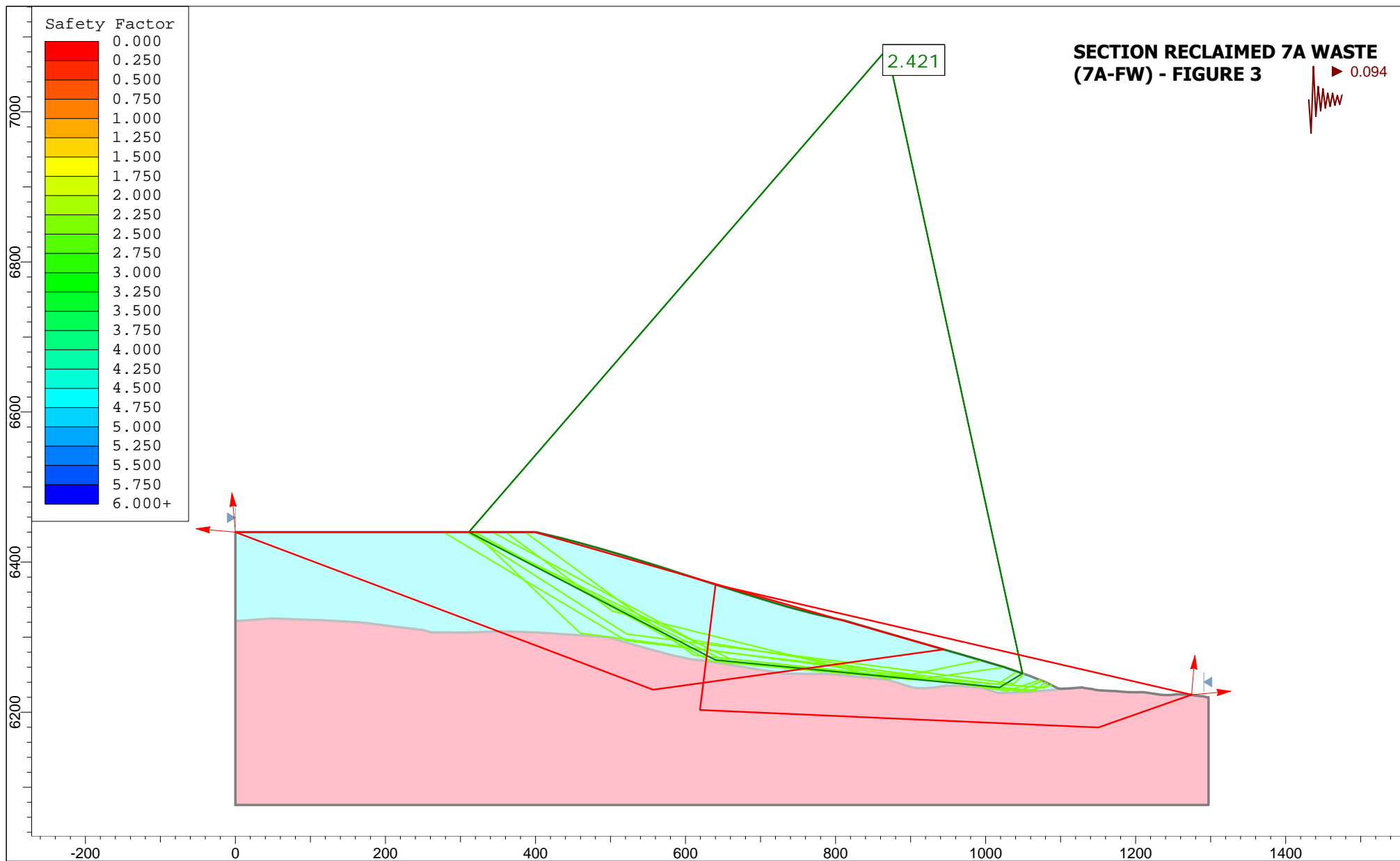
Project			
Tyrone Mine Closure Stockpile Stability - SECTION RECLAIMED 7A WASTE (7A-W)			
Analysis Description			
Seismic - Circular Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	7A west (2).slmd		



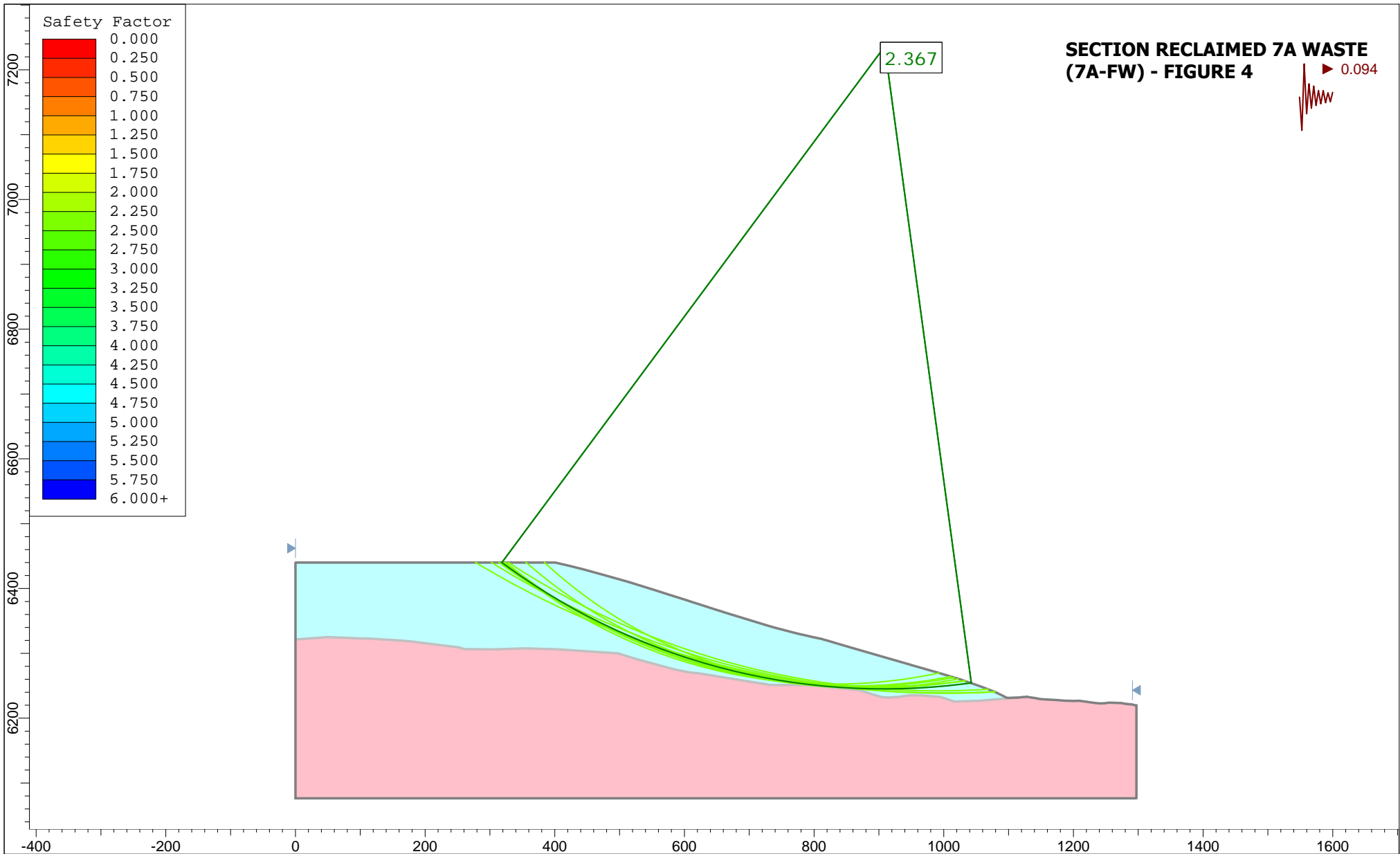
Project			
Tyrone Mine Closure Stockpile Stability - SECTION RECLAIMED 7A WASTE (7A-FW)			
Analysis Description			
Static - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
	1:2110	Golder Associates	
Date	File Name		
1/2/2019	7A-FW.slmd		



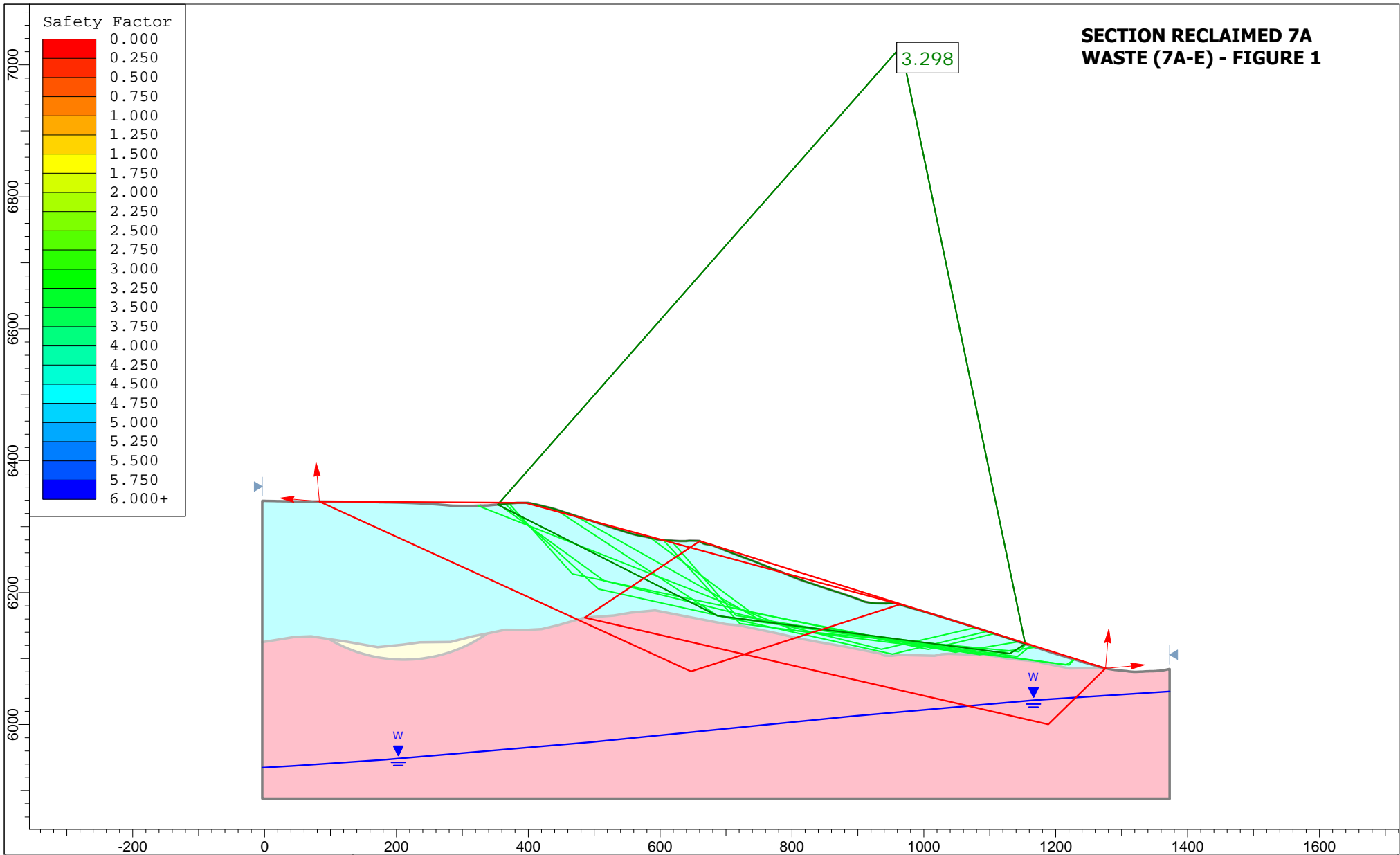
Project				Tyrone Mine Closure Stockpile Stability - SECTION RECLAIMED 7A WASTE (7A-FW)	
Analysis Description				Static - Circular Failure (GLE / Morgenstern-Price)	
Figure	Scale	1:1973	Company	Golder Associates	
Date				File Name	7A-FW.slmd



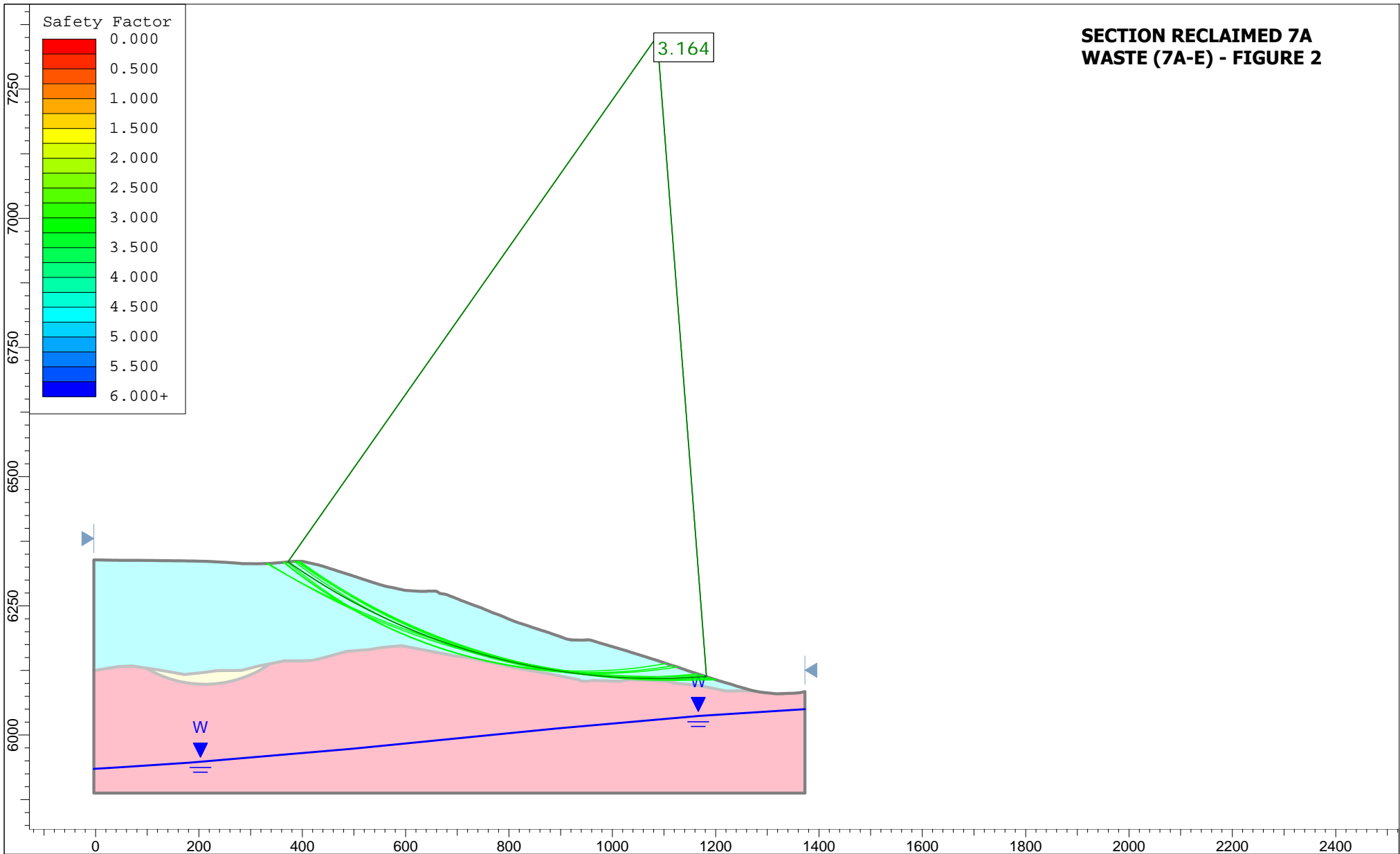
Project			
Tyrone Mine Closure Stockpile Stability - SECTION RECLAIMED 7A WASTE (7A-FW)			
Analysis Description			
Seismic - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
	1:2137	Golder Associates	
Date		File Name	
1/2/2019		7A-FW.slmd	



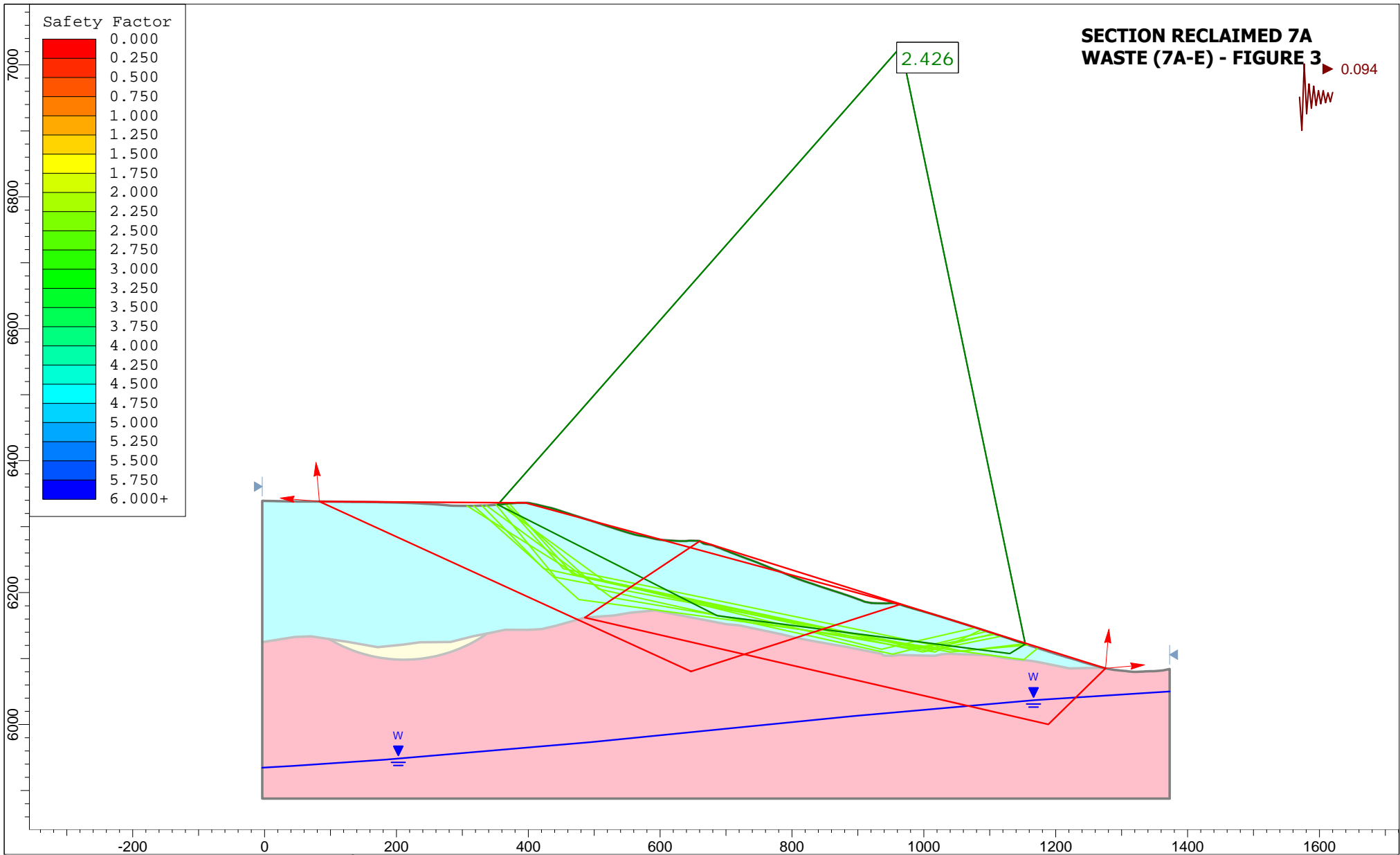
Project			Tyrone Mine Closure Stockpile Stability - SECTION RECLAIMED 7A WASTE (7A-FW)	
Analysis Description			Seismic - Circular Failure (GLE / Morgenstern-Price)	
Figure	Scale	1:2459	Company	Golder Associates
Date	1/2/2019		File Name	7A-FW.slmd



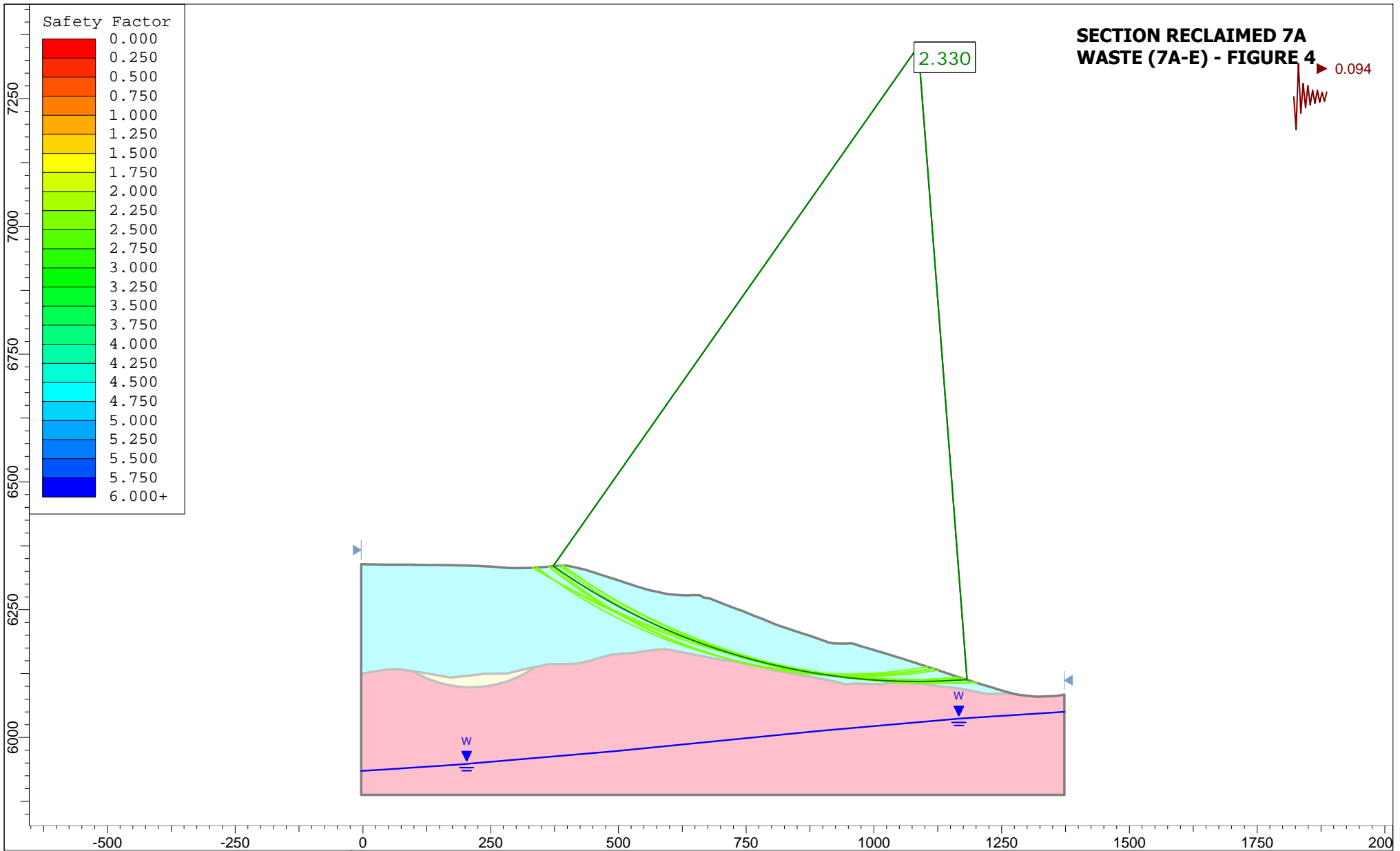
Project			
Tyrone Mine Closure Stockpile Stability - SECTION RECLAIMED 7A WASTE (7A-E)			
Analysis Description			
Static - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	7A-E(2).slmd		



