



## 1 Purpose

The purpose of this calculation is to determine the Live and Dead Load that the typical vent and shaft plug detail can support according to ACI 318-05.

## 2 Assumptions

- Concrete Weight = 150 pcf
- Concrete Strength = 4ksi
- Reinforcement Strength,  $F_y=60$ ksi
- Soil Backfill Unit Weight = 120 pcf
- Assume that soil is free draining away from structure
- Assume a live load 2-ft soil surcharge (equipment load) of 240 psf
- Assume that steel plate and polyurethane foam do not contribute to load distribution.

## 3 Load

Assuming a Live Load of 240 psf and a Soil Cover of 3.0-ft which is a Live Load, Soil of 360 psf, the selected slab thickness of 12-inch is selected. This results in a 150 psf Dead Load, Concrete.

Factored Load

$$P_U = 1.6L + 1.2D$$

$$P_U = 1.6 * 100 \text{ psf} + 1.2 * 150 \text{ psf}$$

$$P_U = 1.140 \text{ ksf}$$

Using the Timoshenko Circular Plates Table,  $M=3wR^2/16$ , Pinned Condition. Use an additional 6-inch to account for uncertainties in actual diameter.

$$M = \frac{3wR^2}{16}$$

$$M = \frac{3 * 1.140 \text{ ksf} * 6.5^2}{16}$$

$$M = 9.03 \text{ k} - \text{ft} / \text{ft}$$

## 4 Concrete Design

Designed Moment = 9.03 k-ft

Assumed thickness,  $t_b = 10$ -inch

Design width,  $b = 12$ -inch

Assumed Rebar, No. 5 bar

Minimum Clear Cover = 1.5-inch for concrete exposed to earth or weather

Reduction Factor,  $\phi = 0.90$

Effective depth,  $d = t_b - C_c - D_b/2 = 8.0$ -inch, round down to the nearest 0.5-inch for construction tolerance

$$A_{s, req} = \frac{M_u}{\phi_y 0.875d}$$

$$A_{s, req} = \frac{9.03k - ft * 12in / ft}{0.90 * 60ksi * 0.875 * 8.0in}$$

$$A_{s, req} = 0.29in^2$$

$$a = \frac{A_s f_y}{0.85 f'_c b}$$

$$a = \frac{0.29 * 60}{0.85 * 4 * 12}$$

$$a = 0.42$$

$$A_{s, req} = \frac{M_u}{\phi_y \left( d - \frac{a}{2} \right)}$$

$$A_{s, req} = \frac{9.03k - ft * 12in / ft}{0.90 * 60ksi * \left( 10.00in - \frac{0.338in}{2} \right)}$$

$$A_{s, req} = 0.26in^2$$

#### Minimum Steel Requirement

$$A_{s_{min 1b}} = \max \left( \frac{3\sqrt{f_c} \text{, } psi * b * d}{f_y}, \frac{200 \text{, } psi * b * d}{f_y} \right)$$

$$A_{s_{min 1b}} = 0.317in^2$$

$$A_{s_{min 2b}} = \frac{4}{3} A_{s_{reqb}}$$

$$A_{s_{min 2b}} = \frac{4}{3} * 0.204in^2$$

$$A_{s_{min 2b}} = 0.35in^2$$

Therefore, the minimum steel required is 0.317 in<sup>2</sup>/ft. Using No. 5 rebar at 12-inch spacing, Each Way (EW) for the bottom steel fulfills this requirement.

#### Shrinkage and Temperature Steel, ACI 7.12.2.1

$$A_{s, min} = 0.0018 * b * d / 2$$

$$A_{s, min} = 0.0018 * 12 * 12 / 2$$

$$A_{s, min} = 0.013in^2$$

The minimum top steel should be #4 @ 12-inch EW.

## 5 Conclusion

For the 12-ft diameter shaft with 3.0-ft maximum soil cover, a concrete slab 12-inch thick with #5 @ 12-inch EW should be used for the bottom slab steel. The minimum steel required for the top of slab for shrinkage and temperature steel is #4 @ 12-inch EW. This design will support a maximum Live Load of 240 psf and a Soil Load of 3-ft of 120 pcf soil, 360 psf.