Project			Tyrone Mine Closure Stockpile Stability - SECTION RECLAIMED 7A WASTE (7A-E)		
Analysis Description			Static - Circular Failure (GLE / Morgenstern-Price)		
Figure	Scale	1:3086	Company	Golder Associates	
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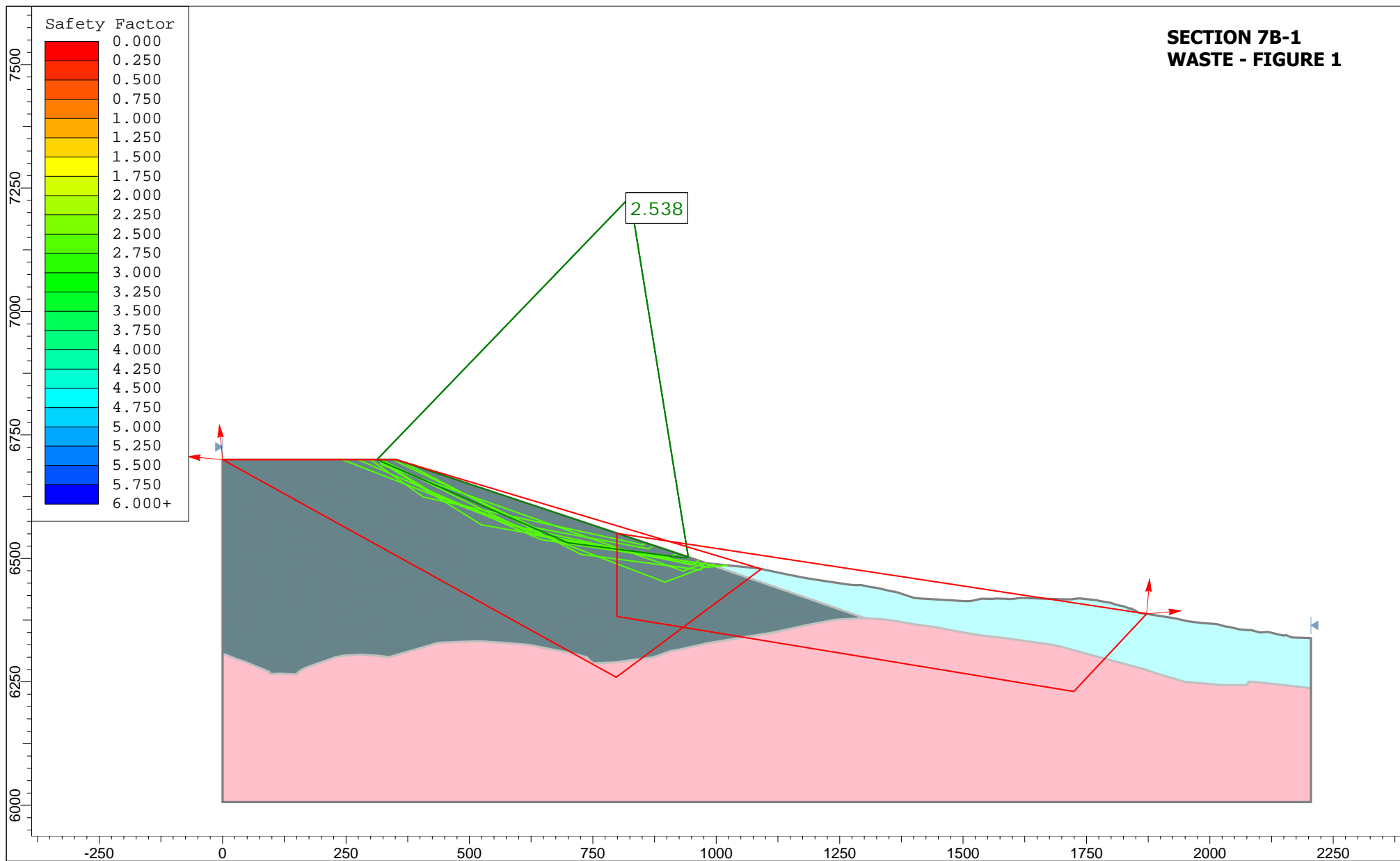


Project			
Tyrone Mine Closure Stockpile Stability - SECTION RECLAIMED 7A WASTE (7A-E)			
Analysis Description			
Seismic - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	7A-E(2).slmd		

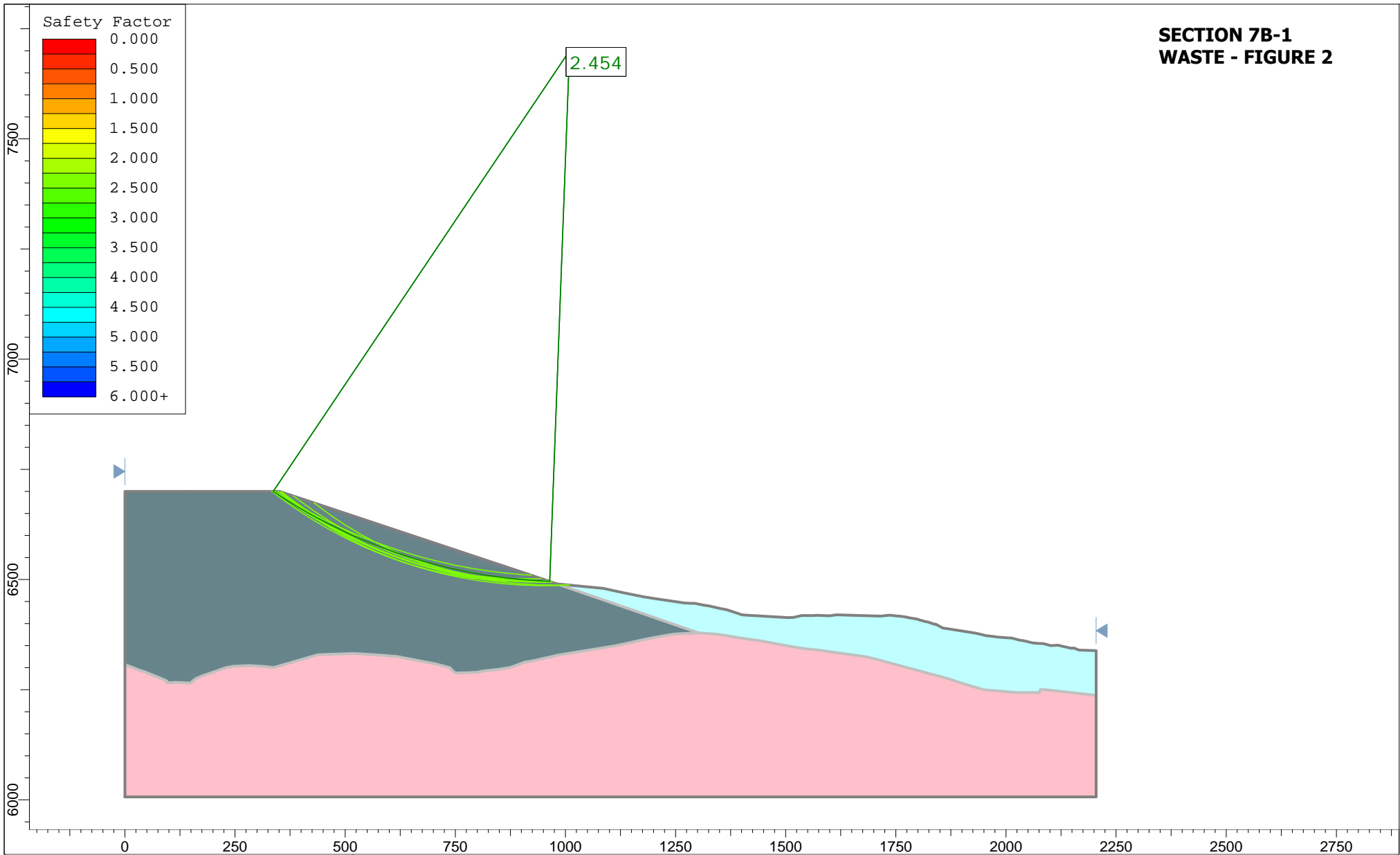


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Tyrone Mine Closure Stockpile Stability - SECTION RECLAIMED 7A WASTE (7A-E)			
Analysis Description			
Seismic - Circular Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
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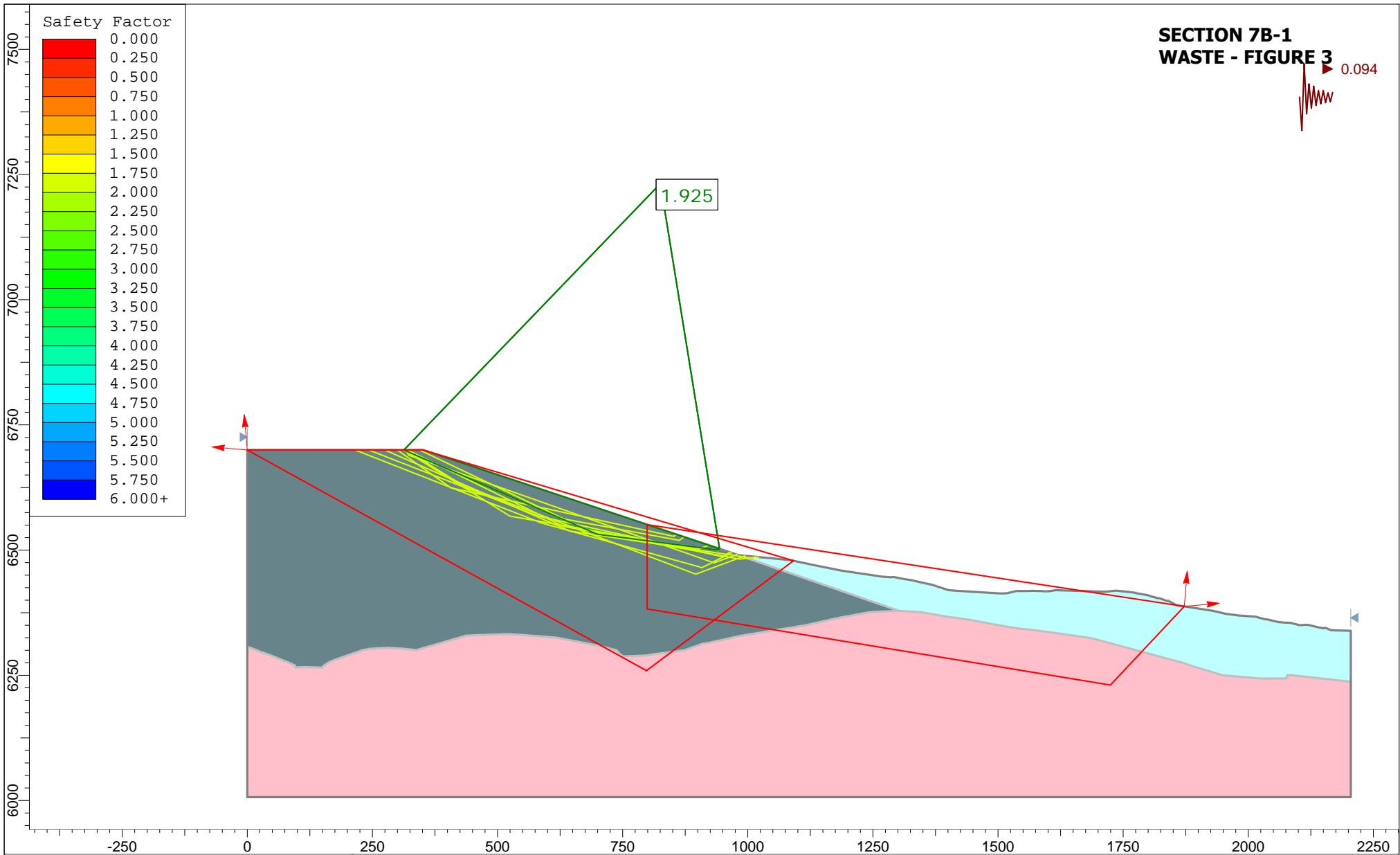
**SECTION 7B-1
WASTE - FIGURE 1**



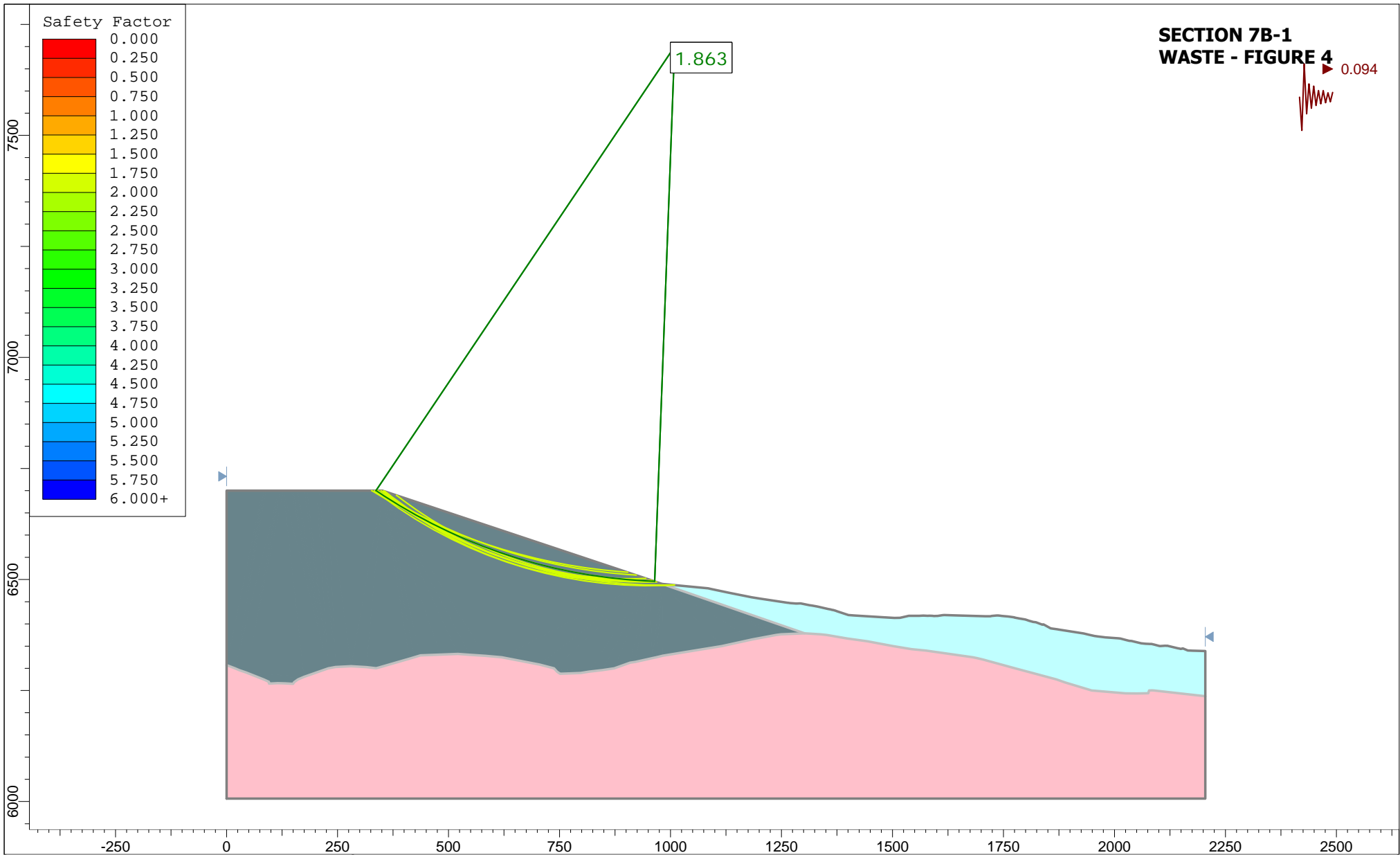
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Analysis Description			
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Figure	Scale	Company	
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2/4/2019	7B.slmd		



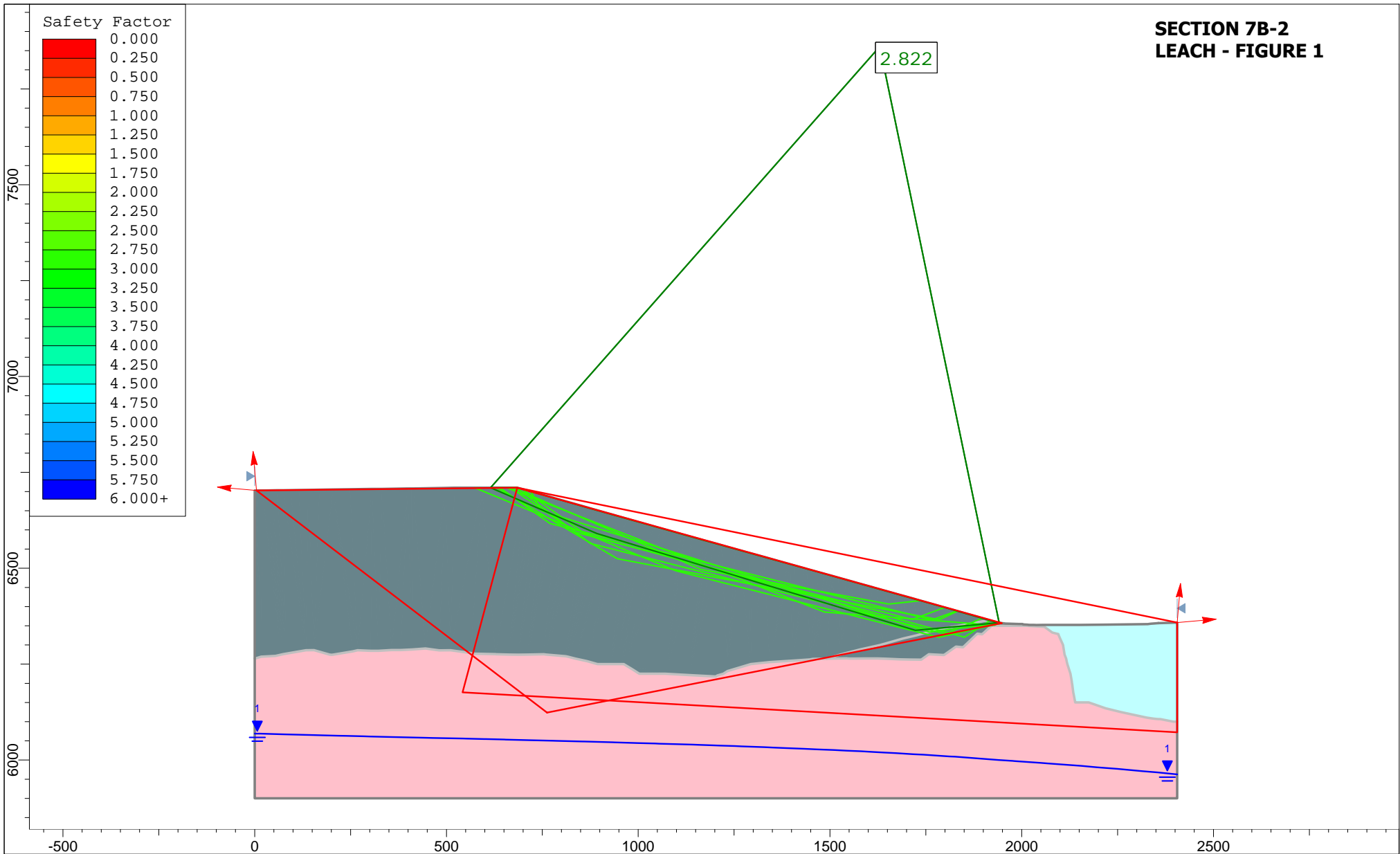
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Analysis Description		
Static - Circular Failure (GLE / Morgenstern-Price)		
Figure	Scale	Company
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Date	File Name	
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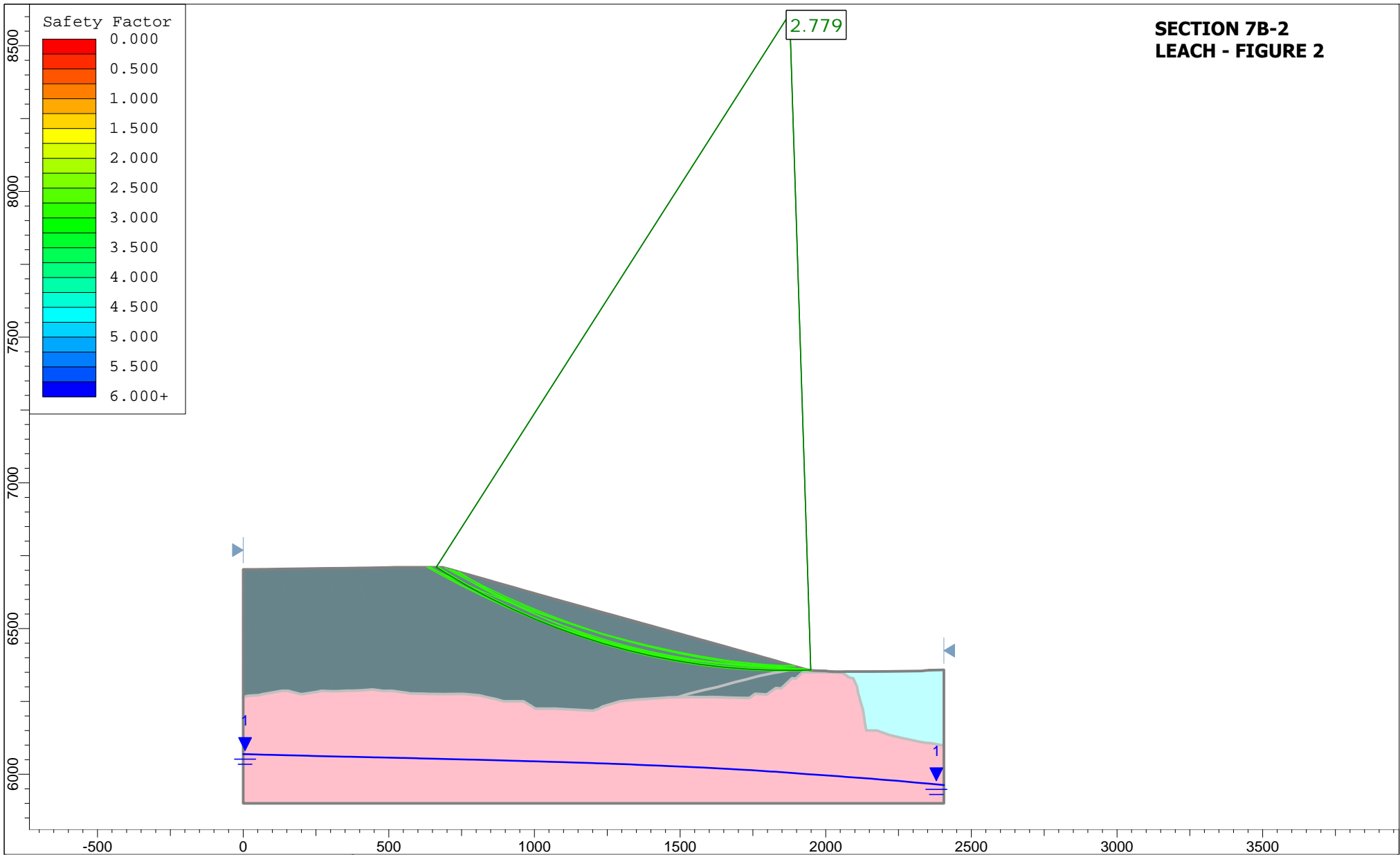
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2/4/2019	7B.slmd		



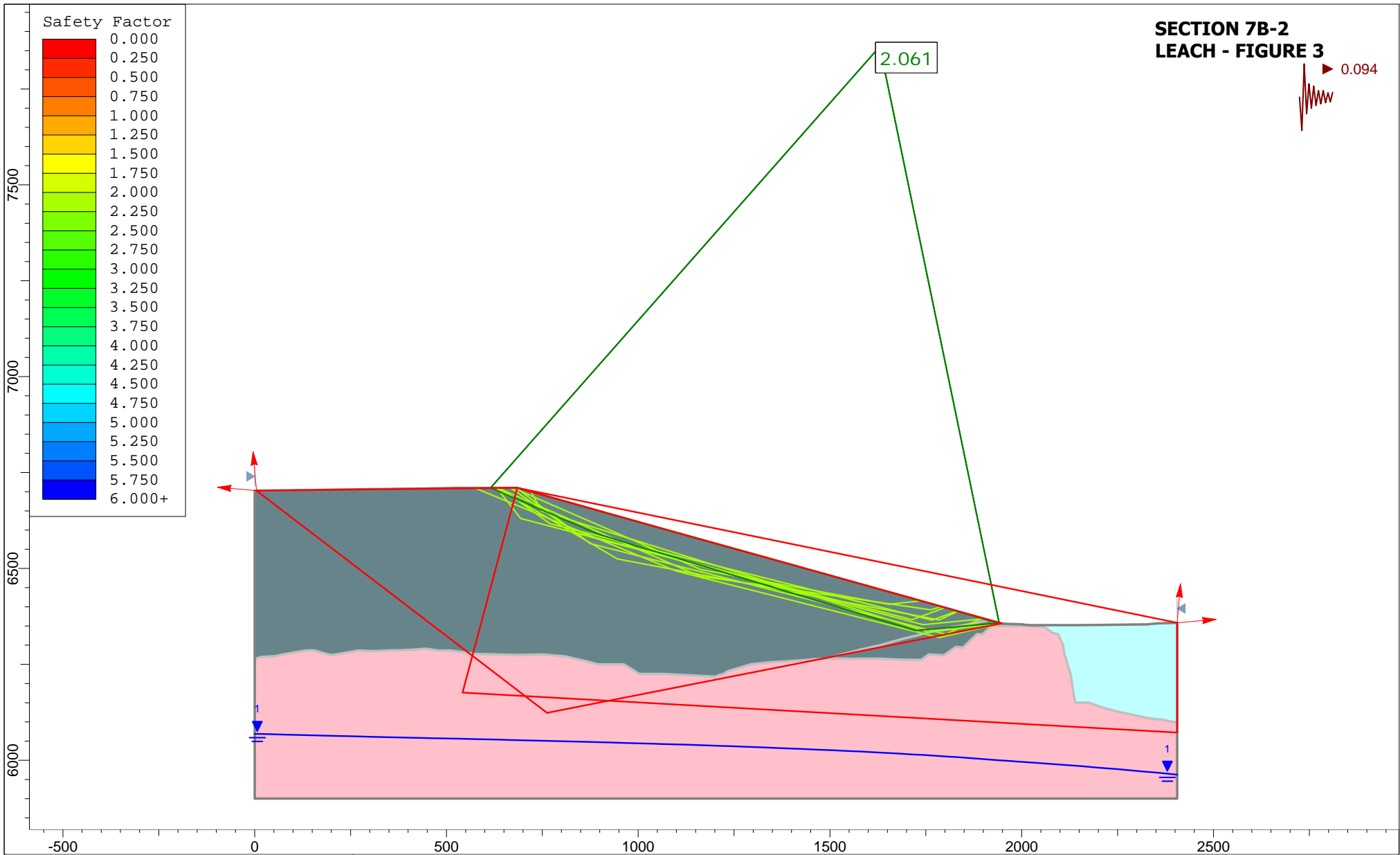
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2/4/2019	7B.slmd		



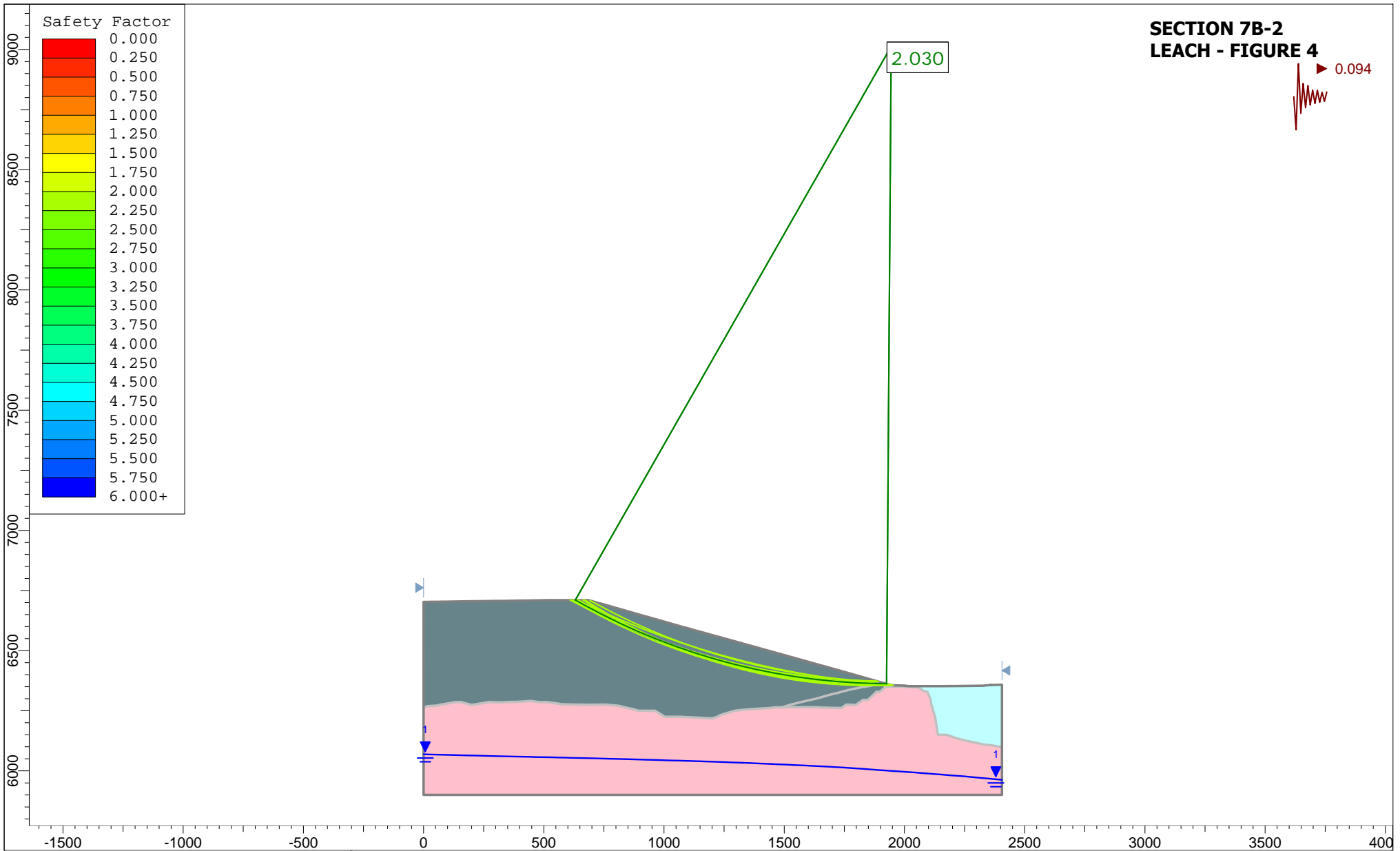
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Analysis Description			
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Figure	Scale	Company	
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Date	File Name		
1/2/2019	2C.slmd		



Project				Tyrone Mine Closure Stockpile Stability - SECTION 7B-2 LEACH	
Analysis Description				Static - Circular Failure (GLE / Morgenstern-Price)	
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Date				File Name	2C.slmd

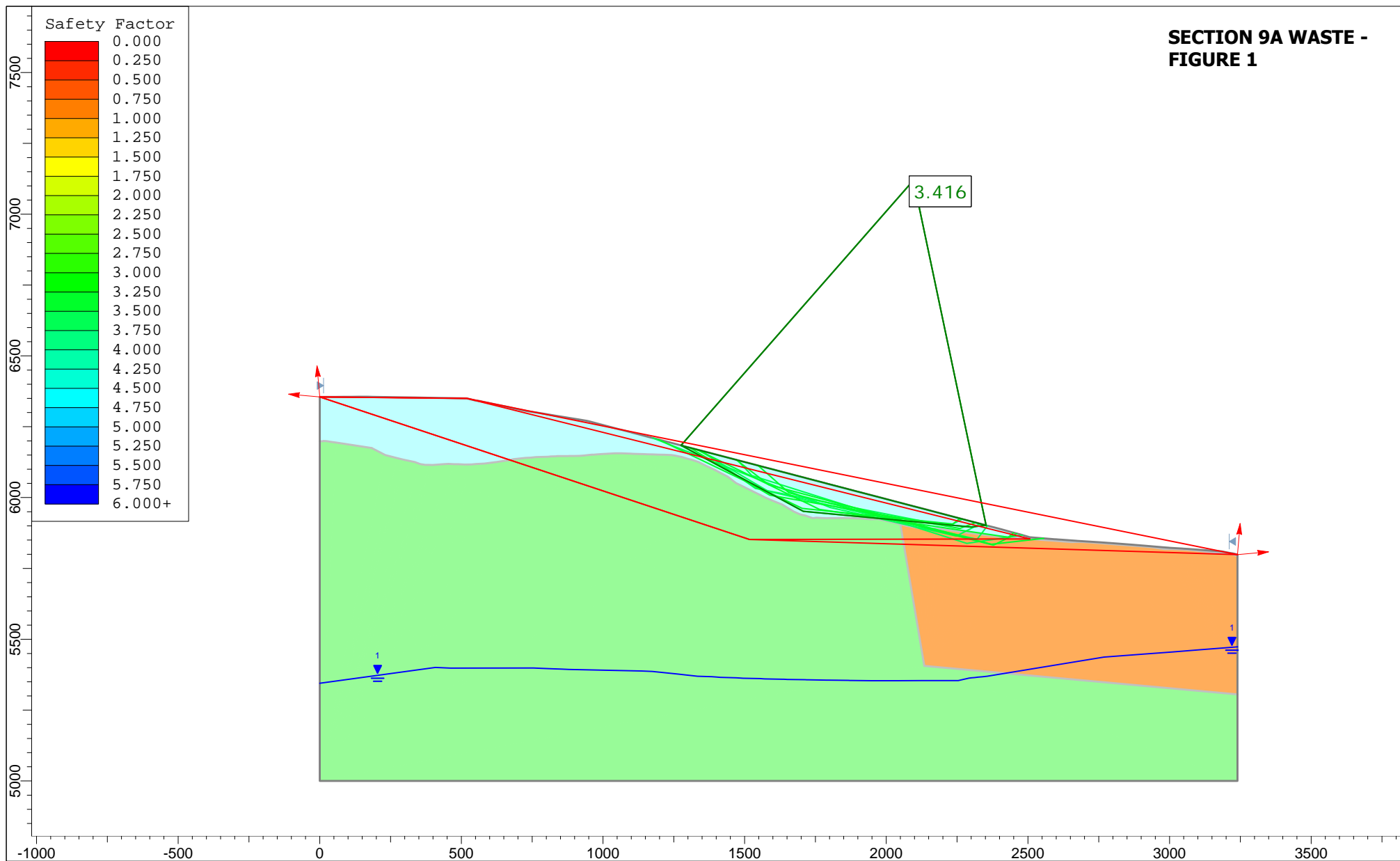


Project			
Tyrone Mine Closure Stockpile Stability - SECTION 7B-2 LEACH			
Analysis Description			
Seismic - Block Failure (GLE / Morgenstern-Price)			
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Date	1/2/2019		File Name
			2C.slmd

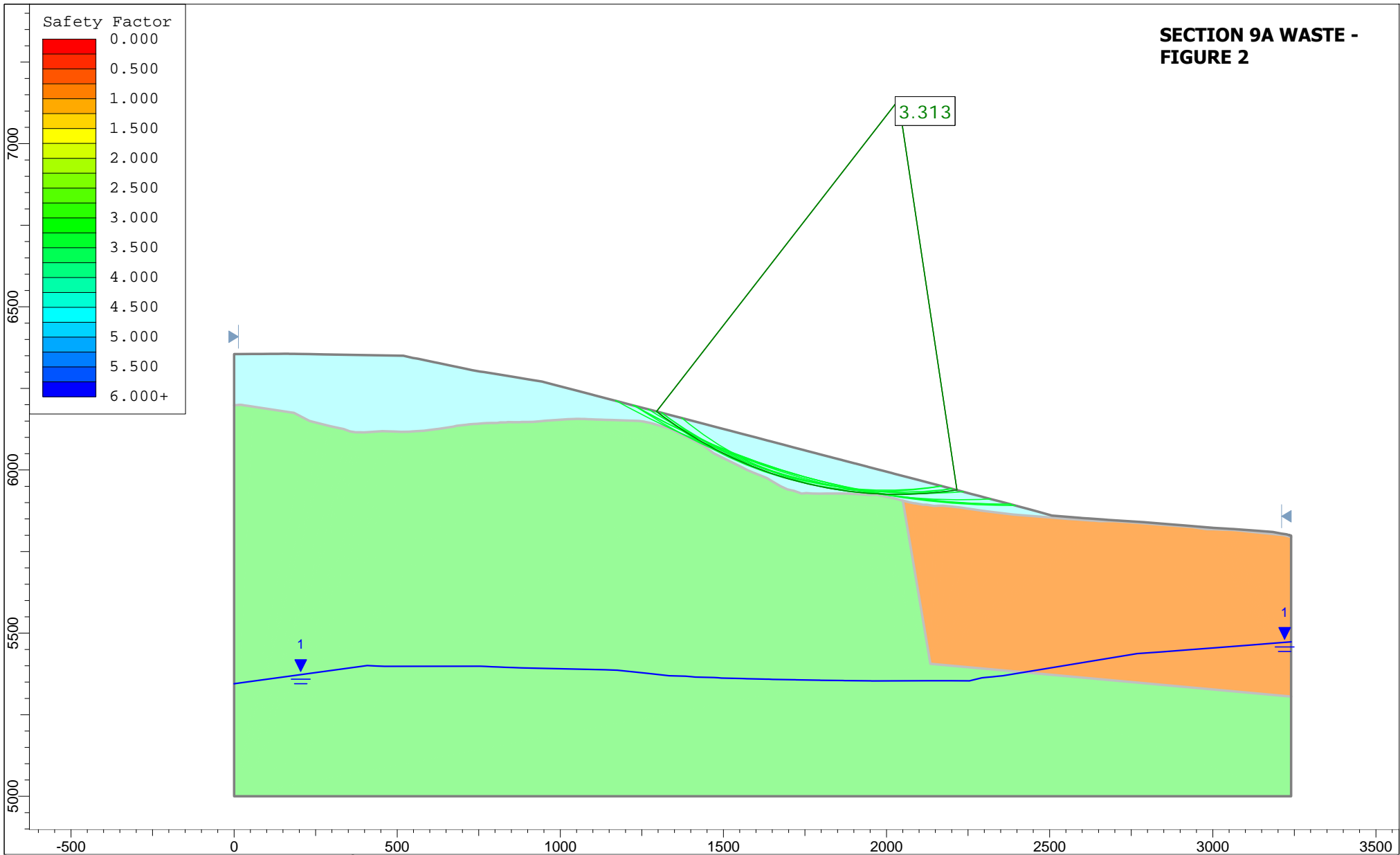


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Analysis Description			
Seismic - Circular Failure (GLE / Morgenstern-Price)			
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Date	1/2/2019		File Name
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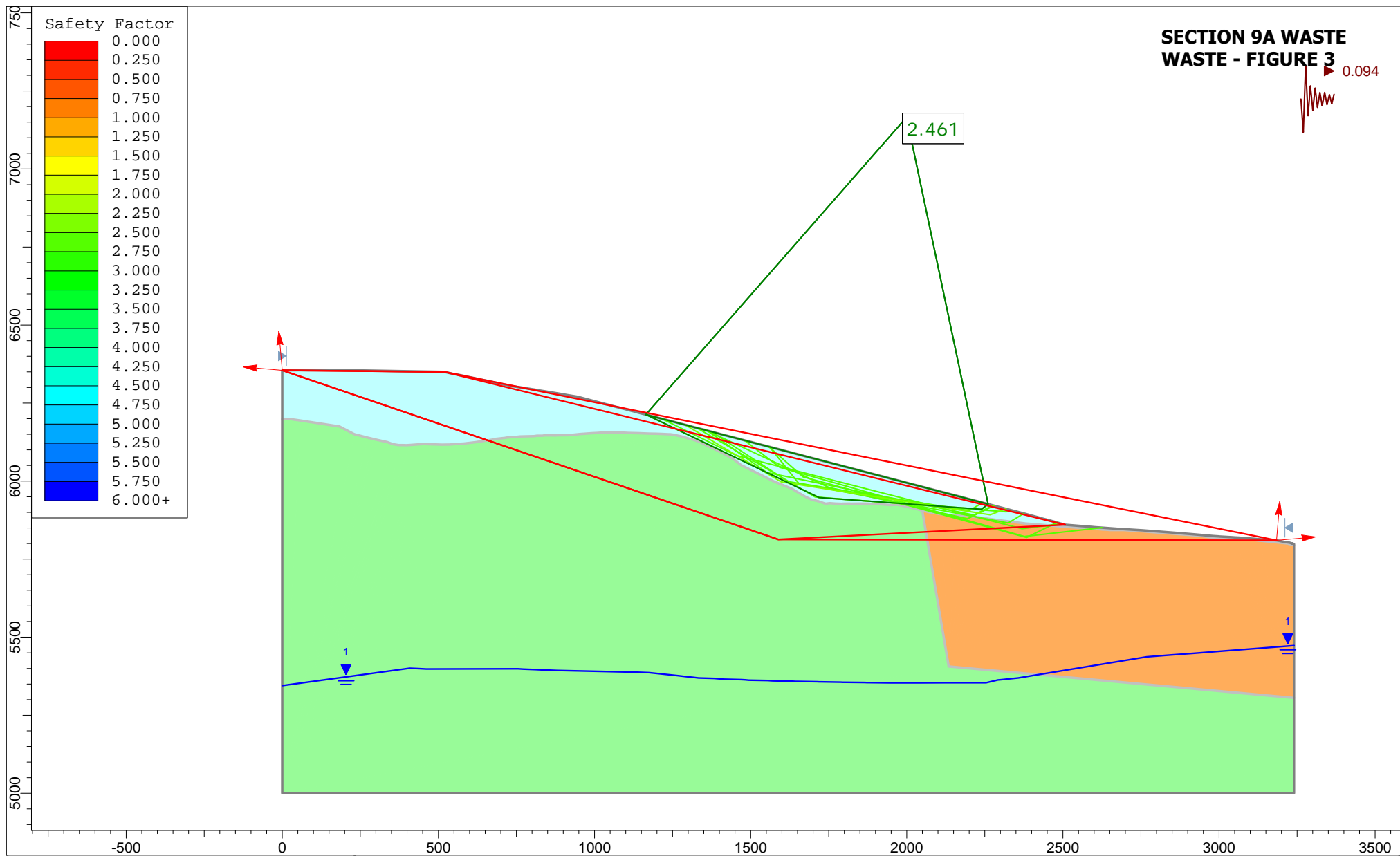
SECTION 9A WASTE - FIGURE 1



Project			
Tyrone Mine Closure Stockpile Stability - SECTION 9A WASTE			
Analysis Description			
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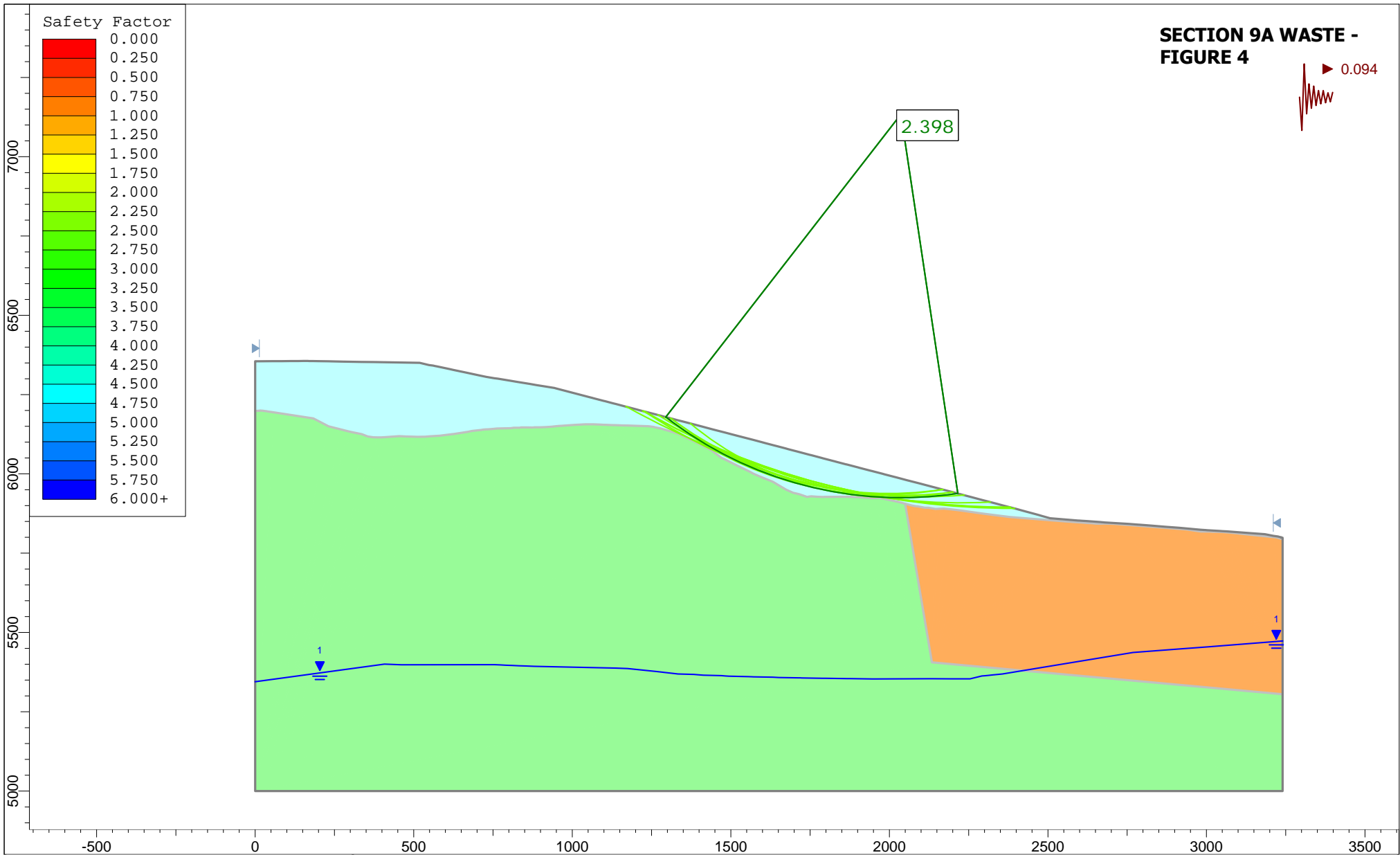


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Date				File Name	9A.slmd

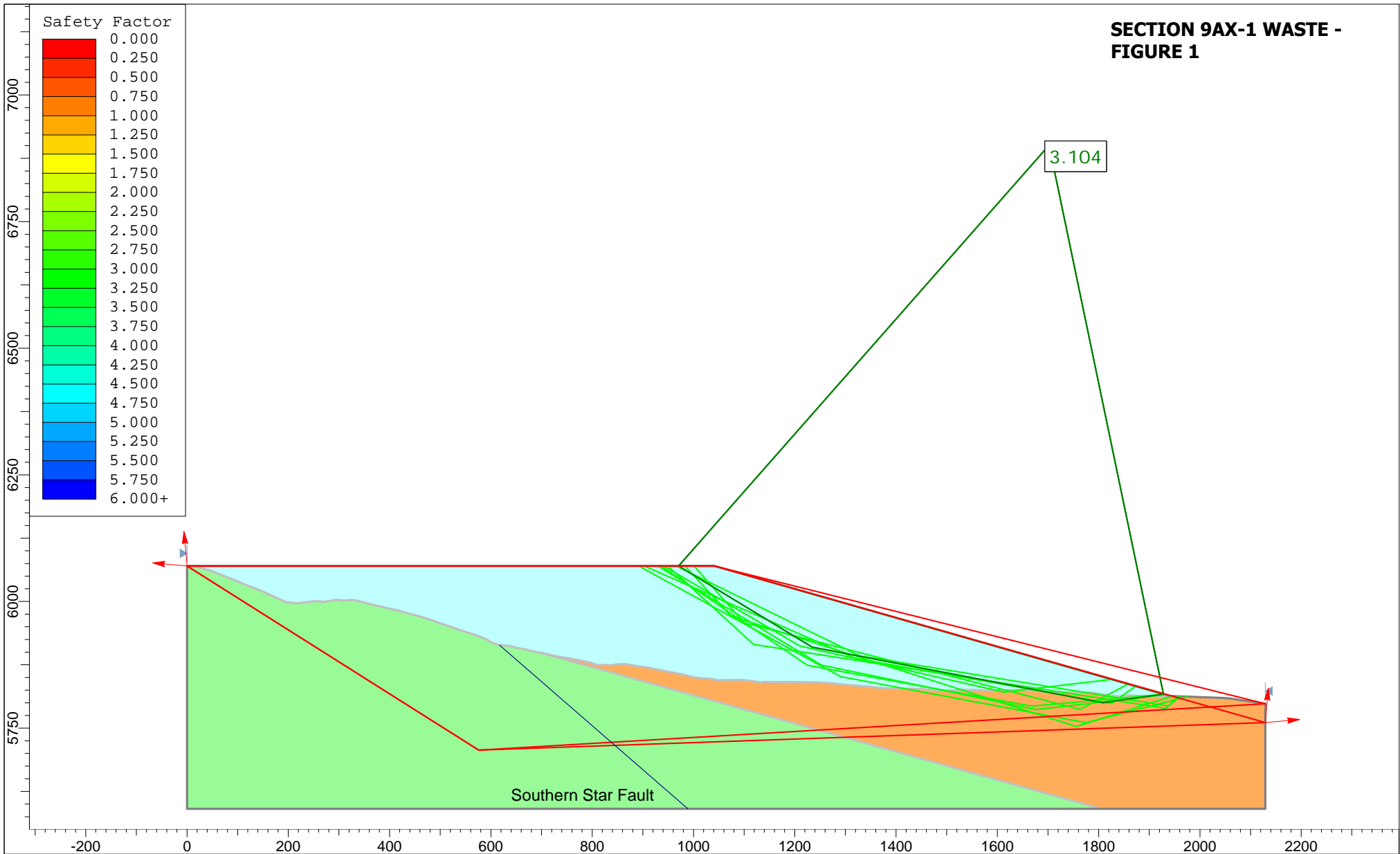


GOLDER

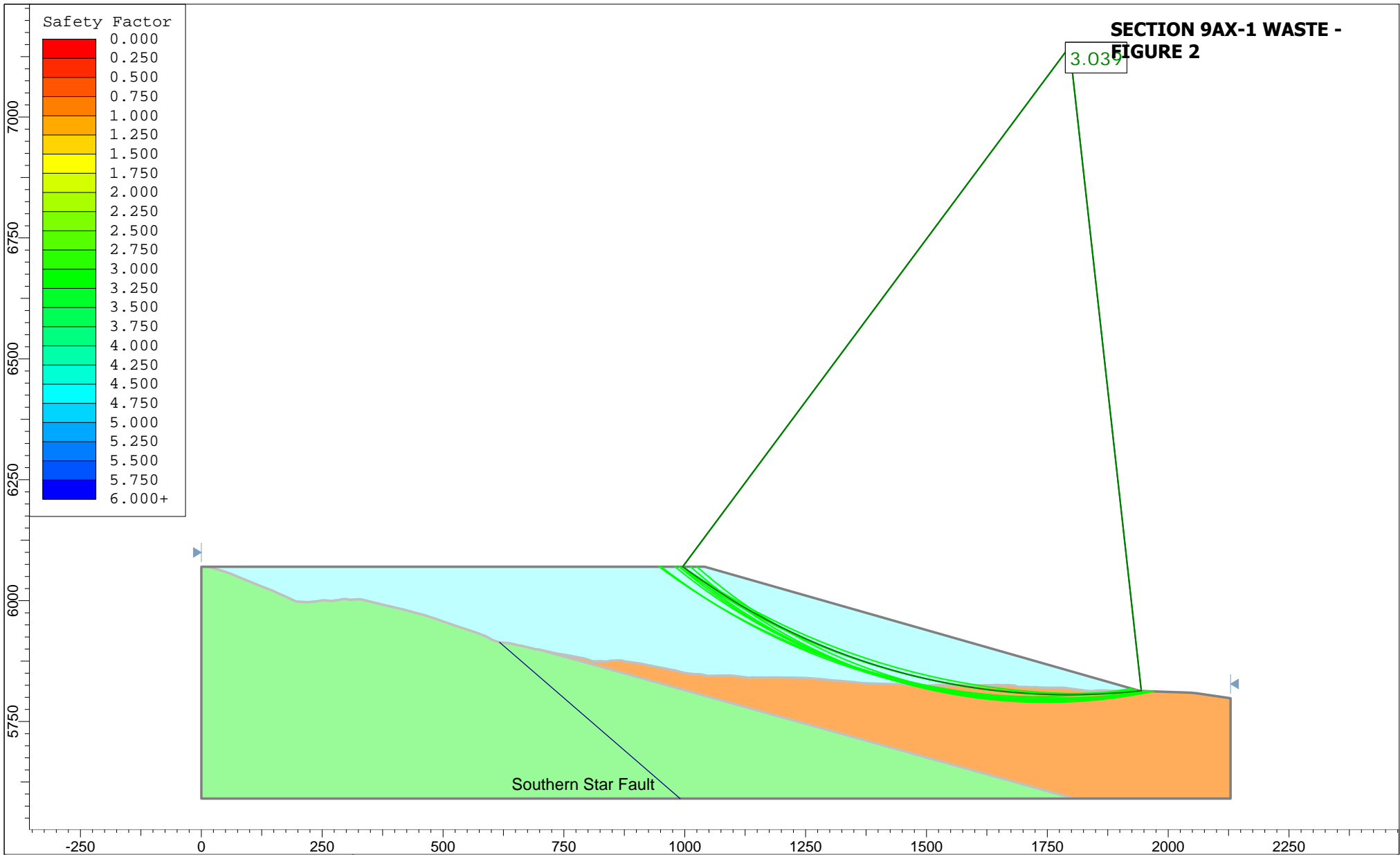
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Analysis Description			
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Date	File Name		
1/2/2019	9A.slmd		



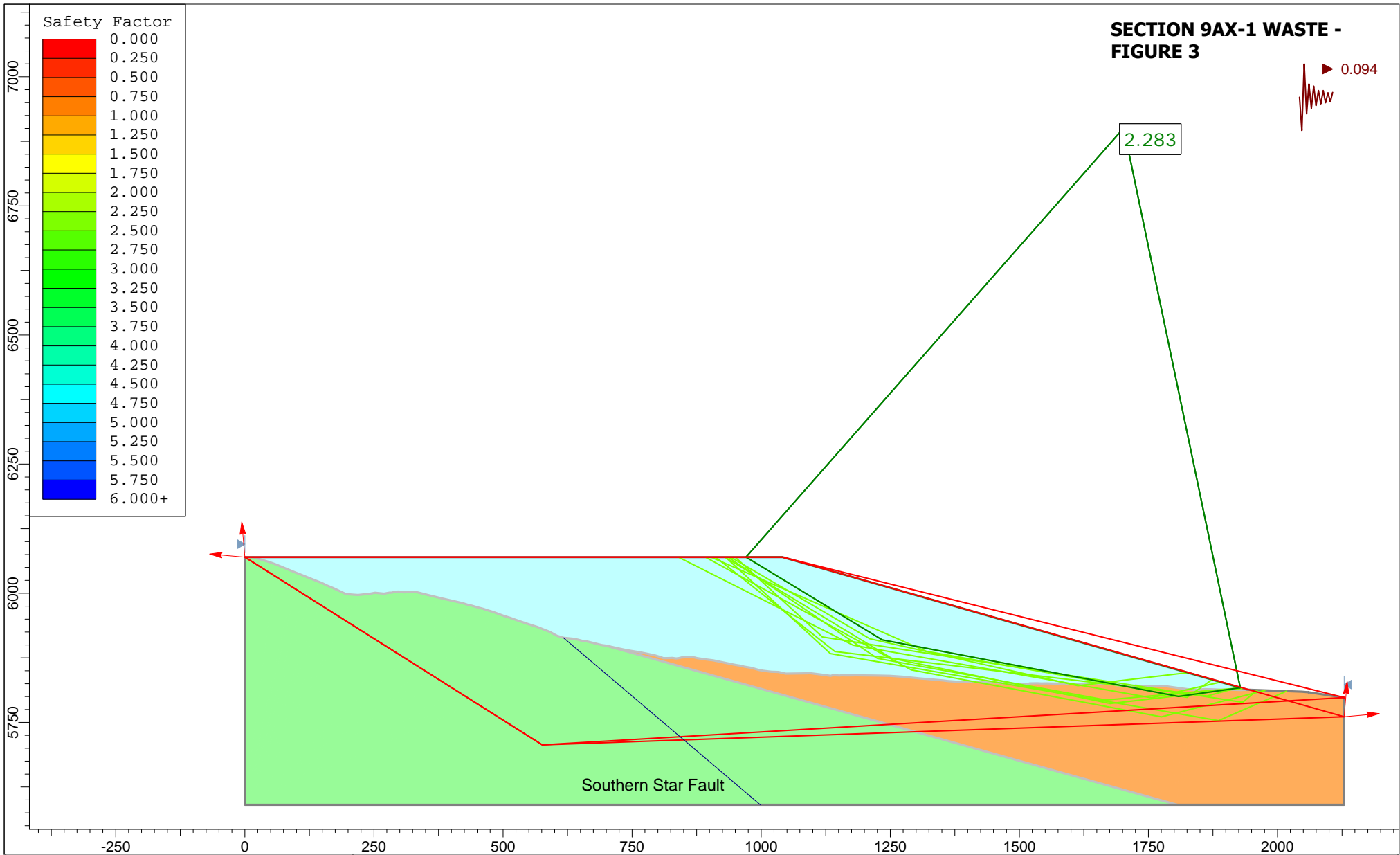
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Analysis Description			
Seismic - Circular Failure (GLE / Morgenstern-Price)			
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1/2/2019	9A.slmd		



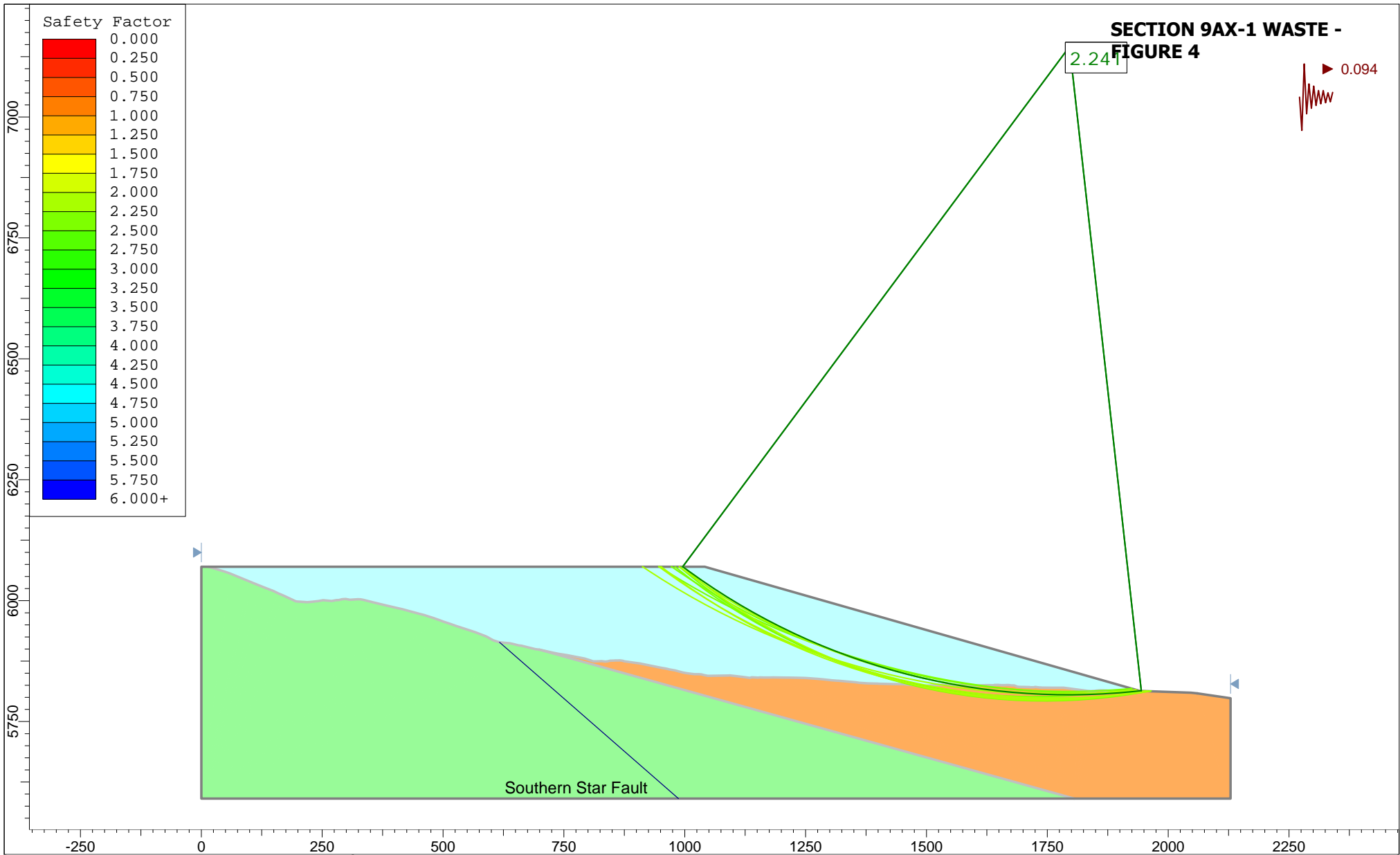
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Analysis Description			
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Date		File Name	
1/2/2019		9AX-1().slmd	



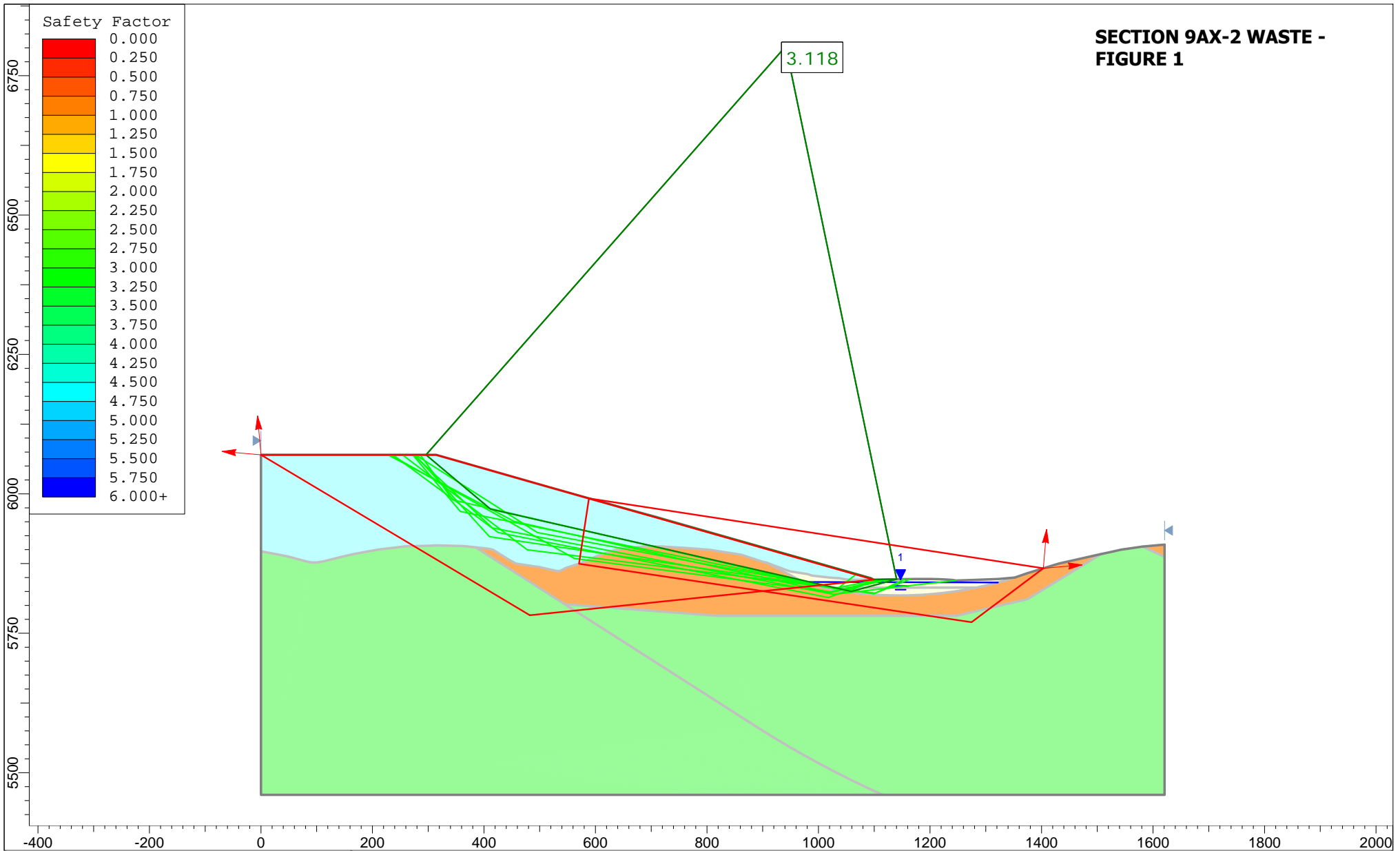
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Analysis Description			
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1/2/2019	9AX-1().slmd		



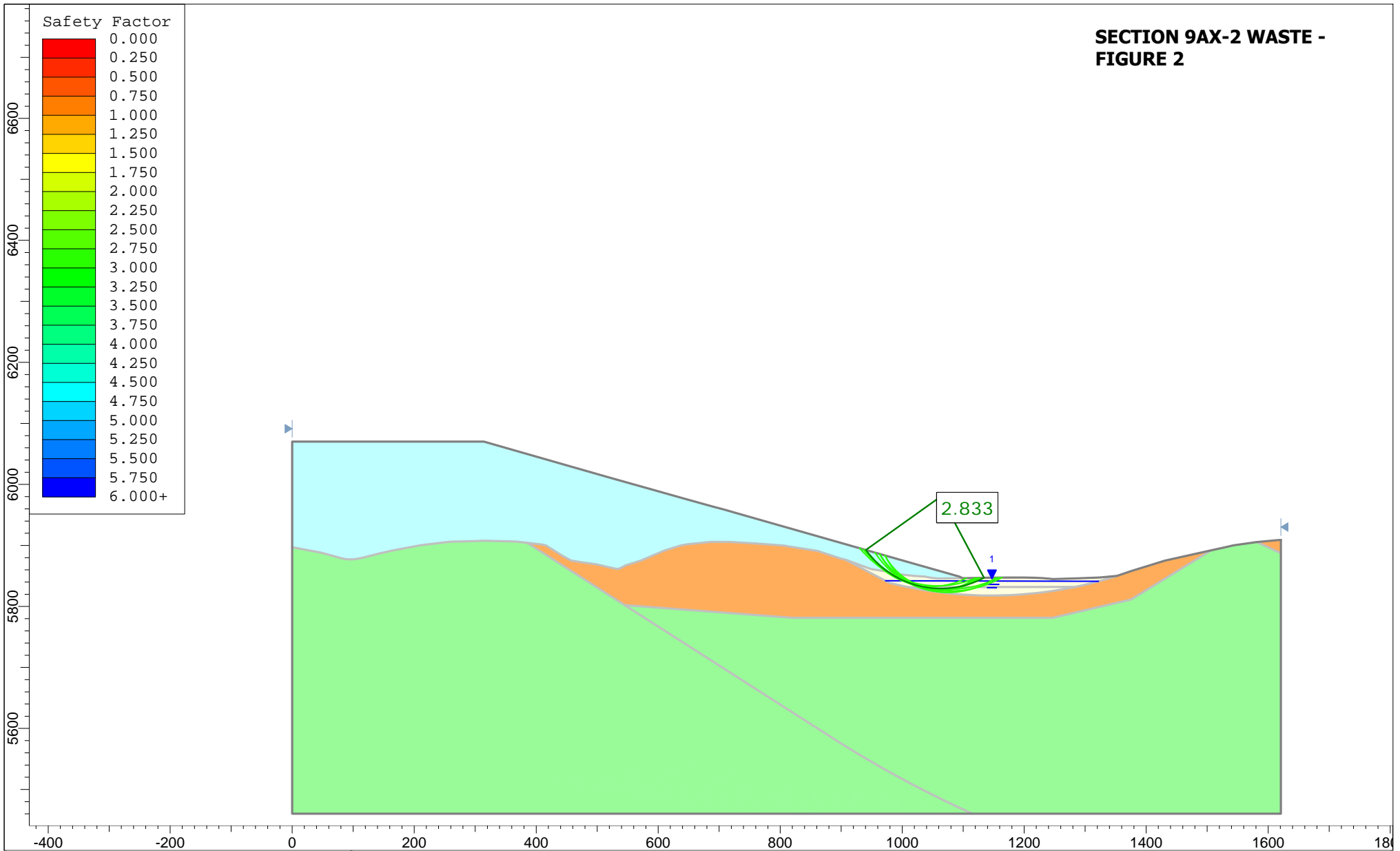
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Analysis Description			
Seismic - Block Failure (GLE / Morgenstern-Price)			
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Date	File Name		
1/2/2019	9AX-1().slmd		



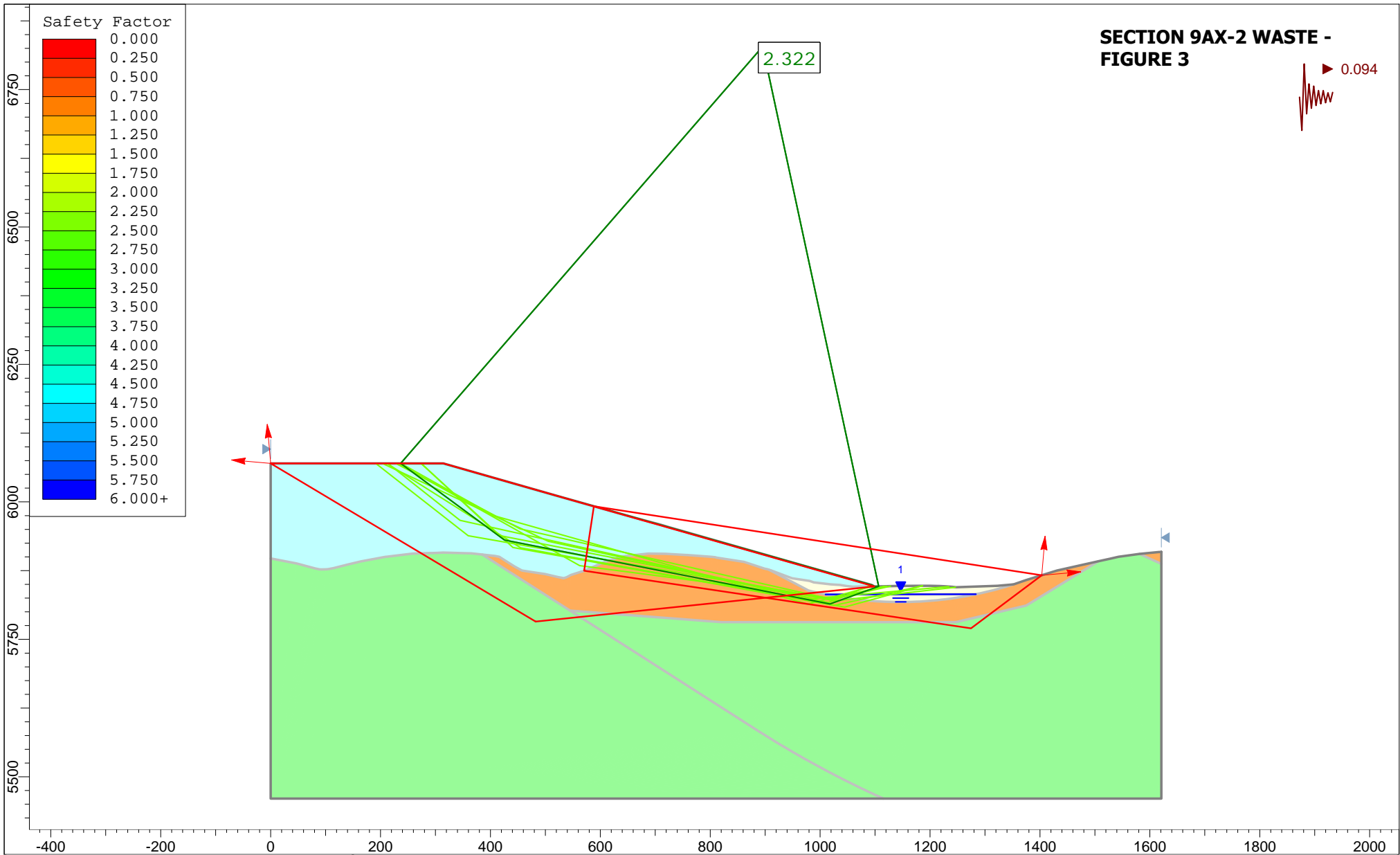
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Analysis Description			
Seismic - Circular Failure (GLE / Morgenstern-Price)			
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Date	1/2/2019		File Name
			9AX-1().slmd



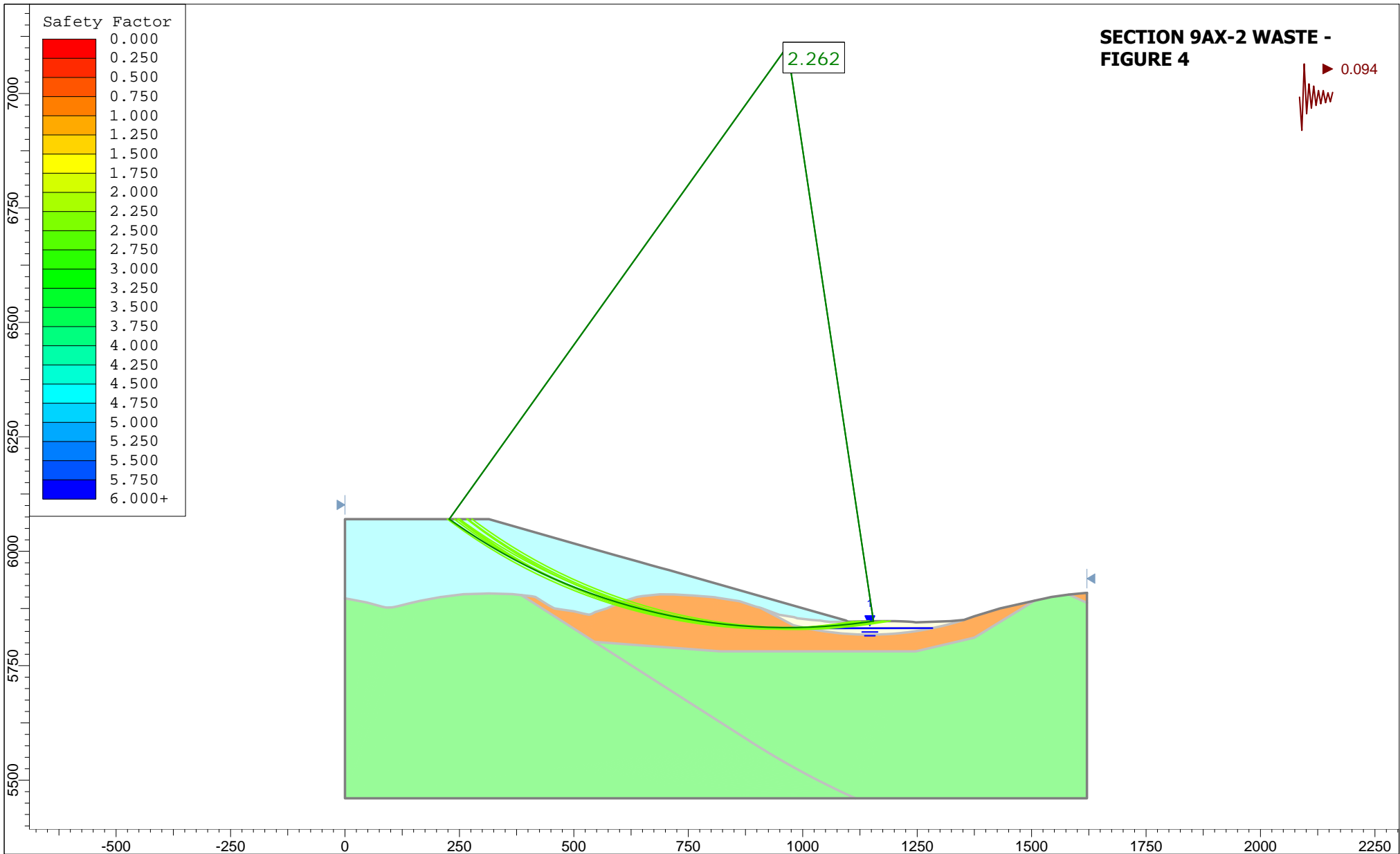
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Analysis Description			
Static - Block Failure (GLE / Morgenstern-Price)			
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Date	File Name		
1/2/2019	9AX-2.slmd		



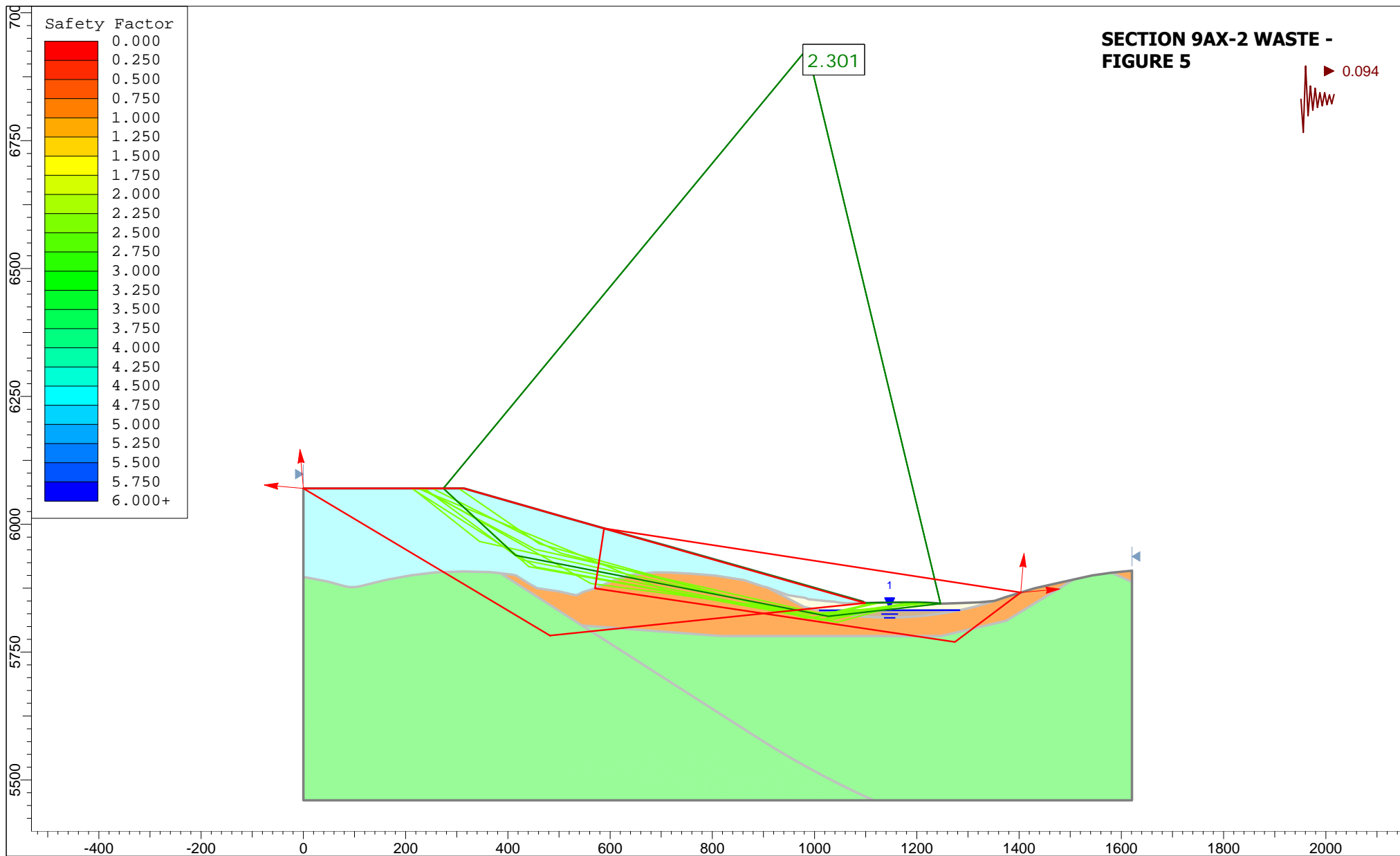
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Analysis Description			
Static - Circular Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	9AX-2.slmd		



Project			
Tyrone Mine Closure Stockpile Stability - Section 9AX (2 of 2)			
Analysis Description			
Seismic - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	9AX-2.slmd		



Project			
Tyrone Mine Closure Stockpile Stability - Section 9AX (2 of 2)			
Analysis Description			
Seismic - Circular Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	9AX-2.slmd		



GOLDER

Project

Tyrone Mine Closure Stockpile Stability - Section 9AX (2 of 2)

Analysis Description

Seismic - Liquefied Qa - Block Failure (GLE / Morgenstern-Price)

Figure

Scale

1:3117

Company

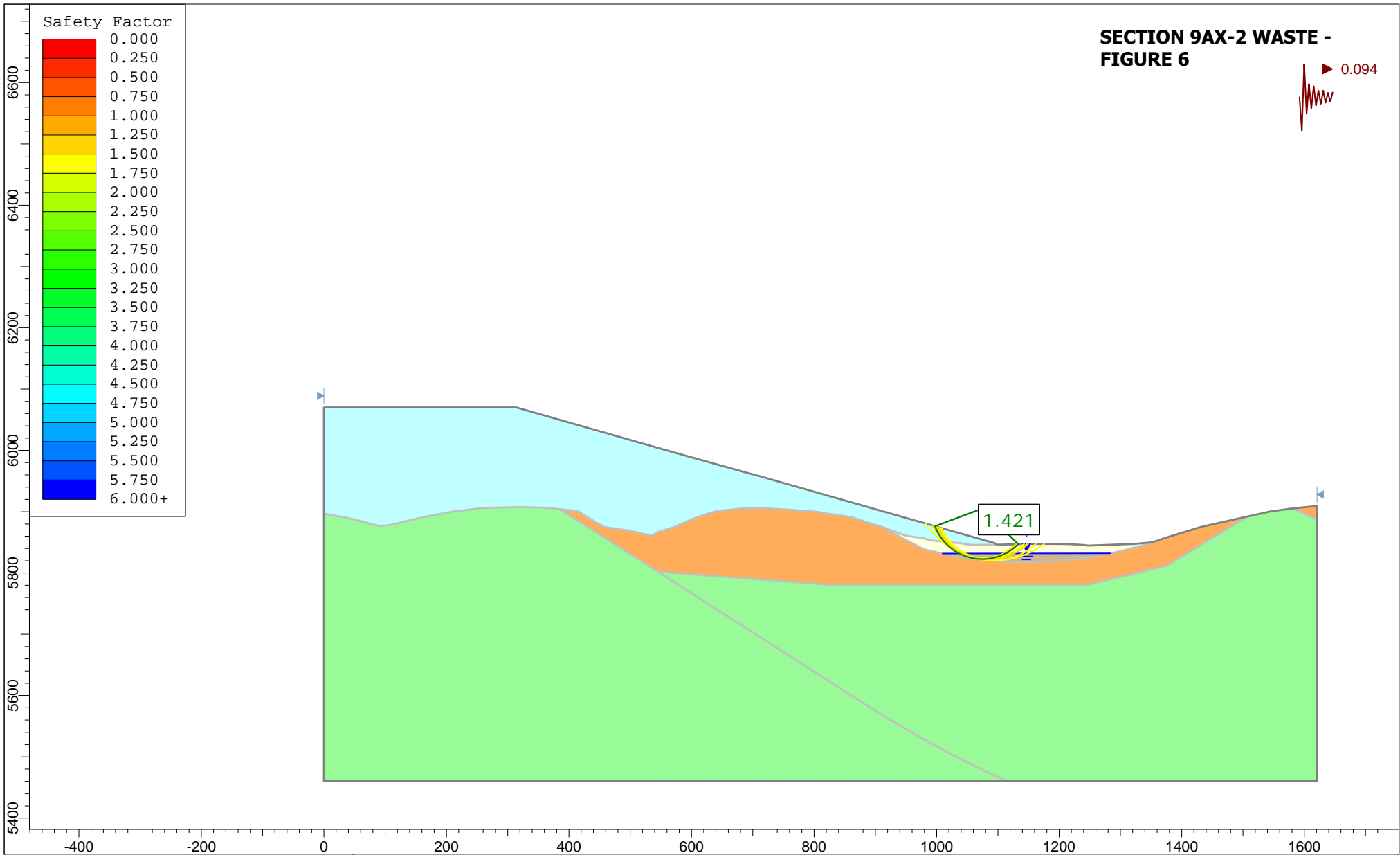
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Date

1/2/2019

File Name

9AX-2.slmd



GOLDER

Project

Tyrone Mine Closure Stockpile Stability - Section 9AX (2 of 2)

Analysis Description

Seismic - Liquefied Qa - Circular Failure (GLE / Morgenstern-Price)

Figure

Scale

1:2602

Company

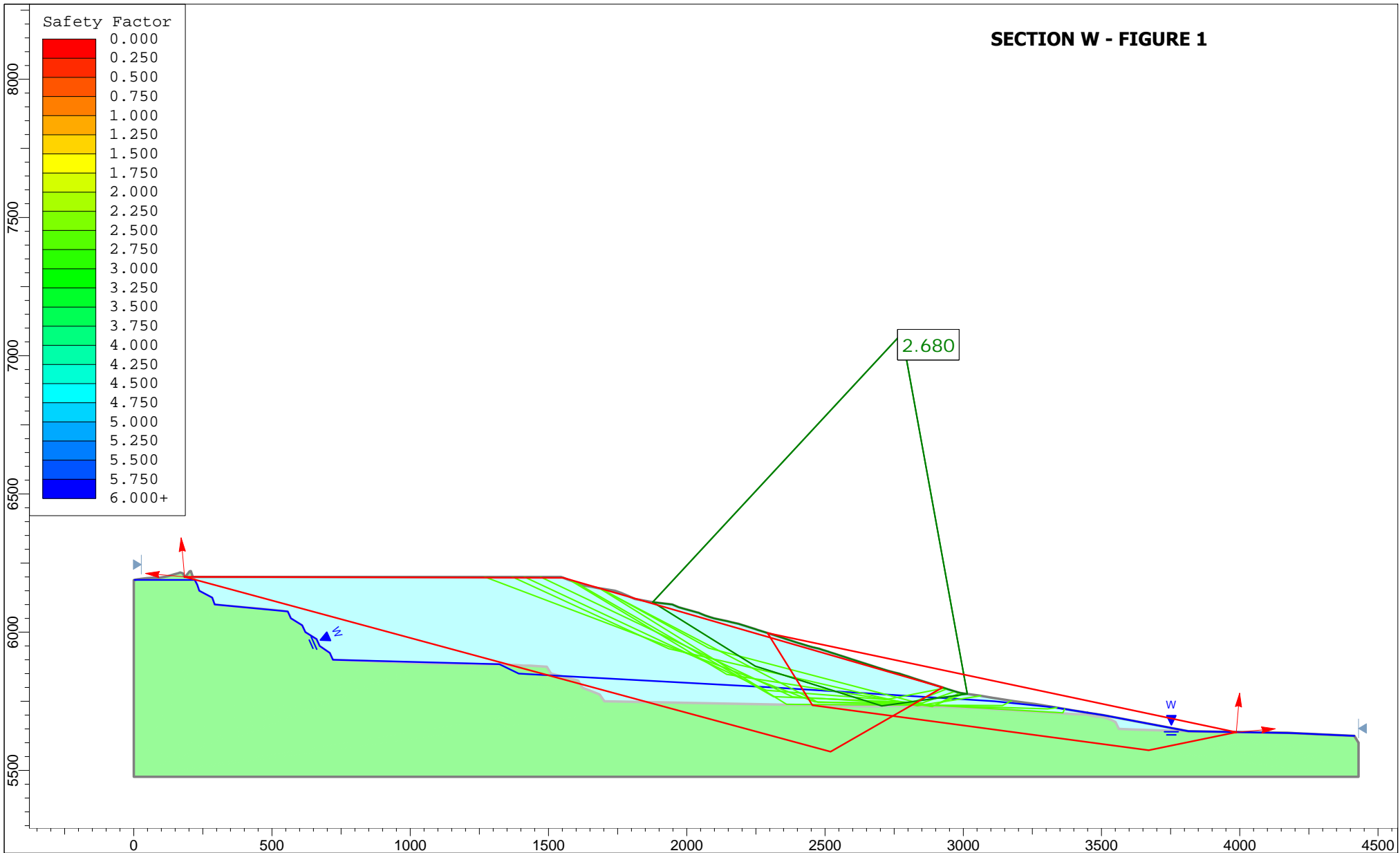
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Date

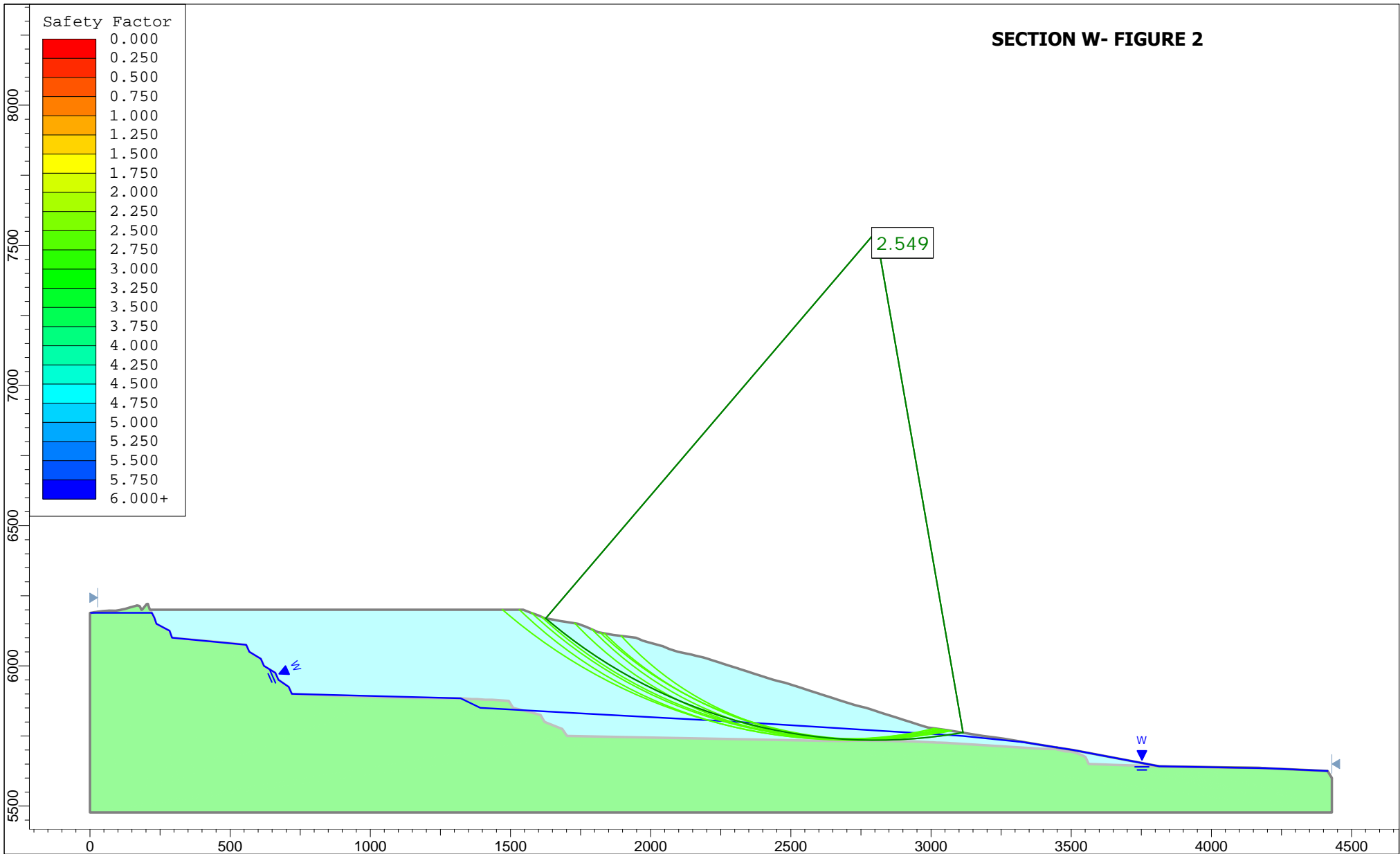
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File Name

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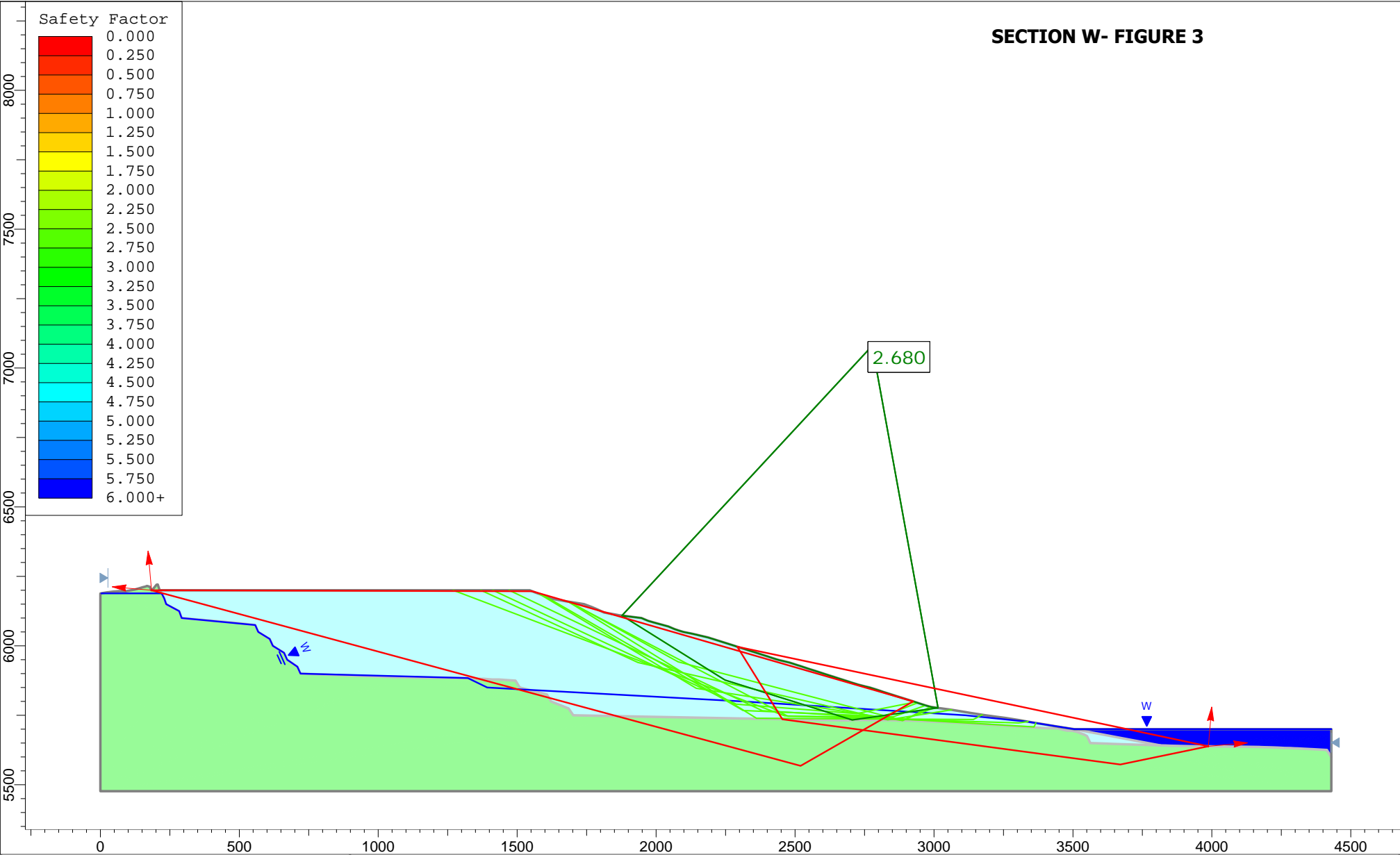



Project			
Tyrone Mine Closure Stockpile Stability - Section West In-Pit Waste (at Little Rock)			
Analysis Description			
Static- No Pit Lake - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
	1:5761	Golder Associates	
Date	File Name		
1/2/2019	LR.slmd		



Project			
Tyrone Mine Closure Stockpile Stability - Section West In-Pit Waste (at Little Rock)			
Analysis Description			
Static- No Pit Lake - Circular Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	LR.slmd		

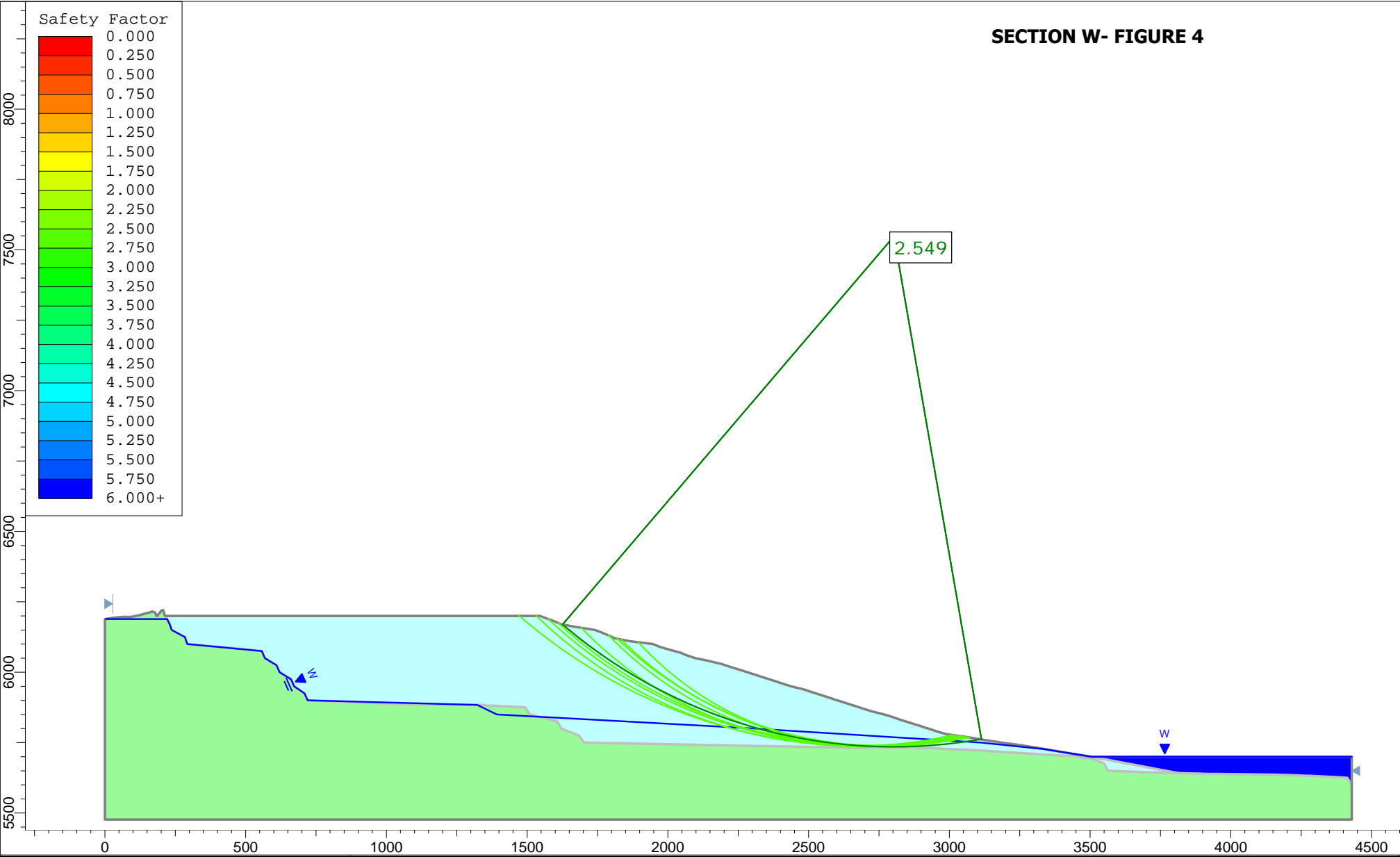
SECTION W- FIGURE 3




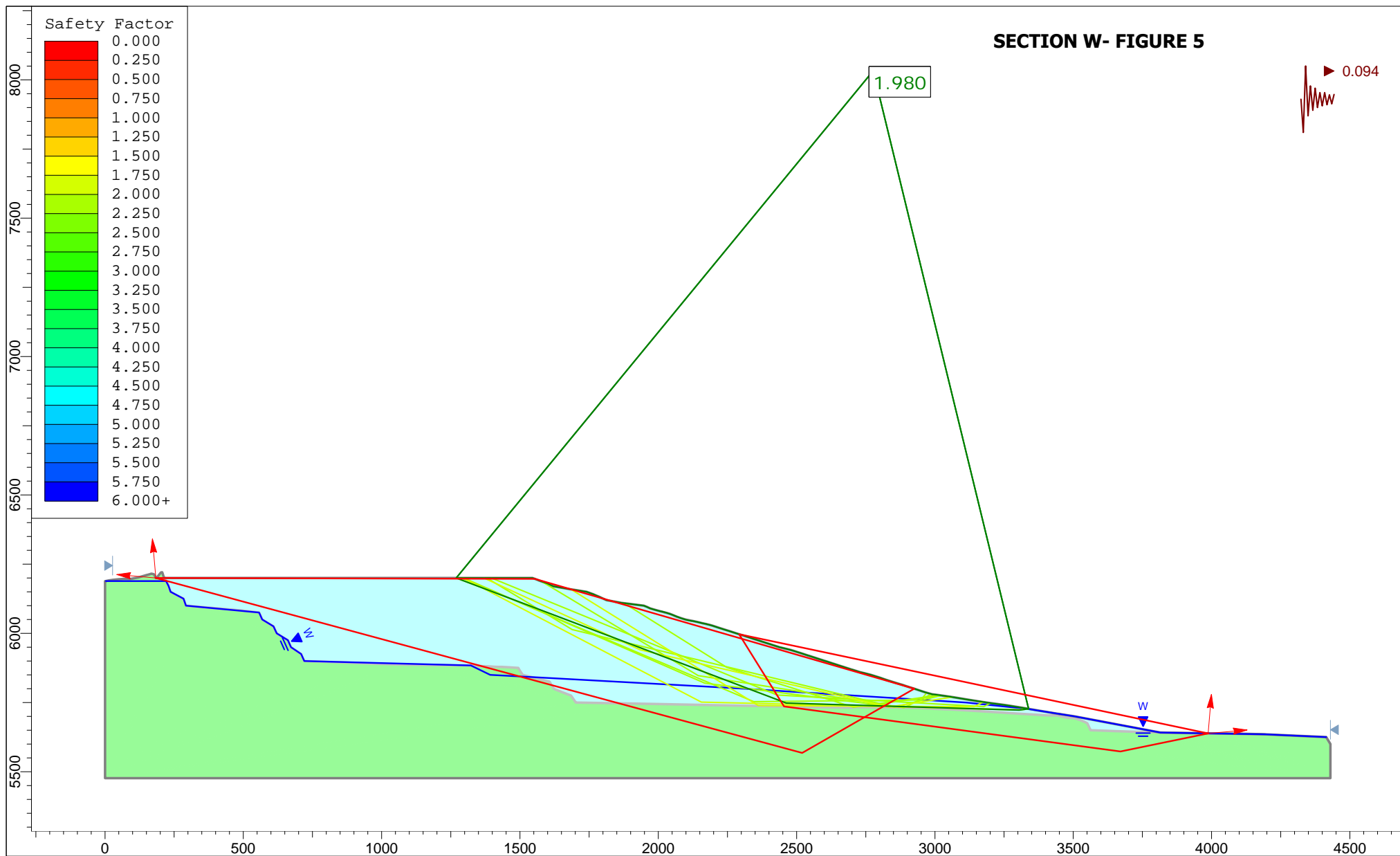
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	Analysis Description				
	Static-Pit Lake 5700 ft - Block Failure (GLE / Morgenstern-Price)				
	Figure		Scale	Company	
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	Date		File Name		
	1/2/2019		LR.slmd		

SLIDEINTERPRET 8.014

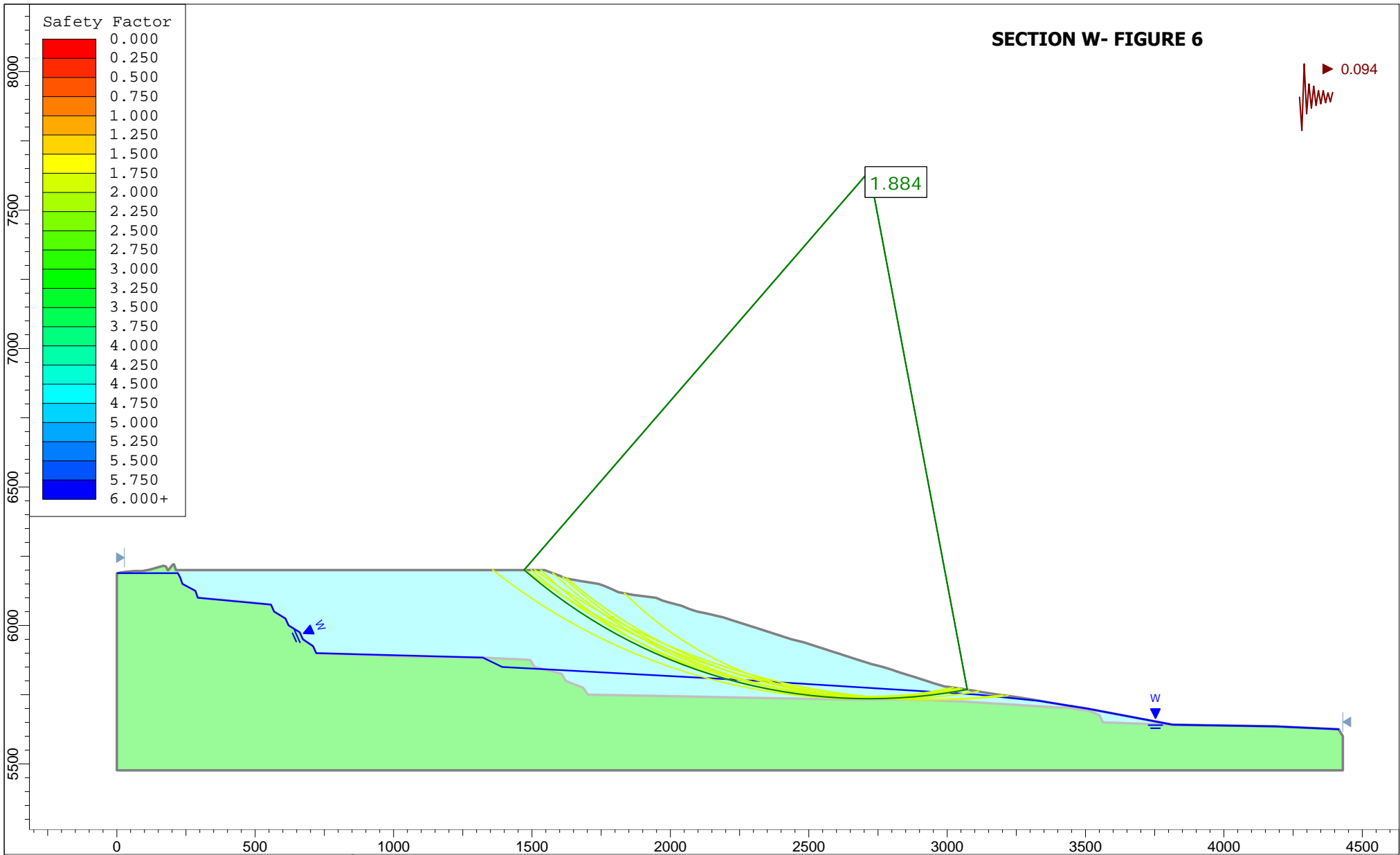
SECTION W- FIGURE 4



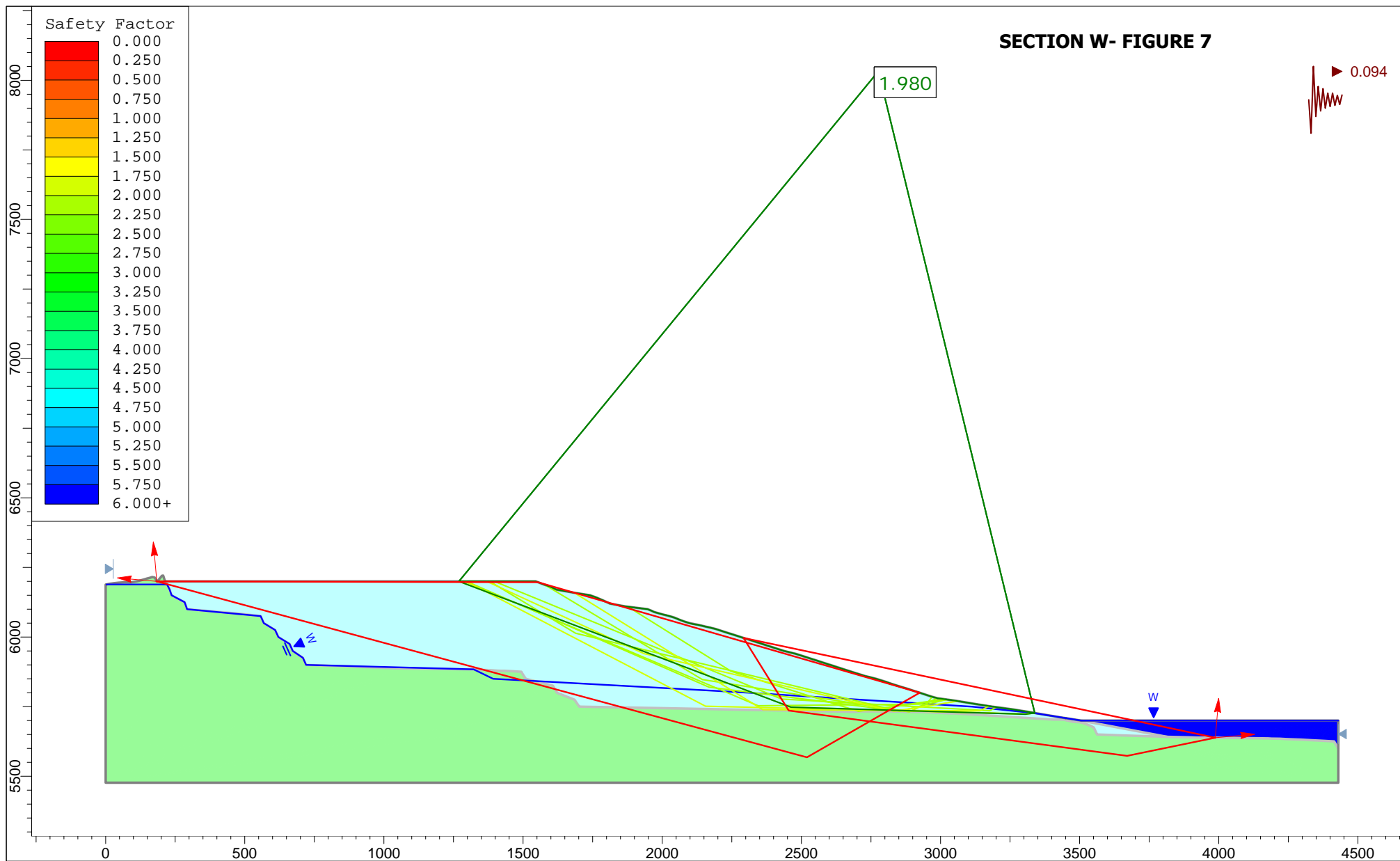
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	Analysis Description			Static-Pit Lake 5700 ft - Circular Failure (GLE / Morgenstern-Price)	
	Figure	Scale	1:5687	Company	Golder Associates
	Date	1/2/2019		File Name	LR.slmd



Project			
Tyrone Mine Closure Stockpile Stability - Section West In-Pit Waste (at Little Rock)			
Analysis Description			
Seismic- No Pit Lake - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
	1:5761	Golder Associates	
Date	File Name		
1/2/2019	LR.slmd		

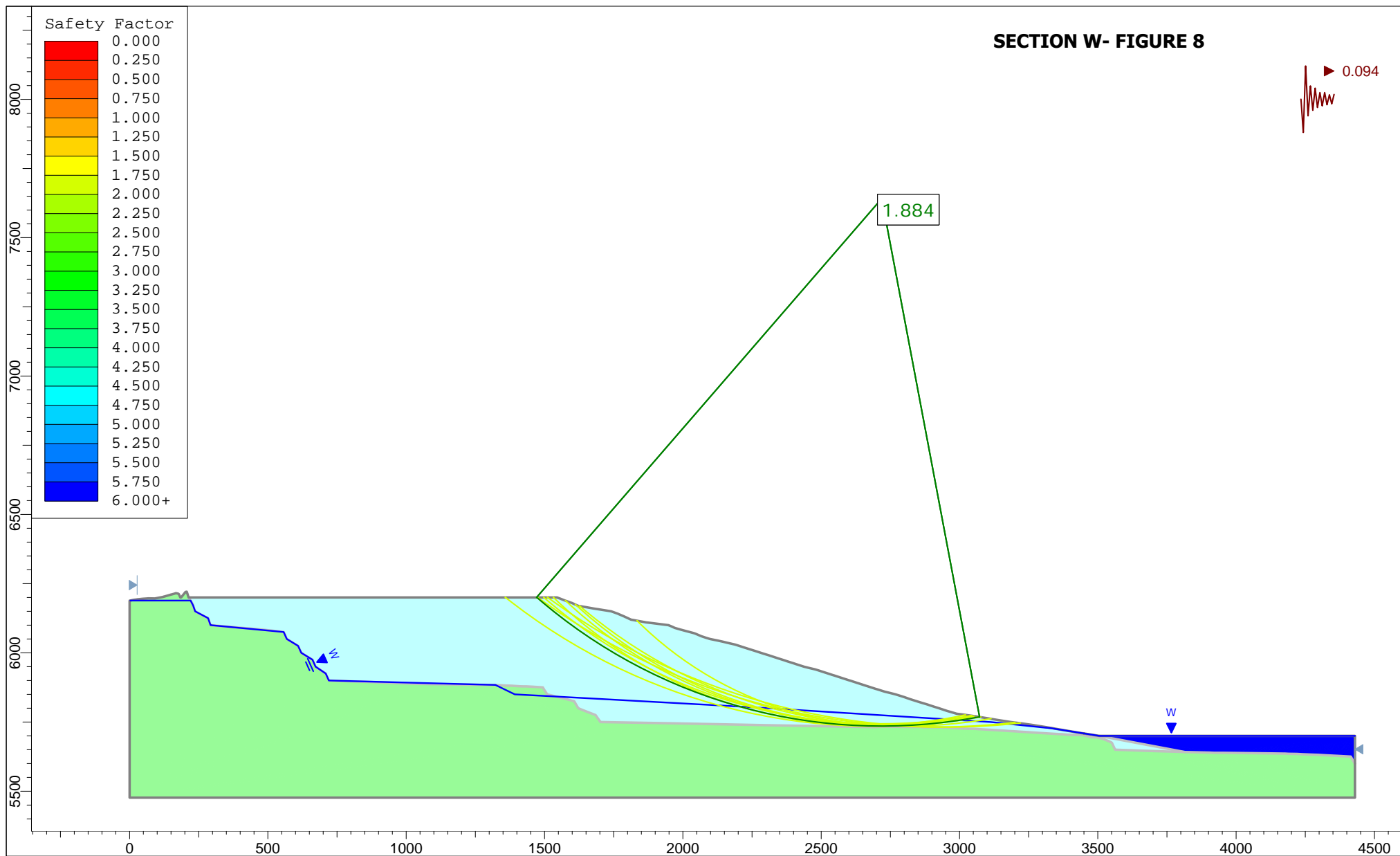


Project			
Tyrone Mine Closure Stockpile Stability - Section West In-Pit Waste (at Little Rock)			
Analysis Description			
Seismic- No Pit Lake - Circular Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
	1:5761	Golder Associates	
Date	File Name		
1/2/2019	LR.slmd		



GOLDER

Project			
Tyrone Mine Closure Stockpile Stability - Section West In-Pit Waste (at Little Rock)			
Analysis Description			
Seismic-Pit Lake 5700 ft - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
	1:5761	Golder Associates	
Date	1/2/2019		File Name
			LR.slmd



GOLDER

Project

Tyrone Mine Closure Stockpile Stability - Section West In-Pit Waste (at Little Rock)

Analysis Description

Seismic-Pit Lake 5700 ft - Circular Failure (GLE / Morgenstern-Price)

Figure

Scale

1:5761

Company

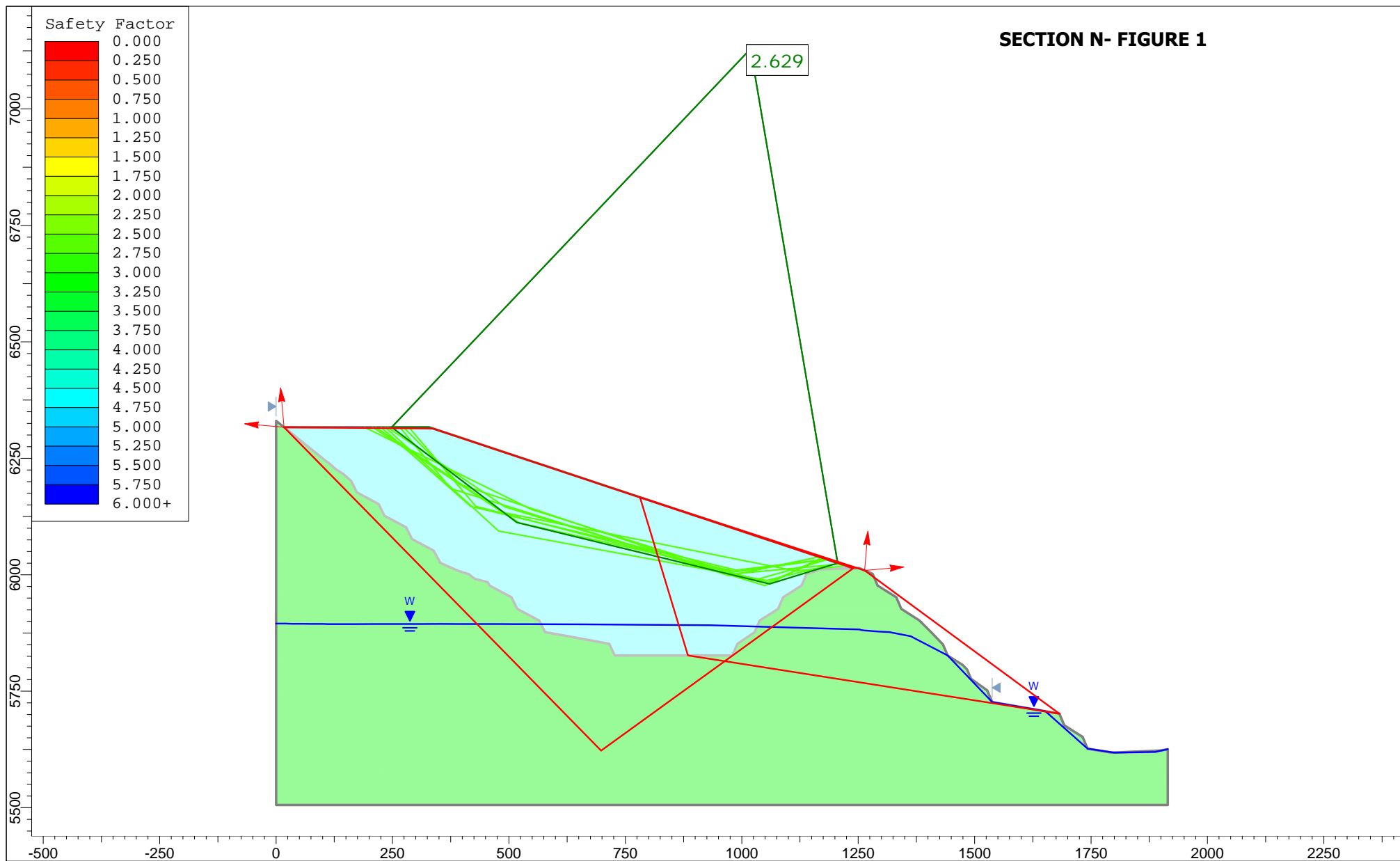
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Date

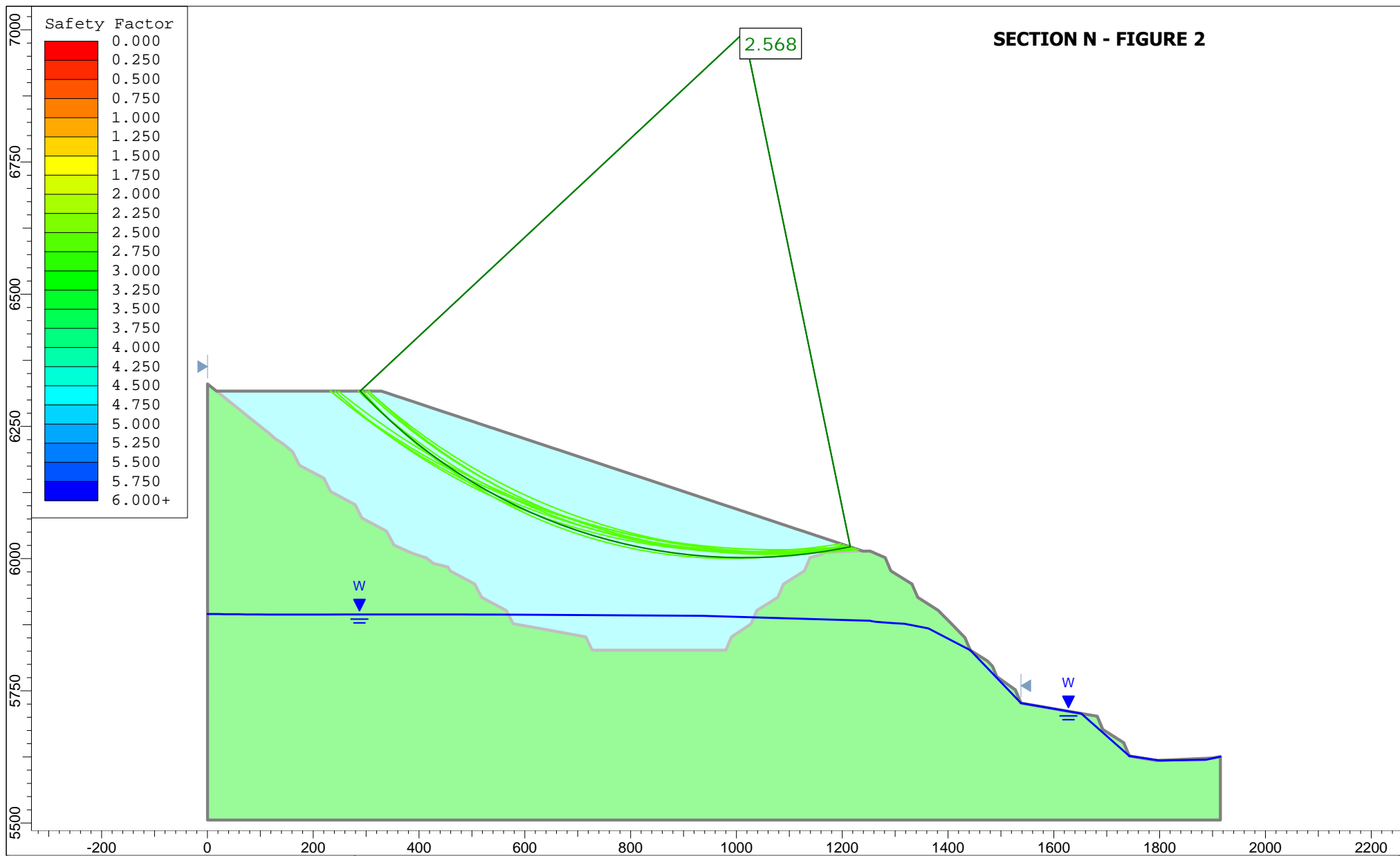
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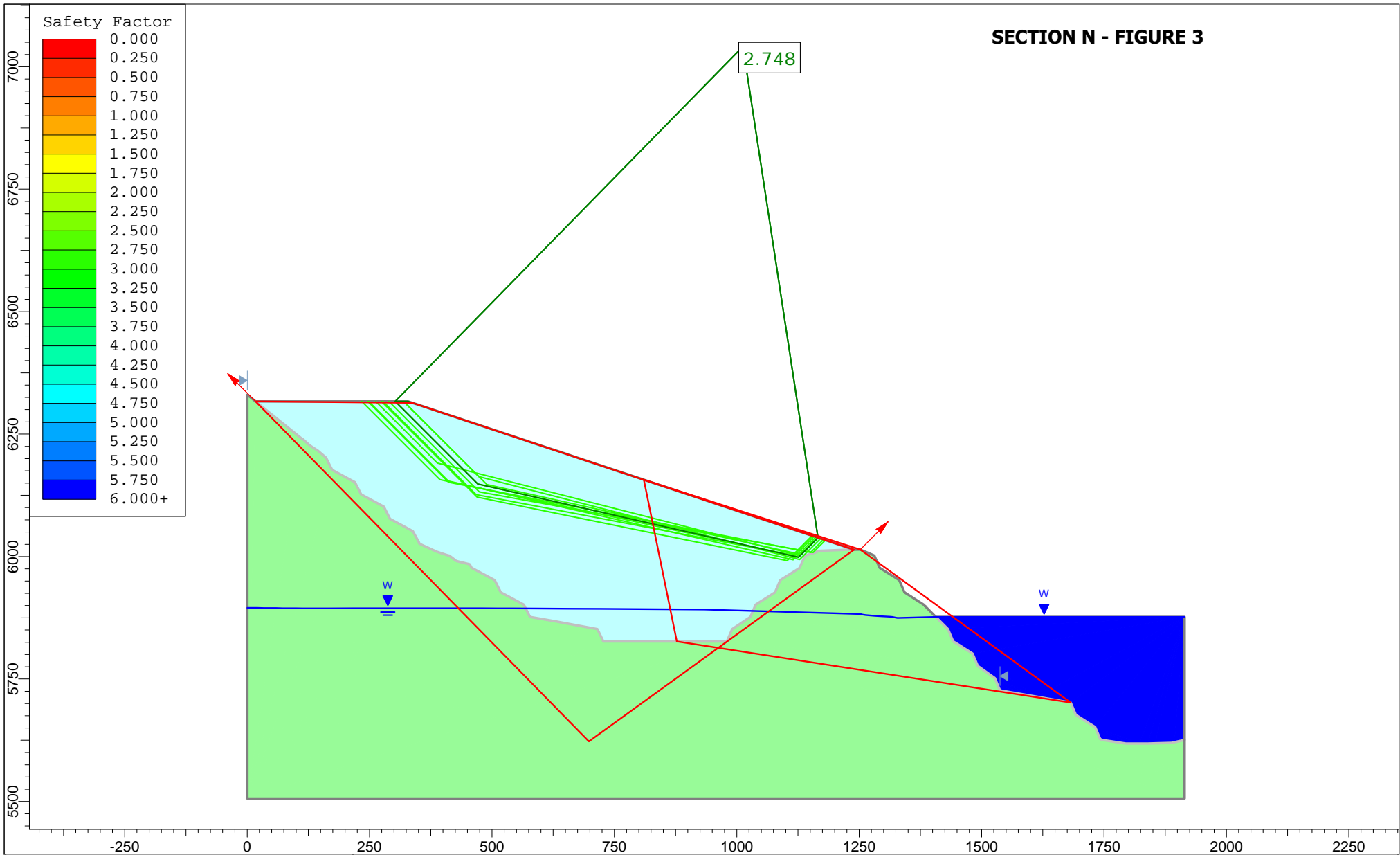


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Tyrone Mine Closure Stockpile Stability - Section North In-Pit Waste (at Little Rock)			
Analysis Description			
Static - No Pit Lake condition - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	LR-n.slmd		

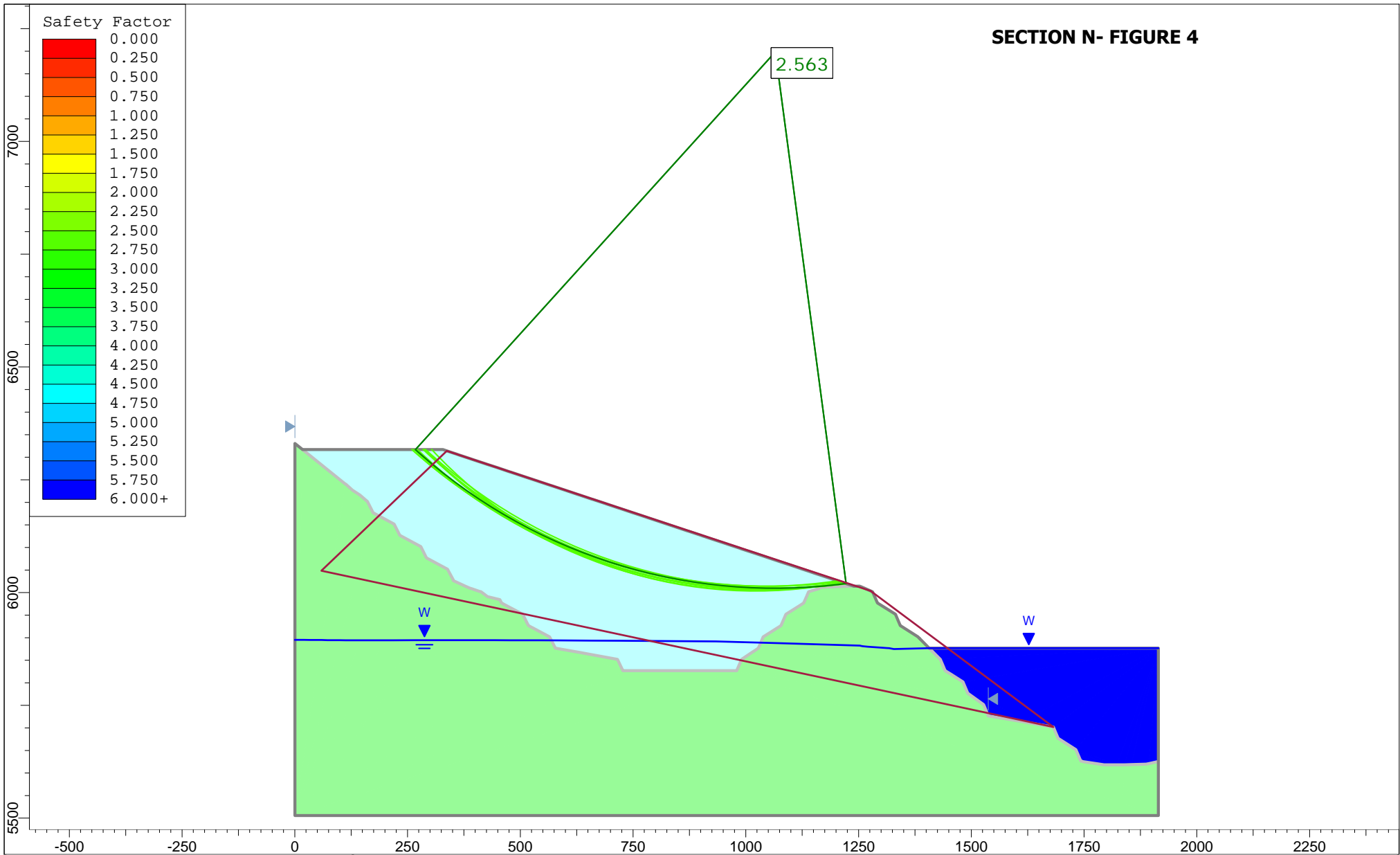


GOLDER

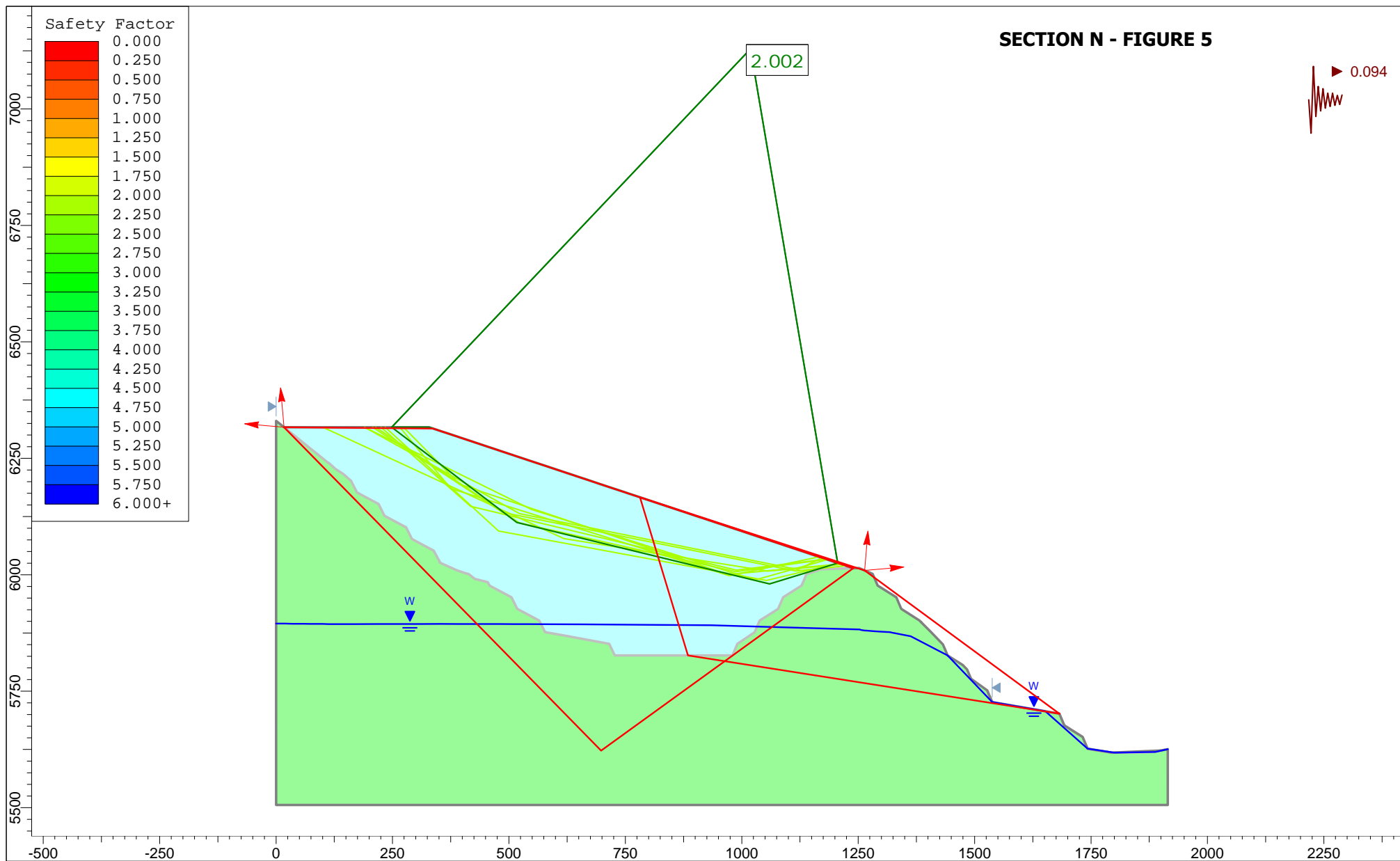
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Tyrone Mine Closure Stockpile Stability - Section North In-Pit Waste (at Little Rock)			
Analysis Description			
Static - No Pit Lake condition - Circular Fail (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	1/2/2019		File Name
			LR-n.slmd



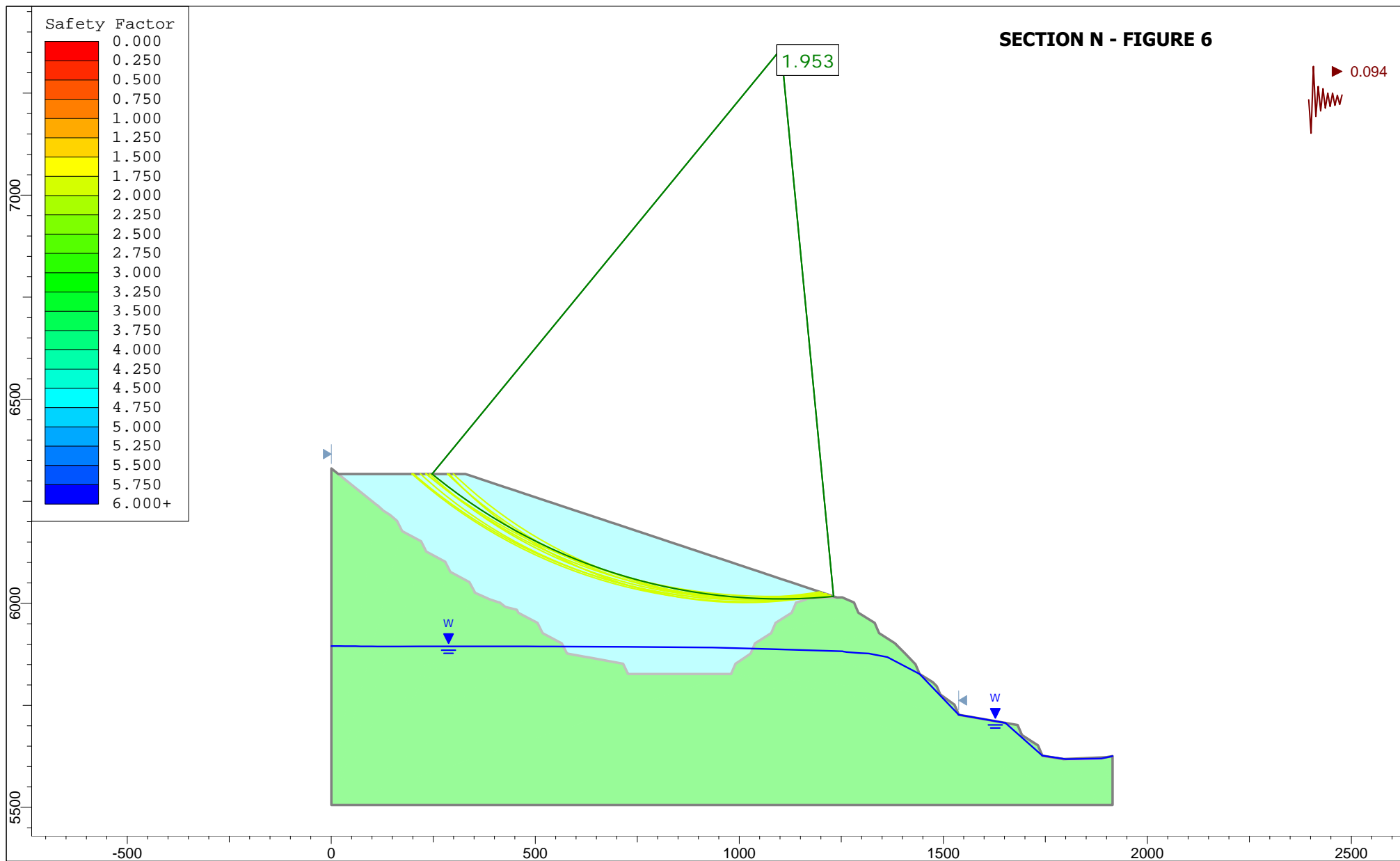
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Tyrone Mine Closure Stockpile Stability - Section North In-Pit Waste (at Little Rock)			
Analysis Description			
Static-Pit Lake 5700 ft - Block Failure (GLE / Morgenstern-Price)			
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Date	File Name		
1/2/2019	LR-n.slmd		



Project			
Tyrone Mine Closure Stockpile Stability - Section North In-Pit Waste (at Little Rock)			
Analysis Description			
Static-Pit Lake 5700 ft - Circular Fail (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	1/2/2019		File Name
			LR-n.slmd

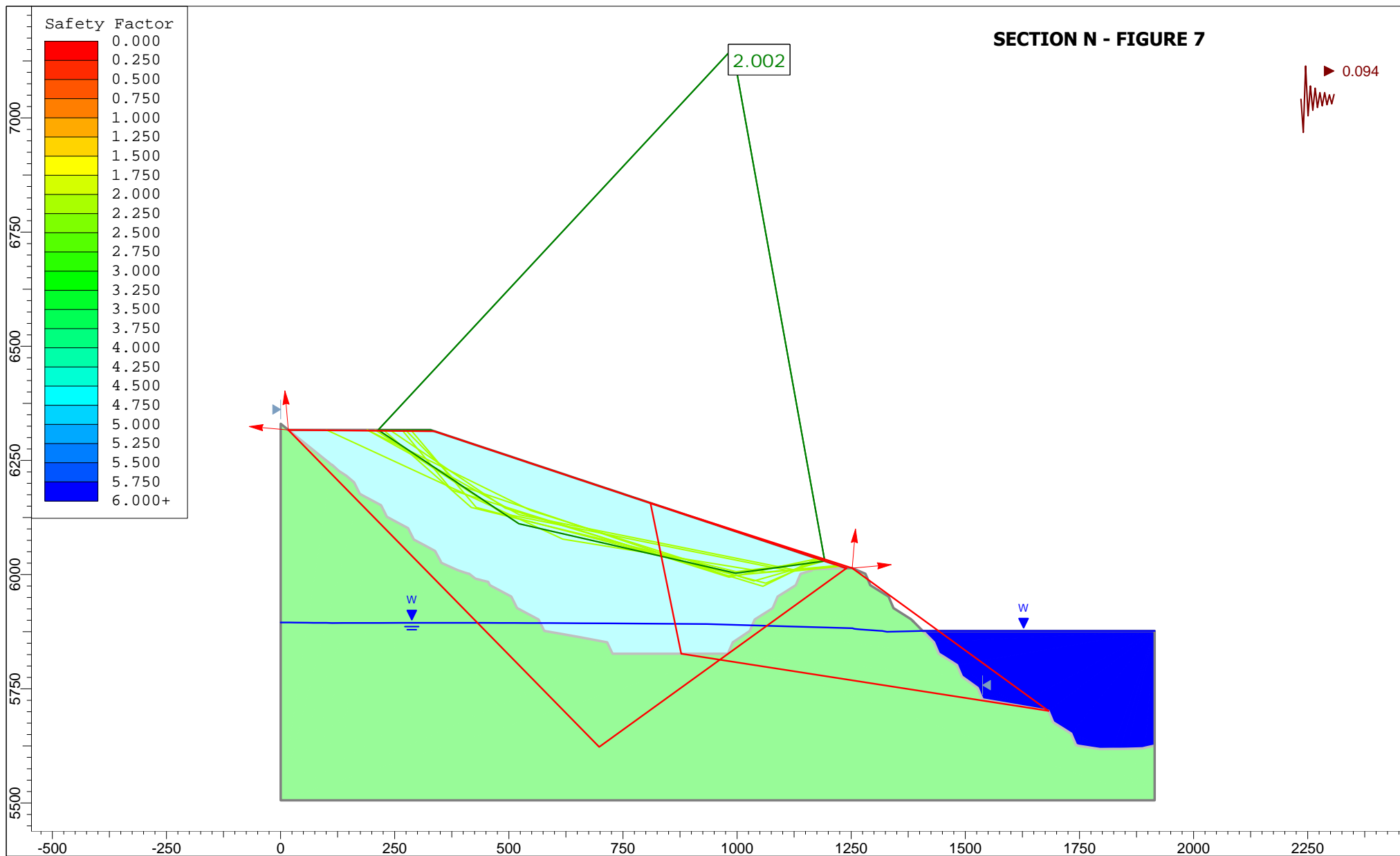


Project			
Tyrone Mine Closure Stockpile Stability - Section North In-Pit Waste (at Little Rock)			
Analysis Description			
Seismic - No Pit Lake condition - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	LR-n.slmd		

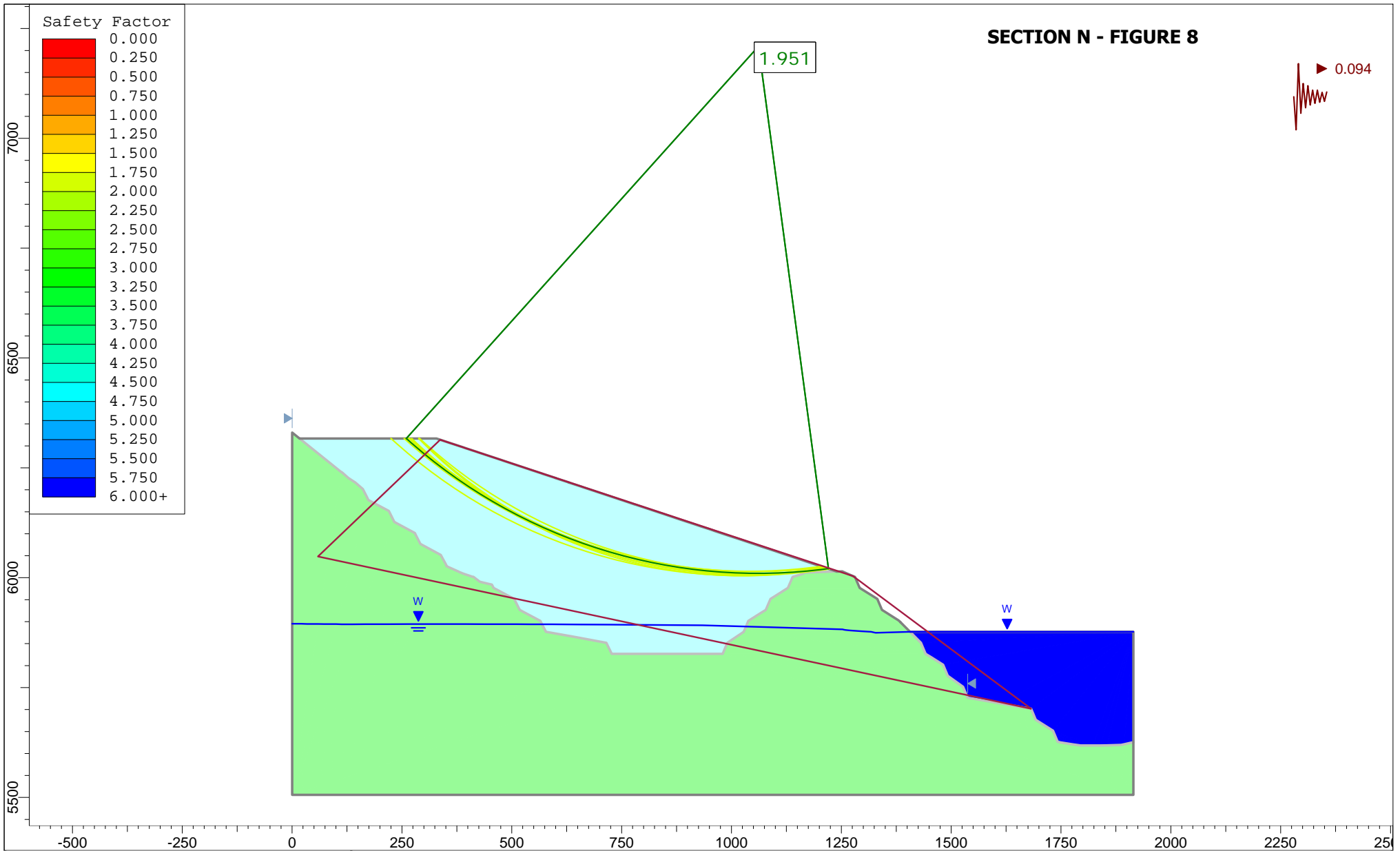


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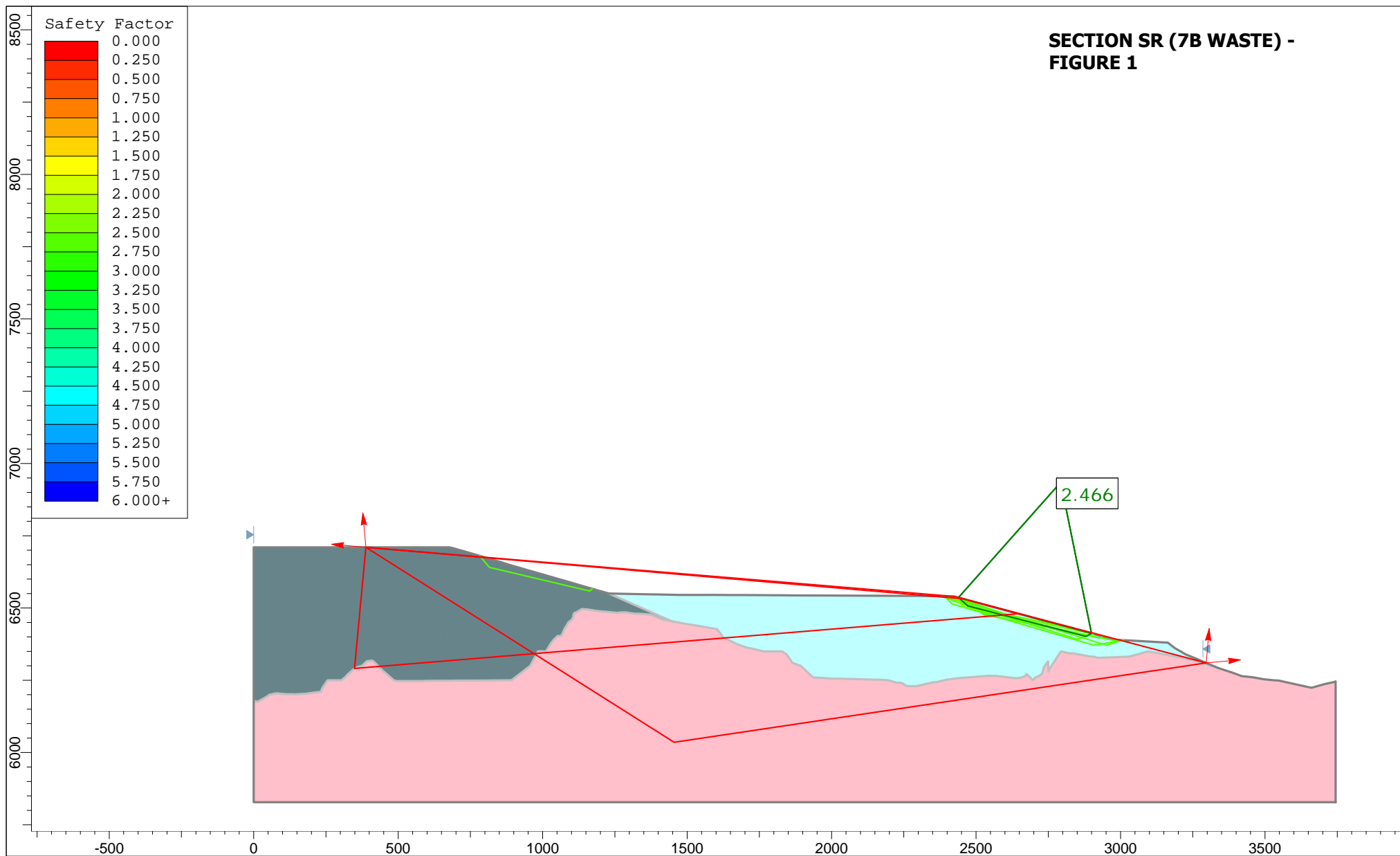
Project			
Tyrone Mine Closure Stockpile Stability - Section North In-Pit Waste (at Little Rock)			
Analysis Description			
Seismic - No Pit Lake condition - Circular Fail (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	LR-n.slmd		



Project			
Tyrone Mine Closure Stockpile Stability - Section North In-Pit Waste (at Little Rock)			
Analysis Description			
Seismic - Pit Lake 5700 ft - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	LR-n.slmd		



Project			
Tyrone Mine Closure Stockpile Stability - Section North In-Pit Waste (at Little Rock)			
Analysis Description			
Seismic - Pit Lake 5700 ft - Circular Fail (GLE / Morgenstern-Price)			
Figure	Scale	Company	
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Date	File Name		
1/2/2019	LR-n.slmd		



GOLDER

Project

Tyrone Mine Closure Stockpile Stability - SECTION SOUTH RIM IN-PIT (7B WASTE)

Analysis Description

Static - Block Failure (GLE / Morgenstern-Price)

Figure

Scale

1:5516

Company

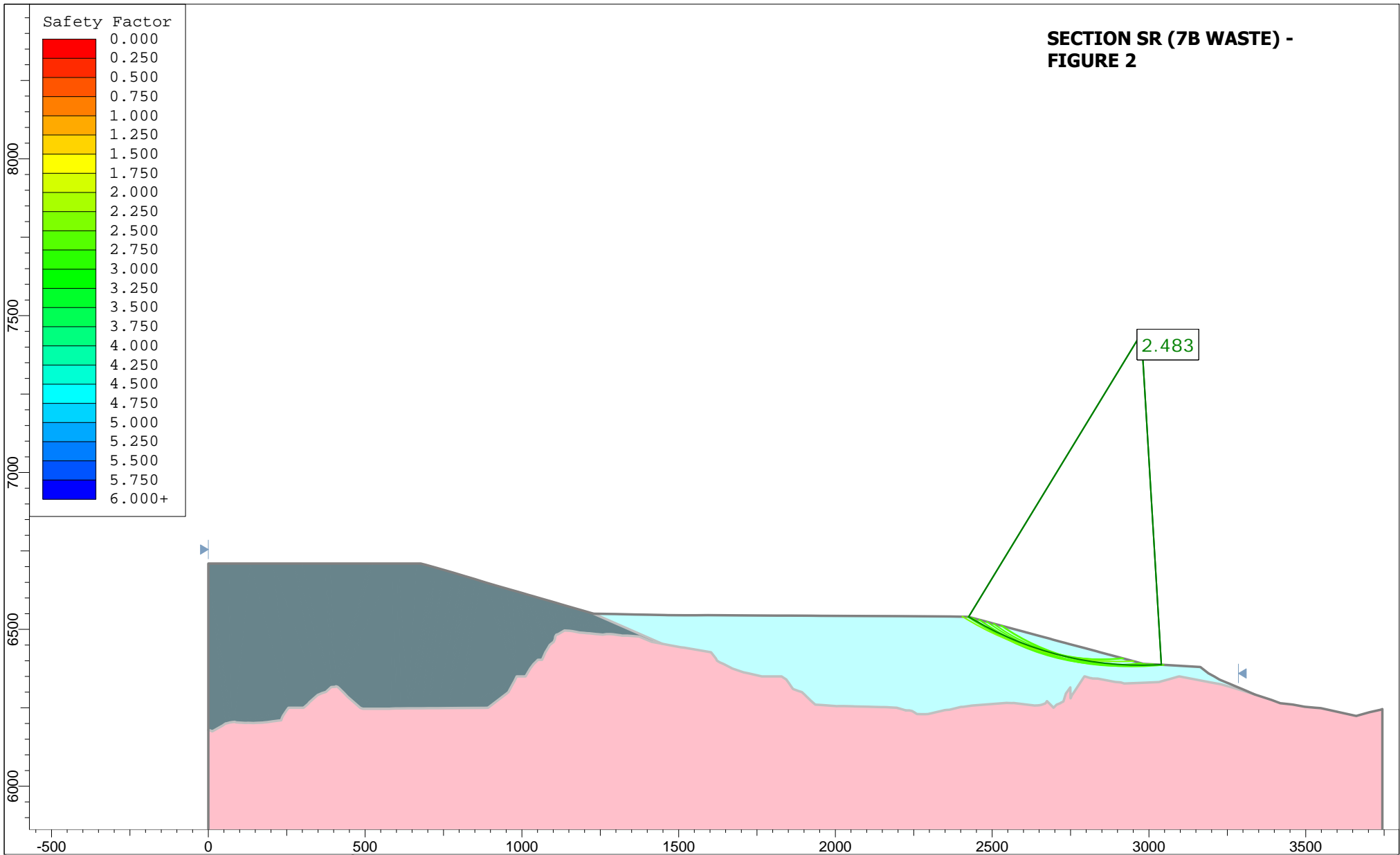
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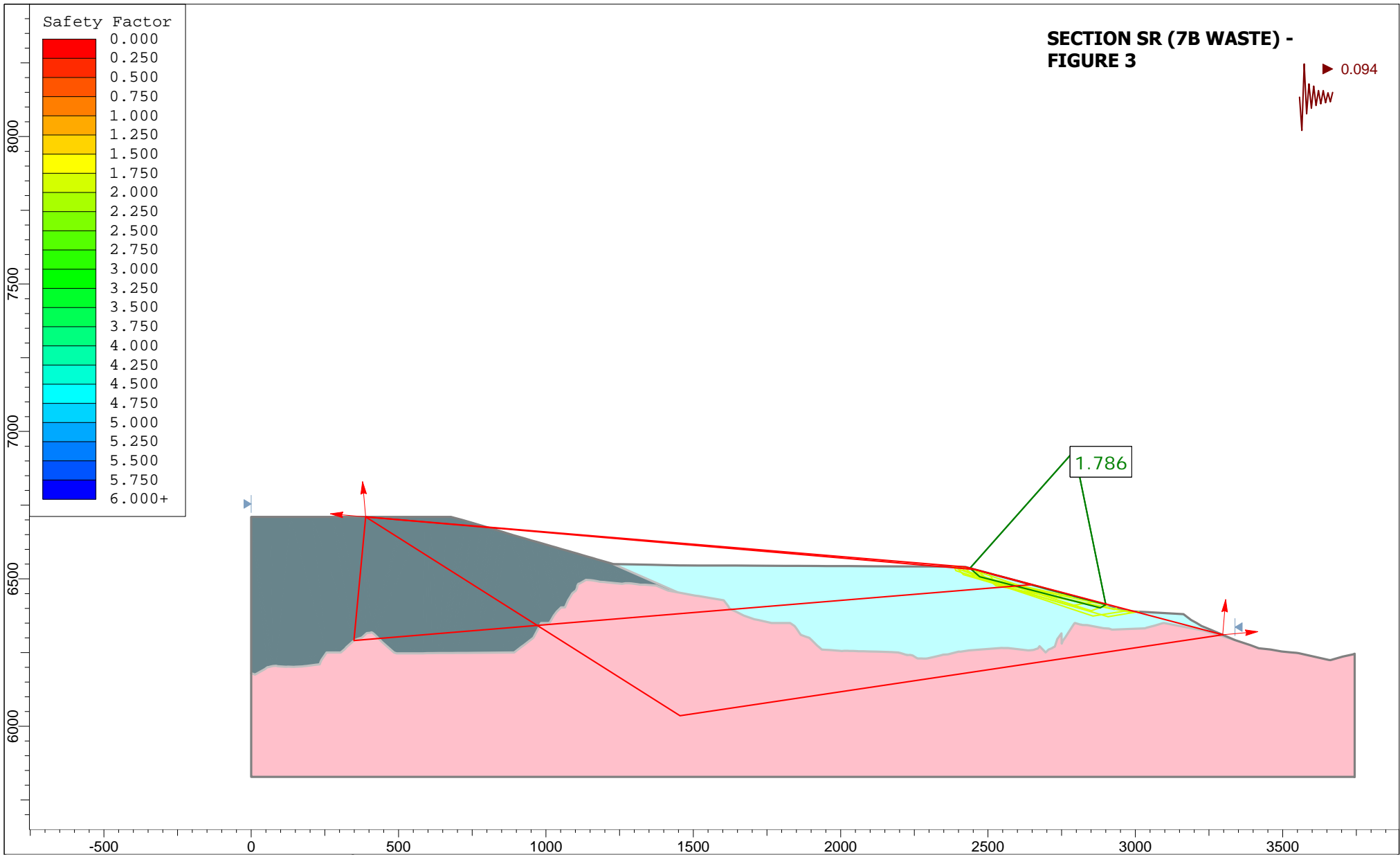
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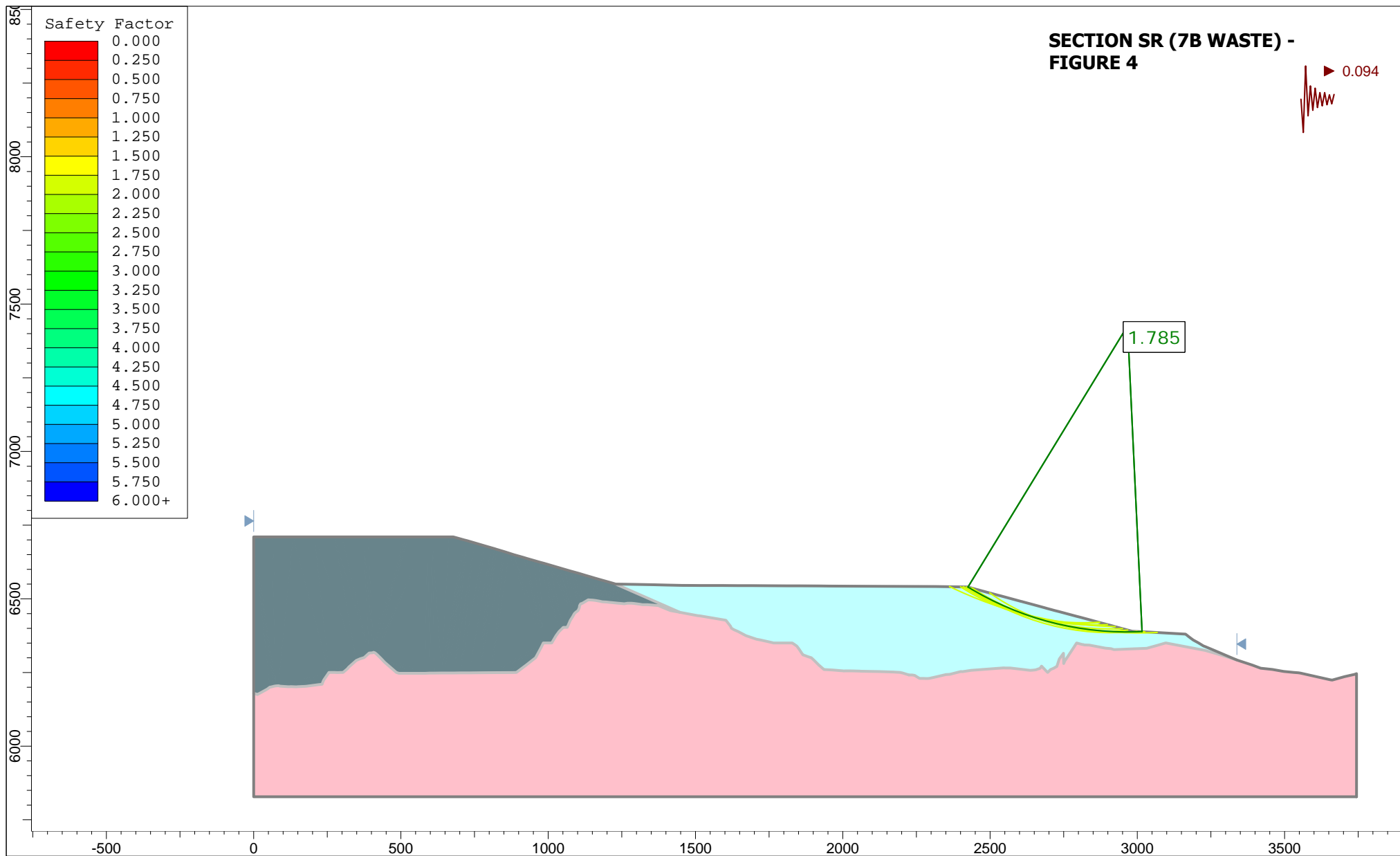
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Project		
Tyrone Mine Closure Stockpile Stability - SECTION SOUTH RIM IN-PIT (7B WASTE)		
Analysis Description		
Static - Circular Failure (GLE / Morgenstern-Price)		
Figure	Scale	Company
	1:5085	Golder Associates
Date	File Name	
	SR.slmd	



Project			
Tyrone Mine Closure Stockpile Stability - SECTION SOUTH RIM IN-PIT (7B WASTE)			
Analysis Description			
Seismic - Block Failure (GLE / Morgenstern-Price)			
Figure	Scale	Company	
	1:5410	Golder Associates	
Date	File Name		
1/2/2019	SR.slmd		



GOLDER

Project

Tyrone Mine Closure Stockpile Stability - SECTION SOUTH RIM IN-PIT (7B WASTE)

Analysis Description

Seismic - Circular Failure (GLE / Morgenstern-Price)

Figure

Scale

1:5410

Company

Golder Associates

Date

1/2/2019

File Name

SR.slmd